FOR INDEX OF SHEETS, SEE SHEET NO. 2

PROJECT LOCATED IN VILLAGES OF MATTESON AND RICHTON PARK

TRAFFIC DATA 2019 ADT = 60,900POSTED SPEED LIMIT = 65 MPH

HBM ENGINEERING CROUP, LLC ROBERT T. BORO, P.E. *062-043749

DATE: 07/30/2021

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SIGNATURE AND SEAL APPLY TO DRAWINGS: 1-6, 23, 84-89

EXPIRATION DATE: 11-30-2021

ROBERT T BORO 062-043749

MOUSSA A

HBM ENGINEERING GROUP, LLC MOUSSA A. ISSA, PH.D. P.E., S.E. *081-005738

DATE: 07/30/2021

Moussa A. Issa

SIGNATURE AND SEAL APPLY TO DRAWINGS: 24-83

EXPIRATION DATE: 11-30-2022

HANSON PROFESSIONAL SERVICES IN KEVIN M. ANDERSON, P.E.

DATE: 07/30/2021

SIGNATURE AND SEAL APPLY TO DRAWINGS:

I-57 AT WCL RR (SN 016-0048,

016-0049) (1 MIS OF US 30)

EXPIRATION DATE: 11-30-2021

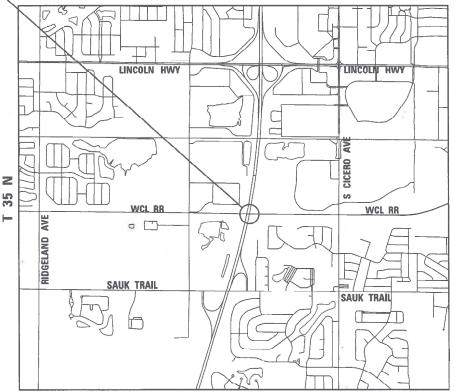
11-5-2021 LETTING ITEM 010 STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PROPOSED HIGHWAY PLANS

FAI ROUTE 57: I-57 AT WCL RR **SECTION 2020–170–BR** PROJECT NHPP-3KSJ(991) BRIDGE JOINT REPLACE / REPAIR AND BRIDGE DECK OVERLAY **COOK COUNTY**

C-91-358-20

R 13 E



NET LENGTH = GROSS LENGTH = 675 FT. = .13 MILES

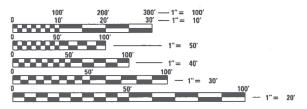
COUNTY TOTAL SHEETS NO.
COOK 89 1
CONTRACT NO. 62M30 SECTION 2020-170-BR

D-91-559-20



STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

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FULL SIZE PLANS HAVE REEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATORS 1-800-892-0123 OR 811

PROJECT ENGINEER: DAN WILGREEN (847) 705-4240 PROJECT MANAGER: FAWAD AQUEEL (847) 705-4247

CONTRACT NO. 62M30

INDEX OF SHEETS

- COVER SHEET
- GENERAL NOTES, INDEX OF SHEETS AND HIGHWAY STANDARDS
- 3**-**5 SUMMARY OF QUANTITIES
- TYPICAL SECTIONS AND HMA CHART
- MAINTENANCE OF TRAFFIC 7-22
- 23 ROADWAY PLAN AND DETAIL
- 24-53 STRUCTURAL PLANS (S.N. 016-0048)
- 54-83 STRUCTURAL PLANS (S.N. 016-0049)
- ENTRANCE AND EXIT RAMP CLOSURE DETAILS (TC-08)
- TRAFFIC CONTROL DETAILS FOR FREEWAY SINGLE & MULTI-LANE WEAVE (TC-09) 85
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- TRAFFIC CONTROL DETAILS FOR FREEWAY SHOULDER CLOSURES AND PARTIAL RAMP CLOSURES (TC-17) 88
- SMART TRAFFIC MONITORING SYSTEM TYPICAL LAYOUT

LIST OF HIGHWAY STANDARDS

STANDARD NO.	<u>DESCRIPTION</u>
701106-02	OFF-ROAD OPERATIONS, MULTILANE, MORE THAN 15' (4.5 M) AWAY
701400-10	APPROACH TO LANE CLOSURE, FREEWAY/EXPRESSWAY
701401-12	LANE CLOSURE, FREEWAY/EXPRESSWAY

LANE CLOSURE, MULTILANE, AT ENTRANCE OR EXIT RAMP, FOR SPEEDS > 45 MPH 701411-09 TRAFFIC CONTROL, SETUP AND REMOVAL, FREEWAY/EXPRESSWAY 701428-01

701901-08 TRAFFIC CONTROL DEVICES TEMPORARY CONCRETE BARRIER

704001-08 781001-04 TYPICAL APPLICATIONS, RAISED REFLECTOR PAVEMENT MARKERS 782006-01 GUARDRAIL AND BARRIER WALL REFLECTOR MOUNTING DETAILS

COMMITMENTS

NONE

GENERAL NOTES

- 1. BEFORE STARTING ANY EXCAVATION, THE CONTRACTOR SHALL CALL "J.U.L.I.E." AT (800) 892-0123 OR 811 FOR FIELD LOCATIONS OF BURIED ELECTRIC, TELEPHONE, AND GAS UTILITIES, 48 HOUR NOTIFICATION IS REQUIRED.
- 2. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITIES WITH UTILITY COMPANIES, AND THE VILLAGES OF MATTESON AND RICHTON PARK.
- 3. BEFORE BEGINNING ANY WORK, THE CONTRACTOR SHALL RETAIN AND RECORD, FOR FUTURE REFERENCE, ALL EXISTING PAVEMENT MARKING LINES (AND RAISED REFLECTIVE PAVEMENT MARKERS) IN ORDER THAT THESE LOCATIONS CAN BE RE-ESTABLISHED FOR STRIPING. EXACT LOCATIONS OF ALL PAVEMENT MARKINGS SHALL BE AS DIRECTED BY THE ENGINEER.
- 4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND CONDITIONS EXISTING IN THE FIELD PRIOR TO CONSTRUCTION AND ORDERING OF MATERIALS.
- 5. THE CONTRACTOR SHALL CONTACT THE DISTRICT ONE TRAFFIC CONTROL SUPERVISOR AT (847) 705-4155 A MINIMUM OF 72 HOURS IN ADVANCE OF BEGINNING WORK.
- 6. THE RESIDENT ENGINEER SHALL CONTACT REGINA COOPER, AREA TRAFFIC FIELD ENGINEER. AT REGINA.COOPER2@ILLINOIS.GOV A MINIMUM OF TWO (2) WEEKS PRIOR TO PLACEMENT OF PERMANENT PAVEMENT MARKINGS.
- 7. THE CONTRACTOR IS REQUIRED TO PROVIDE ACCESS TO ABUTTING PROPERTY AT ALL TIMES DURING THE CONSTRUCTION OF THIS PROJECT.
- 8. DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS.
- 9. ALL DAMAGE TO EXISTING PAVEMENT MARKINGS OR RAISED REFLECTIVE PAVEMENT MARKERS OUTSIDE THE REMOVAL LINE SHOWN ON THE PLANS SHALL BE REPLACED AT NO ADDITIONAL COST TO THE DEPARTMENT.
- 10. THE CONTRACTOR WILL NOT BE ALLOWED TO SET UP A YARD OR FIELD OFFICE ON STATE PROPERTY WITHOUT WRITTEN PERMISSION FROM THE DEPARTMENT.
- 11. SHOULDER PATCHING LOCATIONS WILL BE DETERMINED IN THE FIELD BY THE ENGINEER.
- 12. RESURFACING OF SHOULDERS ARE INCLUDED WITH THE RESURFACING OF THE APPROACH PAVEMENTS.

HBM	
ENGINEERING GROUP, LLC	

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GENERAL NOTES, INDEX OF SHEETS & HIGHWAY STANDARDS	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
I-57 OVER WCL RR	57	2020-170-BR	соок	89	2
I-37 OVER VVCE IIII			CONTRACT	F NO. 62	2M30
SCALE: 24.0000 / in SHEET 1 OF 1 SHEETS STA. TO STA.		ILLINOIS FED. A	ID PROJECT		

				С	ONSTRUCTION	CODE
				9	0% FED 10% S	TATE
CODE			TOTAL	ROADWAY	BRIDGE	BR I DGE
NO.	ITEM	UNIT	QUANT I TY	0005	0059	0059
				URBAN	SN 016-0048	SN 016-0049
20700220	POROUS GRANULAR EMBANKMENT	CU YD	26		11	15
40600290	BITUMINOUS MATERIALS (TACK COAT)	POUND	908	908		
40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SQ YD	1481	1481		
	POLYMERIZED HOT-MIX ASPHALT SURFACE					
40605026	POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, STONE MATRIX ASPHALT, 9.5, MIX "F", N80	TON	198	198		
50102400	CONCRETE REMOVAL	CU YD	40.1		19.5	20.6
50104650	SLOPE WALL REMOVAL	SQ YD	70		28	42
50157300	PROTECTIVE SHIELD	SQ YD	480		240	240
50300355	CONCRETE CURENCE VIDEO	CH VD	44.0		22	22.0
50300255	CONCRETE SUPERSTRUCTURE	CU YD	44.9		22	22.9
50300260	BRIDGE DECK GROOVING	SQ YD	3410		1705	1705
50300300	PROTECTIVE COAT	SQ YD	4424		2212	2212
50500405	FURNISHING AND ERECTING STRUCTURAL STEEL	POUND	4960		2590	2370
50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	7520		3750	3770
E0000515	DAD CDL LCEDC	EACH	112		5.6	5.0
30800515	BAR SPLICERS	EACH	112		56	56
51100100	SLOPE WALL 4 INCH	SQ YD	70		28	42
* SPECIALT	V ITEM					

	1	1		CONSTRUCTION CODE		
				2	90% FED 10% S	TATE
CODE			TOTAL	ROADWAY	BR I DGE	BRIDGE
NO.	ITEM	UNIT	QUANT I TY	0005	0059	0059
				URBAN	SN 016-0048	SN 016-0049
52000110	PREFORMED JOINT STRIP SEAL	FOOT	264		132	132
58700300	CONCRETE SEALER	SQ FT	768		384	384
50000200	EDOVY CDACK INJECTION	FOOT	222		110	214
59000200	EPOXY CRACK INJECTION	FOOT	333		119	214
64200116	SHOULDER RUMBLE STRIPS, 16 INCH	FOOT	4740	4740		
67000400	ENGINEER'S FIELD OFFICE, TYPE A	CAL MO	12	12		
67100100	MOBILIZATION	L SUM	1	1		
	MODILIZATION	L 30W	1	<u> </u>		
70107025	CHANGEABLE MESSAGE SIGN	CAL DA	360	360		
70300904	PAVEMENT MARKING TAPE, TYPE IV 4"	FOOT	20252	20252		
70300906	PAVEMENT MARKING TAPE, TYPE IV 6"	FOOT	2900	2900		
70300908	PAVEMENT MARKING TAPE, TYPE IV 8"	FOOT	1282	1282		
70400100	TEMPORARY CONCRETE BARRIER	FOOT	1750	1750		
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	FOOT	1750	1750		
70600250	IMPACT ATTENUATORS, TEMPORARY (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2	2		
70600350	IMPACT ATTENUATORS, RELOCATE (NON-REDIRECTIVE), TEST LEVEL 3	EACH	2	2		

HBM ENGINEERING GROUP, ILC

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SUMMARY OF QUANTITIES	F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
I–57 OVER WCL RR			2020-170-BR		соок	89	3
F-57 OVER TIGE IIII				CONTRAC	r N O. 62	2M30	
SCALE: 100.0000 ' / insheet 1 OF 3 SHEETS STA. TO STA.	İ		(ILLINOIS (F	FED. AID	PROJECT		

CONSTRUCTION CODE

^{*} SPECIALTY ITEM

					CONSTRUCTION CODE		
					9	0% FED 10% S	TATE
	CODE			TOTAL	ROADWAY	BRIDGE	BRIDGE
	NO.	ITEM	UNIT	QUANT I TY	0005	0059	0059
					URBAN	SN 016-0048	SN 016-0049
*	70000000	THERMOPLASTIC PAVEMENT MARKING - LINE	FOOT	2026	2026		
	78000200	4"	FOOT	2926	2926		
						ļ	
*	78000500	THERMOPLASTIC PAVEMENT MARKING - LINE 8"	FOOT	950	950		
ŀ		8"		330	330		
*	78004220	PREFORMED PLASTIC PAVEMENT MARKING, TYPE B - INLAID - LINE 5"	FOOT	2440	2440		
ŀ		D - INCATO LINE 3					
-							
*	78004240	PREFORMED PLASTIC PAVEMENT MARKING, TYPE B - INLAID - LINE 8"	FOOT	242	242		
ļ							
	70000	MODIFIED URETHANE PAVEMENT MARKING -					
*	78009004	LINE 4"	FOOT	1646	1646		
*	78100100	RAISED REFLECTIVE PAVEMENT MARKER	EACH	36	36		
ŀ	70100100	TOTAL TOTAL PRODUCTION OF THE	L/ (C/)	30			
*	78100300	REPLACEMENT REFLECTOR	EACH	172	172		
f							
-		<u> </u>					
*	78200011	BARRIER WALL REFLECTORS, TYPE C	EACH	400	400		
f	78300202	PAVEMENT MARKING REMOVAL - WATER	CO ET	2833	2833		
-	76300202	BLASTING	SQ FT	2033	2033		
Ī	X0325201	SHOULDER RUMBLE STRIP REMOVAL	SQ YD	904	904		
}							
							1
	X1700085	CLASS D PATCHES, TYPE IV, 14 INCH (SPECIAL)	SQ YD	300	300		
f							
-			:				
	X2700001	TEMPORARY RUMBLE STRIPS (SPECIAL)	EACH	32	32		
*	X2700003	GROOVING FOR RECESSED PAVEMENT MARKING	FOOT	200	200		
-		8" 	. 551	200	250		
	X4400501	COMBINATION CURB AND GUTTER REMOVAL AND REPLACEMENT LESS THAN OR EQUAL TO 10	FOOT	32	32		
-							
	* SPECIALT	FEET					

_			1		CONSTRUCTION CODE			
					9	90% FED 10% S	TATE	
	CODE			TOTAL	ROADWAY	BR I DGE	BR I DGE	
	NO .	ITEM	UNIT	QUANTITY	0005	0059	0059	
L					URBAN	SN 016-0048	SN 016-0049	
L								
	X7011015	TRAFFIC CONTROL AND PROTECTION (EXPRESSWAYS)	L SUM	1	1			
	X7013820	TRAFFIC CONTROL SURVEILLANCE, EXPRESSWAYS	CAL DA	180	180			
	X7030005	TEMPORARY PAVEMENT MARKING REMOVAL	SQ FT	9079	9079			
	X7800956	PREFORMED PLASTIC PAVEMENT MARKING, TYPE B - INLAID - CONTRAST - LINE 7"	FOОТ	200	200			
	X7830072	GROOVING FOR RECESSED PAVEMENT MARKING 6"	FOOT	2440	2440			
	X7830076	GROOVING FOR RECESSED PAVEMENT MARKING 9"	FOOT	242	242			
	X8570000	SMART TRAFFIC MONITORING SYSTEM	L SUM	1	1			
L	Z0001800	APPROACH SLAB REPAIR (PARTIAL DEPTH)	SQ YD	0.1			0.1	
	Z0001903	STRUCTURAL STEEL REMOVAL	POUND	5390		2900	2490	
	Z0001905	STRUCTURAL STEEL REPAIR	POUND	1640		820	820	
	Z0006016	BRIDGE DECK LATEX CONCRETE OVERLAY, 2 3/4 INCHES	SQ YD	3590		1795	1795	
	Z0012130	BRIDGE DECK SCARIFICATION 3/4"	SQ YD	3590		1795	1795	
	Z0012754	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT	509		294	215	
	Z0012755	STRUCTURAL REPAIR OF CONCRETE (DEPTH GREATER THAN 5 INCHES)	SQ FT	338		190	148	

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SUMMARY OF QUANTITIES	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEET NO.
I–57 OVER WCL RR	57	2020-170-BR	соок	89 4
I-27 OVER WOL HIL			CONTRAC	T N O. 62M30
SCALE: 100.0000 ' / iħSHEET 2 OF 3 SHEETS STA. TO STA.	İ	(ILLINOIS (FED. A	ID PROJECT	

CONSTRUCTION CODE

^{*} SPECIALTY ITEM

				C	ONSTRUCTION	CODE
				9	0% FED 10% S	TATE
CODE			TOTAL	ROADWAY	BRIDGE	BR I DGE
NO.	ITEM	UNIT	QUANT I TY	0005	0059	0059
				URBAN	SN 016-0048	SN 016-0049
Z0016001	DECK SLAB REPAIR (FULL DEPTH, TYPE I)	SQ YD	3.7		2.6	1.1
Z0016002	DECK SLAB REPAIR (FULL DEPTH, TYPE II)	SQ YD	5.4		2.4	3
Z0018051	DRAINAGE SCUPPERS TO BE ADJUSTED	EACH	4		2	2
Z0030850	TEMPORARY INFORMATION SIGNING	SQ FT	80	80		
Z0048665	RAILROAD PROTECTIVE LIABILITY INSURANCE	L SUM	1	1		
Z0073200	TEMPORARY SHORING AND CRIBBING	EACH	11		5	6
Z0076600	TRAINEES	HOUR	500	500		
Z0076604	TRAINEES - TRAINING PROGRAM GRADUATE	HOUR	500	500		

^{*} SPECIALTY ITEM

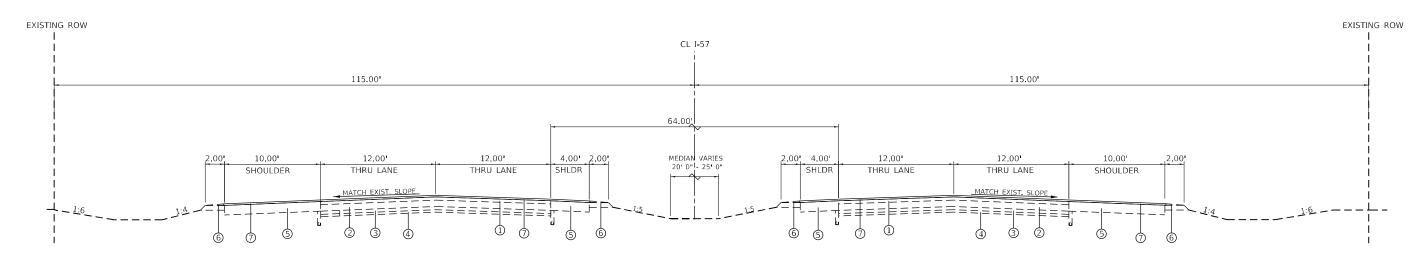
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SI	JMMARY	OF QU	ANTITIES	s	F.A.I. RTE.	SECTION	V	
	57	2020-170-	BR					
	I-37 U	VER WC						
SCALE: 100.0000 ' / ih.SHEET 3	OF 3	SHEETS	STA.	TO STA.		ILLI	NOIS (FED. /

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I-57 EXISTING TYPICAL SECTION



I-57 PROPOSED TYPICAL SECTION

LEGEND

- ① EXISTING HMA PAVEMENT, 4" & VARIES
- ② EXISTING CRC PAVEMENT, 8" & VARIES
- 3 EXISTING STABILIZED SUBBASE, 4" & VARIES
- 4 EXISTING AGGREGATE SUBGRADE, VARIES
- ⑤ PAVED SHOULDER, 12", VARIES
- **(6)** EXISTING AGGREGATE SHOULDER
- 7 PROPOSED POLYMERIZED HMA SURFACE COURSE, SMA, 9.5, MIX "F", N80, 2"

HOT-MIX ASPHALT MIXTURE REQUIREMENTS

PAY ITEM DESCRIPTION	MIX TYPE	% AIR VOIDS @ Ndes	QMP DESIGNATION
APPROACH RESURFACING	POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, SMA, 9.5; MIX "F", N80, 2"	3.5% @80 GYR	QC/QA
SHOULDER RUMBLE STRIP REMOVAL	HMA SURFACE COURSE IL-9.5, MIX D, N70	4% @70 GYR	QC/QA
CLASS D PATCHING (SPECIAL)	HMA SURFACE COURSE, MIX "D", IL-9.5, N70, TOP 2"	4% @70 GYR	QC/QA
	HMA BINDER COURSE, IL-19.0, N90, 12"	4% @90 GYR	QC/QA

QMP DESIGNATION: QUALITY CONTROL/QUALITY ASSURANCE (QC/QA) QUALITY CONTROL FOR PERFORMANCE (QCP)

QUALITY MANAGEMENT PROGRAM (QMP) IDENTIFIES THE PARTICULAR QUALITY CONTROL SPECIFICATION THAT APPLIES TO THE HMA MIXTURE. THE UNIT WEIGHT USED TO CALCULATE ALL HMA SURFACE MIX QUANTITIES IS 112 LBS/SQVD/IN. THE AC TYPE FOR POLYMERIZIED HMA MIXTURES SHALL BE "SBS/SBR PG 76-22" AND FOR NON-POLYMERIZED HMA THE "AC TYPE" SHALL BE "PG 64-22" UNLESS MODIFIED BY SPECIAL PROVISIONS.

FOR USE OF RECYCLE MATERIALS SEE SPECIAL PROVISIONS.

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,	TYPICAL	SECTIO	NS AND	нма	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
	I–57 OVER WCL RR					57	2020-170-BR	COOK	89	6
		1-37 0	VLII VV	L IIII				CONTRACT	NO. 62	2M30
00 /in S⊦	HEET 1	OF 1	SHEETS	STA.	TO STA.		ILLINOIS FED. AI	D PROJECT		

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P:\2004-825 PTB195-014 HBM\WO #1 I57 over WCL RR\SF

MAINTENANCE OF TRAFFIC GENERAL NOTES

- 1. THE STATIONING USED IN THESE MAINTENANCE-OF-TRAFFIC PLANS DO NOT MATCH THE AS-BUILT PLANS OR THE STRUCTURAL PLANS. THEY SHOULD BE USED FOR THE BENEFIT OF LOCATING MOT ELEMENTS RELATIVE TO ONE ANOTHER. THE STATIONING AND ALIGNMENT ARE FROM RESURFACING CONTRACT 62M54.
- 2. THE CONTRACTOR SHALL PROVIDE ALL SIGNS, VERTICAL PANELS, TYPE II BARRICADES, TYPE III BARRICADES, CHANNELIZATION DRUMS, ALL TEMPORARY CONCRETE BARRIERS, AND PROTECTION NECESSARY FOR WORK ZONE TRAFFIC CONTROL AND PROTECTION, OR AS DIRECTED BY THE ENGINEER.
- THE CONTRACTOR SHALL CONTACT THE DISTRICT ONE TRAFFIC CONTROL SUPERVISOR AT Carlos.Munoz@illinois.gov A MINIMUM OF 72 HOURS IN ADVANCE OF BEGINNING OF WORK."
- 4. A MAINTENANCE OF TRAFFIC PLAN SHALL BE SUBMITTED TO THE DISTRICT ONE EXPRESSWAY TRAFFIC CONTROL SUPERVISOR (847-705-4155) 14 DAYS IN ADVANCE OF ANY STAGES CHANGES OR FULL EXPRESSWAY CLOSURES ON INTERSTATE 57. THE MAINTENANCE OF TRAFFIC PLAN SHALL INCLUDE, BUT NOT BE LIMITED TO: LANE AND RAMP CLOSURES, EXISTING GEOMETRICS, AND EQUIPMENT AND MATERIAL LOCATION. PLEASE SEE CONTRACT SPECIAL PROVISIONS.
- 5. THE CONTRACTOR SHALL REQUEST AND GAIN APPROVAL FROM THE ILLINOIS DEPARTMENT OF TRANSPORTATION'S EXPRESSWAY TRAFFIC OPERATIONS ENGINEER AT WWW.IDOTLCS.COM TWENTY-FOUR (24) HOURS IN ADVANCE OF ALL DAILY LANE, RAMP AND SHOULDER CLOSURES AND 7 DAYS IN ADVANCE OF ALL PERMANENT AND WEEKEND CLOSURES ON ALL FREEWAYS AND/OR EXPRESSWAYS IN DISTRICT ONE. THIS ADVANCE NOTIFICATION IS CALCULATED BASED ON MONDAY THROUGH FRIDAY AND SHALL NOT INCLUDE WEEKENDS OR HOLIDAYS.
- TWO WEEKS PRIOR TO PLACEMENT OF PERMANENT PAVEMENT MARKINGS THE ENGINEER SHALL CONTACT THE IDOT AREA TRAFFIC FIELD ENGINEER AT 847-705-4412 AND THE IDOT EXPRESSWAY AREA ENGINEER AT (847)-705-4153.
- 7. THE CONTRACTOR SHALL USE DURING ALL WINTER STAGING MONTHS (NOVEMBER 1 APRIL 1) PERMANENT PAVEMENT MARKINGS. THE PERMANENT PAVEMENT MARKING MATERIAL SHALL BE APPROVED BY THE ENGINEER. PERMANENT PAVEMENT MARKING ON NEW OR EXISTING PAVEMENT TO REMAIN SHALL BE REMOVED BY WATER BLASTING OR AS APPROVED BY THE ENGINEER.
- 3. THE CONTRACTOR SHALL USE PAVEMENT MARKING TAPE, TYPE IV ON EXISTING SURFACES TO REMAIN, AREAS IN CONFLICT WITH OTHER STAGES AND PERMANENT SURFACES EXCEPT DURING WINTER MONTHS.
- 9. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING AND PROVIDING ACCESS POINTS TO THE WORK ZONE. ACCESS POINTS MUST BE APPROVED BY THE ENGINEER PRIOR TO INSTALLATION. ANY SIGNING OR ADDITIONAL TRAFFIC CONTROL DEVICES AND FLAGGERS REQUIRED TO PROVIDE CONTRACTOR ACCESS TO THE WORK ZONE IS INCLUDED IN THE COST OF "TRAFFIC CONTROL AND PROTECTION (EXPRESSWAYS)" AND WILL NOT BE PAID FOR SEPARATELY.
- 10. ALL DRUMS, VERTICAL PANELS, AND TYPE II BARRICADES SHALL BE SPACED AT 50 FEET CENTER TO CENTER THROUGHOUT THE WORK ZONE, EXCEPT IN TAPER AREAS, GORE AREAS, AND ALONG CORNER RADII, WHERE THEY SHALL BE SPACED AT 20 FEET CENTER TO CENTER EXCEPT AS OTHERWISE INDICATED ON THE PLANS.

- 11. TEMPORARY CONCRETE BARRIERS SHALL BE EQUIPPED WITH TYPE "C"
 REFLECTORS, IN ACCORDANCE WITH HIGHWAY STANDARD 704001 SPACED AT 25'
 CENTERS.
- 12. ALL CONSTRUCTION WARNING SIGNS SHALL BE BLACK LEGEND ON ORANGE BACKGROUND.
- 13. ALL CONSTRUCTION WARNING SIGN DIMENSIONS SHALL BE 48" X 48", EXCEPT AS OTHERWISE STATED ON THE PLANS.
- 14. ALL "ROAD CONSTRUCTION AHEAD" WARNING SIGNS SHALL BE EQUIPPED WITH HIGH INTENSITY MONO-DIRECTIONAL FLASHING LIGHTS.
- 15. ALL EXISTING GUIDE SIGNS (i.e. STREET NAME SIGNS, ADVANCE STREET NAME SIGNS, ROUTE MARKERS, ETC.) SHALL BE MAINTAINED AND VISIBLE TO TRAFFIC DURING CONSTRUCTION.
- 16. EXISTING SIGNS IN CONFLICT WITH STAGING SHALL BE COVERED AS INDICATED ON THE MAINTENANCE OF TRAFFIC PLANS AND/OR AS DIRECTED BY THE ENGINEER. THE COST OF THIS WORK SHALL BE CONSIDERED INCLUDED IN THE CONTRACT COST FOR TRAFFIC CONTROL AND PROTECTION (EXPRESSWAYS).
- 17. DIRECTION INDICATOR BARRICADES SHALL BE USED AT LANE CLOSURE TAPER LOCATIONS FOR ROADWAYS WITH SPEEDS GREATER THAN OR EQUAL TO 45 MPH OR AS OTHERWISE DIRECTED BY THE ENGINEER.
- 18. ALL FULL EXPRESSWAY CLOSURES AND STAGE CHANGES REQUIRING THE STOPPING AND/OR THE PACING OF TRAFFIC SHALL TAKE PLACE DURING THE ALLOWABLE HOURS FOR FULL EXPRESSWAY CLOSURES AND SHALL BE APPROVED BY THE DEPARTMENT. THE CONTRACTOR SHALL NOTIFY THE DISTRICT ONE EXPRESSWAY TRAFFIC CONTROL SUPERVISOR AT LEAST 3 WORKING DAYS (WEEKENDS AND HOLIDAYS DO NOT COUNT INTO THIS 72 HOURS NOTIFICATION) IN ADVANCE OF ANY PROPOSED STAGE CHANGE. PLEASE SEE THE "KEEPING THE EXPRESSWAY OPEN TO TRAFFIC (D1)" SPECIAL PROVISION.
- 19. TEMPORARY CONCRETE BARRIERS AND IMPACT ATTENUATORS SHALL NOT BE PINNED TO BRIDGE DECK, APPROACH SLABS, OR SLEEPER SLABS.
- 20. FORM OPER 2410 ROAD RESTRICTION INFORMATION FORM SHALL BE SUBMITTED PER INSTRUCTIONS 21 DAYS PRIOR TO GOING INTO STAGE II.



SCALE: NTS

SUGGESTED CONSTRUCTION SEQUENCING

PRE-STAGE

INTERSTATE 57

CONSTRUCTION TO BE COMPLETED

- REMOVE THE RUMBLE STRIPS ALONG THE INSIDE AND OUTSIDE EDGES OF PAVEMENT.
- REMOVE REFLECTORS IN RAISED REFLECTOR PAVEMENT MARKERS.

MAINTENANCE OF TRAFFIC

- PRE-STAGE WORK WILL BE PERFORMED DURING OFF-PEAK HOURS IN ACCORDANCE WITH HIGHWAY STANDARD 701401 AND DISTRICT ONE DETAIL TC-17.
- WORK ZONE SPEED LIMIT IS 55 MPH.

STAGE 1

INTERSTATE 57

CONSTRUCTION TO BE COMPLETED

• PERFORM BRIDGE REHABILITATION WORK AS DESCRIBED IN THE GP&E SHEETS WITHIN THE LIMITS OF STAGE 1 CONSTRUCTION (INSIDE LANE AND SHOULDER).

MAINTENANCE OF TRAFFIC

- REDUCE THE 2 LANES OF TRAFFIC TO 1 LANE BEFORE SHIFTING THE LANE PARTIALLY ONTO THE OUTSIDE SHOULDER AS SHOWN IN THE PLANS.
- WORK ZONE SPEED LIMIT IS 45 MPH.
- LANE REDUCTION FOR NORTHBOUND IS TO BE COMPLETED SOUTH OF THE EXIT RAMP TO SAUK TRAIL. THE EXIT AT SAUK TRAIL WILL UTILIZE EXISTING STRIPING.
- NB ENTRANCE RAMP FROM SAUK TRAIL IS TO BE SHORTENED TO A 400' AS SHOWN IN THE PLANS.
- LANE REDUCTION FOR SOUTHBOUND IS TO BE COMPLETED BETWEEN THE EXIT FOR EB US ROUTE 30 AND THE SB ENTRANCE FROM US ROUTE 30. THE RAMPS WILL UTILIZE EXISTING STRIPING.

STAGE 2

INTERSTATE 57

CONSTRUCTION TO BE COMPLETED

 PERFORM BRIDGE REHABILITATION WORK AS DESCRIBED IN THE GP&E SHEETS WITHIN THE LIMITS OF STAGE 2 CONSTRUCTION (OUTSIDE LANE AND SHOULDER).

MAINTENANCE OF TRAFFIC

- REDUCE THE 2 LANES OF TRAFFIC TO 1 LANE BEFORE SHIFTING THE LANE PARTIALLY ONTO THE INSIDE SHOULDER AS SHOWN IN THE PLANS.
- WORK ZONE SPEED LIMIT IS 45 MPH.
- LANE REDUCTION FOR NORTHBOUND IS TO BE COMPLETED SOUTH OF THE EXIT RAMP TO SAUK TRAIL. THE EXIT AT SAUK TRAIL WILL UTILIZE EXISTING STRIPING.
- NB ENTRANCE RAMP FROM SAUK TRAIL IS TO BE SHORTENED TO A 400'
 AND IS TO CROSS OVER THE EXISTING OUTSIDE LANE AS SHOWN IN THE
 PLANS
- LANE REDUCTION FOR SOUTHBOUND IS TO BE COMPLETED BETWEEN THE EXIT FOR EB US ROUTE 30 AND THE SB ENTRANCE FROM US ROUTE 30. THE RAMPS WILL UTILIZE EXISTING STRIPING.

POST-STAGE

INTERSTATE 57

CONSTRUCTION TO BE COMPLETED

- REMOVAL OF TEMPORARY CONCRETE BARRIER.
- PLACEMENT OF PERMANENT PAVEMENT MARKINGS AND MARKERS.
- REPLACEMENT OF RUMBLE STRIPS.
- REPLACEMENT OF REFLECTORS IN THE RAISED REFLECTOR PAVEMENT MARKERS.

MAINTENANCE OF TRAFFIC

- POST-STAGE WORK WILL BE PERFORMED IN ACCORDANCE WITH HIGHWAY STANDARD 701401 AND DISTRICT ONE DETAIL TC-17.
- WORK ZONE SPEED LIMIT IS 55 MPH.

JSER NAME = AnderØØ833	DESIGNED - CDM	REVISED -
	DRAWN - CDM	REVISED -
PLOT SCALE = 99.9998 '/ in.	CHECKED - KMA	REVISED -
PLOT DATE = 6/28/2021	DATE - 06/28/21	REVISED -

		_	TEM	IPORARY PA	VEMENT M	ARKING SCH	HEDULE
			PAVEMENT	PAVEMENT	PAVEMENT	TEMPORARY	
			MARKING	MARKING	MARKING	PAVEMENT	DESCRIPTION
			TAPE, TYPE IV	TAPE, TYPE IV	TAPE, TYPE IV	MARKING	DESCRIPTION
		_	4"	6"	8"	REMOVAL	
STAGE	FROM	ТО	70300904	70300906	70300908	X7030005	
STAGE	STA	STA	FOOT	FOOT	FOOT	SQ FT	
SB STAGE 1	105+45	118+15	1267			422.33	WHITE SOLID OUTSIDE EDGE
SB STAGE 1	107+25	114+50		725		362.50	YELLOW SOLID ON SIDE OF TCB
SB STAGE 1	107+25	161+50	5415				YELLOW SOLID INSIDE EDGE
SB STAGE 1	107+25	124+35				568.67	YELLOW SOLID INSIDE EDGE NOT PRESENT IN STAGE 2
SB STAGE 2	105+65	124+35	1867				YELLOW SOLID INSIDE EDGE NOT PRESENT IN STAGE 1
SB STAGE 2	105+65	161+50				1858.67	YELLOW SOLID INSIDE EDGE
SB STAGE 2	107+25	114+50		725		362.50	YELLOW SOLID ON SIDE OF TCB
SB STAGE 2	107+25	124+35	1707			569.00	WHITE SOLID OUTSIDE EDGE
NB STAGE 1	63+27	114+70	5148				YELLOW SOLID INSIDE EDGE
NB STAGE 1	107+45	114+70		725		362.50	YELLOW SOLID ON SIDE OF TCB
NB STAGE 1	63+27	114+70				791.67	YELLOW SOLID INSIDE EDGE NOT PRESENT IN STAGE 2
NB STAGE 1	99+25	103+73			450	266.67	WHITE SOLID RAMP DUTSIDE EDGE
NB STAGE 1	103+73	116+50	1281			443.67	WHITE SOLID OUTSIDE EDGE
NB STAGE 2	91+00	97+00	601				YELLOW SOLID INSIDE EDGE NOT PRESENT IN STAGE 1
NB STAGE 2	63+27	97+00				1124.67	YELLOW SOLID INSIDE EDGE
NB STAGE 2	91+00	97+70	672			224.00	WHITE SOLID OUTSIDE EDGE
NB STAGE 2	97+70	99+56			186	144.00	WHITE SOLID GORE EDGE
NB STAGE 2	98+43	99+56			114	96.00	WHITE SOLID GORE EDGE
NB STAGE 2	98+75	104+05			532	341.33	WHITE SOLID RAMP OUTSIDE EDGE
NB STAGE 2	104+05	114+70	1067			362.33	WHITE SOLID OUTSIDE EDGE
NB STAGE 2	107+45	114+70		725		362.50	YELLOW SOLID ON SIDE OF TCB
NB STAGE 2	104+05	116+30	1227			415.67	YELLOW SOLID INSIDE EDGE
	PI	ROJECT TOTAL:	20,252	2,900	1,282	9,079	

TEMPORARY CONCRETE BARRIER SCHEDULE								
			TEMPORARY CONCRETE BARRIER	RELOCATE TEMPORARY CONCRETE BARRIER	BARRIER WALL REFLECTORS, TYPE C			
STAGE	FROM	ТО	70400100	70400200	78200011			
tioning trapiting	STA	STA	FOOT	FOOT	EACH			
SB STAGE 1	107+25	116+00	875		116			
SB STAGE 2	107+25	116+00		875	58			
NB STAGE 1	106+00	114+70	875		116			
NB STAGE 2	106+00	114+70		875	58			
Replacement	:S				52			
	F	ROJECT TOTAL:	1750	1750	400			

RUMBLE STRIPS SCHEDULE								
				SHOULDER RUMBLE STRIPS, 16 INCH	SHOULDER RUMBLE STRIP REMOVAL			
STAGE	FROM	TO	SIDE	64200116	X0325201			
STAGE	STA	STA		FOOT	SQ YD			
NB STAGE 1	107+00	108+57	Outside		52			
NB STAGE 1	113+67	116+50	Outside		94			
NB STAGE 2	103+85	108+54	Inside		156			
NB STAGE 2	113+61	116+35	Inside		91			
SB STAGE 1	105+45	108+34	Outside		96			
SB STAGE 1	113+34	118+15	Outside		160			
SB STAGE 2	105+65	108+39	Inside		91			
SB STAGE 2	113+49	118+35	Inside		162			
NB Post-Stage	107+00	116+50	Outside	950				
NB Post-Stage	103+85	116+35	Inside	1250				
SB Post-Stage	105+45	118+15	Outside	1270				
SB Post-Stage	105+65	118+35	Inside	1270				
	PROJ	4740	904					

SCALE: NTS

SHEET 1

	PAVEMENT MARKING REMOVAL SCHEDULE									
			PAVEMENT							
			MARKING							
			REMOVAL -	DESCRIPTION						
			WATER							
			BLASTING							
STAGE	FROM	TO	X0327980							
STAGE	STA	STA	SQ FT							
NORTHBOUND	63+27	107+75	462.50	WHITE 30' SKIP-10' DASH, LANE LINE						
NORTHBOUND	98+59	108+22	160.67	6' SKIP-2' DASH, LANE DROP AT ENTRANCE RAMP						
NORTHBOUND	98+75	108+22	631.33	WHITE SOLID OUTSIDE EDGE ALONG RAMP						
NORTHBOUND	103+85	107+74	130.00	YELLOW SOLID INSIDE EDGE						
NORTHBOUND	114+40	116+30	63.67	YELLOW SOLID INSIDE EDGE						
NORTHBOUND	108+22	116+50	276.00	WHITE SOLID OUTSIDE EDGE						
SOUTHBOUND	105+45	118+15	423.33	WHITE SOLID OUTSIDE EDGE						
SOUTHBOUND	105+65	107+58	64.00	YELLOW SOLID INSIDE EDGE						
SOUTHBOUND	114+27	118+15	129.33	YELLOW SOLID INSIDE EDGE						
SOUTHBOUND	114+25	161+50	491.67	WHITE 30' SKIP-10' DASH, LANE LINE						
	PROJ	ECT TOTAL:	2833							

TEMPORARY ATTENUATOR SCHEDULE					
		IMPACT ATTENUATORS,	IMPACT ATTENUATORS,		
		TEMPORARY	RELOCATE (NON-		
		(NON-REDIRECTIVE),	REDIRECTIVE,		
		TEST LEVEL 3	TEST LEVEL 3		
STAGE		70600250	70600350		
STAGE	STA	EACH	EACH		
SB STAGE 1	116+00	1			
SB STAGE 2	116+00		1		
NB STAGE 1 106+00		1			
NB STAGE 2 106+00			1		
PF	ROJECT TOTAL:	2	2		

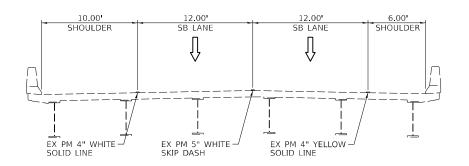
REFLECTOR REPLACEMENT SCHEDULE				
			REPLACEMENT REFLECTOR	
STAGE	FROM	то	78100300	
STAGE	STA	STA	EACH	
NORTHBOUND 63+27		107+50	112	
SOUTHBOUND	60			
PROJECT TOTAL: 172				

	PERMANENT PAVEMENT MARKING SCHEDULE												
			THERMOPLASTIC PAVEMENT MARKING - LINE 4"	THERMOPLASTIC PAVEMENT MARKING - LINE 8"	PLASTIC PAVEMENT	PREFORMED PLASTIC PAVEMENT MARKING, TYPE B - INLAID - CONTRAST - LINE 7"	PREFORMED PLASTIC PAVEMENT MARKING, TYPE B- INLAID - LINE 8"	MODIFIED URETHANE PAVEMENT MARKING - LINE 4"	RAISED REFLECTIVE PAVEMENT MARKER	GROOVING FOR RECESSED PAVEMENT MARKING 6"	GROOVING FOR RECESSED PAVEMENT MARKING 8"	GROOVING FOR RECESSED PAVEMENT MARKING 9"	DESCRIPTION
STAGE	FROM	TO	78000200	78000500	78004355	X7800956	78004358	78000004	78100100	X7830072	X2700003	X7830076	
	STA	STA	FOOT	FOOT	FOOT	FOOT	FOOT	FOOT	EACH	FOOT	FOOT	FOOT	
NORTHBOUND	63+27	109+06			1,150					1,150			WHITE 30' SKIP-10' DASH, LANE LINE
NORTHBOUND	98+59	108+22					242					242	6' SKIP-2' DASH, LANE DROP AT ENTRANCE RAMP
NORTHBOUND	98+75	108+22		950									WHITE SOLID OUTSIDE EDGE ALONG RAMP
NORTHBOUND	104+05	109+03	498										YELLOW SOLID INSIDE EDGE
NORTHBOUND	107+70	114+40							18				RAISED REFLECTIVE MARKERS ON OVERLAY
NORTHBOUND	108+22	109+08	86										WHITE SOLID OUTSIDE EDGE
NORTHBOUND	109+03	113+13						411					YELLOW SOLID INSIDE EDGE
NORTHBOUND	109+06	113+16				100					100		WHITE 30' SKIP-10' DASH, LANE LINE
NORTHBOUND	109+08	113+18						411					WHITE SOLID OUTSIDE EDGE
NORTHBOUND	113+13	116+30	317										YELLOW SOLID INSIDE EDGE
NORTHBOUND	113+16	114+70			40					40			WHITE 30' SKIP-10' DASH, LANE LINE
NORTHBOUND	113+18	116+50	333										WHITE SOLID OUTSIDE EDGE
SOUTHBOUND	105+45	108+83	337										WHITE SOLID OUTSIDE EDGE
SOUTHBOUND	105+65	108+88	323										YELLOW SOLID INSIDE EDGE
SOUTHBOUND	107+26	108+86			40					40			WHITE 30' SKIP-10' DASH, LANE LINE
SOUTHBOUND	107+50	114+20							18				RAISED REFLECTIVE MARKERS ON OVERLAY
SOUTHBOUND	108+83	112+96						412					WHITE SOLID OUTSIDE EDGE
SOUTHBOUND	108+86	112+98				100					100		WHITE 30' SKIP-10' DASH, LANE LINE
SOUTHBOUND	108+88	113+01						412					YELLOW SOLID INSIDE EDGE
SOUTHBOUND	112+96	118+15	518										WHITE SOLID OUTSIDE EDGE
SOUTHBOUND	112+98	161+50			1,210					1,210			WHITE 30' SKIP-10' DASH, LANE LINE
SOUTHBOUND	113+01	118+15	514										YELLOW SOLID INSIDE EDGE
	PROJ	ECT TOTAL:	2,926	950	2,440	200	242	1,646	36	2,440	200	242	

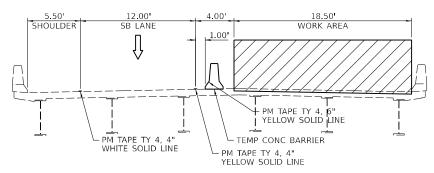
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PLOT DATE = 6/28/2021	DATE - 06/28/21	REVISED -

STATE OI	F ILLINOIS
DEPARTMENT OF	TRANSPORTATION

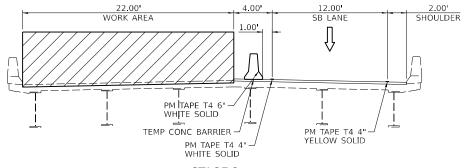
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SCHEDULE OF QUANTITIES		57	2020-170-BR	COOK	89	9
SCHEDOLL OF GOANTITIES				CONTRACT	NO. 62	M30
1 OF 1 SHEETS STA	TO STA.		TILINOIS EED AT	D PPO IECT		



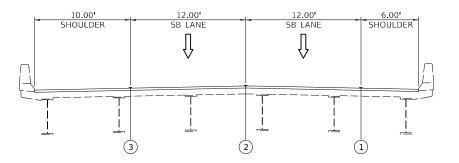
EXISTING CONDITIONS



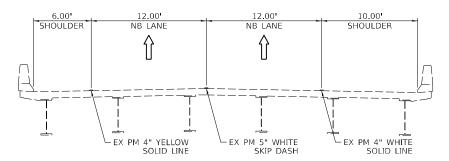
STAGE 1



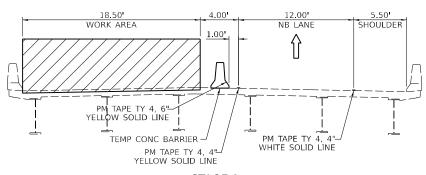
STAGE 2
SB INTERSTATE 57



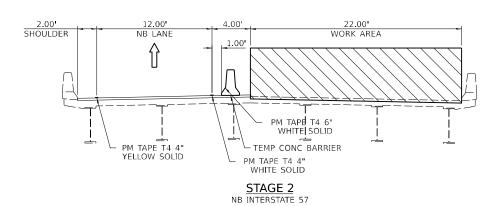
FINAL CONDITIONS SB INTERSTATE 57



EXISTING CONDITIONS



STAGE 1 NB INTERSTATE 57



10.00' SHOULDER 12.00' NB LANE 12.00' NB LANE SHOULDER

FINAL CONDITIONS

NB INTERSTATE 57

PERMANENT PAVEMENT MARKING LEGEND

ON HMA ON LATEX CONCRETE OVERLAY

1 THERMOPLASTIC PM 4" YELLOW SOLID LINE

NOTES:

1. NORTHBOUND AND SOUTHBOUND STAGES MAY BE SIMULTANEOUS OR INDEPENDENT OF ONE ANOTHER.

TEMPORARY CONCRETE BARRIERS AND IMPACT ATTENUATORS SHALL NOT BE PINNED INTO THE BRIDGE DECK, SLEEPER SLAB, OR APPROACH SLAB.

MODIFIED URETHANE PM 4" YELLOW SOLID LINE

PR THERMOPLASTIC PM 5" WHITE SKIP DASH

PREFORMED PLASTIC PM 7" TYPE B - INLAID - CONTRAST - SKIP DASH

GROOVING FOR RECESSED PAVEMENT MARKING 6"

GROOVING FOR RECESSED PAVEMENT MARKING 8"

3 THERMOPLASTIC PM 4" WHITE SOLID LINE

MODIFIED URETHANE PM 4" WHITE SOLID LINE

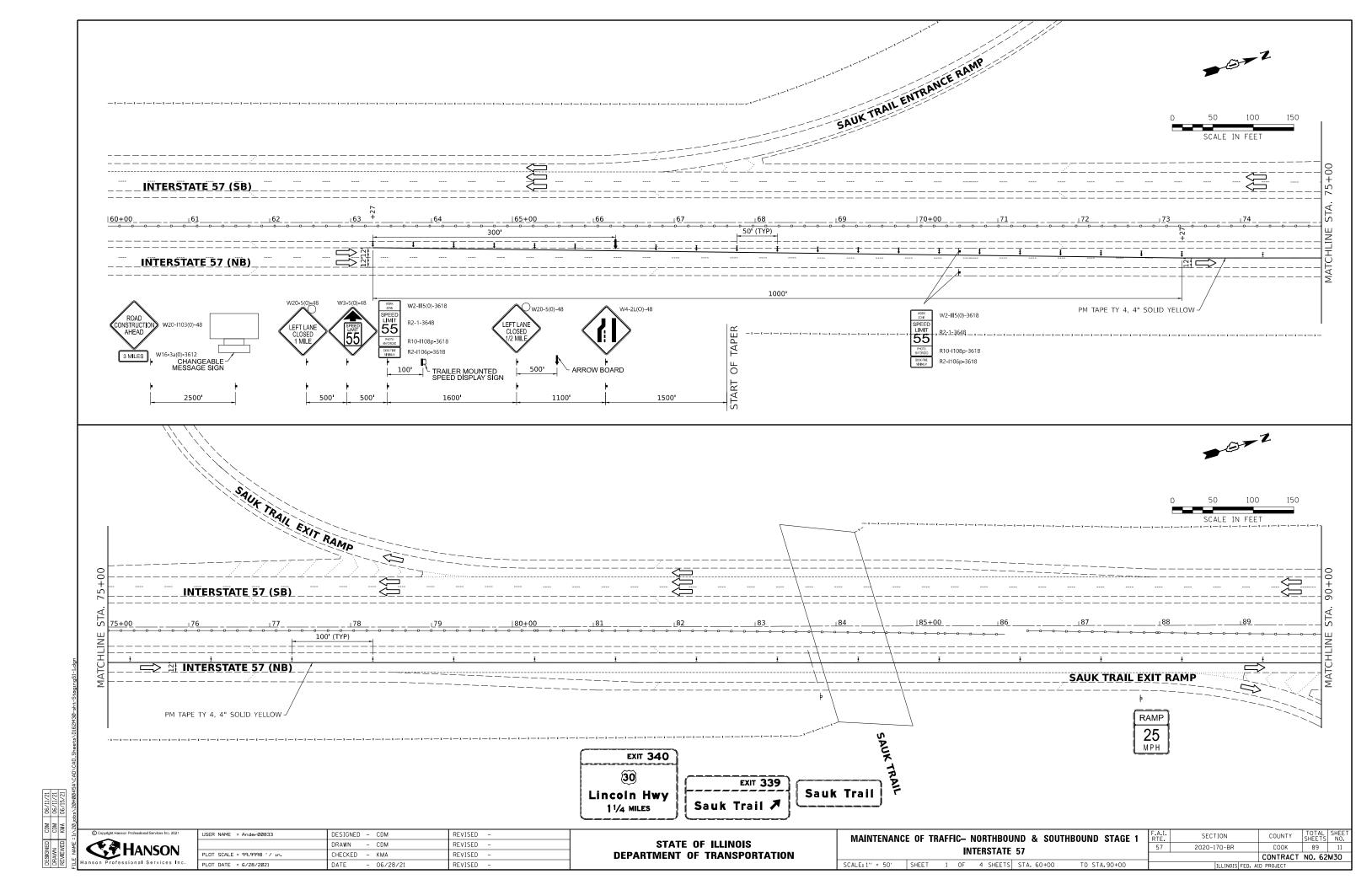


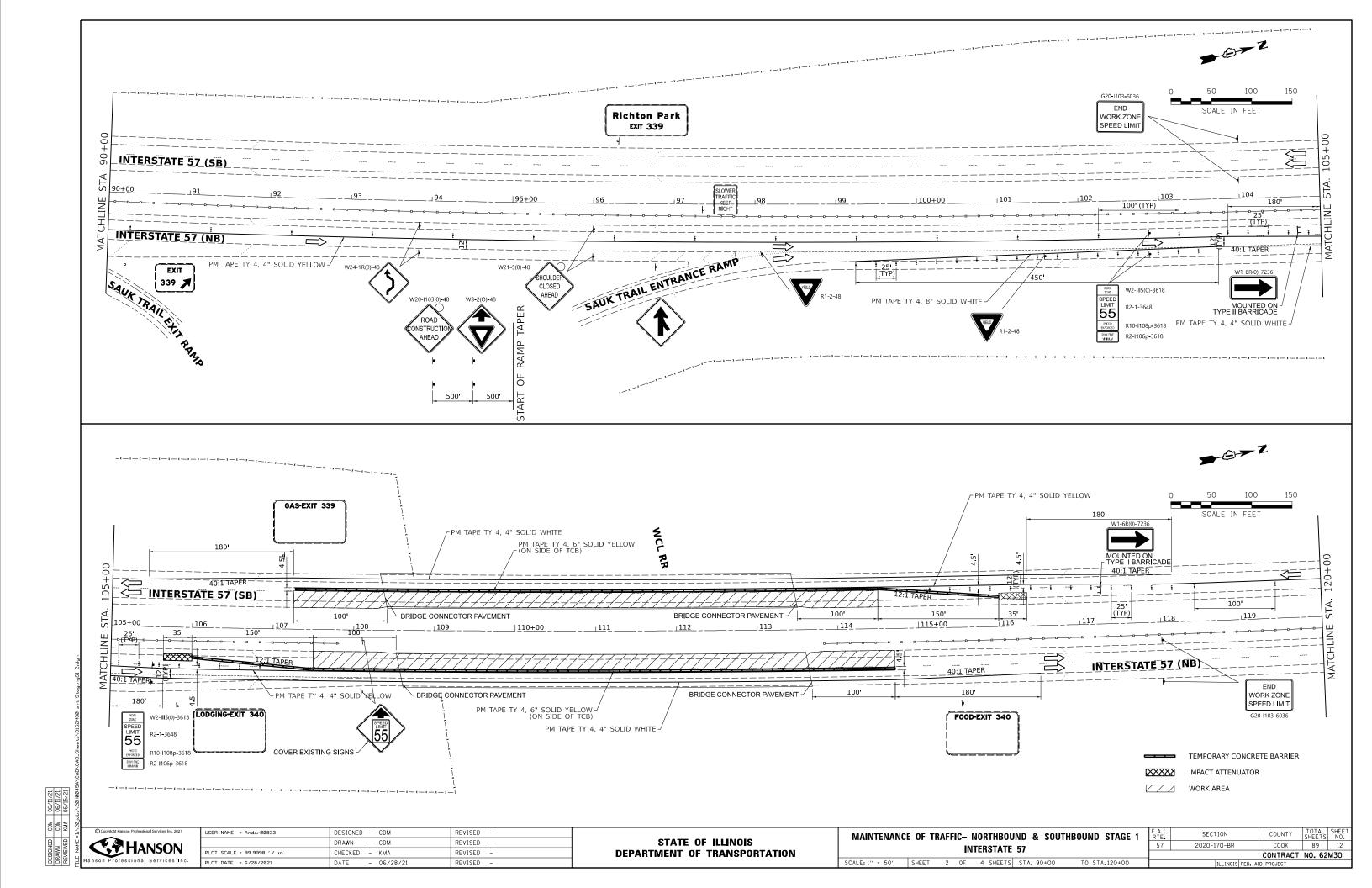
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PLOT DATE = 6/28/2021	DATE - 06/28/21	REVISED -

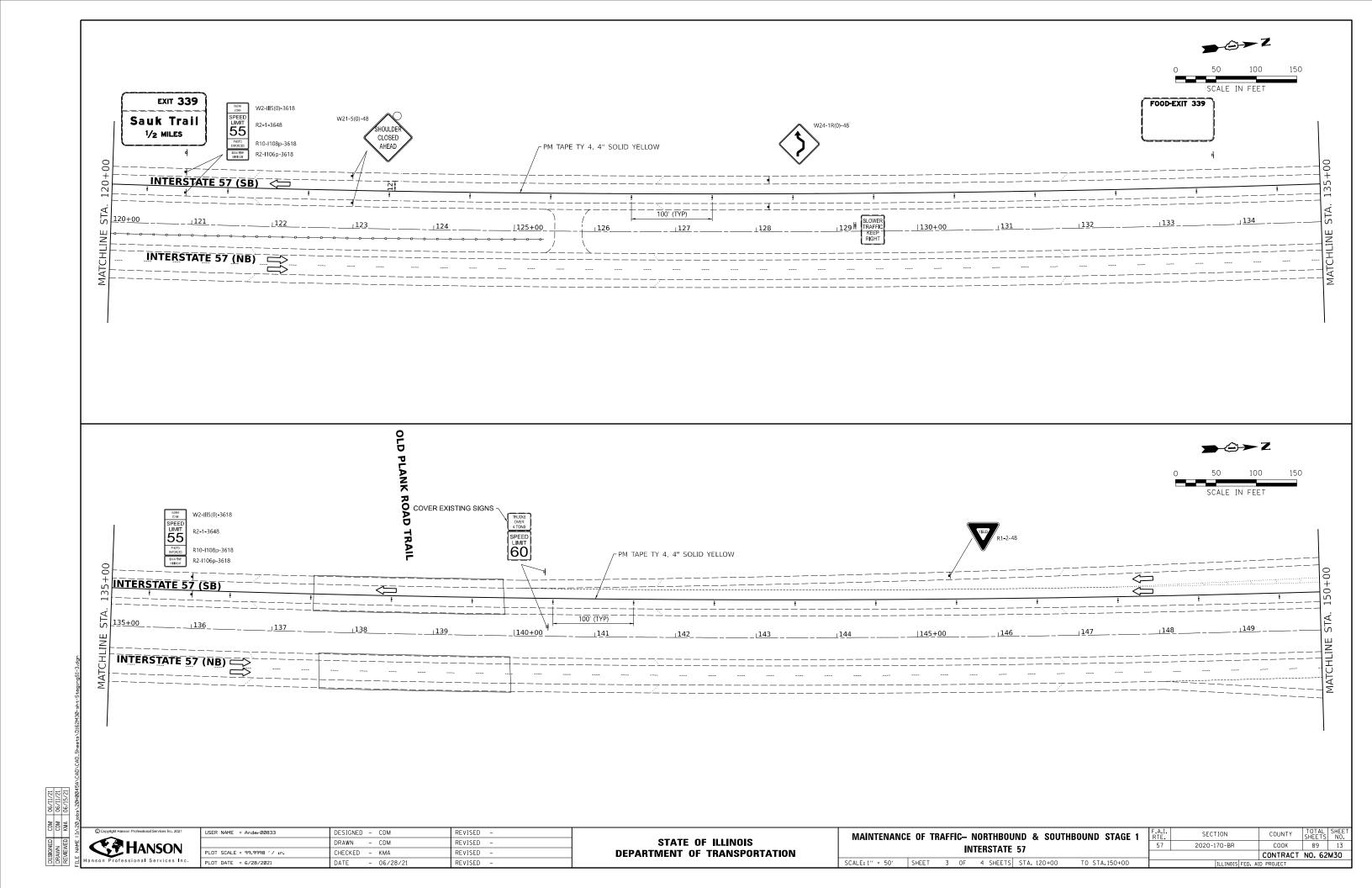
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

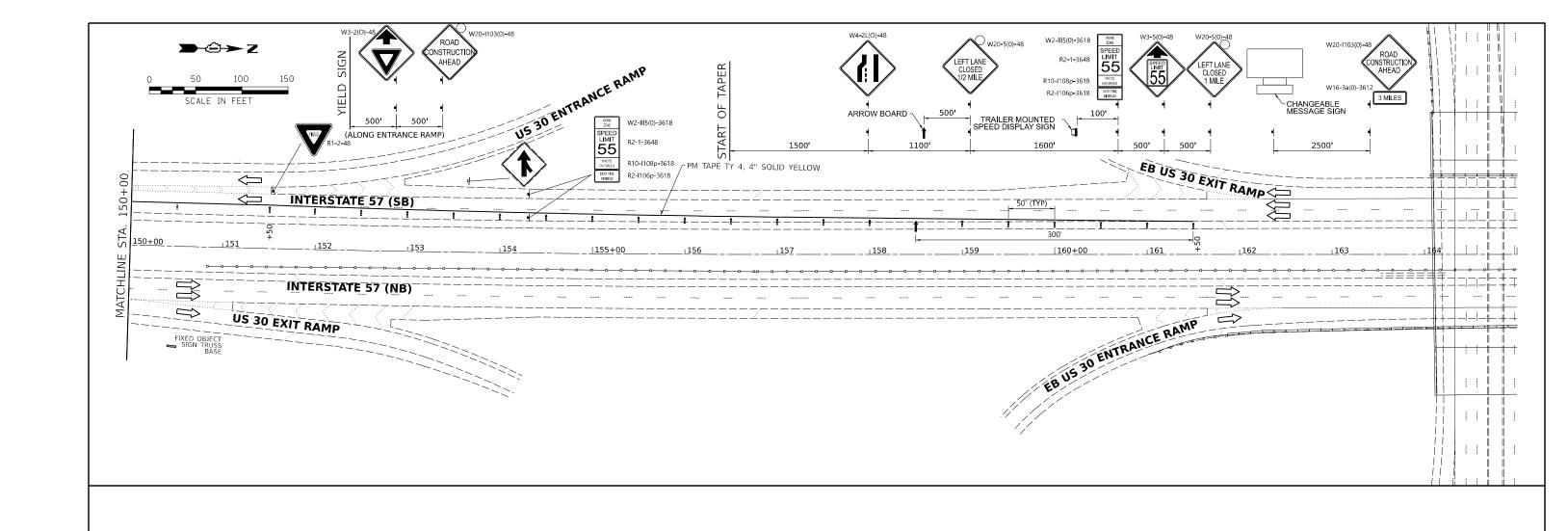
MAINTENANCE OF TRAFFIC— TYPICAL SECTION RT	.A.I.		
INTERSTATE 57			
INTERSTATE 37			

2020-170-BR 89 10 CONTRACT NO. 62M30 SCALE: NTS SHEET 1 OF 1 SHEETS STA. TO STA.









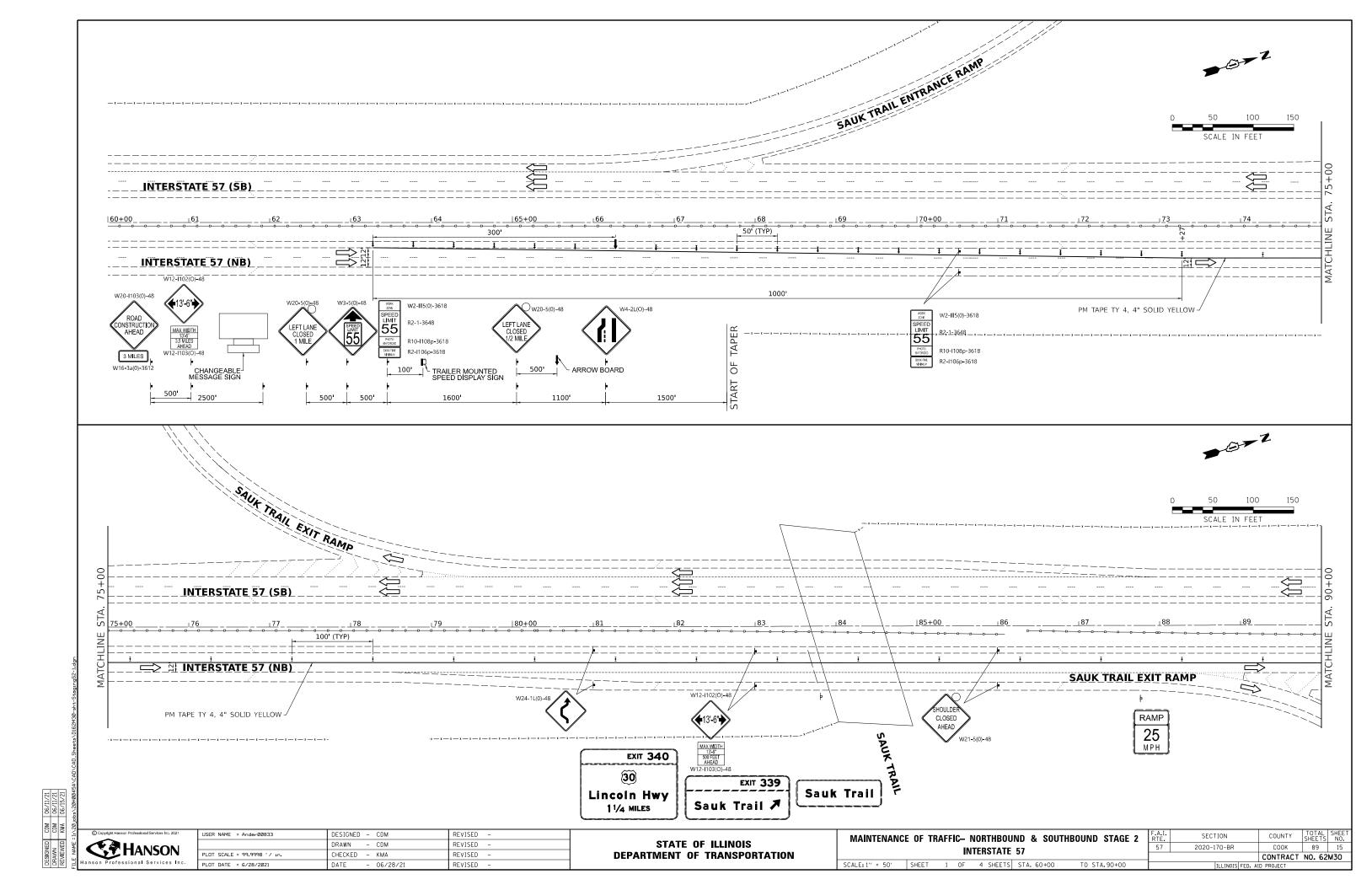
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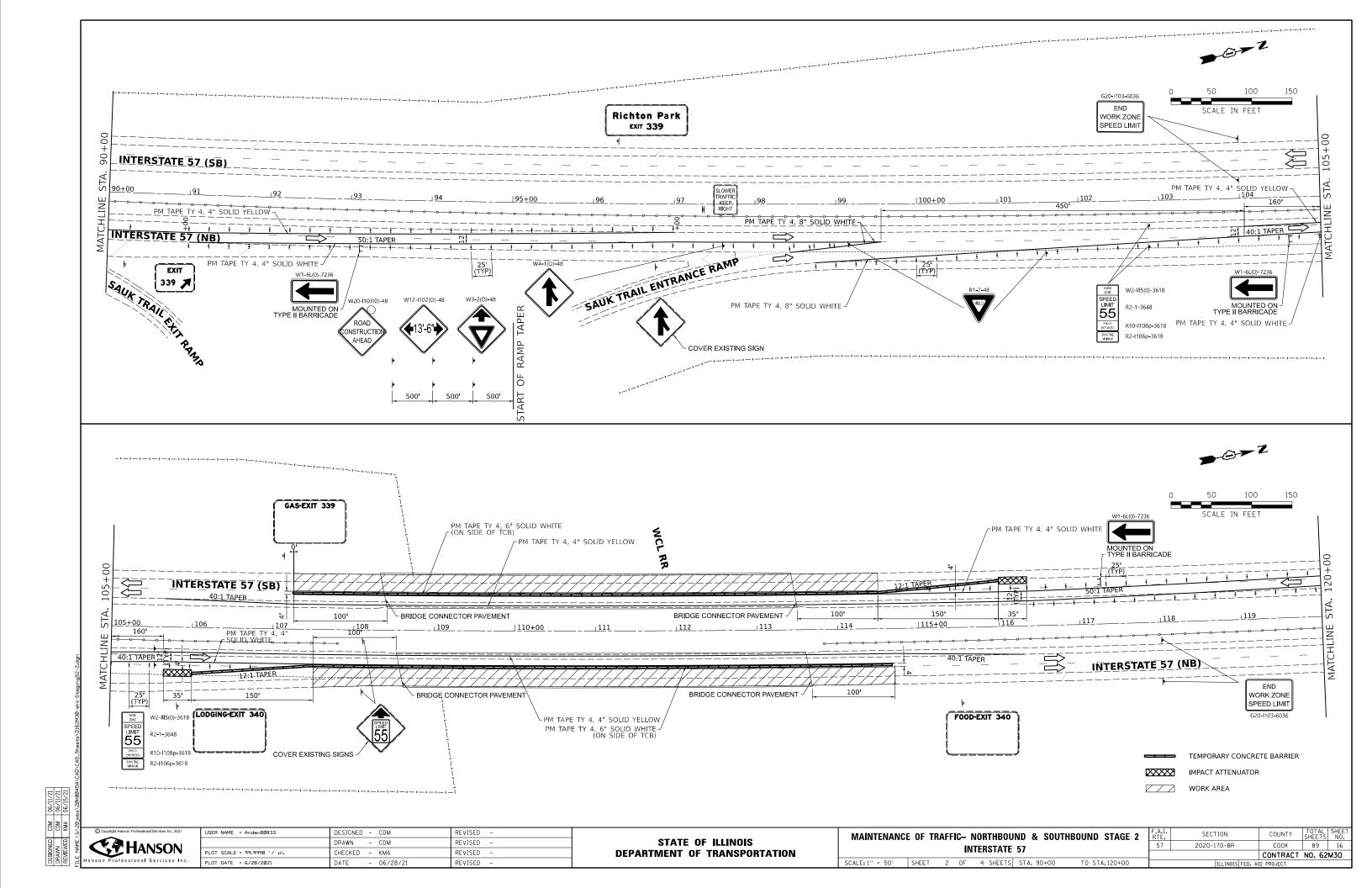
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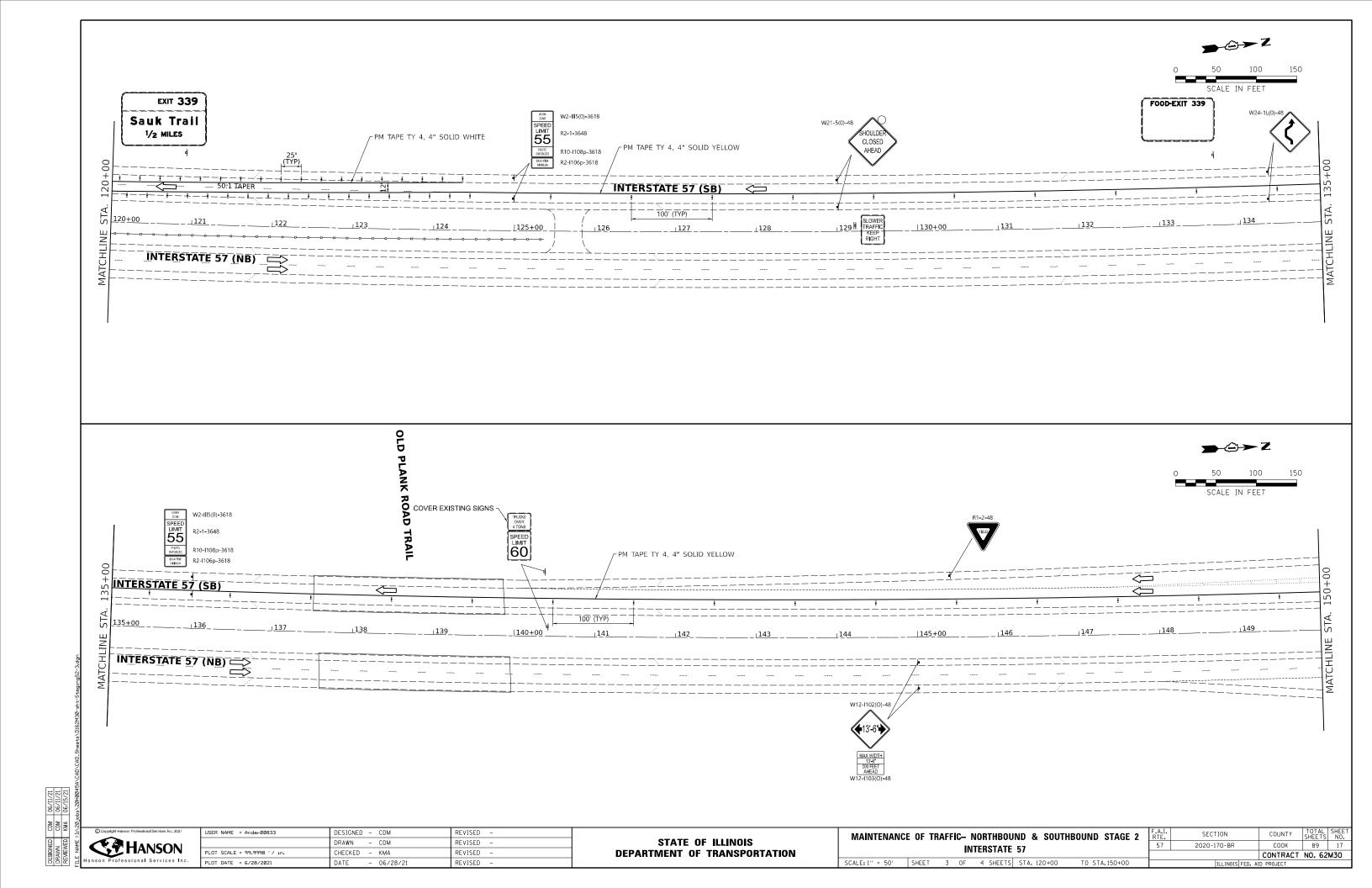
STATE OF	ILLINOIS
DEPARTMENT OF	TRANSPORTATION

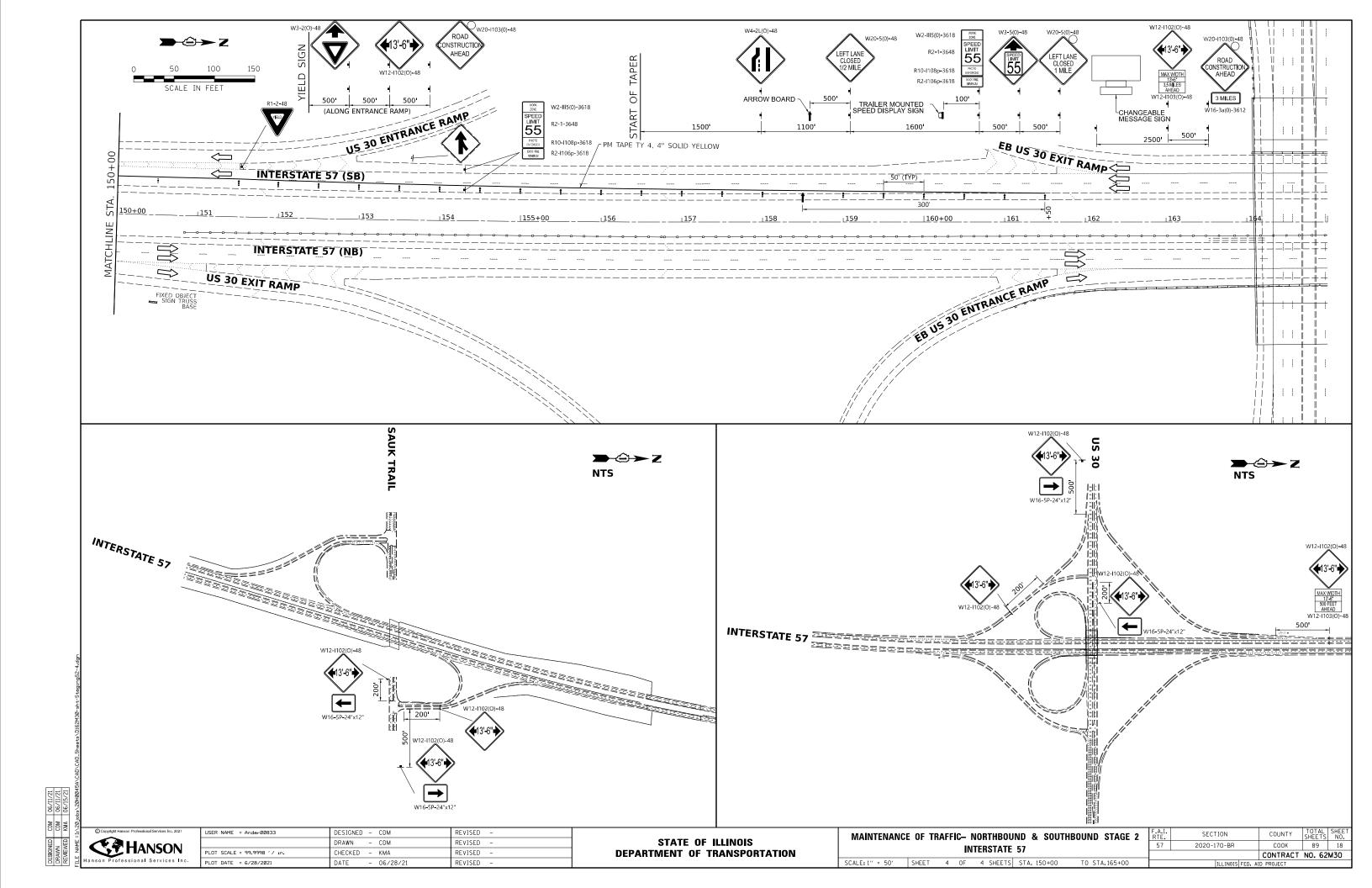
MAINTENANC	E OF TE	RAFF	IC- I	NORTHBOU	ND & SOU	THBOUND S	TAGE 1	F.A.I. RTE.	SECTION	
			IN	TERSTATE	57			57	2020-170-BR	
			Į.v	ILIIOIAIL	37					1
CALE: 1" = 50"	SHEET	4	OF	4 SHEETS	STA. 150+00	TO STA.16	65+00		TILLINOIS FED. AI	ID.

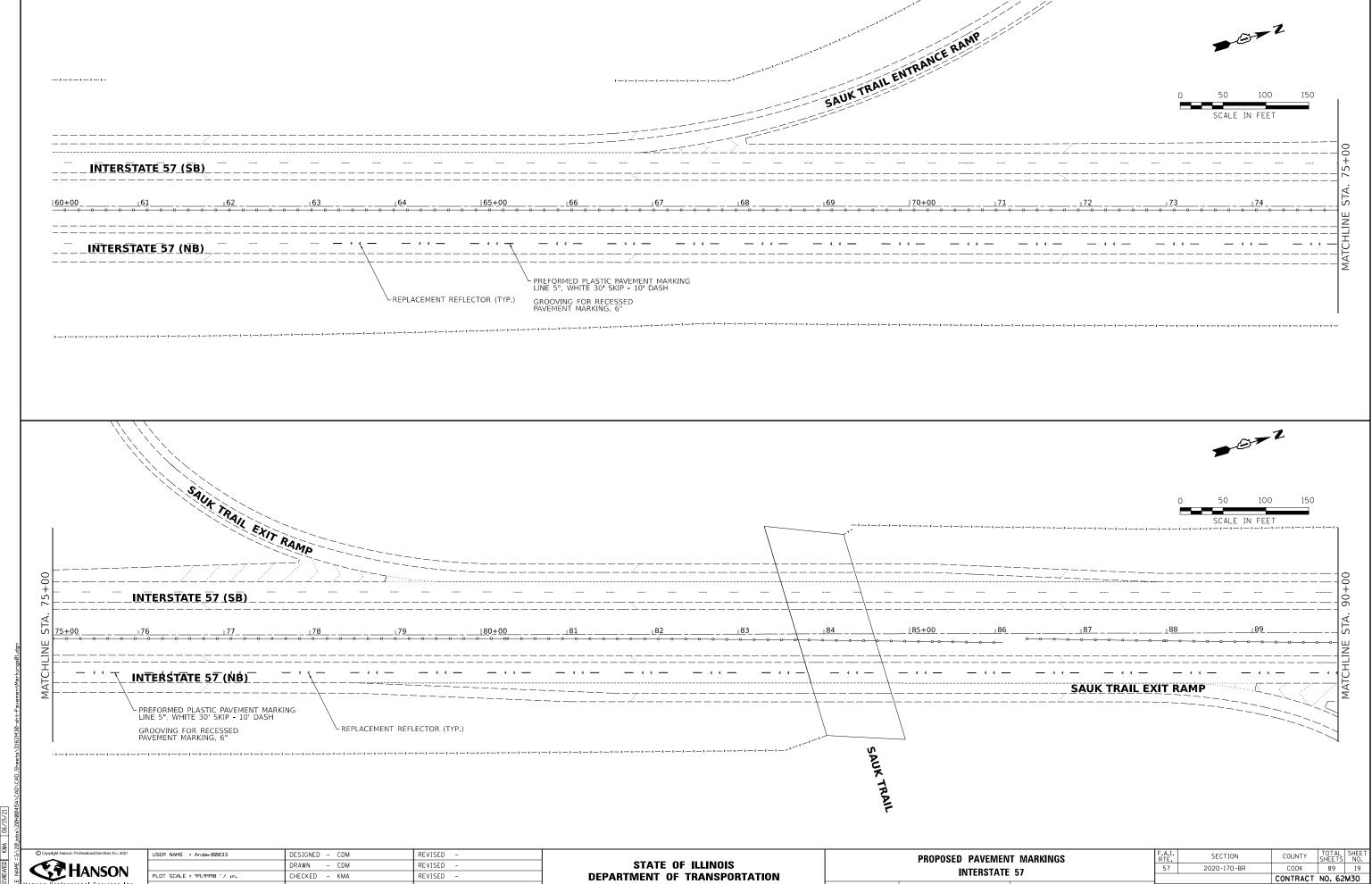
	ILLINOIS FED. A	ID PROJECT		
!				
		CONTRACT	NO. 62	M30
57	2020-170-BR	COOK	89	14
RTE.	SECTION	COUNTY	SHEETS	NO.









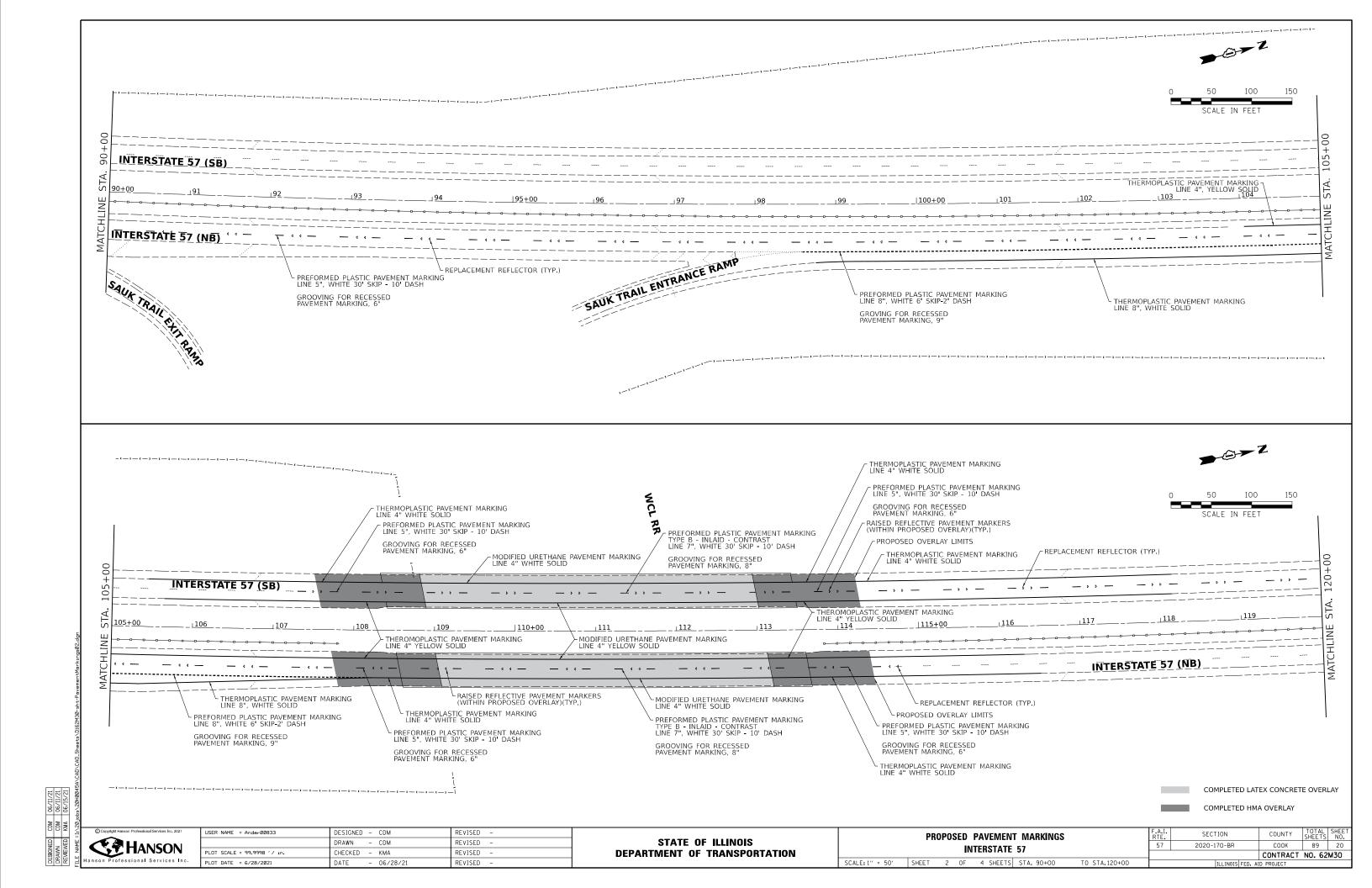


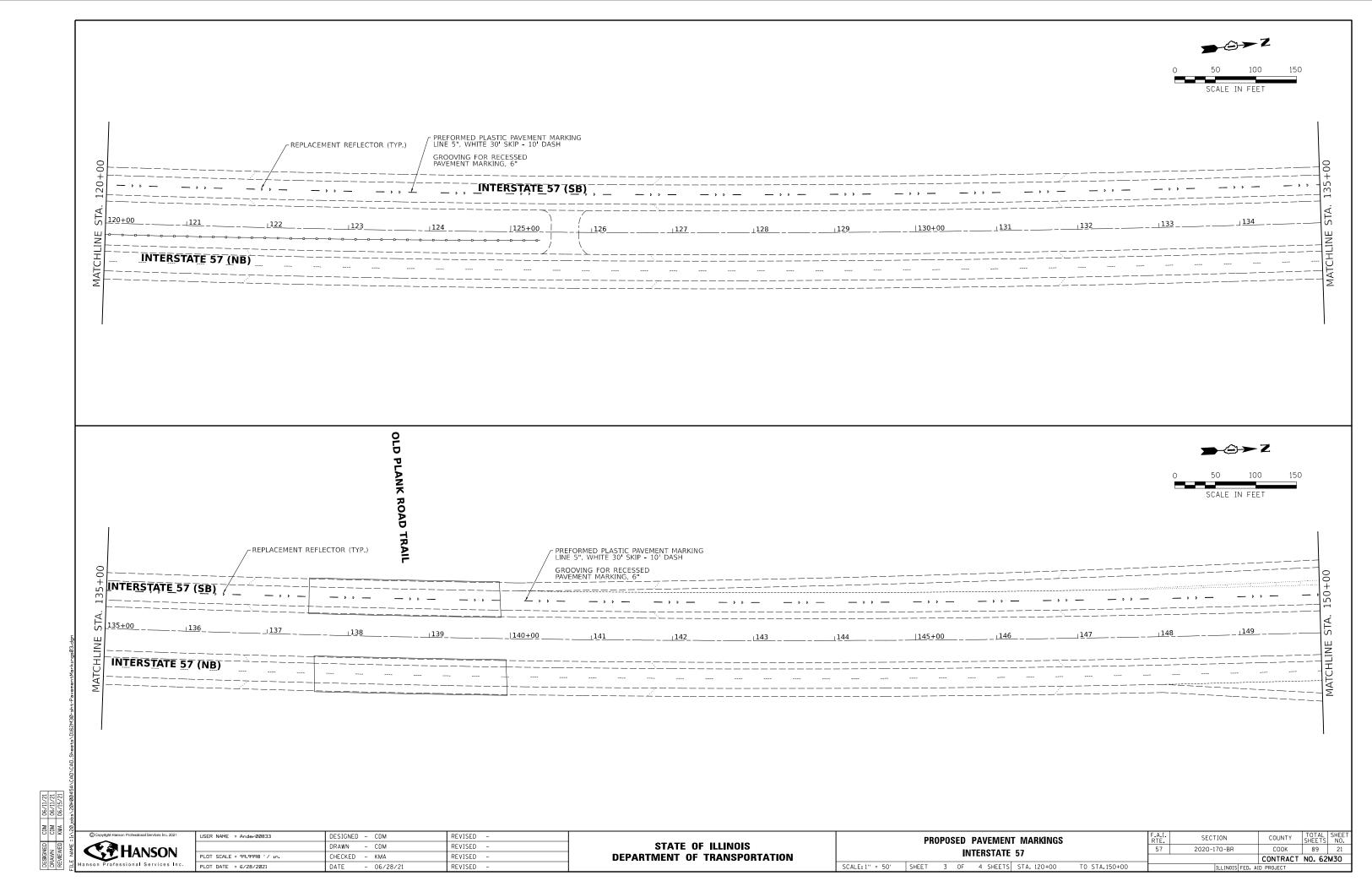
SCALE: 1" = 50' SHEET 1 OF 4 SHEETS STA. 60+00

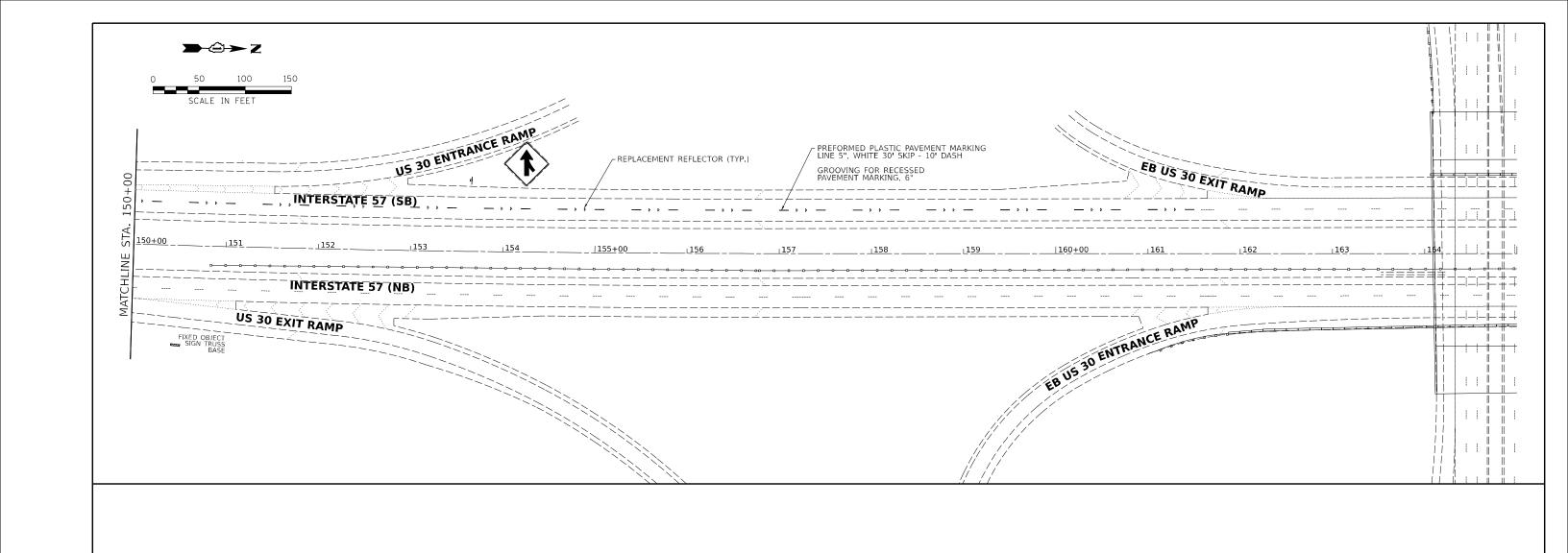
 DESIGNED
 CDM
 06/11/21

 DRAWN
 CDM
 06/11/21

 REVIEWED
 KMA
 06/15/21







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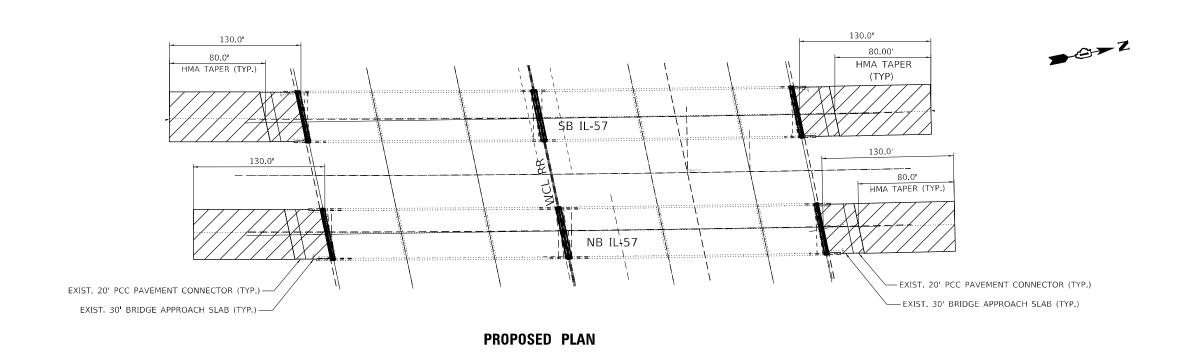
HANSON

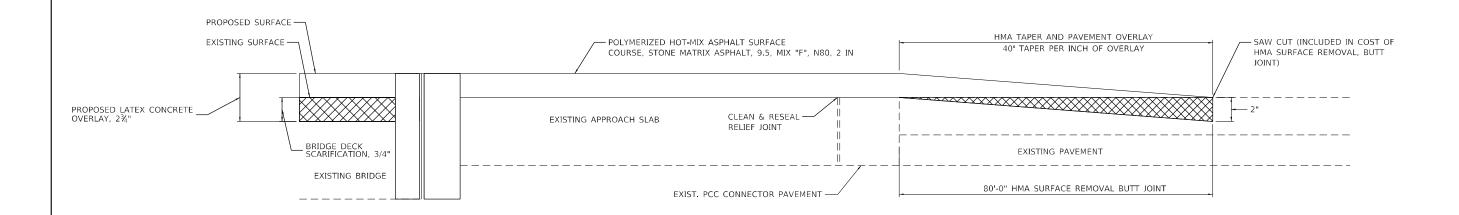
Hanson Professional Services Inc.

USER NAME = AnderØØ833	DESIGNED - CDM	REVISED -
	DRAWN - CDM	REVISED -
PLOT SCALE = 100.0000 '/ in.	CHECKED - KMA	REVISED -
PLOT DATE = 6/28/2021	DATE - 06/28/21	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

	PROPOS	ED PAVEMEN	T MARKINGS		F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
		INTERSTATE	57		57	2020-170-BR	COOK	89	22
		INTERISTALE	37				CONTRACT	NO. 62	M30
SCALE: 1" = 50" S	SHEET 4	OF 4 SHEETS	STA. 120+00	TO STA.150+00		ILLINOIS FED. A	ID PROJECT		





LEGEND

APPROACH PAVEMENT SMA OVERLAY DETAIL

NOTE

 \bigotimes

HMA SURFACE REMOVAL, BUTT JOINT

POLYMERIZED HOT-MIX ASPHALT SURFACE COURSE, STONE MATRIX ASPHALT, 9.5, MIX "F", N80, 2 IN



BRIDGE DECK SCARIFICATION, 3/4"

HBM ENGINEERING GROUP LICE

USER NAME = all.issa	DESIGNED	-	JSA	REVISED -
	DRAWN	-	JSA	REVISED -
PLOT SCALE = 80.0015 / in.	CHECKED	-	RTB	REVISED -
PLOT DATE = 8/11/2021	DATE	-	8/11/2021	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ROADWAY PLAN AND DETAIL	F.A.I. RTE.	SEC [*]	ΓΙΟΝ		COUNTY	TOTAL SHEETS	
I-57 OVER WCL RR	57	2020-170-BR COOK		соок	89	23	
I-37 OVER VVOL IIII					CONTRACT	NO. 62	2M30
SCALE: 80.0015 / in SHEET 1 OF 1 SHEETS STA. TO STA.			ILLINO15	FED. AI	D PROJECT		

REMOVING AND RE-ERECTING THE GUARDRAIL IS INCLUDED IN THE COST OF DRAINAGE SCUPPERS TO BE ADJUSTED (SEE SHEETS 31

S.N. 016-0048 was originally build in 1966 and reconstructed/widened in 1996. The structure has a back-to-back SCOPE OF WORK Structure: abutment length of 415'-4" along the tangent line at Sta. 111+77.31 and an out-to-out deck width of 43'-2". The 1. Provide Protective shield within limits superstructure consists of a 7½" thick reinforced concrete deck supported on three span continuous steel beams indicated on the plans. of span lengths 60'-11", 73'-3" and 60'-11" in Unit I and 82'-1", 82'-10" and 48'-8" in Unit II. The substructure consists of reinforced concrete abutments supported on reinforced concrete piles and reinforced concrete piers 2. Scarify $\frac{3}{4}$ " from the bridge deck slab. on timber piles. 3. Perform Deck and Approach Slab Repairs. Traffic is to be maintained utilizing stage construction. 4. Reconstruct Expansion Joints at the South No salvage. and North abutments and at Pier 3 and 196'-71/8" 215'-1" install new preformed joint strip seals. Unit 2 Unit 1 5. Apply a 2¾" Bridge Deck Latex Concrete 30'-0" 60'-11" 73'-3" 60'-11" 82'-1" 82'-10" 48'-8" 30'-0" Approach slab Span 1 Span 2 Span 3 Span 4 Span 5 Span 6 Approach slab Overlay on Bridge Deck and see Roadway Plans for 2" Stone-Matrix Asphalt (SMA) 50'-0" Overlay on the approach Slabs. Reconstruct exist.-Reconstruct exist. Reconstruct exist. Limits of expansion joint expansion joint expansion joint Exist. W36 protective shield 6. Perform Bridge Deck Grooving. – Ç Brg. N. Abut. ¢ Brg. S. Abut. — Apply protective coat to the top of € Pier 4 – € Pier 5 – © Pier 1 @ Pier 2 @ Pier 3reconstructed transverse joint areas and Perform Structural top and inside faces of parapets and top of Perform Structural - Perform Structural Perform Structural 22'-0"± Repair of Concrete - Perform Slopewall Latex Overlay. Repair of Concrete Repair of Concrete Repair of Concrete (typ. at South and Repair (typ. at at Pier 3 at Pier 1 North Abutments) at Pier 2 both abutments) 8. Perform Structural steel beam end repairs, and replace diaphragm as shown on the Exist. Creosoted Perform Structural WCL Railroad plans. Repair of Concrete Piles, typ. Perform Structural Repair of Concrete at Pier 5 Exist. Ground Line 9. Perform structural concrete repairs for the at Pier 4 abutments and piers as noted on the plans. ELEVATION 10. Apply Concrete Sealer at North and South Abutments and at Pier 3 as shown on the plans. 11. Perform Slope Wall repairs. 30'-0" 415'-4" (Bk. to Bk. Abutments along tangent) 30'-0" Approach Slab Approach Slab 48'-8" 12. Clean all deck drains. 60'-11" 73'-3" 82'-10" Span 1 Span 2 Span 3 Span 4 Span 5 Span 6 -WCL Railroad Apply 2" Stone-Matrix Asphalt Bridge name (SMA) Overlay, typ. both ends. plates, see @ WCL ROW -Exist. deck For SMA items, see Roadway Plans. Sheet S1-15 Perform Bridge drain to be'. Deck Grooving cleaned, typ. 11°3′16″ Local Tangent at € S. Brg. Pier 3 € N. Brg. Pier 3 Sta. 111+77.31 Sta. 110+55.35 Sta. 110+53.42 @ Pier 1 © Pier 2 © Pier 3 ⊊ SB I-57 Roadway 🤈 î Pier 4 @ Pier 5 Sta. 109+18.95 Sta. 109+92.50 Sta. 110+54.37 Sta. 111+37.43 Sta. 112+20.44 Range 13E, 3rd P.M. Bk. of S. Abut. Perform ¾" Bridge Deck -Bk. of N. Abut. Reconstruct Exist. Scarification and apply $2\frac{3}{4}$ " Joint at Pier 3 111 Bridge Deck Latex Concrete Reconstruct Exist. Joint Overhead Overlay and Protective Coat 105+00 between Bridge Deck and 1 106+00 Increase 107+00 Power Lines 108+00 109+00 - Approach Slab (Typ. at both Abut. Joints) Structure Location LOCATION SKETCH 415'-4" (Bk. to Bk. Abutments along tangent) Roadway West Bridge SB 82'-1" 1'-6" GENERAL PLAN AND ELEVATION Tangent Sta. 111+77.31 I-57 SB OVER WCL RR ₩ Ç Pier 5 Moussa A. Isa G Brg. S. Abut. -F.A.I. ROUTE 57 ₩ ⊈ Brg. N. Abut. Signed MOUSSA A. SECTION 2020-170-BR Dr. Moussa A. Issa, S.E. Expires 11-30-2022 II. Lic. No. 081-005738 € F.A.I. Rt. 57 ISSA - Sta. 111+77.31 081-005738 ★ Curve C COOK COUNTY ⊊ Pier 2 Local Tangent CHICAGO. Date <u>08/11/</u>2021 Sta.111+77.31 STATION 111+77.31 EXISTING LAYOUT SKETCH S.N. 016-0048 DESIGNED - EBK REVISED SECTION COUNTY STATE OF ILLINOIS CHECKED - MAI, RTB REVISED -2020-170-BR COOK 89 24 **STRUCTURE NO. 016-0048** REVISED **DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 62M30 SHEET S1-01 OF S1-30 SHEETS - 8/11/2021 REVISED .

GENERAL NOTES

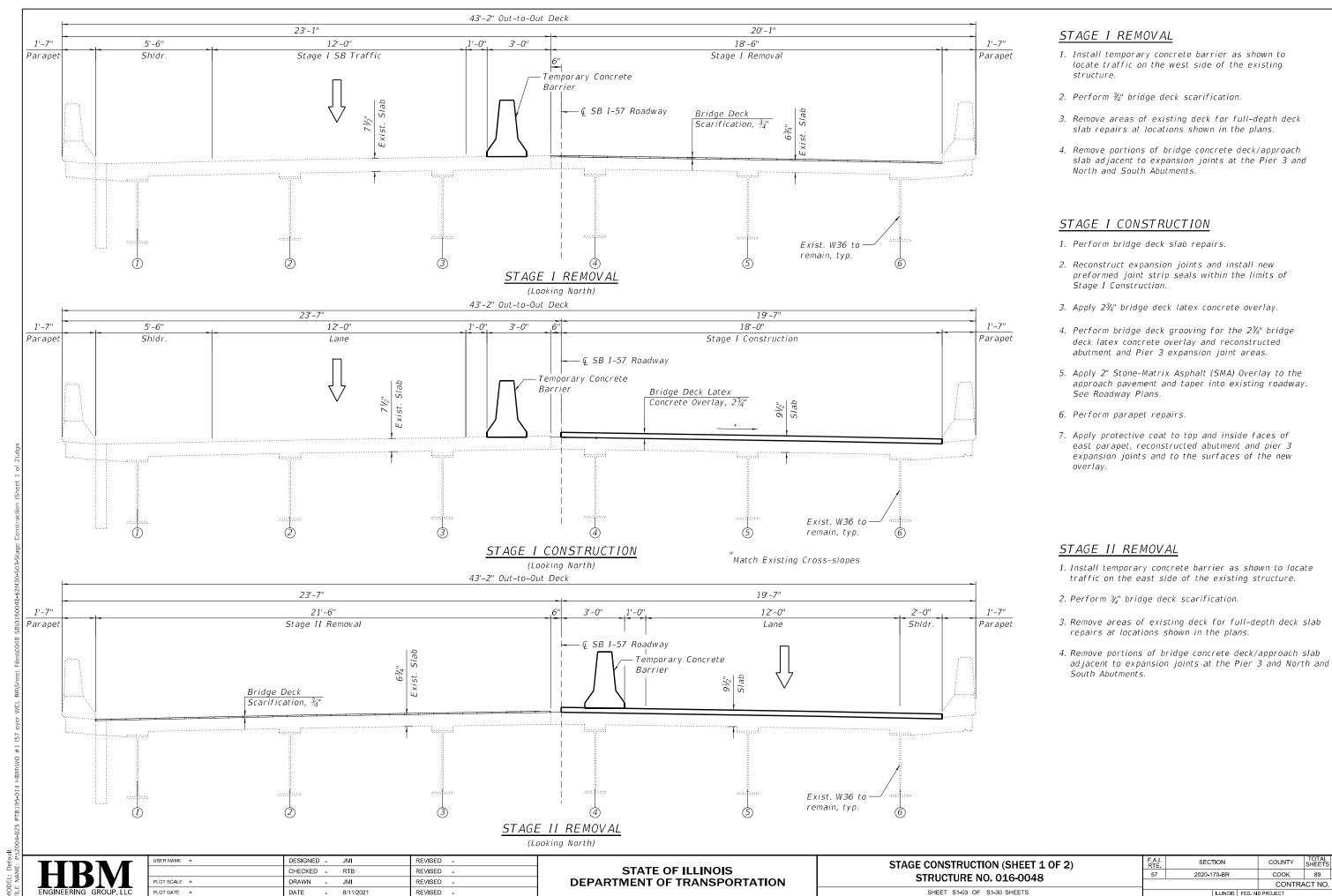
- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 3. Bars noted thus, 3x2-#5, indicates 3 lines of #5 bars with 2 lengths of bars per line.
- 4. For SMA overlay on Approach Slab, see Civil Sheets.
- 5. Protective Coat shall be applied to the top of reconstructed transverse joint areas and top and inside faces of parapets and top of Latex Overlay.
- 6. Concrete Sealer shall be applied to the abutments and Pier 3 at locations shown in the plans.
- 7. Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.
- 8. Prior to pouring the new concrete deck for expansion joint reconstruction and deck slab repairs, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding ¼ deep shall be identified and reported to the Bureau of Bridges and Structures for further dispositions. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 9. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 10. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning And Painting Contact Surface Areas Of Existing Steel Structures".
- 11. All new structural steel shall be hot-dip galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel".
- 12. Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts $\frac{3}{4}$ ", open holes $\frac{13}{16}$ ", unless otherwise noted.
- 13. Cost of cleaning existing drains shall not be paid separately, but shall be included with Bridge Deck Scarification.
- 14. The Contractor shall exercise extreme caution under the Aerial Power Lines. The Contractor is responsible to coordinate with the utilities to deenergize these lines if required. The Contractor shall make necessary adjustments to maintain adequate clearance to the Aerial Power Lines as specified in OSHA safety requirements.
- 15. All proposed beam end repair plates and bottom flange repair angles shall conform to the requirements of AASHTO M270 Grade 50. All proposed diaphragms and their connection angles shall conform to the requirements of AASHTO M270 Grade 36.

INDEX OF SHEETS

- S1-01 General Plan and Elevation
- S1-02 General Notes, Index of Sheets & Total Bill of Material
- S1-03 Stage Construction (Sheet 1 of 2)
- S1-04 Stage Construction (Sheet 2 of 2)
- S1-05 Temporary Concrete Barrier for Stage Construction
- S1-06 Deck Repair Plan (Sheet 1 of 2) S1-07 Deck Repair Plan (Sheet 2 of 2)
- S1-08 Drainage Scupper Adjustment Details
- S1-09 S. Abut. Joint Removal and Replacement (Sheet 1 of 2)
- S1-10 S. Abut. Joint Removal and Replacement (Sheet 2 of 2)
- 51-11 N. Abut. Joint Removal and Replacement (Sheet 1 of 2) 51-12 N. Abut. Joint Removal and Replacement (Sheet 2 of 2)
- S1-13 Pier 3 Joint Removal and Replacement (Sheet 1 of 2)
- S1-14 Pier 3 Joint Removal and Replacement (Sheet 2 of 2)
- S1-15 Parapet Repairs (Sheet 1 of 2)
- S1-16 Parapet Repairs (Sheet 2 of 2)
- S1-17 Preformed Joint Strip Seal
- S1-18 Framing Plan Steel Repairs (Sheet 1 of 2)
- S1-19 Framing Plan Steel Repairs (Sheet 2 of 2)
- S1-20 Structural Steel Repair Details (Sheet 1 of 2)
- S1-21 Structural Steel Repair Details (Sheet 2 of 2)
- S1-22 South Abutment Repairs
- S1-23 North Abutment Repairs
- S1-24 Pier 1 Repairs
- S1-25 Pier 2 Repairs
- S1-26 Pier 3 Repairs
- S1-27 Pier 4 Repairs
- S1-28 Pier 5 Repairs
- S1-29 Slope Wall Repairs
- S1-30 Bar Splicer Assembly and Mechanical Splicer Details

TOTAL BILL OF MATERIAL

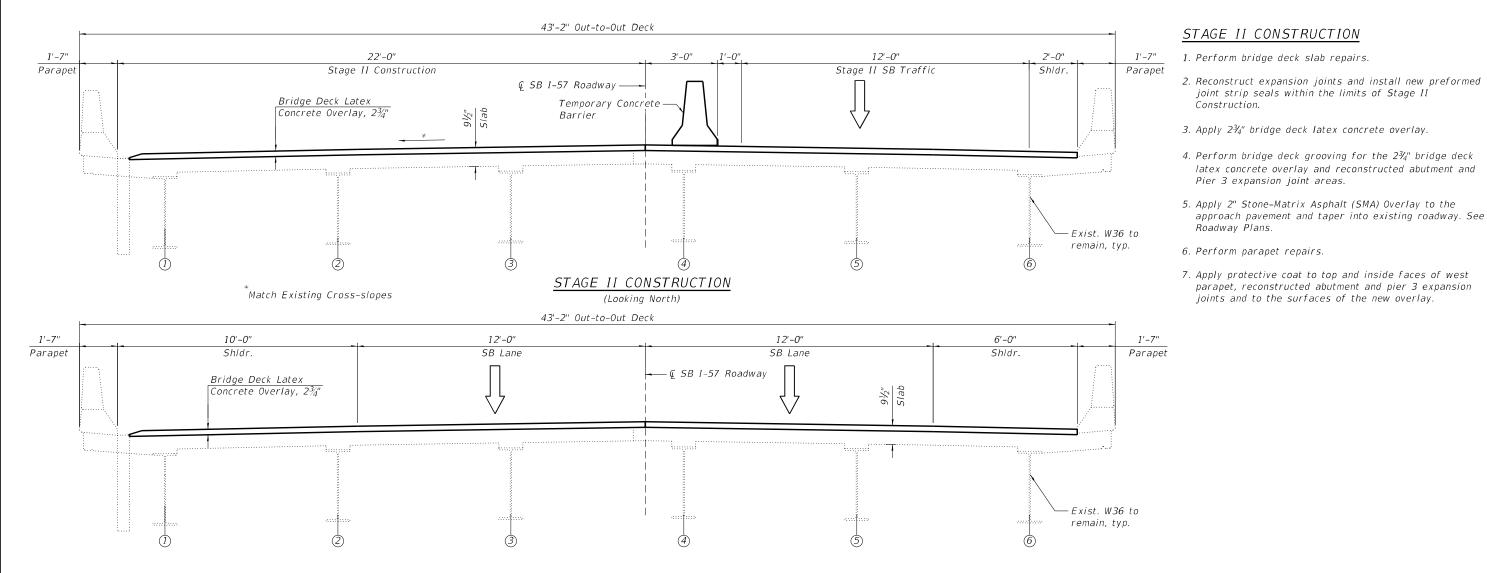
ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	Cu Yd	-	11	11
Concrete Removal	Cu Yd	19.5	-	19.5
Slope Wall Removal	Sq Yd	-	28	28
Protective Shield	Sq Yd	240	-	240
Concrete Superstructure	Cu Yd	22.0	-	22.0
Bridge Deck Grooving	Sq Yd	1705	-	1705
Protective Coat	Sq Yd	2212	-	2212
Furnishing And Erecting Structural Steel	Pound	2900	-	2900
Reinforcement Bars, Epoxy Coated	Pound	3750	-	3750
Bar Splicers	Each	56	-	56
Slope Wall 4 Inch	Sq Yd	-	28	28
Preformed Joint Strip Seal	Foot	132	-	132
Concrete Sealer	Sq Ft	-	384	384
Epoxy Crack Injection	Foot	-	119	119
Structural Steel Removal	Pound	2900	-	2900
Structural Steel Repair	Pound	820	-	820
Bridge Deck Latex Concrete Overlay, 2 3/4 Inches	Sq Yd	1795	-	1795
Bridge Deck Scarification 3/4"	Sq Yd	1795	-	1795
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	1	293	294
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	-	190	190
Deck Slab Repair (Full Depth, Type I)	Sq Yd	2.6	-	2.6
Deck Slab Repair (Full Depth, Type II)	Sq Yd	2.4	-	2.4
Drainage Scuppers To Be Adjusted	Each	2	-	2
Temporary Shoring And Cribbing	Each	5	-	5



TOTAL SHEE SHEETS NO.

89 26

CONTRACT NO. 62M30



FINAL DECK CROSS SECTION

(Looking North)

ENGINEEDING CROUP LLC

USER NAME =	DESIGNED	-	JMI	REVISED -
	CHECKED	-	RTB	REVISED -
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PLOT DATE =	DATE	-	8/11/2021	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** STAGE CONSTRUCTION (SHEET 2 OF 2) STRUCTURE NO. 016-0048 SHEET S1-04 OF S1-30 SHEETS

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
57	2020-170-BR		соок	89	27
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joint strip seals within the limits of Stage II

latex concrete overlay and reconstructed abutment and

approach pavement and taper into existing roadway. See

parapet, reconstructed abutment and pier 3 expansion

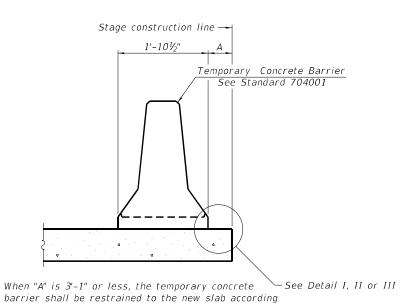
joints and to the surfaces of the new overlay.

Construction.

Roadway Plans.

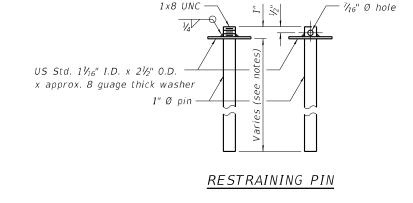
Pier 3 expansion joint areas.

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to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

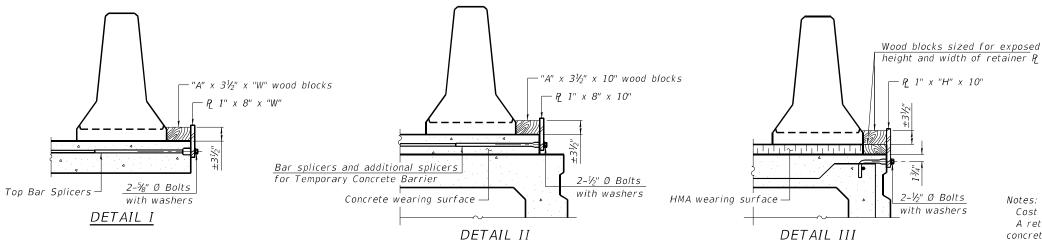
← Stage removal line ← Stage removal line 1'-101/2" 1'-101/2" Temporary Concrete Barrier See Standard 704001 6" min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint * When hot-mix asphalt wearng surface is present, embedment is required when "A" is greater than 3'-1".

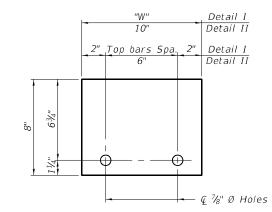


NEW SLAB OR NEW DECK BEAM

EXISTING SLAB

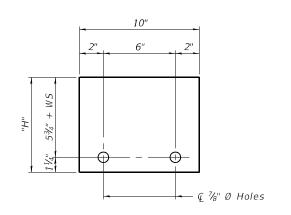
SECTIONS THRU SLAB OR DECK BEAM





STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)



shall be 3" plus the wearing surface depth.

EXISTING DECK BEAM

STEEL RETAINER P 1" x "H" x 10" (Detail III)

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate Q of each temporary concrete barrier.

BAR SPLICER FOR #4 BAR - DETAIL III

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

Detail I - Installation for a new bridge deck or bridge slab.

Detail II - Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

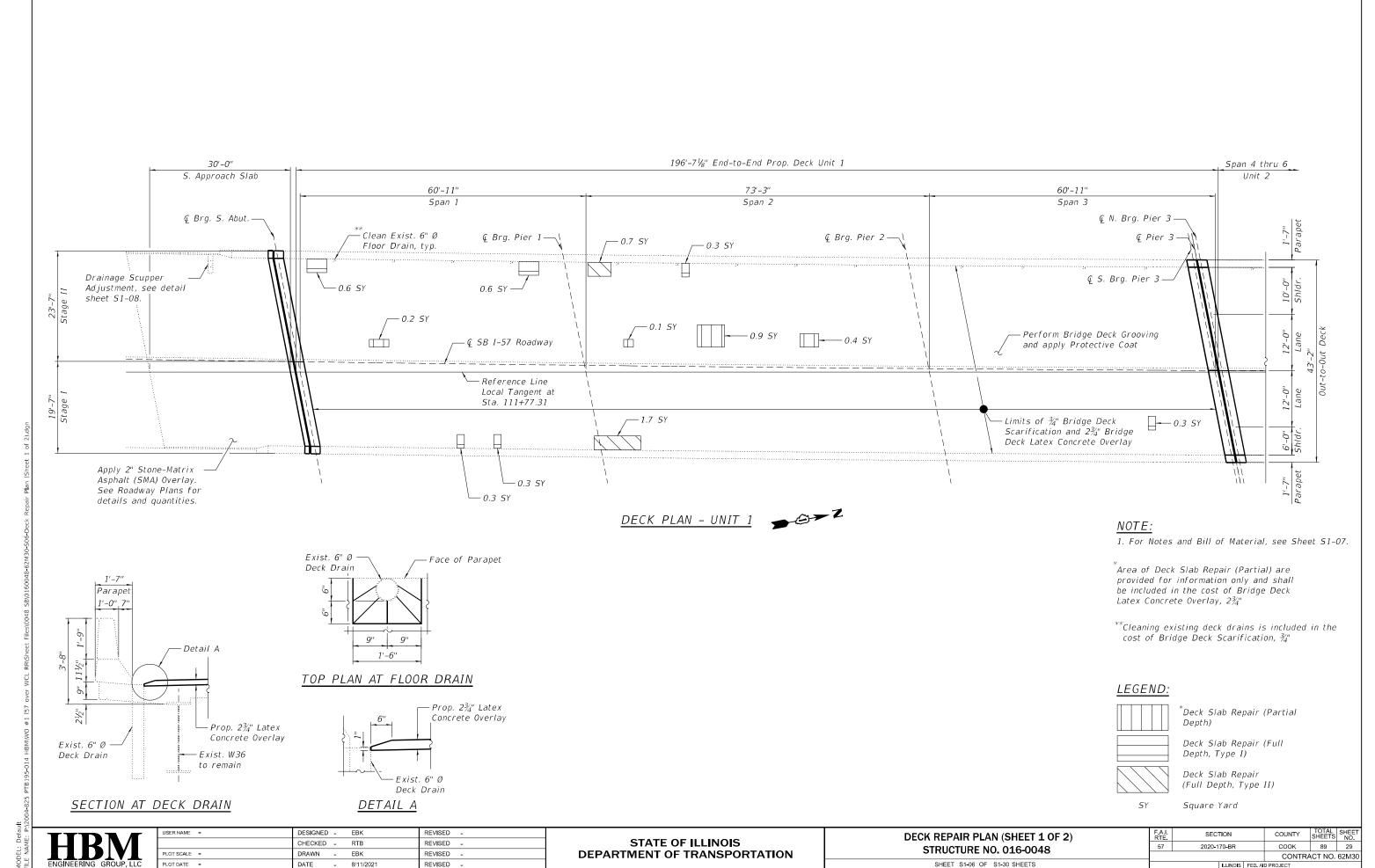
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION 2020-170-BR **STRUCTURE NO. 016-0048**

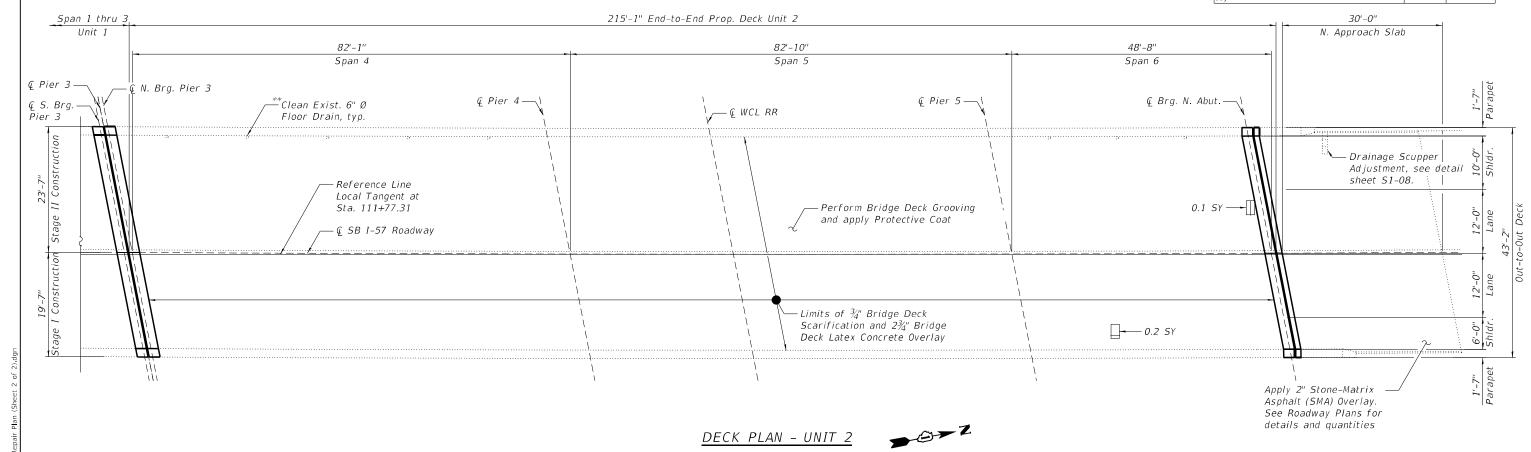
COUNTY COOK 89 28 CONTRACT NO. 62M30 SHEET S1-05 OF S1-30 SHEETS



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BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Bridge Deck Grooving	Sq Yd	1,705
Protective Coat	Sq Yd	2,212
Bridge Deck Latex Concrete Overlay, 2 3/4 Inches	Sq Yd	1,795
Bridge Deck Scarification 3/4"	Sq Yd	1,795
Deck Slab Repair (Full Depth, Type I)	Sq Yd	2.6
Deck Slab Repair (Full Depth, Type II)	Sq Yd	2.4



Area of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 2¾

***Cleaning existing deck drains is included in the cost of Bridge Deck Scarification, 3/4"

NOTES:

- 1. Areas of deck repair shown are estimated. The Engineer shall show actual locations of deck repairs at the time of construction.
- 2. For North, South and Pier 3 transverse joint removal and reconstruction, see Sheets S1-09 thru S1-14.
- Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.

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*Deck Slab Repair (Partial Depth)

Deck Slab Repair (Full
Depth, Type I)

SY Square Yard

HBM ENGINEERING GROUP, LLC

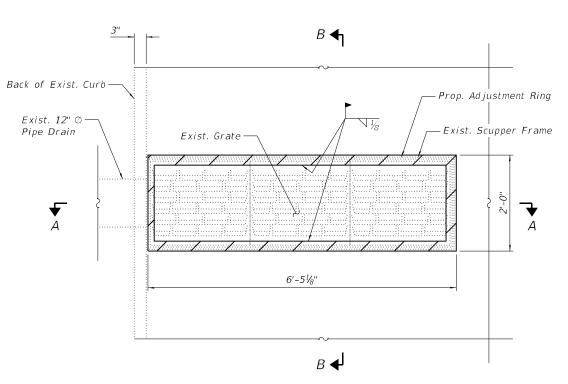
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DECK REPAIR PLAN (SHEET 2 OF 2) STRUCTURE NO. 016-0048
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 SECTION
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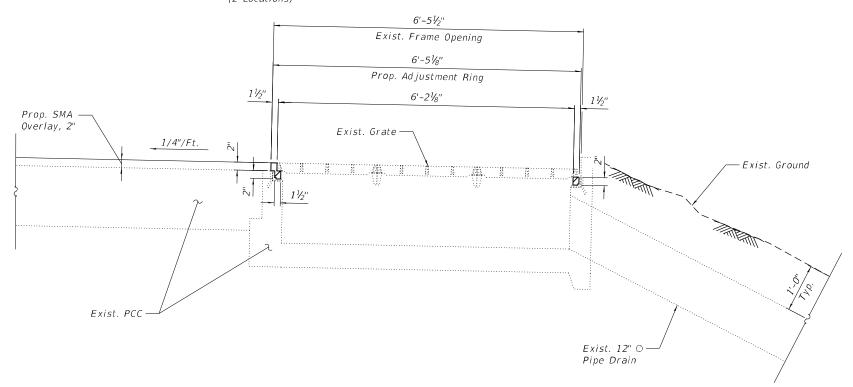
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 CONTRACT NO. 62M30



TYPICAL SCUPPER PLAN

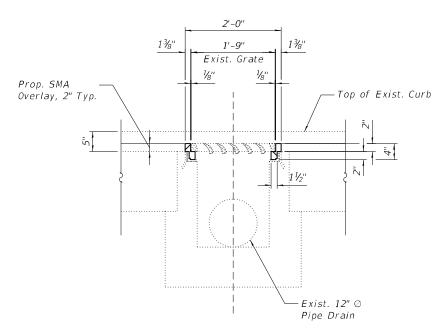
(2 Locations)



SECTION A-A

NOTES

- 1. The Contractor shall field verify Existing Dimensions and Details of the Existing Scuppers and make necessary adjustments prior to construction of New Adjusting Ring or ordering of material for Adjusting Drainage Scuppers.
- 2. All Cast Iron Parts shall be Grey Iron conforming to the requirements of AASHTO M 105, Class 35B.
- 3. Cast Iron Parts shall be unfinished.
- 4. The Contractor shall take appropriate measures to ensure that Protective Coat is not applied to the scuppers.
- 5. Adjusting Ring shall be from Neenah or approved equal. Structural steel weldments or equal section and of the same configuration may be submitted in place of Cast Iron. Fillet or full penetration welds may be used for weldments. Details shall be submitted to the Engineer for approval.
- 6. Provide a $\frac{1}{6}$ " Fillet Weld around perimeter of new Adjusting Ring to secure to existing Scupper.
- 7. Cost of all labor and materials necessary to clean all existing floor drains and scuppers, install adjusting scupper rings, remove and reinstall grates is included in the cost for Drainage Scupper to be Adjusted.



SECTION B-B

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scuppers To Adjusted	Be Each	2

HBM ENGINEERING GROUP, LLC

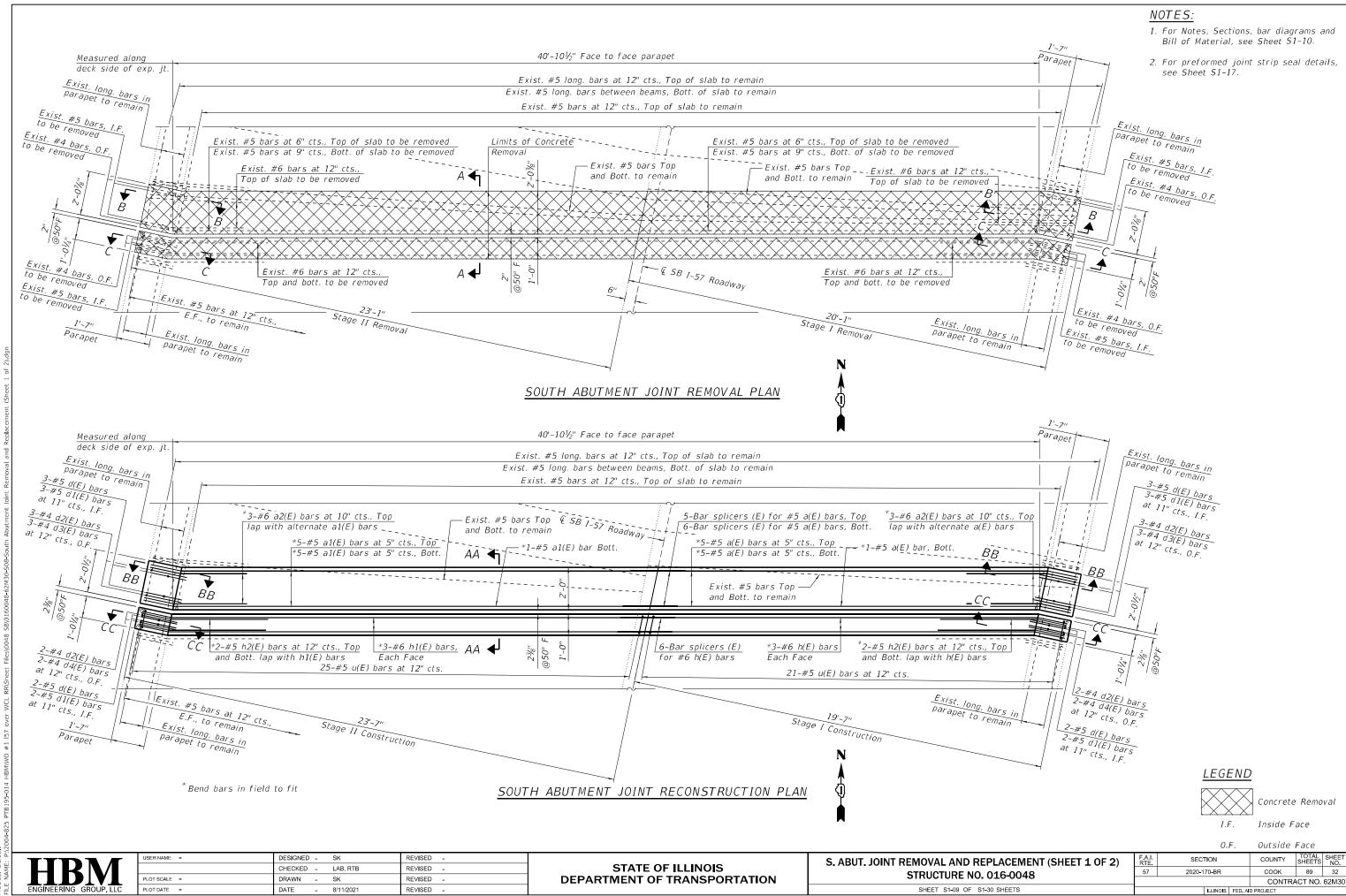
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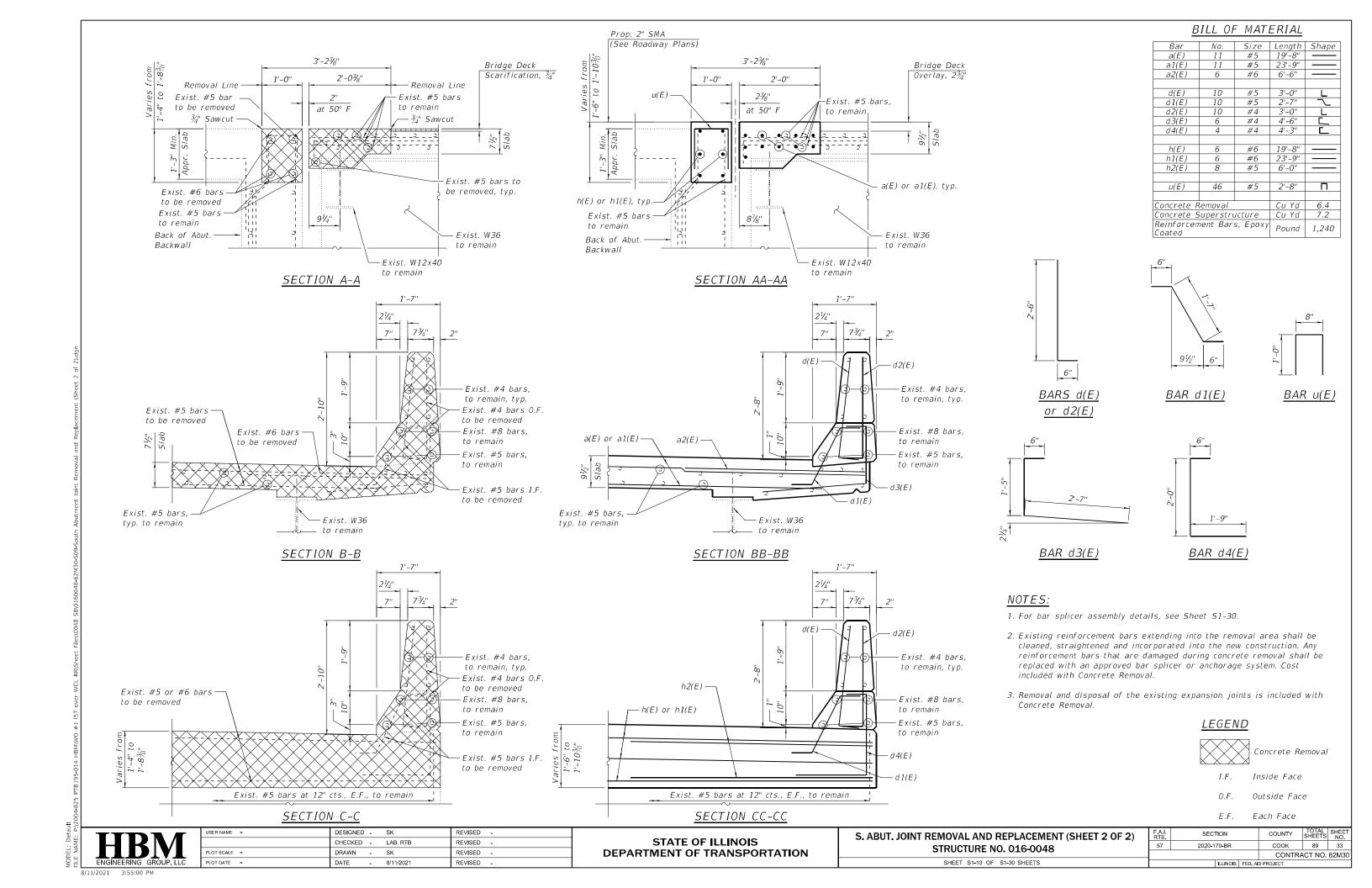
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

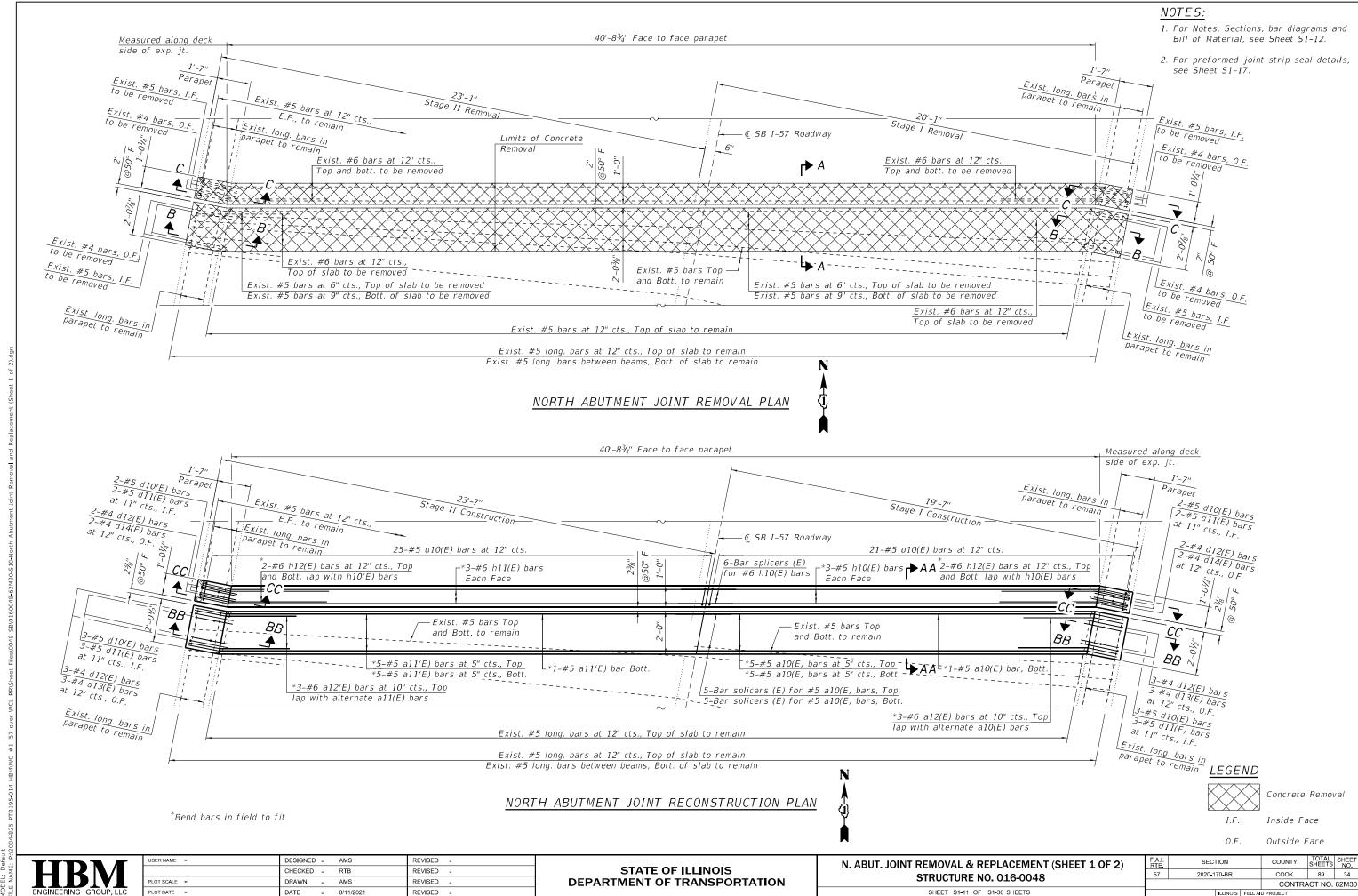
DRAINAGE SCUPPER ADJUSTMENT DETAILS
STRUCTURE NO. 016-0048

SHEET \$1-08 OF \$1-30 SHEETS

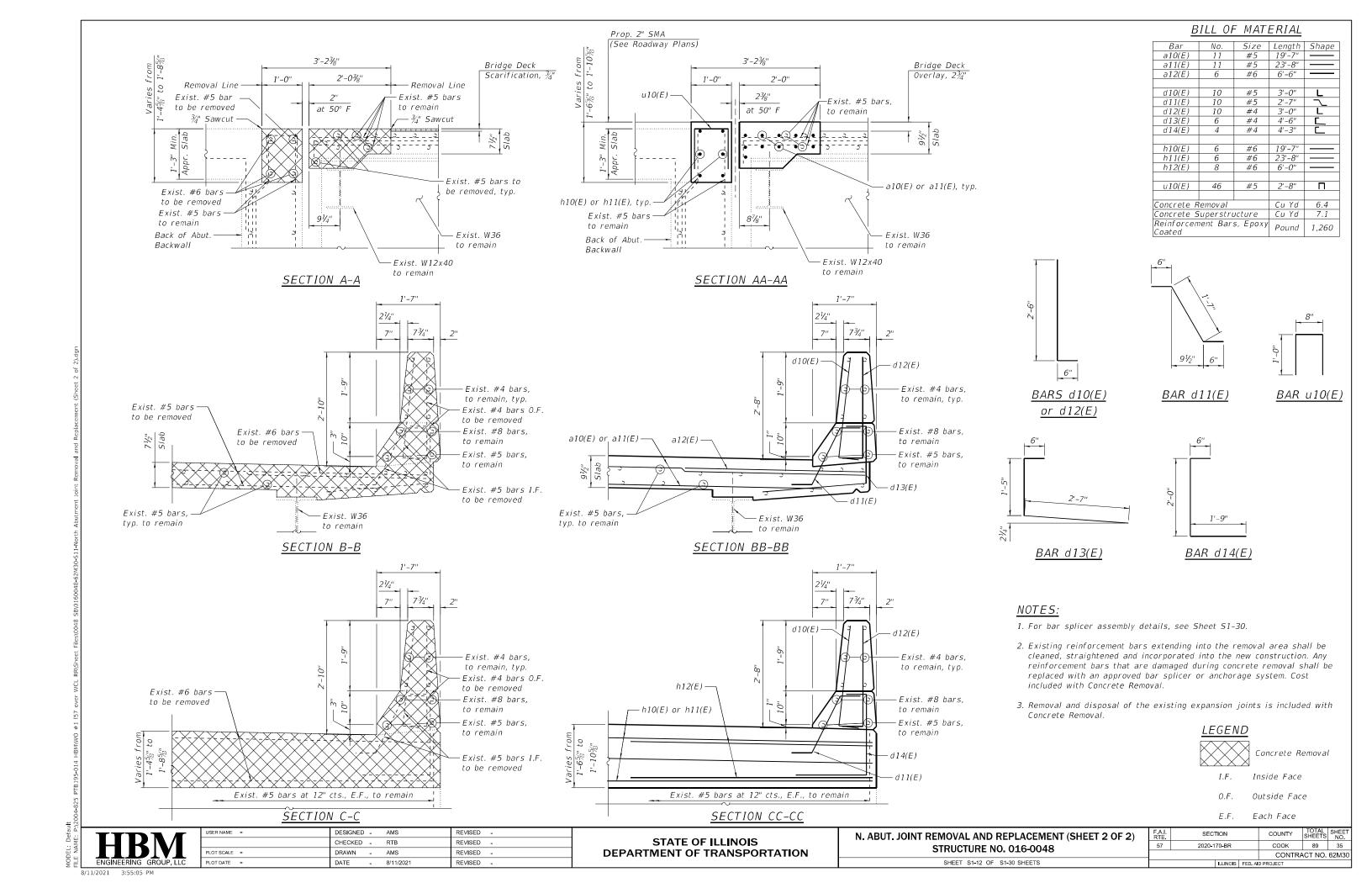
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57	2020-170-BR		соок	89	31
CONTRACT NO. 62					

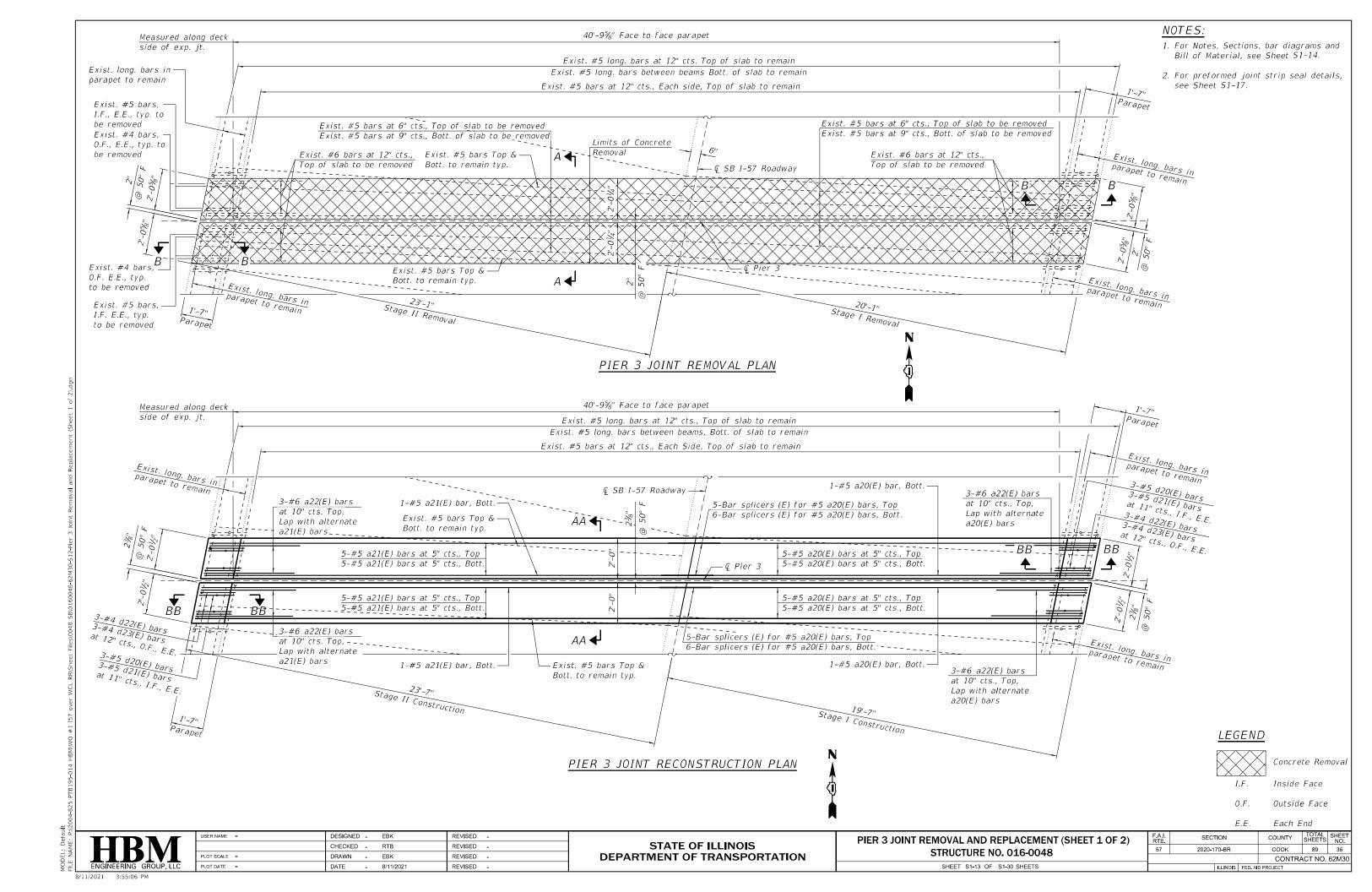


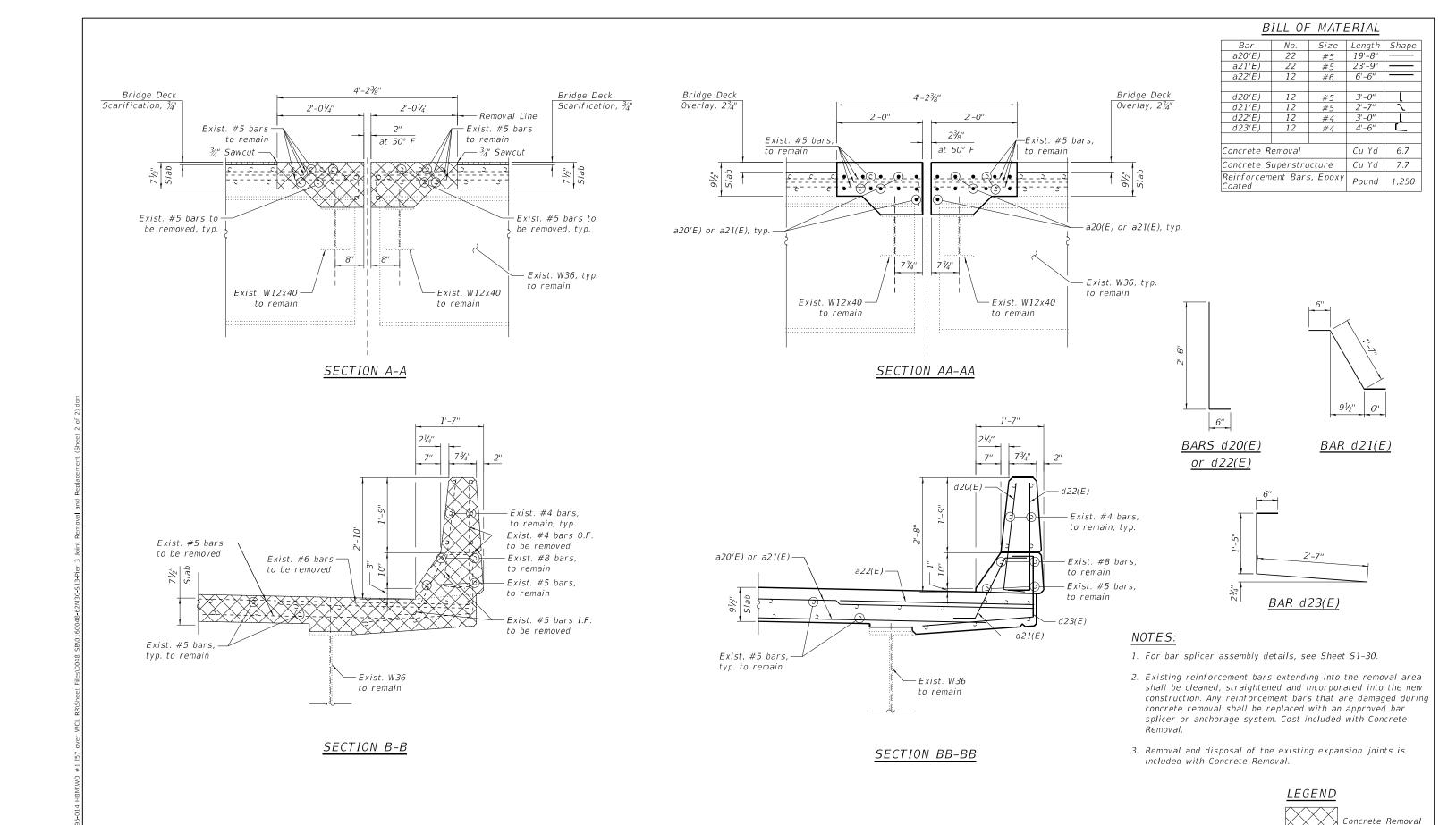




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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

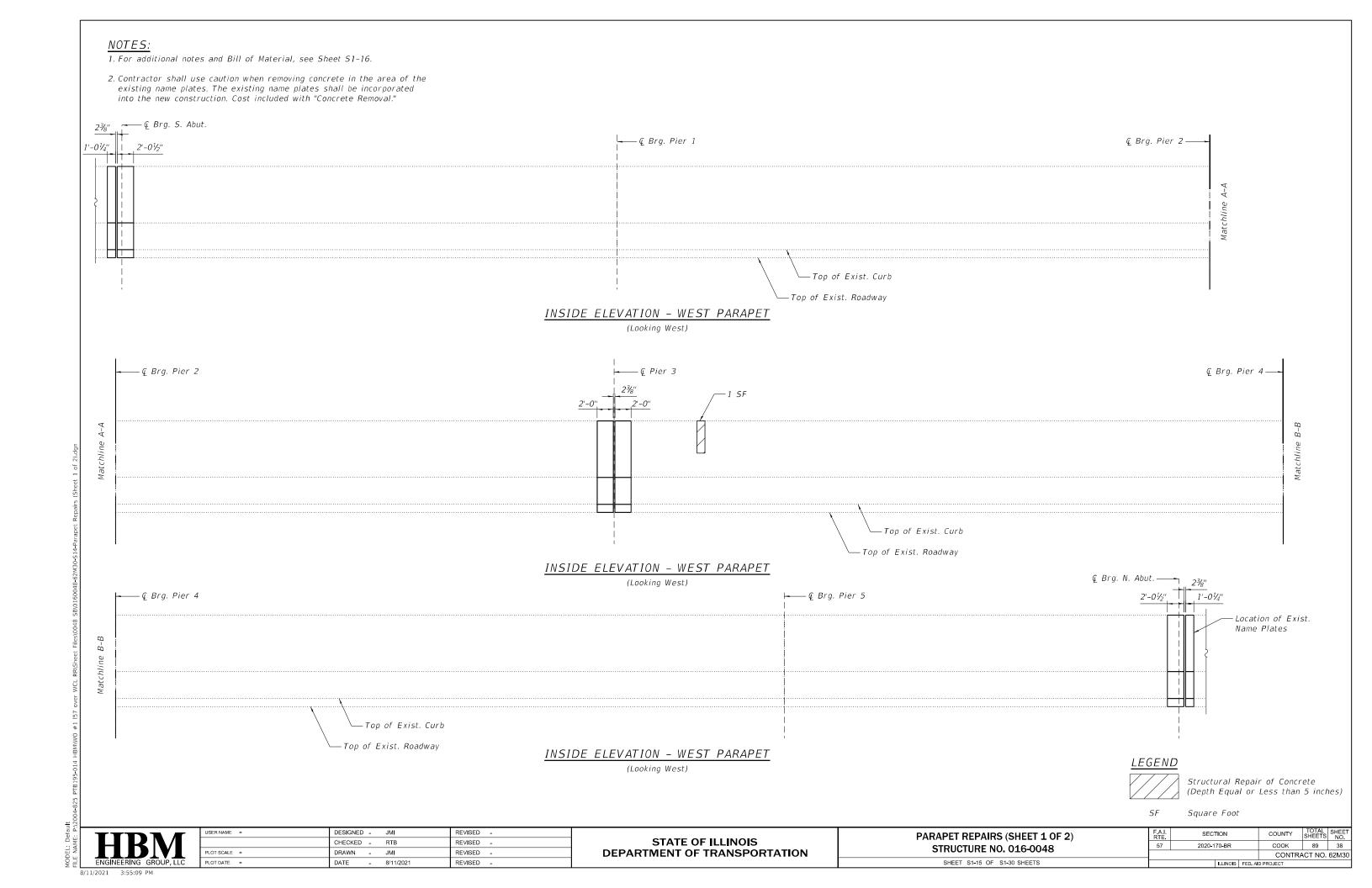
PIER 3 JOINT REMOVAL AND REPLACEMENT (SHEET 2 OF 2)
STRUCTURE NO. 016-0048

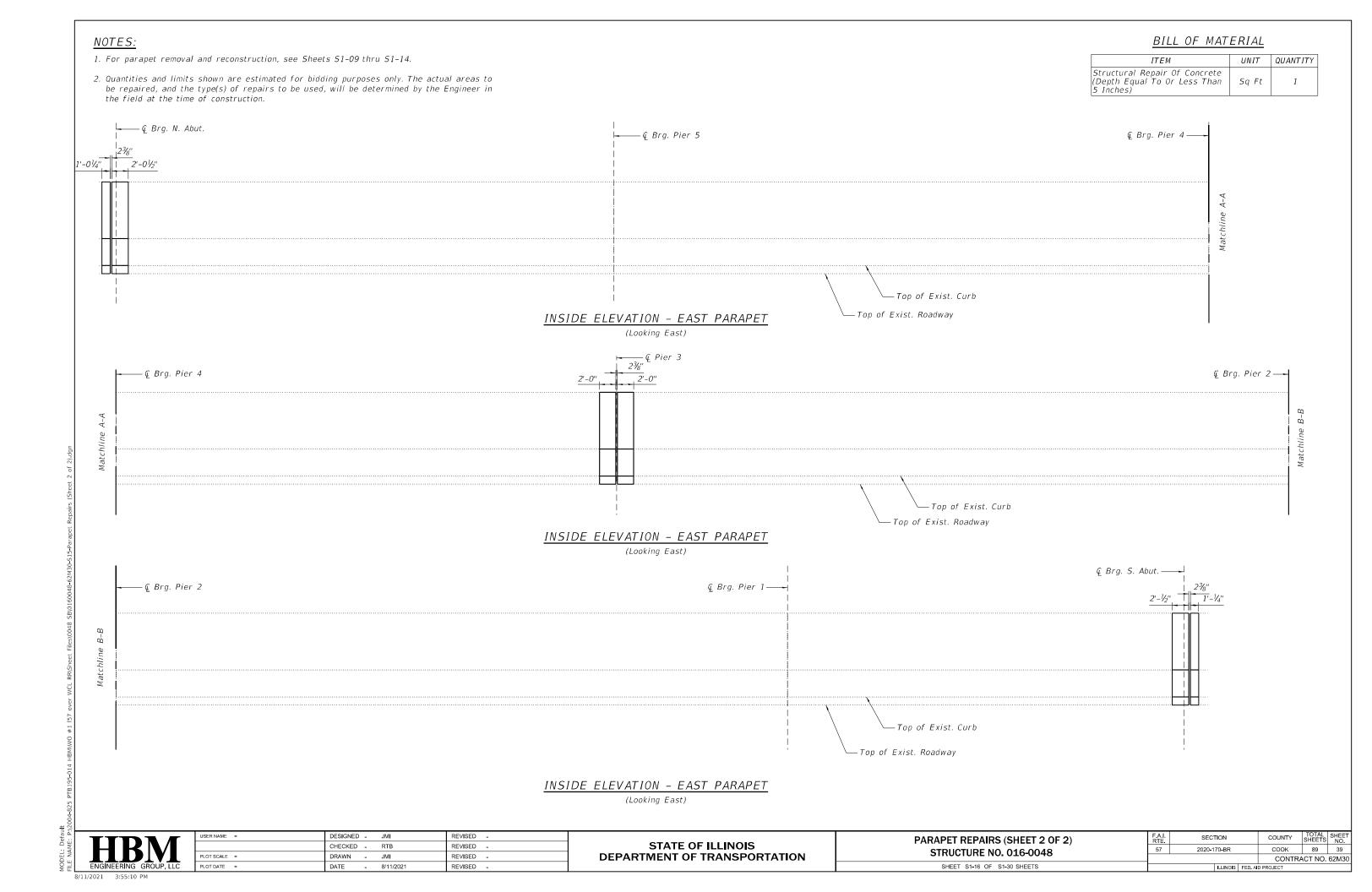
SHEET S1-14 OF S1-30 SHEETS

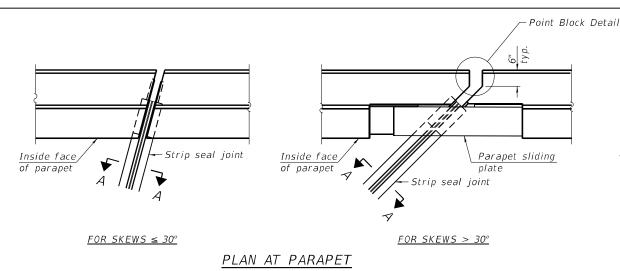
I.F. 0.F. Inside Face

Outside Face

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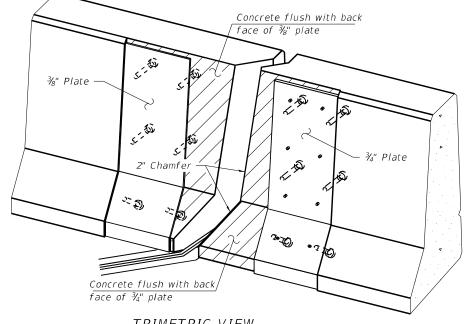
Top of locking

Top of deck

edge rail

^{*}¾" Ø x 6" Studs '¾" Embedded plate full depth ¾" Embedded plate, full depth 6" 1/2" Parapet sliding plate 1'-0" ¾" Ø Countersunk bolts-

<u>Direction</u> of traffic



SECTION B-B

seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The strip seal shall be made continuous and shall have a minimum thickness of 1/4". The configuration of the strip

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal. 39" constant slope barrier shown, 44" constant slope barrier

similar as noted.

Notes:

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

ELEVATION AT PARAPET

Parapet sliding

Inside Face

of Parapet

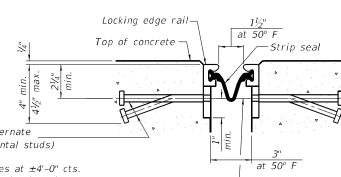
(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

2" Max.

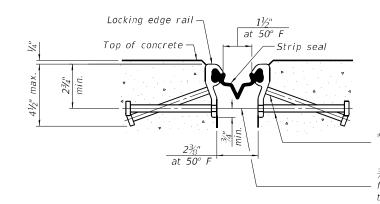
В

%" Ø x 6" Studs

Detail A



TRIMETRIC VIEW (Showing embedded plates only)



SHOWING ROLLED RAIL JOINT

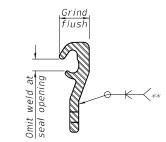
* $\frac{1}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs) %" ϕ threaded rods in %6" ϕ holes at $\pm 4'$ -0" cts. for holding the proper joint opening based on

the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed SHOWING WELDED RAIL JOINT off flush with the plates after concrete is set.

<u>ROLLED</u> WELDED RAIL (EXTRUDED) RAIL

LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	132

SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

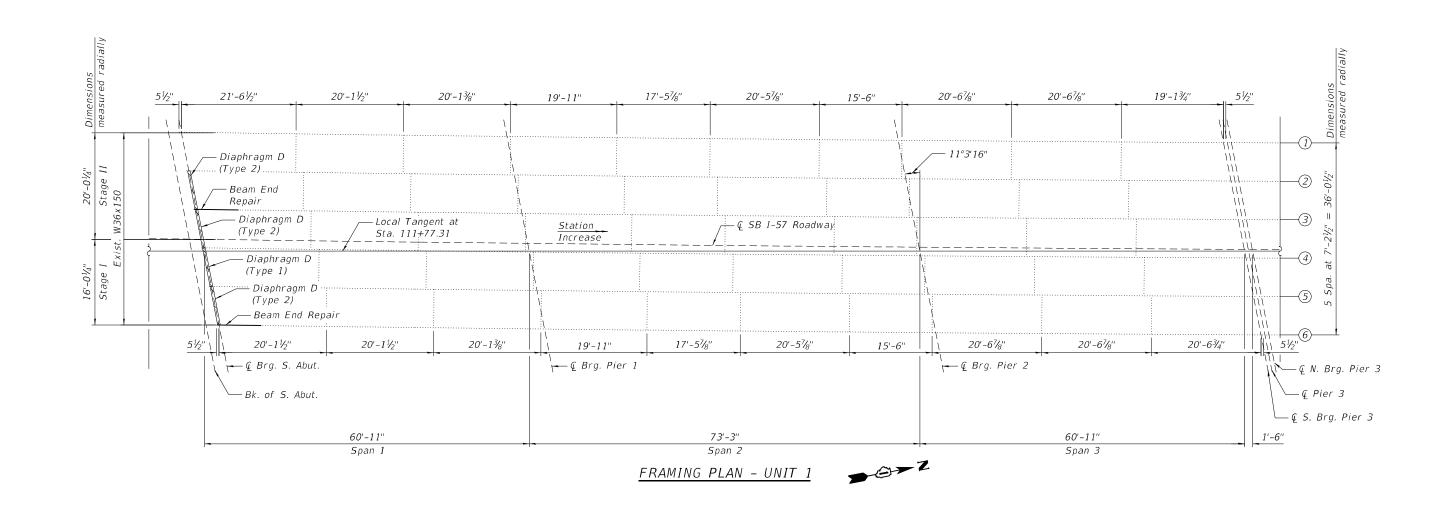
DETAIL A

USER NAME =	DESIGNED	-	JMI	REVISED	-
	CHECKED	-	RTB	REVISED	-
PLOT SCALE =	DRAWN	-	JMI	REVISED	-
PLOT DATE =	DATE	-	8/11/2021	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

		INT STRIP SEAL IO. 016-0048
SHEET	S1-17 OF	S1-30 SHEETS

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
57	2020-170-BR		соок	89	40
			CONTRA	CT NO.	62M3
	III INOIE	D DDO JECT			



NOTE:

1. For Notes and Bill of Material, see Sheet S1-19.

DESIGNED - JMI REVISED -CHECKED - RTB REVISED -DRAWN - JMI, EBK REVISED -DATE - 8/11/2021 REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** FRAMING PLAN STEEL REPAIRS (SHEET 1 OF 2) STRUCTURE NO. 016-0048

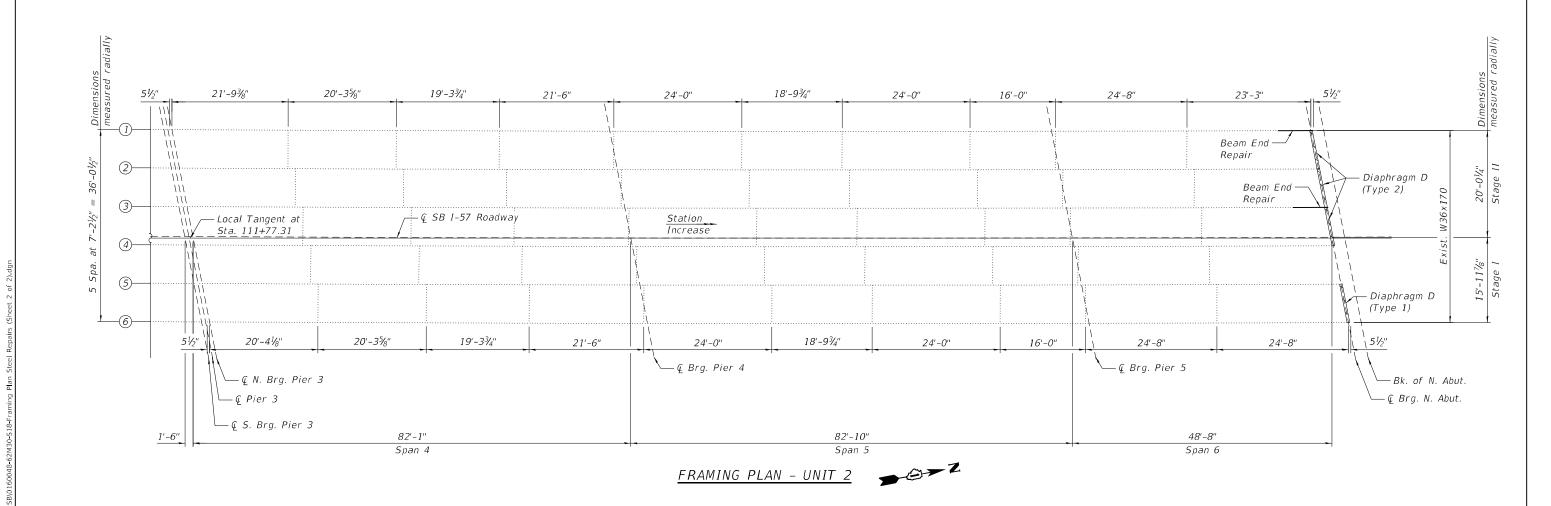
Remove and Replace Exist. Diaphragm TOTAL SHEET NO. COUNTY

Perform Beam End Repairs

F.A.I. RTE 57 SECTION COOK 89 41 2020-170-BR CONTRACT NO. 62M30 SHEET S1-18 OF S1-30 SHEETS

LEGEND:

ITEM	UNIT	QUANTITY
Furnishing And Erecting Structural Steel	Pound	2,590
Structural Steel Removal	Pound	2,660
Structural Steel Renair	Pound	1080



NOTES:

1. All work is to be performed utilized staged construction. See Sheets S1-03 and S1-04 for details.

2. For Beam End Repairs and Diaphragm Removal and Replacement Details, see Sheet S1-20 and S1-21.

DESIGNED - JMI CHECKED - RTB

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** FRAMING PLAN STEEL REPAIRS (SHEET 2 OF 2) STRUCTURE NO. 016-0048

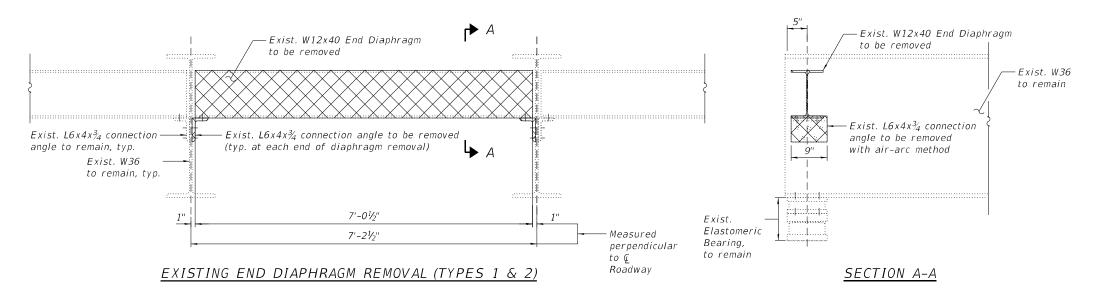
Remove and Replace Exist. Diaphragm TOTAL SHEET NO. COUNTY

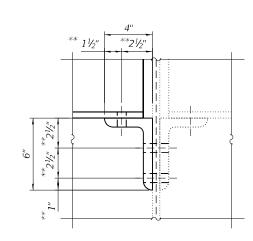
Perform Beam End Repairs

REVISED -REVISED -REVISED -- 8/11/2021 REVISED -

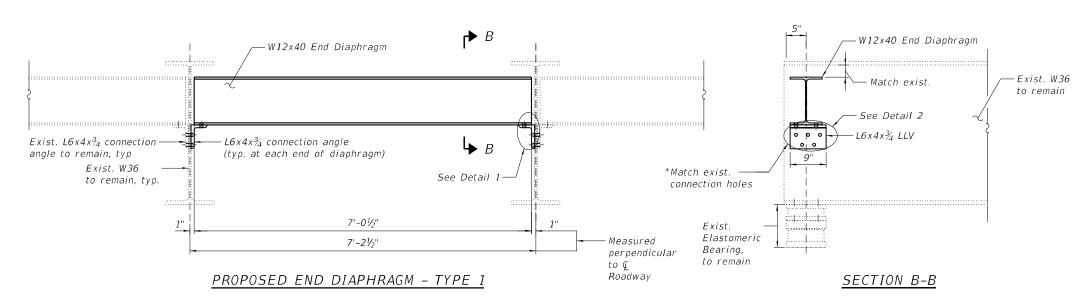
SECTION SHEET S1-19 OF S1-30 SHEETS

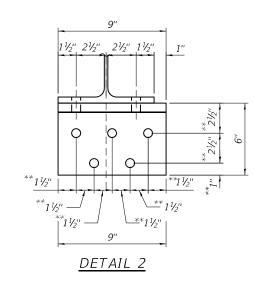
LEGEND:





DETAIL 1





- Field Drill Holes in Clip Angle to match existing holes in Beam or Diaphragm
- Dimension may vary in the field based on existing hole locations in beam web.

NOTES:

- 1. For locations of Diaphragm Removal/Replacement and Beam Repairs and Bill of Material, see Sheets S1-18 and S1-19.
- 2. All proposed beam end repair plates and bottom flange repair angles shall conform to the requirements of AASHTO M270 Grade 50. All proposed diaphragms and their connection angles shall conform to the requirements of AASHTO M270 Grade 36.
- 3. Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Diaphragm connection holes shall be $^{15}\!\!/_{6}$ " for $^{3}\!\!/_{4}$ " bolts. Two hardened washers shall be required at diaphragm connections.
- 4. Existing diaphragm and connection angle removal shall be paid for as Structural Steel Removal.
- 5. All proposed beam end repair plates, bottom flange repair angles and associated bolts/fasteners shall be paid for as Structural Steel Repair. All proposed diaphragms, connection angles and associated bolts/fasteners shall be paid for as Furnishing and Erecting Structural Steel.

- 6. Load carrying components designated "CVN" shall conform to the Charpy V-Notch Impact Testing Requirements, Zone 2.
- 7. Use holes in new steel as template to field drill holes in existing steel unless noted otherwise.
- 8. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the special provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures". Cost included in Structural Steel Repair.



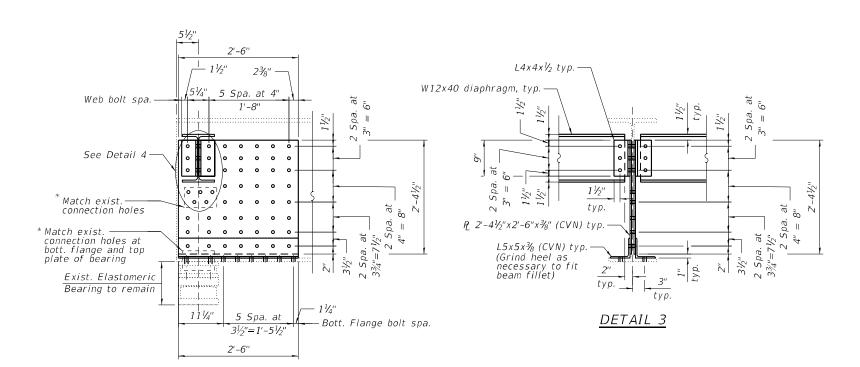


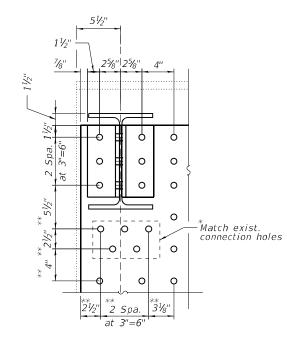
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	CHECKED	-	RTB	REVISED -
PLOT SCALE =	DRAWN	-	AMS	REVISED -
PLOT DATE =	DATE	-	8/11/2021	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION STRUCTURAL STEEL REPAIR DETAILS (SHEET 1 OF 2) **STRUCTURE NO. 016-0048** SHEET S1-20 OF S1-30 SHEETS

SECTION COUNTY SHEETS 2020-170-BR COOK 89 43 CONTRACT NO. 62M30

PROPOSED END DIAPHRAGM - TYPE 2





<u>DETAIL 4</u>

<u>SECTION C-C</u> <u>BEAM END REPAIR DETAIL</u>

NOTES:

- 1. For diaphragm removal, notes and Bill of Material, see Sheets S1-19 and S1-20.
- 2. Cost of field drilling shall be included in the cost of Structural Steel Repair.

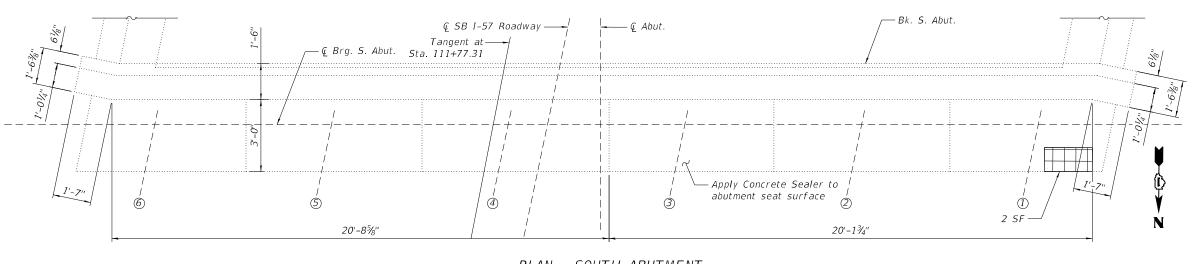
HBM	
ENGINEERING GROUP, LLC	

USER NAME =	DESIGNED	-	AMS	REVISED -
	CHECKED	-	RTB	REVISED -
PLOT SCALE =	DRAWN	-	AMS	REVISED -
PLOT DATE =	DATE	-	8/11/2021	REVISED -

STRUCTURAL STEEL REPAIR DETAILS (SHEET 2 OF 2)	F.A.I. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-0048		2020-1	70 - BR		соок	89	44
					CONTR	ACT NO.	62M30
SHEET S1-21 OF S1-30 SHEETS			PLUNOIS	EED A	D PPO IECT		

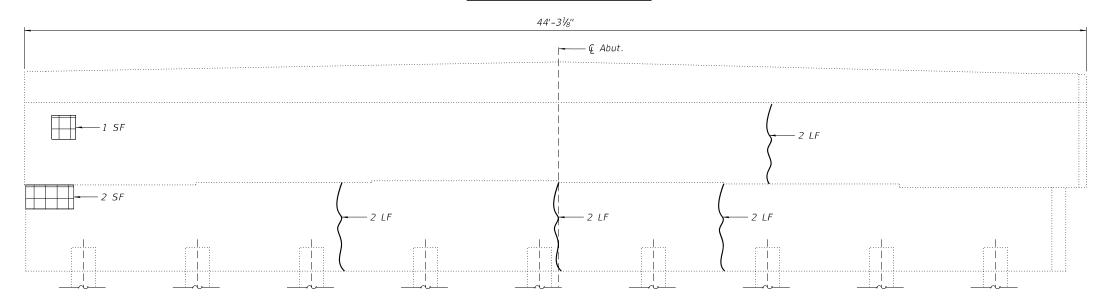
^{*} Field Drill Holes in Repair Plate and/or Angle to match existing holes in Beam or Diaphragm

^{**} Dimension may vary in the field based on existing hole locations in beam web



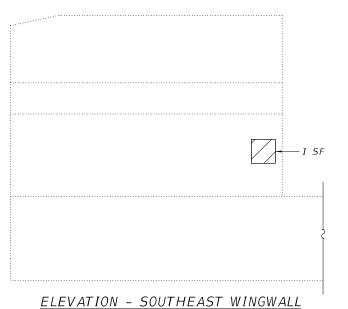
	ITEM	UNIT	QUANTITY
	Concrete Sealer	Sq Ft	129
	Epoxy Crack Injection	Foot	8
T	Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	SqFi	1
	Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	5

PLAN - SOUTH ABUTMENT



ELEVATION - SOUTH ABUTMENT

(Looking South)



(Looking West)

NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)

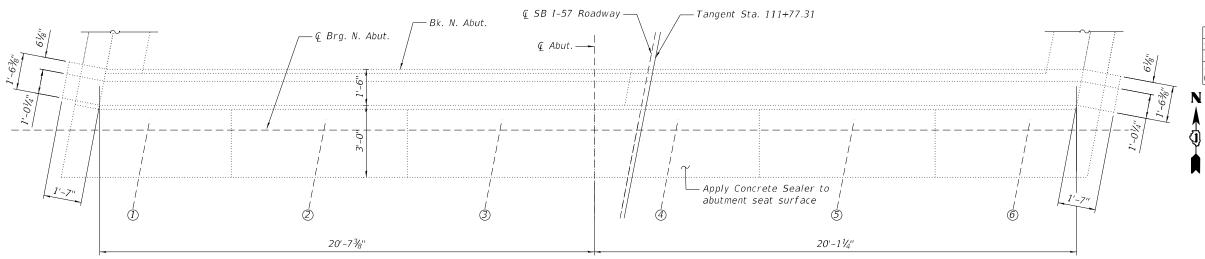


Epoxy Crack Injection (Width > 0.06")

Square Foot

USER NAME =	DESIGNED	-	JMI	REVISED -
	CHECKED	-	RTB	REVISED -
PLOT SCALE =	DRAWN	-	JMI	REVISED -
PLOT DATE =	DATE	-	8/11/2021	REVISED -

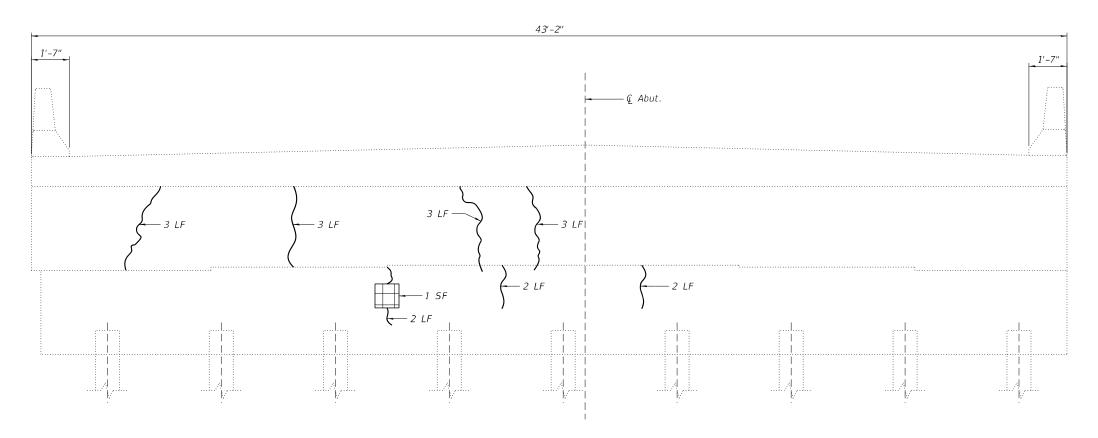
A.I. TE	SECT	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEET NO.			
57	2020-1	70 - BR		соок	89	45			
			CONTRA	CT NO.	62M30				



<u>BILL OF MATERIAL</u>

ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	129
Epoxy Crack Injection	Foot	18
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	1

PLAN - NORTH ABUTMENT



ELEVATION - NORTH ABUTMENT

(Looking North)

NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND



Structural Repair of Concrete (Depth Greater than 5 inches)



Epoxy Crack Injection (Width > 0.06")

F Square Foot

HBM ENGINEERING GROUP, LLC

USER NAME =	DESIGNED	-	JMI	REVISED -	
	CHECKED	-	RTB	REVISED -	
PLOT SCALE =	DRAWN	-	JMI	REVISED -	
PLOT DATE =	DATE	-	8/11/2021	REVISED -	

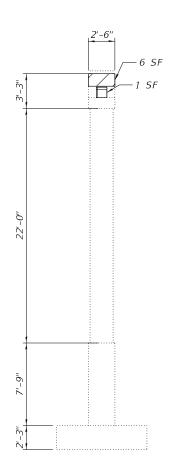
A.I. ΓΕ.	SECTIO	N		COUNTY	TOTAL SHEETS	SHEET NO.
7 2020-170-BR		соок	89	46		
		CONTRACT NO. 62			62M30	
	ILL	INOIS	FED. Al	D PROJECT		

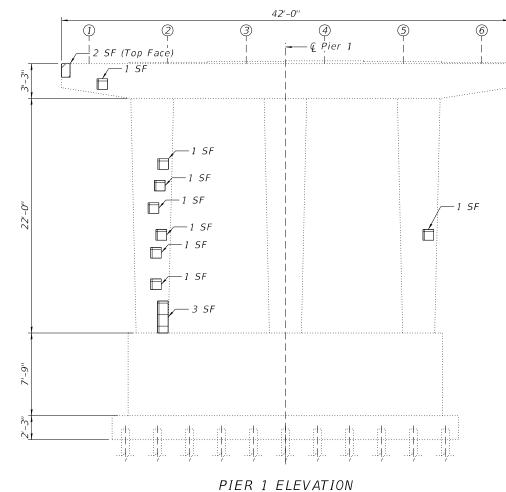
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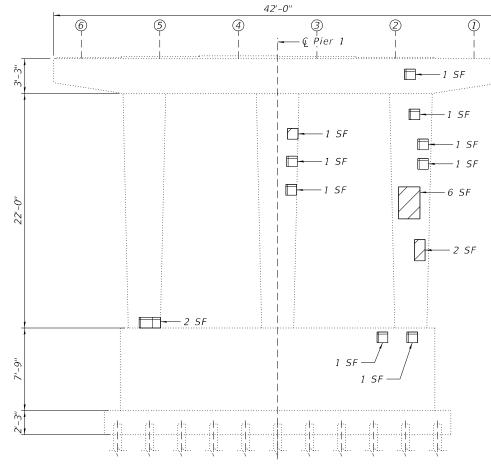
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ITEM	UNIT	QUANTITY
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	25
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	22







END ELEVATION (Looking East)

(Looking North)

END ELEVATION (Looking West)

PIER 1 ELEVATION

(Looking South)

NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)

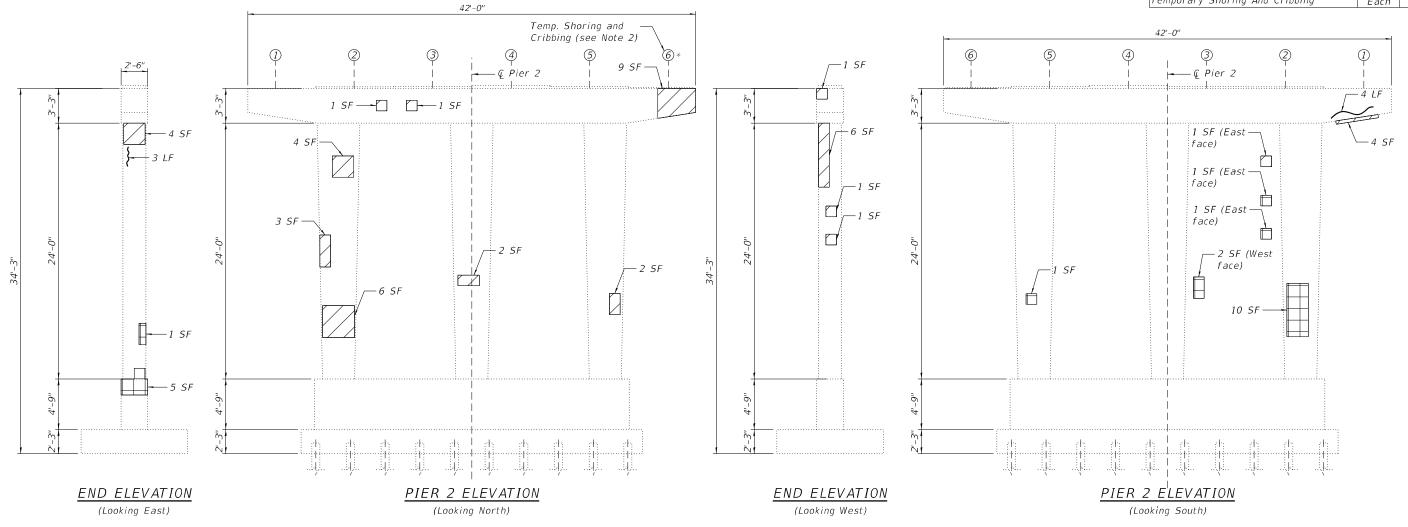
SF Square Foot



USER NAME =	DESIGNED - JMI	REVISED -
	CHECKED - RTB	REVISED -
PLOT SCALE =	DRAWN - JMI	REVISED -
PLOT DATE =	DATE - 8/11/2021	REVISED -

A.I. SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
7	2020-170-BR		соок	89	47
			CONTRA	CT NO.	62M30
	ILLINOIS.	EED AU	D PPO IECT		

ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	7
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	46
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	21
Temporary Shoring And Cribbing	Fach	1



NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only.

 The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

SUMMAR	Y OF RE PIER 2	ACTIONS
R DL	(k)	89.5
R LL	(k)	50.0
R IM	(k)	13.0
R Total	(k)	152.5

UMMAR)	OF RE	ACTIONS
DL	(k)	89.5
LL	(k)	50.0
IM	(k)	130

<u>LEGEND</u>	
	Structural Repair of Concrete (Depth Equal to or Less than 5 inches)
	Structural Repair of Concrete (Depth Greater than 5 inches)
\sim	Epoxy Crack Injection (Width > 0.06")
C.F.	Course Free

Square Foot

LF Linear Foot

Temporary Shoring and Cribbing



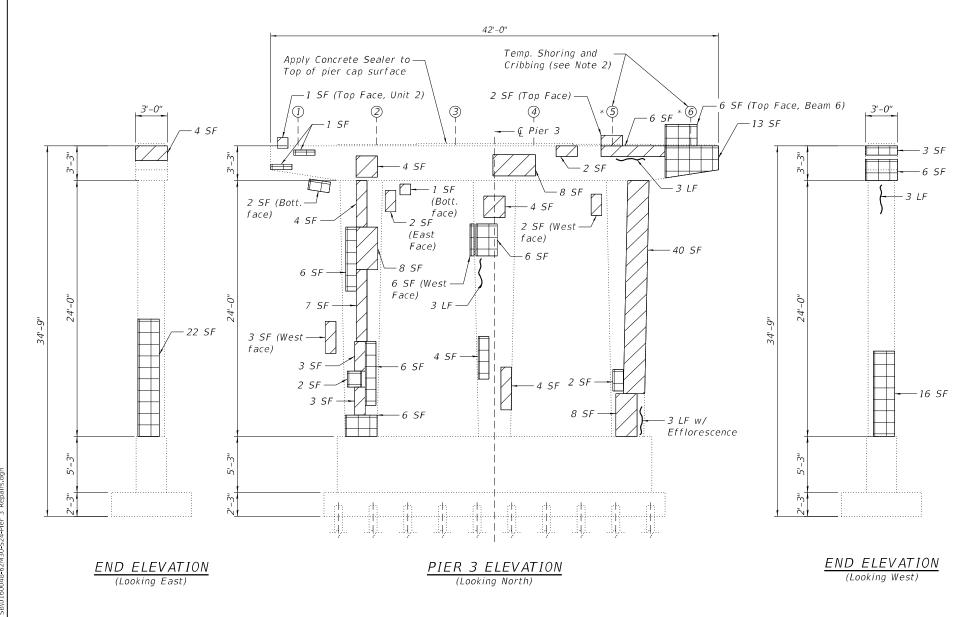
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PLOT SCALE =	DRAWN	-	JMI	REVISED -	
PLOT DATE =	DATE	-	8/11/2021	REVISED -	

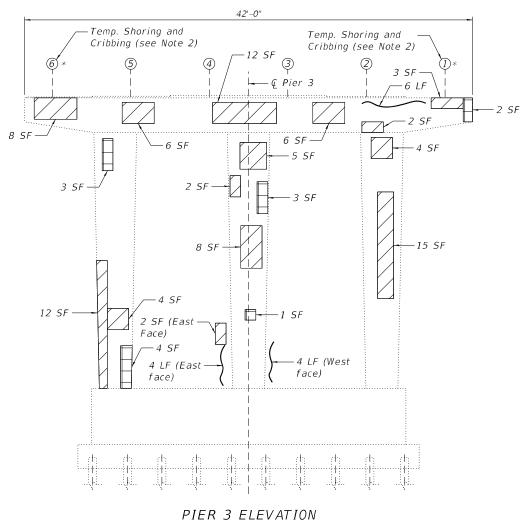
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PIER 2 REPAIRS
STRUCTURE NO. 016-0048
CHEET CARE OF CARO CHEETO

A.I. ΓΕ.	SECT	ION		COUNTY	TOTAL SHEETS	SHEET NO.
7	2020-17	70 - BR		соок	89	48
				CONTRA	CT NO.	62M30

ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	126
Epoxy Crack Injection	Foot	26
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	205
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	121
Temporary Shoring And Cribbing	Each	4





NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

SUMMARY OF REACTIONS						
		Pier 3, Span 3	Pier 3, Span 4			
R DL	(k)	27.4	38.5			
R LL	(k)	43.6	45.77			
R IM	(k)	11.7	11.05			
R Total	(k)	82.7	95.32			



(Looking South)

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Structural Repair of Concrete (Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

SF Square Foot

LF Linear Foot

Temporary Shoring and Cribbing



USER NAME =	DESIGNED	-	JMI	REVISED -	ı
	CHECKED	-	RTB	REVISED -	ı
PLOT SCALE =	DRAWN	-	JMI	REVISED -	ı
PLOT DATE =	DATE	-	8/11/2021	REVISED -	l
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

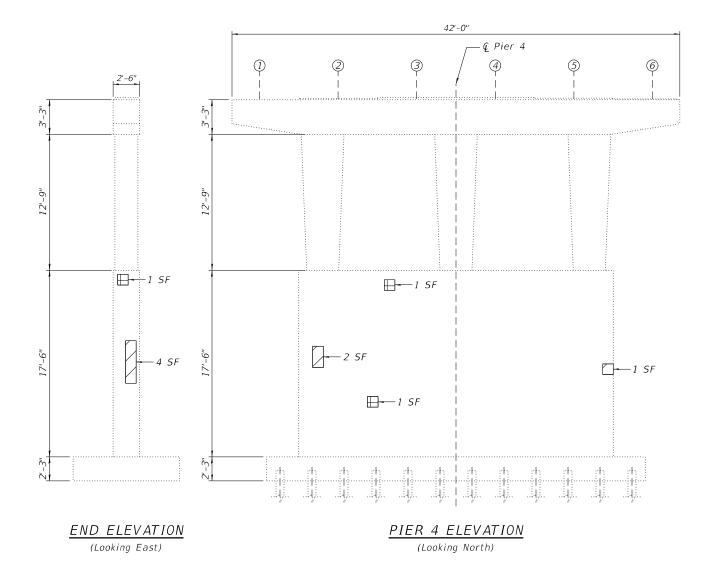
PIER 3 REPAIRS							
STRUCTURE NO. 016-0048							
CHEET C1 36 OF C1 30 CHEETC							

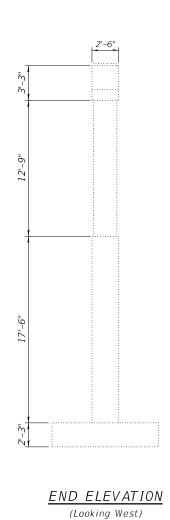
A.I. TE.	SECT	TION		COUNTY	TOTAL SHEETS	SHEET NO.
57	2020-1	70-BR		соок	89	49
				CONTRA	ACT NO.	62M30
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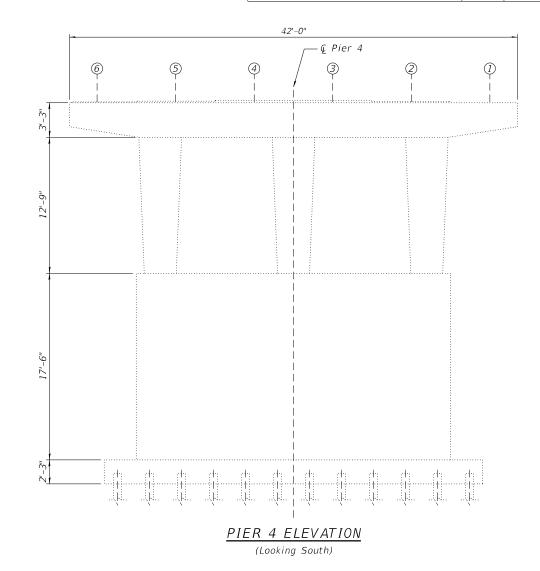
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ENGINEERING GR

ITEM	UNIT	QUANTIT
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	7
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	3







NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

SF

Structural Repair of Concrete (Depth Greater than 5 inches)

Square Foot

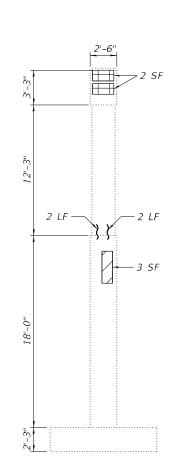
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PLOT SCALE =	DRAWN - JMI	REVISED -
PLOT DATE =	DATE - 8/11/2021	REVISED -

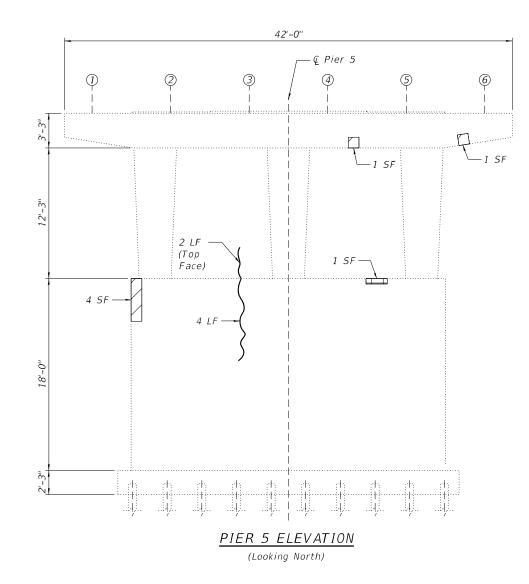
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

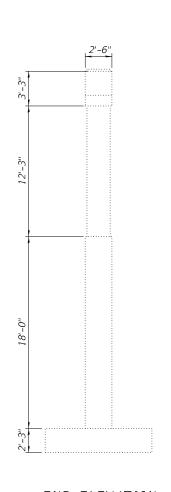
PIER 4 REPAIRS STRUCTURE NO. 016-0048 SHEET S1-27 OF S1-30 SHEETS

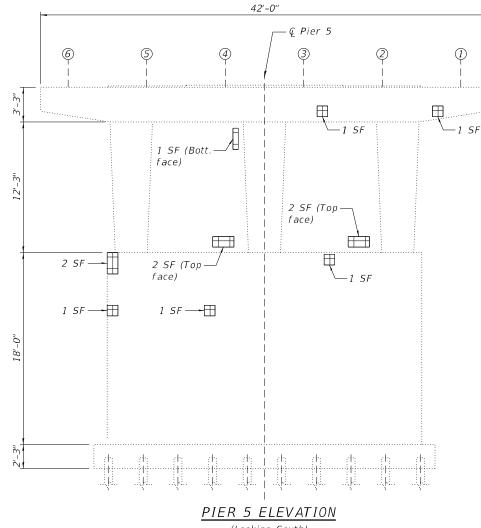
SECTION COUNTY 2020-170-BR COOK 89 50 CONTRACT NO. 62M30

ITEM	UNIT	QUANTITY
Epoxy Crack Injection	FOOT	10
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	9
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	17









END ELEVATION (Looking East)

END ELEVATION (Looking West)

(Looking South)

NOTES:

Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

SF

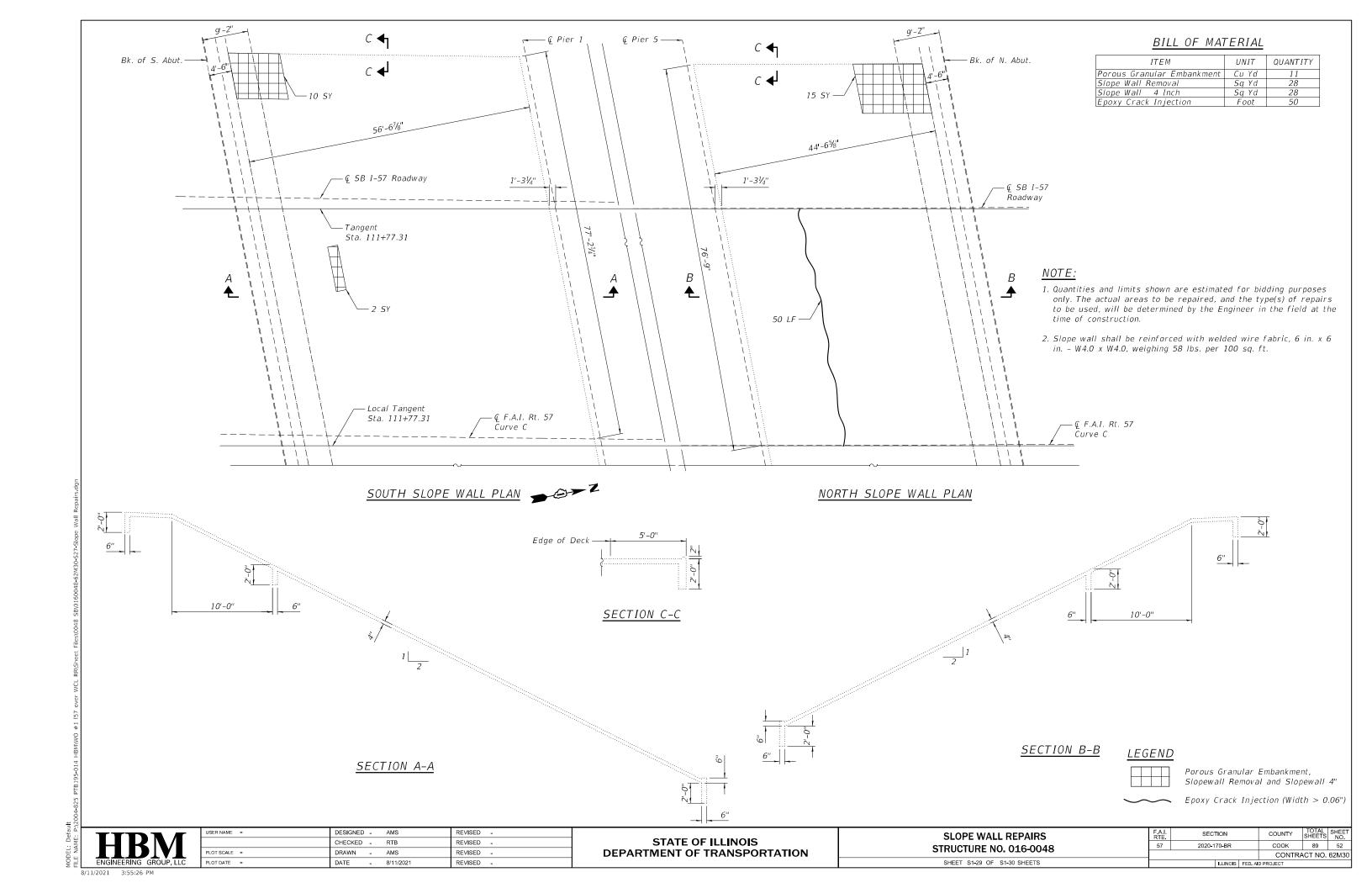
Square Foot

HBM	
ENGINEERING GROUP, LLC	

	USER NAME =	DESIGNED	-	JMI	REVISED -	
		CHECKED	-	RTB	REVISED -	
	PLOT SCALE =	DRAWN	-	JMI	REVISED -	
	PLOT DATE =	DATE	-	8/11/2021	REVISED -	
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F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE
57	2020-170-BR		соок	89	51
			CONTRA	CT NO.	62M3
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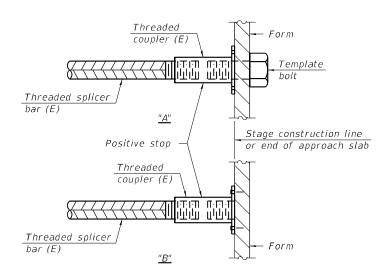
STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

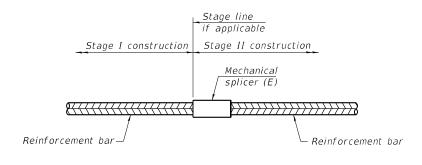
Location	Bar size	No. assemblies required	Minimum lap length
South Abutment Deck	#5	11	3'-6"
South Abutment Bk. Wall	#6	6	4'-0"
North Abutment Deck	#5	11	3'-6"
North Abutment Bk. Wall	#6	6	4'-0"
Pier 3	#5	22	3'-6"



INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum $60\ ksi$ yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

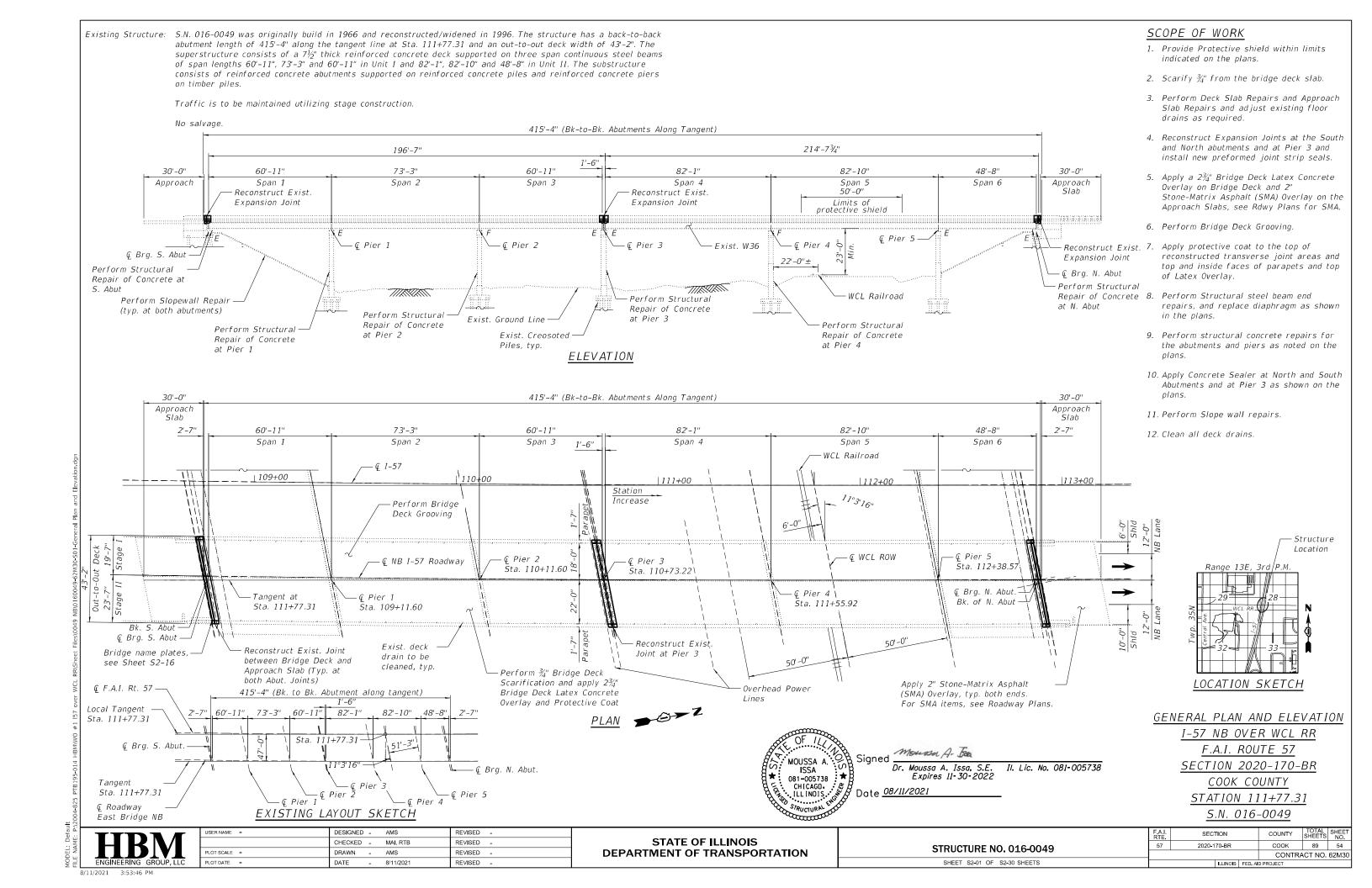
HBM ENGINEERING GROUP I.C. 1-1-2020

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
STRUCTURE NO. 016-0048

SHEET S1-30 OF S1-30 SHEETS

I-825 PTB195-014 HBM\WO #1157 over WCL RR\Sheet Files\0048 SB\0160048-62M30-528-Bar



GENERAL NOTES

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 3. Bars noted thus, 3x2-#5, indicates 3 lines of #5 bars with 2 lengths of bars per line.
- 4. For SMA overlay on Approach Slab, see Civil Sheets.
- 5. Protective Coat shall be applied to the top of reconstructed transverse joint areas and top and inside faces of parapets and top of Latex Overlay.
- 6. Concrete Sealer shall be applied to the abutments and Pier 3, at locations shown in the plans.
- 7. Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50°F.
- 8. Prior to pouring the new concrete deck for expansion joint reconstruction and deck slab repairs, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding \(\frac{1}{2} \) deep shall be identified and reported to the Bureau of Bridges and Structures for further dispositions. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 9. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 10. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures".
- 11. All new structural steel shall be hot-dip galvanized. See Special Provisions for "Hot Dip Galvanizing for Structural Steel."
- 12. Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts $\frac{3}{4}$ ", open holes $\frac{13}{6}$ ", unless otherwise noted.
- 13. Cost of cleaning existing drains shall not be paid separately, but shall be included with Bridge Deck Scarification
- 14. The Contractor shall exercise extreme caution under the Aerial Power Lines. The Contractor is responsible to coordinate with the utilities to deenergize these lines if required. The Contractor shall make necessary adjustments to maintain adequate clearance to the Aerial Power Lines as specified in OSHA safety requirements.
- 15. All proposed beam end repair plates and bottom flange repair angles shall conform to the requirements of AASHTO M270 Grade 50. All proposed diaphragms and their connection angles shall conform to the requirements of AASHTO M270 Grade 36.

INDEX OF SHEETS

52-01	General Plan an	d Elevation			
52-02	General Notes 1	Inday of Shoots	& Total	Rill of	Materia

S2-03 Stage Construction (Sheet 1 of 2) S2-04 Stage Construction (Sheet 2 of 2)

S2-05 Temporary Concrete Barrier for Stage Construction

S2-06 Deck Repair Plan (Sheet 1 of 2) S2-07 Deck Repair Plan (Sheet 2 of 2)

S2-08 Drainage Scupper Adjustment Detail

S2-09 S. Abut. Joint Removal and Replacement (Sheet 1 of 2)

52-10 S. Abut. Joint Removal and Replacement (Sheet 2 of 2)

S2-11 N. Abut. Joint Removal and Replacement (Sheet 1 of 2) S2-12 N. Abut. Joint Removal and Replacement (Sheet 2 of 2)

S2-13 Pier 3 Joint Removal and Replacement (Sheet 1 of 2)

S2-14 Pier 3 Joint Removal and Replacement (Sheet 2 of 2) S2-15 Parapet Repairs (Sheet 1 of 2)

S2-16 Parapet Repairs (Sheet 2 of 2)

S2-17 Preformed Joint Strip Seal

S2-18 Framing Plan Steel Repairs (Sheet 1 of 2)

S2-19 Framing Plan Steel Repairs (Sheet 2 of 2)

S2-20 Structural Steel Repair Details (Sheet 1 of 2) S2-21 Structural Steel Repair Details (Sheet 2 of 2)

52-22 South Abutment Repairs

S2-23 North Abutment Repairs

S2-24 Pier 1 Repairs S2-25 Pier 2 Repairs

52-26 Pier 3 Repairs

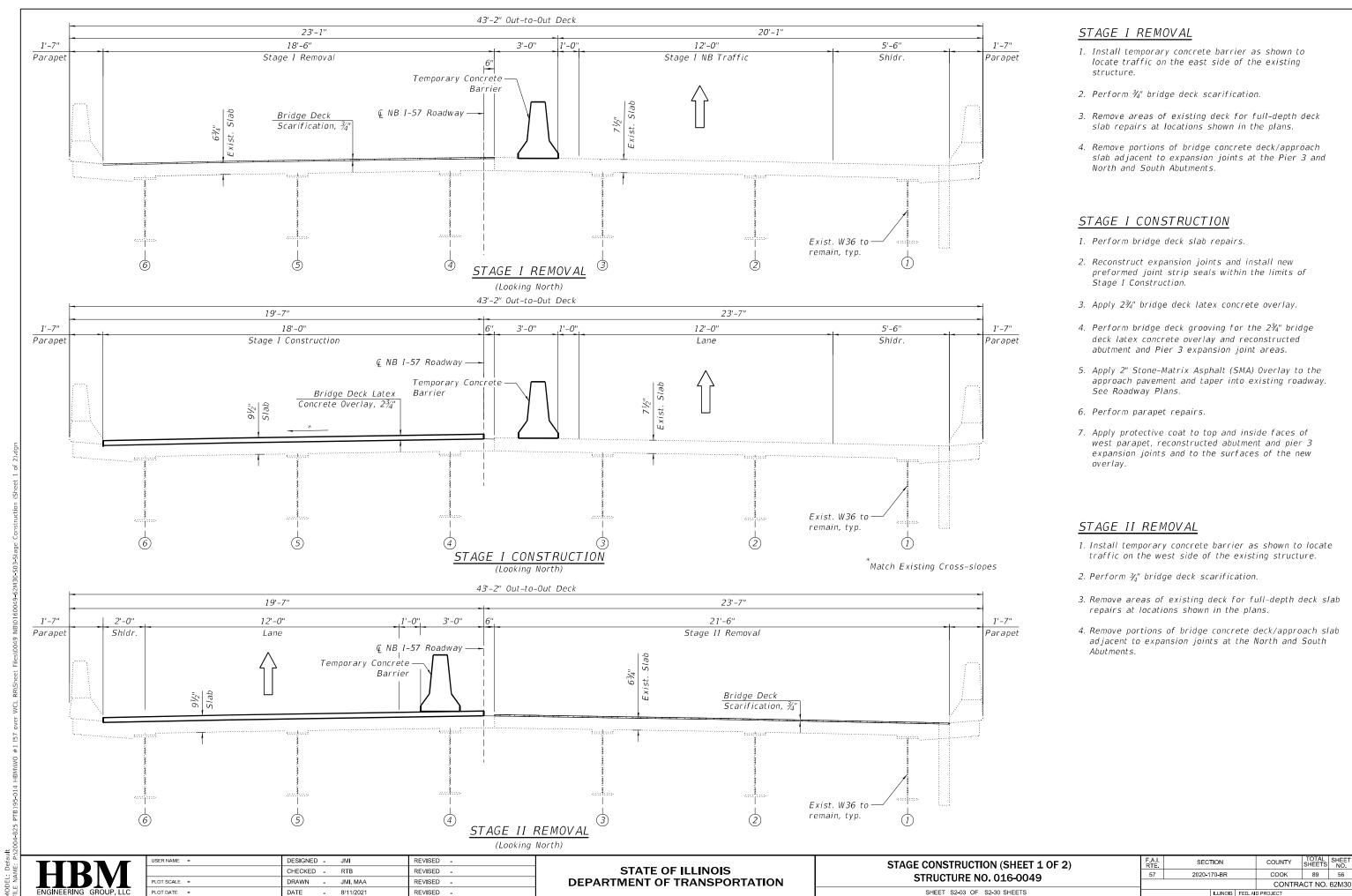
52-27 Pier 4 Repairs S2-28 Pier 5 Repairs

52-29 Slope Wall Repairs

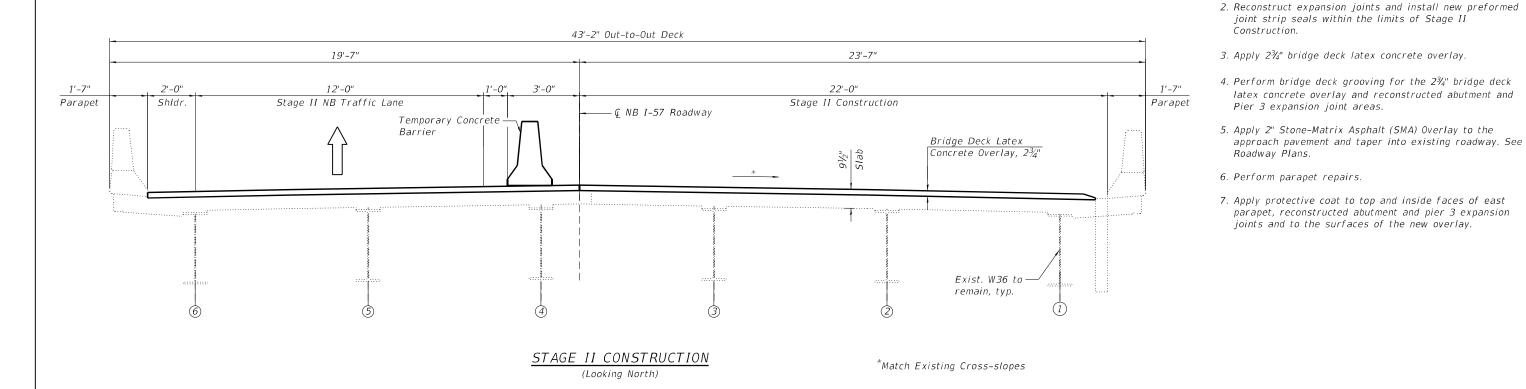
S2-30 Bar Splicer Assembly and Mechanical Splicer Details

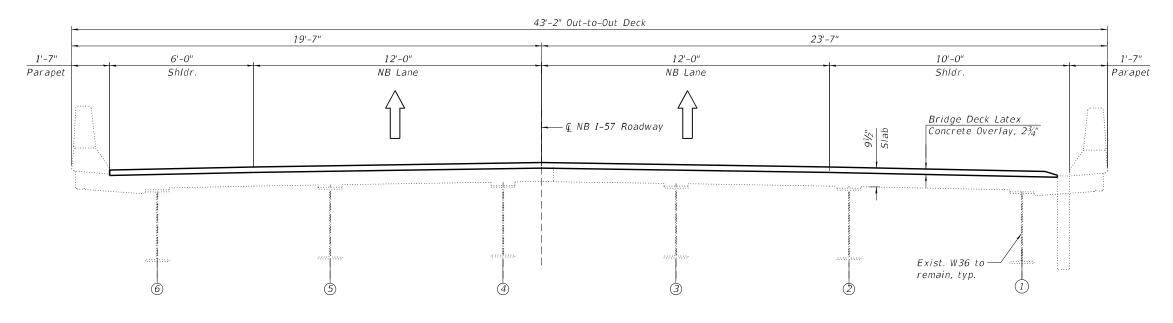
TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	Cu Yd	-	15	15
Concrete Removal	Cu Yd	20.6	-	20.6
Slope Wall Removal	Sq Yd	-	42	42
Protective Shield	Sq Yd	240	-	240
Concrete Superstructure	Cu Yd	22.9	-	22.9
Bridge Deck Grooving	Sq Yd	1705	-	1705
Protective Coat	Sq Yd	2212	-	2212
Furnishing And Erecting Structural Steel	Pound	2310	-	2310
Reinforcement Bars, Epoxy Coated	Pound	3770	-	3770
Bar Splicers	Each	56	-	56
Slope Wall 4 Inch	Sq Yd	-	42	42
Preformed Joint Strip Seal	Foot	132	-	132
Concrete Sealer	Sq Ft	-	384	384
Epoxy Crack Injection	Foot	-	214	214
Approach Slab Repair (Partial Depth)	Sq Yd	0.1	-	0.1
Structural Steel Removal	Pound	2310	-	2310
Structural Steel Repair	Pound	820	-	820
Bridge Deck Latex Concrete Overlay, 2 3/4 Inches	Sq Yd	1795	-	1795
Bridge Deck Scarification 3/4"	Sq Yd	1795	-	1795
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	2	213	215
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	-	148	148
Deck Slab Repair (Full Depth, Type I)	Sq Yd	1.1	-	1.1
Deck Slab Repair (Full Depth, Type II)	Sq Yd	3.0	-	3.0
Drainage Scuppers To Be Adjusted	Each	2	-	2
Temporary Shoring And Cribbing	Each	6	-	6



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FINAL DECK CROSS SECTION

(Looking North)

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ENGINEERING GROUP, LLC

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PLOT SCALE =	DRAWN	-	JMI, MAA	REVISED -
PLOT DATE =	DATE	-	8/11/2021	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STAGE CONSTRUCTION (SHEET 2 OF 2)						
STRUCTURE NO. 016-0049						
SHEET SOLM OF SOLM SHEETS	_					

A.I. TE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
57	2020-170-BR		COOK	89	57
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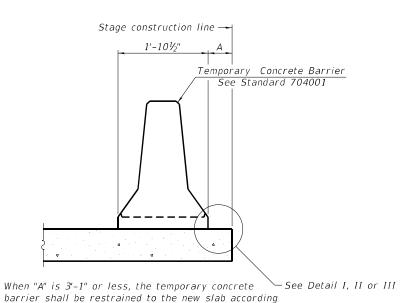
STAGE II CONSTRUCTION

1. Perform bridge deck slab repairs.

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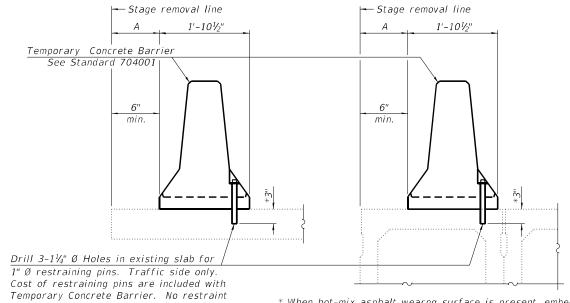
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to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

NEW SLAB OR NEW DECK BEAM



* When hot-mix asphalt wearng surface is present, embedment shall be 3" plus the wearing surface depth.

EXISTING DECK BEAM

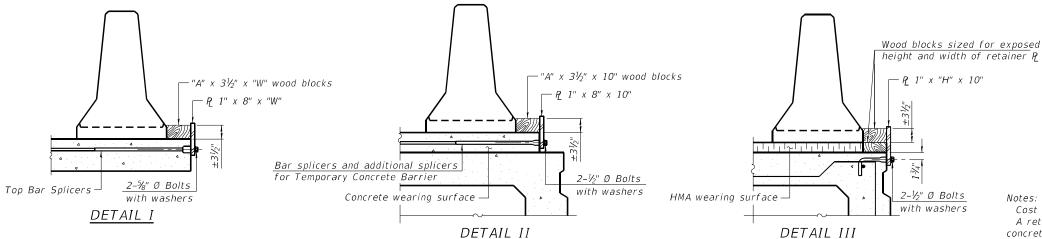
1x8 UNC US Std. 11/16" I.D. x 21/2" O.D. x approx. 8 guage thick washer RESTRAINING PIN

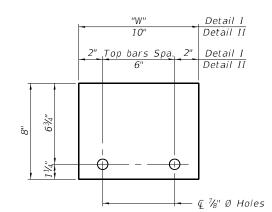
2-17-2017

SECTIONS THRU SLAB OR DECK BEAM

is required when "A" is greater than 3'-1".

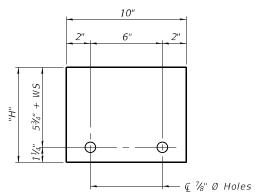
EXISTING SLAB





STEEL RETAINER P. 1" x 8" x "W"

(Detail I and II)



STEEL RETAINER P 1" x "H" x 10" (Detail III)

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate Q of each temporary concrete barrier.

BAR SPLICER FOR #4 BAR - DETAIL III

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

Detail I - Installation for a new bridge deck or bridge slab.

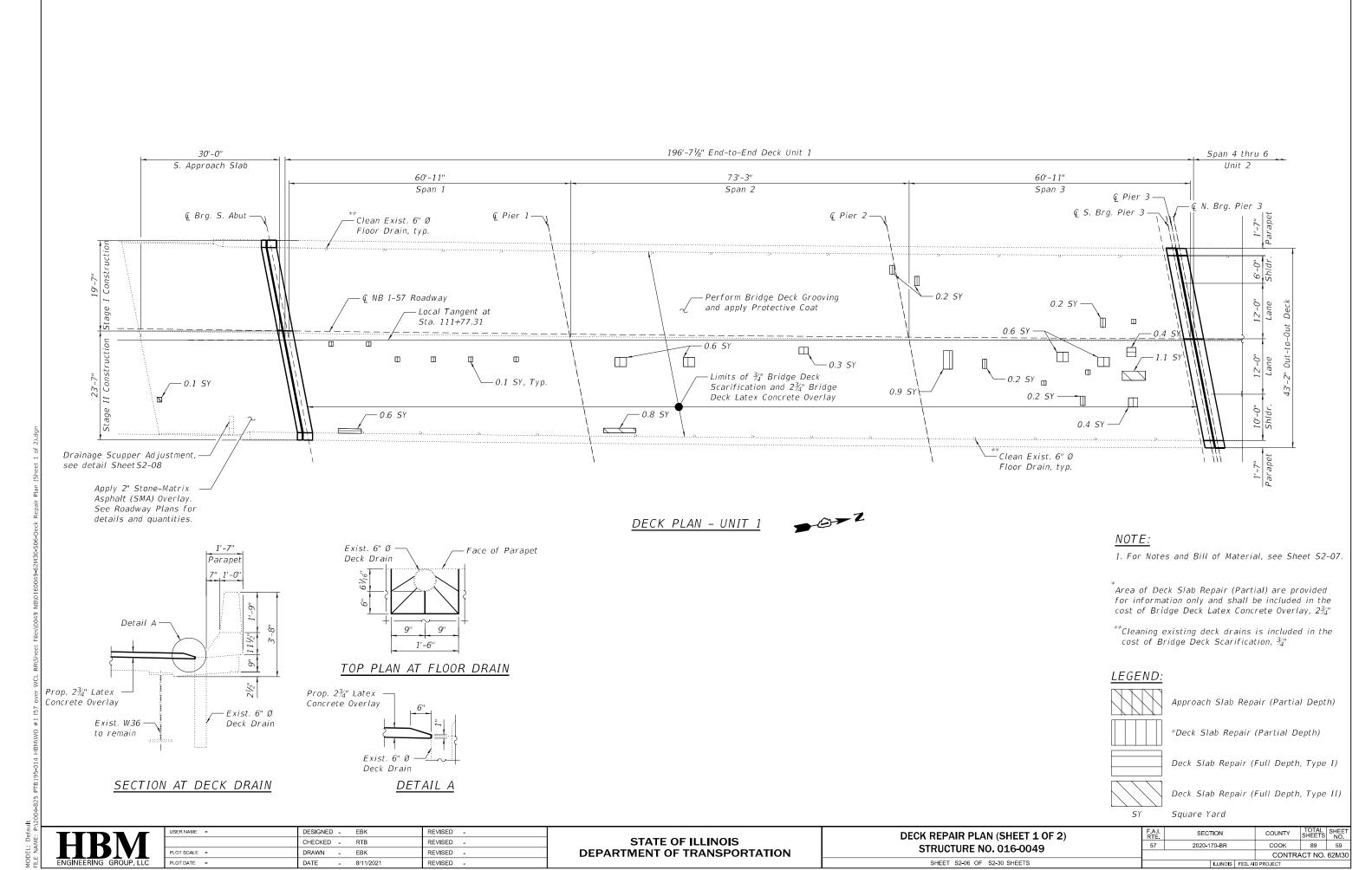
Detail II - Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

DESIGNED - JMI REVISED . CHECKED - RTB REVISED -REVISED DATE - 8/11/2021 REVISED -

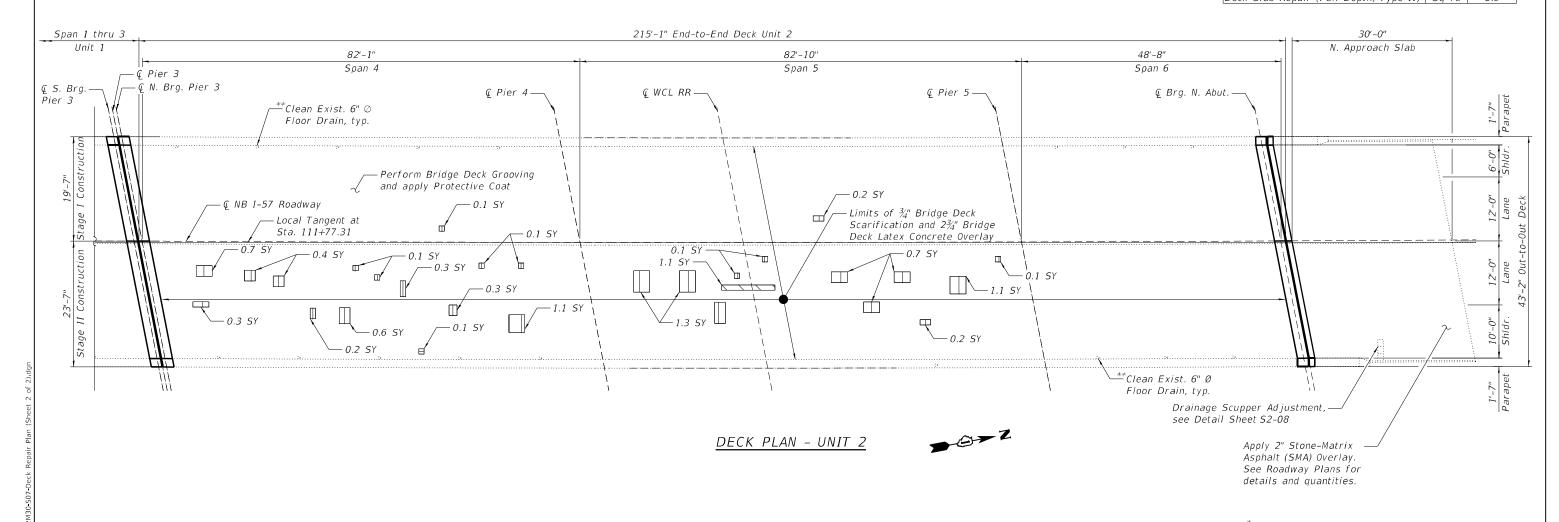
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION COUNTY TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION 2020-170-BR COOK 89 58 **STRUCTURE NO. 016-0049** CONTRACT NO. 62M30 SHEET S2-05 OF S2-30 SHEETS



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ITEM	UNIT	QUANTITY
Bridge Deck Grooving	Sq Yd	1705
Protective Coat	Sq Yd	2212
Approach Slab Repair (Partial Depth)	Sq Yd	0.1
Bridge Deck Latex Concrete Overlay, 2 3/4 Inches	Sq Yd	1795
Bridge Deck Scarification 3/4"	Sq Yd	1795
Deck Slab Repair (Full Depth, Type I)	Sq Yd	1.1
Deck Slab Repair (Full Depth, Type II)	Sa Yd	3.0



^{*}Area of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 23/4"

NOTES:

- 1. Areas of deck repair shown are estimated. The Engineer shall show actual locations of deck repairs at the time of construction.
- 2. For South, North and Pier 3 transverse joint removal and reconstruction, see Sheets S2-09 thru S2-14.
- 3. Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.

LEGEND:

* Deck Slab Repair (Partial Depth)

Deck Slab Repair (Full Depth, Type I)

Deck Slab Repair (Full Depth, Type II)

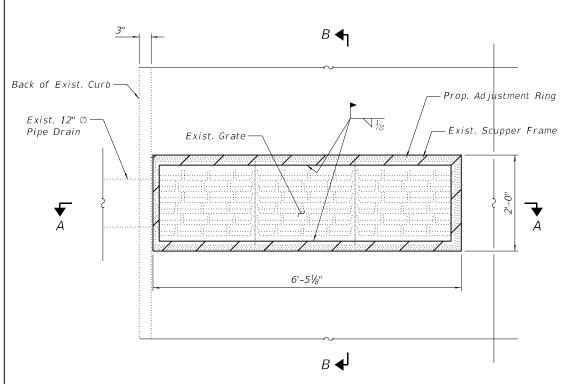
Square Yard



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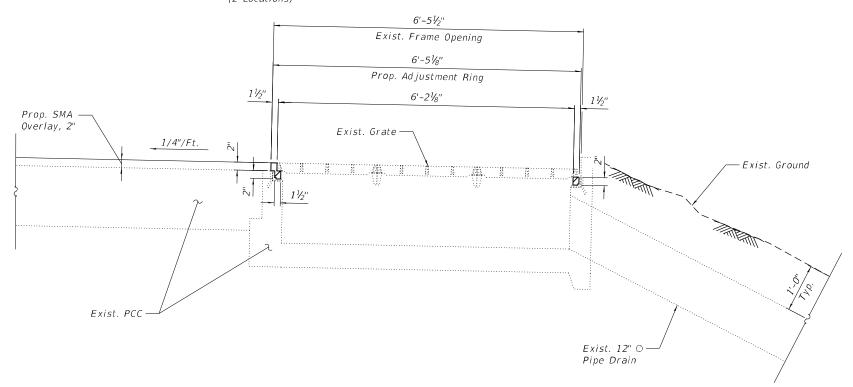
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57	2020-170-BR		соок	89	60
			CONTRA	CT NO.	62M3
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^{***}Cleaning existing deck drains is included in the cost of Bridge Deck Scarification, ¾"



TYPICAL SCUPPER PLAN

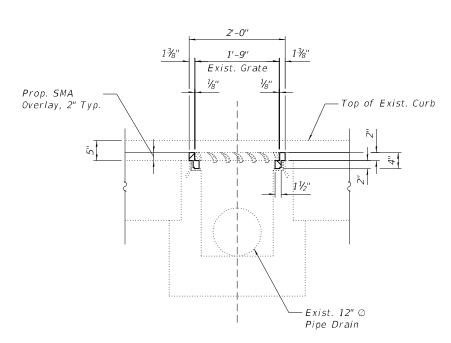
(2 Locations)



SECTION A-A

NOTES

- 1. The Contractor shall field verify Existing Dimensions and Details of the Existing Scuppers and make necessary adjustments prior to construction of New Adjusting Ring or ordering of material for Adjusting Drainage Scuppers.
- 2. All Cast Iron Parts shall be Grey Iron conforming to the requirements of AASHTO M 105, Class 35B.
- 3. Cast Iron Parts shall be unfinished.
- 4. The Contractor shall take appropriate measures to ensure that Protective Coat is not applied to the scuppers.
- 5. Adjusting Ring shall be from Neenah or approved equal. Structural steel weldments or equal section and of the same configuration may be submitted in place of Cast Iron. Fillet or full penetration welds may be used for weldments. Details shall be submitted to the Engineer for approval.
- 6. Provide a $\frac{1}{6}$ " Fillet Weld around perimeter of new Adjusting Ring to secure to existing Scupper.
- 7. Cost of all labor and materials necessary to clean all existing floor drains and scuppers, install adjusting scupper rings, remove and reinstall grates is included in the cost for Drainage Scupper to be Adjusted.



SECTION B-B

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Drainage Scuppers To Be Adjusted	Each	2

HBM ENGINEERING GROUP, LLC

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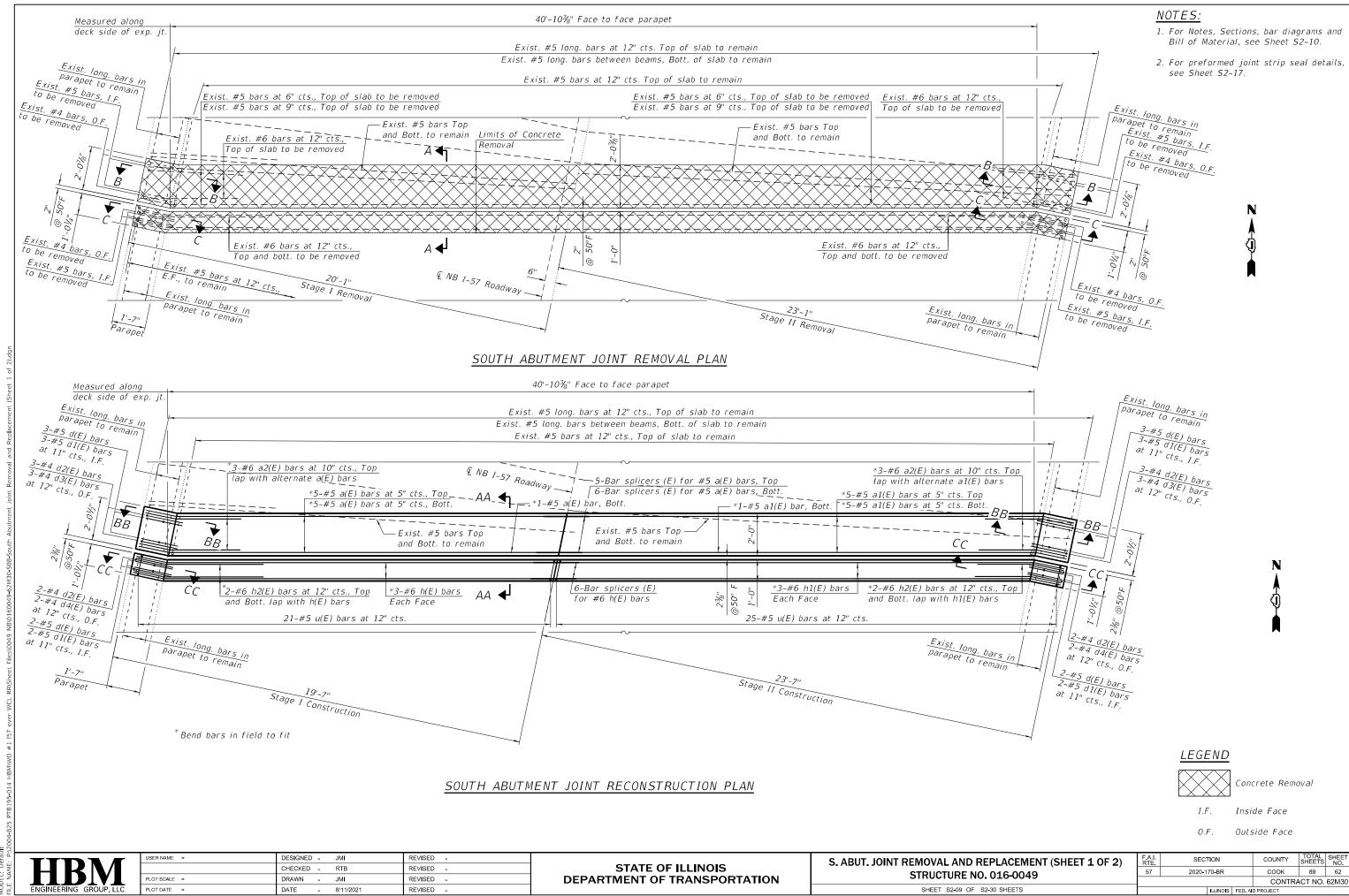
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DRAINAGE SCUPPER ADJUSTMENT DETAILS
STRUCTURE NO. 016-0049

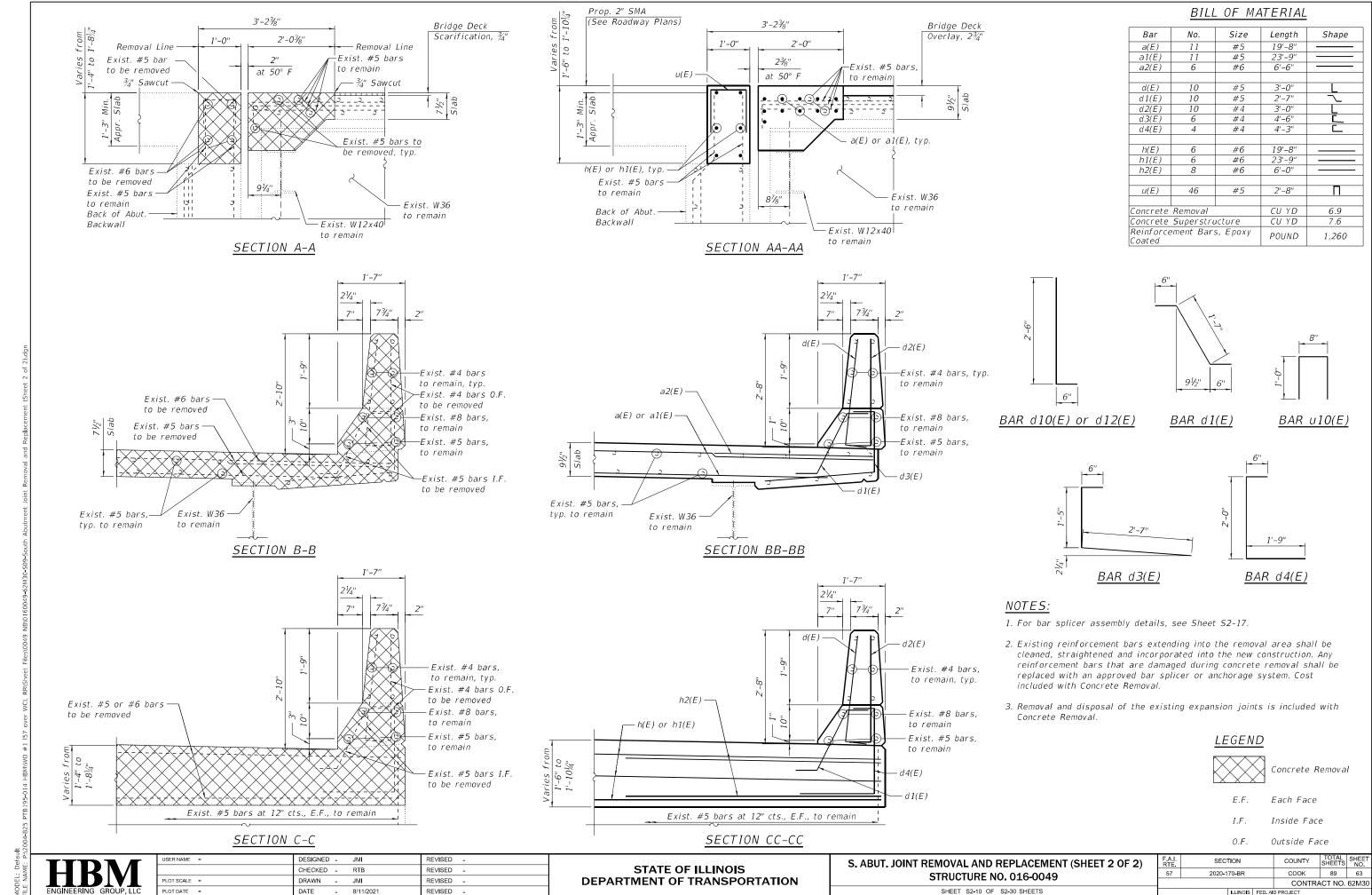
SHEET \$2-08 OF \$2-30 SHEETS

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
57	2020-170-BR		соок	89	61
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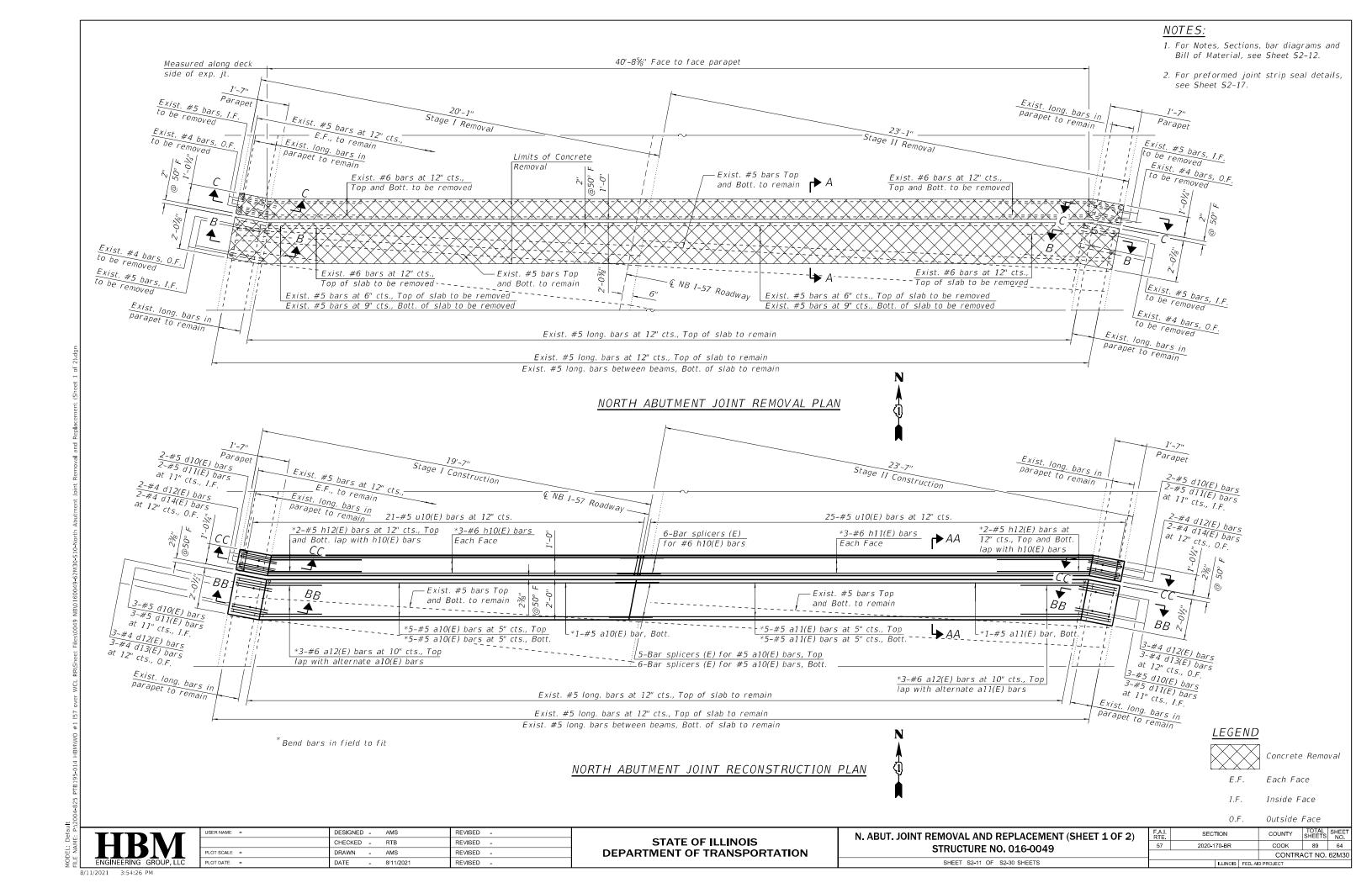
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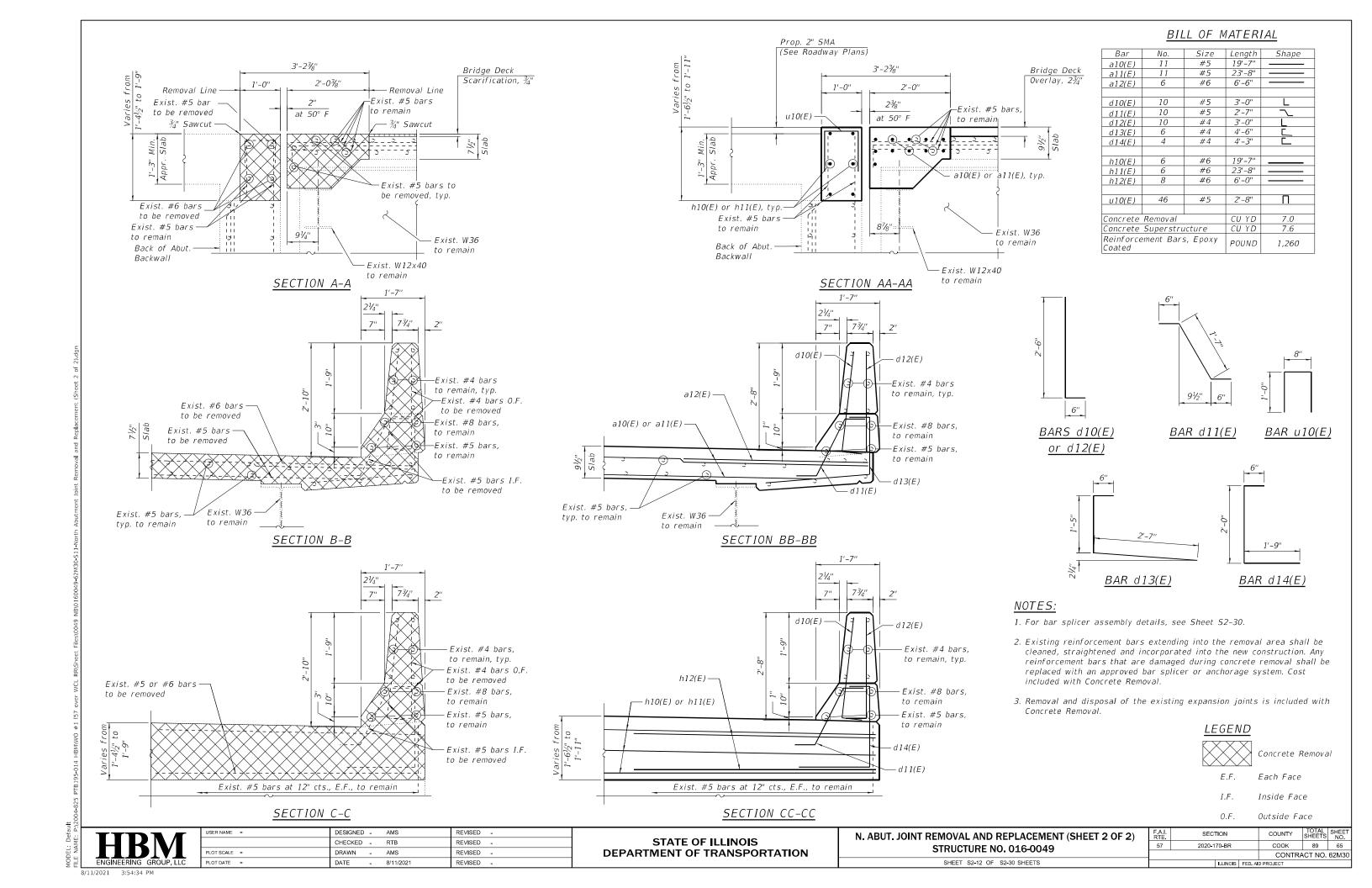


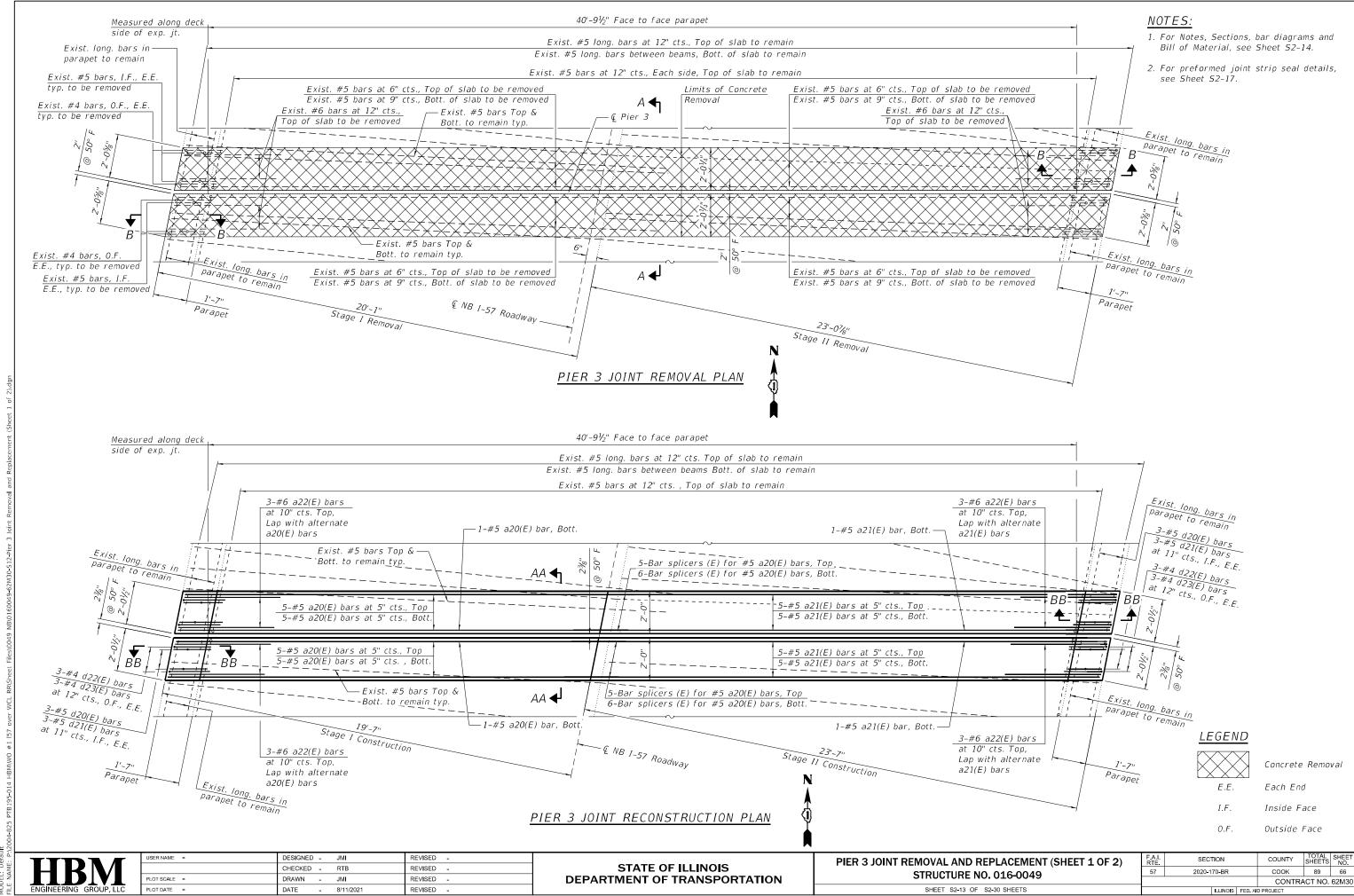
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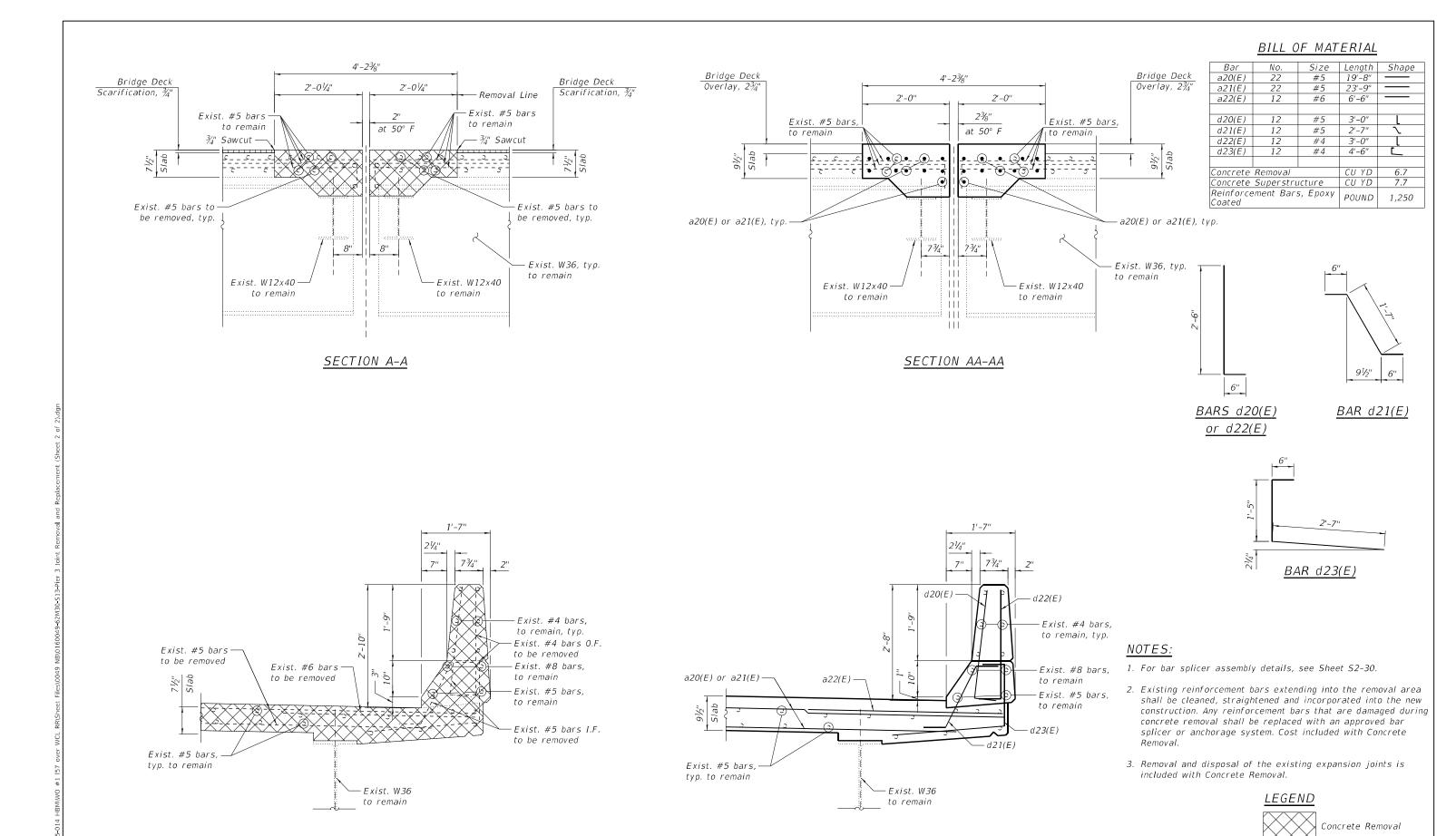


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SECTION B-B

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIER 3 JOINT REMOVAL AND REPLACEMENT (SHEET 2 OF 2)
STRUCTURE NO. 016-0049

SHEET \$2-14 OF \$2-30 SHEETS

SECTION BB-BB

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 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 57
 2020-170-BR
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 67

 CONTRACT NO. 62M30

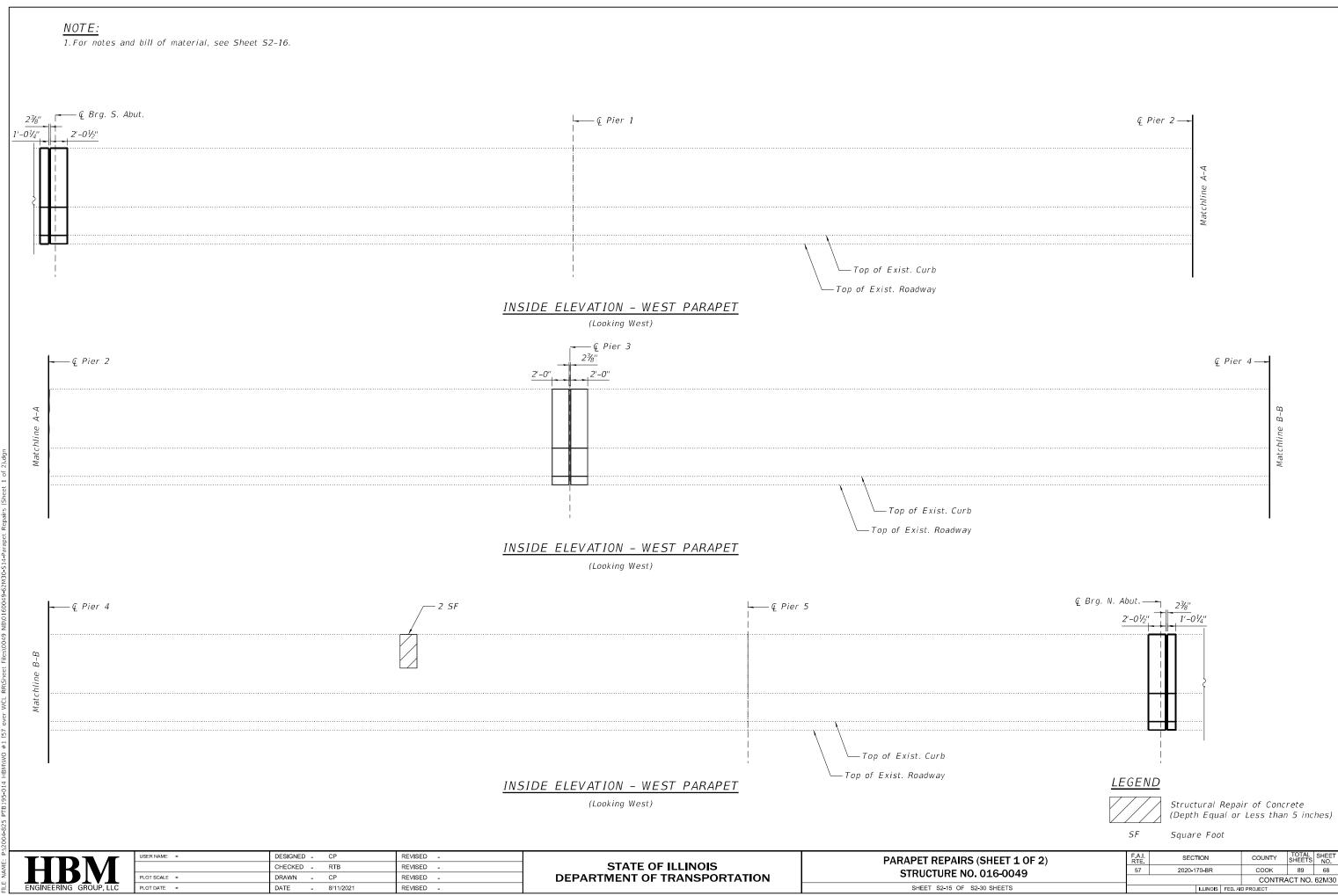
Inside Face

Outside Face

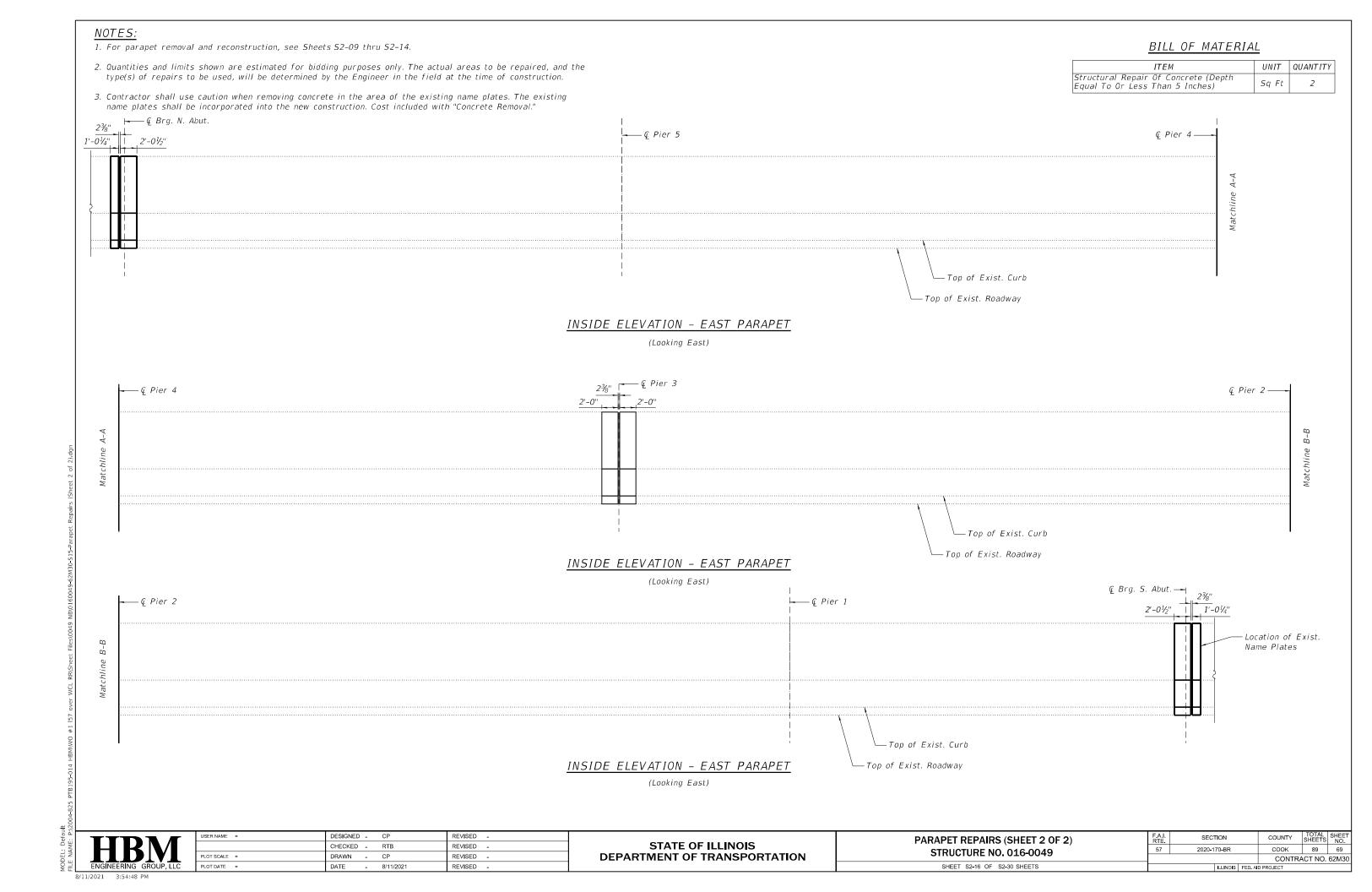
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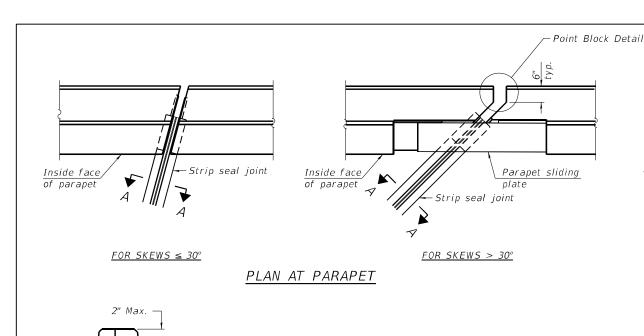
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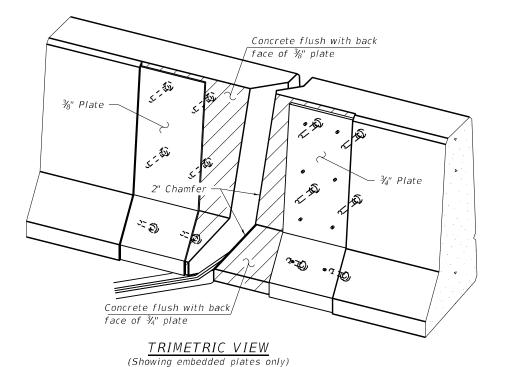


Top of locking

Top of deck

edge rail

SECTION B-B



Notes:

The strip seal shall be made continuous and shall have a minimum thickness of V_4 ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the $4\frac{1}{2}$ maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

inchorage stats included with Preformed Joint Strip Seal.

39" constant slope barrier shown, 44" constant slope barrier similar as noted.

The constant appoint below the strip seal will years based.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

ELEVATION AT PARAPET

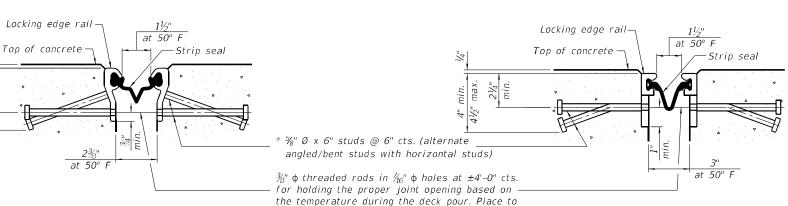
Parapet sliding

Inside Face

of Parapet

plate

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)



DETAIL A

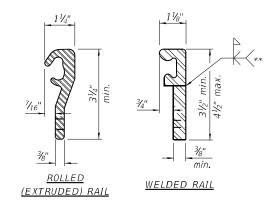
SHOWING ROLLED RAIL JOINT

off flush with the plates after concrete is set.

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

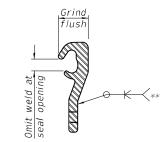
SECTION A-A

miss studs. All rods shall be burned, or sawed



LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	132



5/8" Ø x 6" Studs

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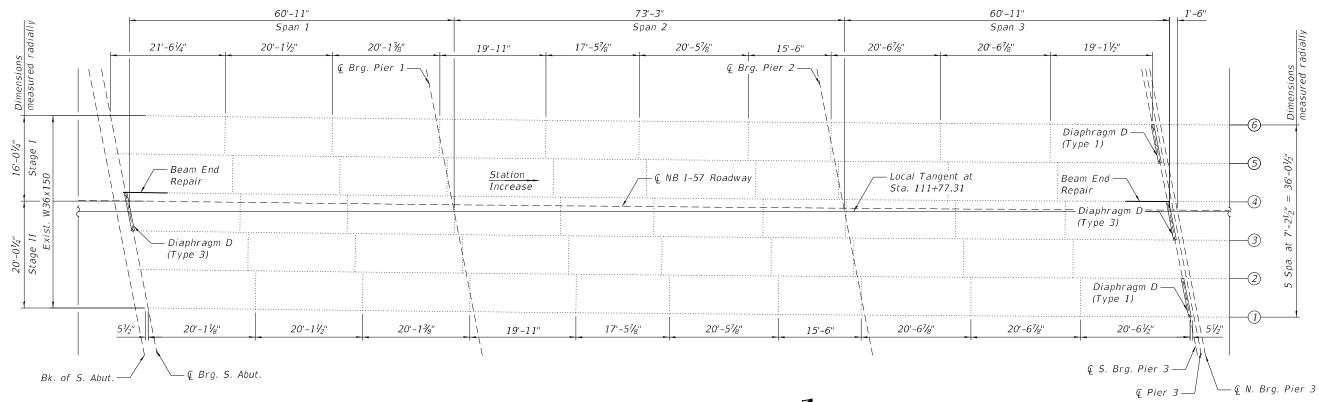
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SHOWING WELDED RAIL JOINT

PREFORMED JOINT STRIP SEAL STRUCTURE NO. 016-0049

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.	
57	2020-170-BR		соок	89	70	
CONTRACT NO.					62M3	

ENGINEERING GR



FRAMING PLAN - UNIT 1

1. For Notes and Bill of Material, see Sheet S2-19.

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

FRAMING PLAN STEEL REPAIRS (SHEET 1 OF 2) **STRUCTURE NO. 016-0049**

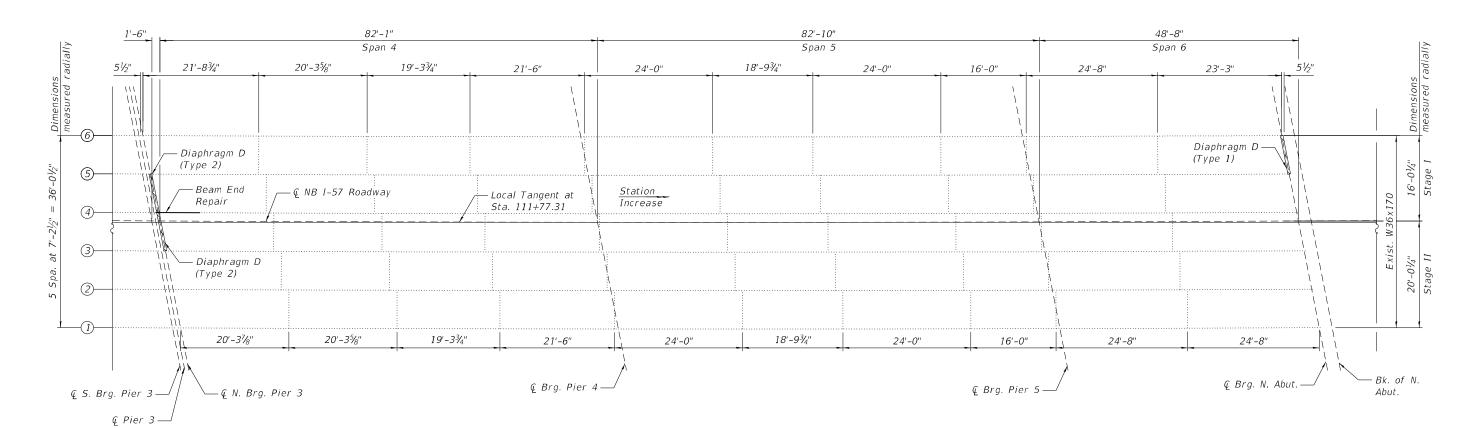
Remove and Replace Exist. Diaphragm TOTAL SHEET NO. COUNTY

Perform Beam End Repairs

SECTION COOK 89 71 2020-170-BR CONTRACT NO. 62M30 SHEET S2-18 OF S2-30 SHEETS

LEGEND:

ITEM	UNIT	QUANTIT
Furnishing And Erecting Structural Steel	Pound	2,370
Structural Steel Removal	Pound	2,490
Structural Steel Renair	Pound	820



FRAMING PLAN - UNIT 2

NOTES:

1. All work is to be performed utilized staged construction. See Sheets S2-03 and S2-04 for details.

2. For Beam End Repairs and Diaphragm Removal and Replacement Details, see Sheets S2-20 and S2-21.

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN STEEL REPAIRS (SHEET 2 OF 2)
STRUCTURE NO. 016-0049

SHEET S2-19 OF S2-30 SHEETS

Remove and Replace Exist.

Diaphragm

SECTION COUNTY SHEETS NO.

2020-170-BR COOK 89 72

CONTRACT NO. 62M30

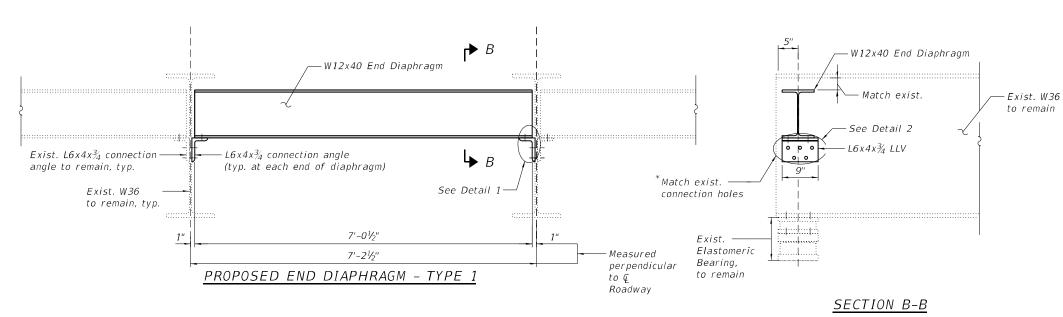
Perform Beam End Repairs

LEGEND:

MODEL: Default FILE NAME: P:\2004-825 PTB195-014 EXISTING END DIAPHRAGM REMOVAL (TYPES 1,2 & 3)

SECTION A-A

DETAIL 1



11/2" 21/2" || 21/2" 11/2" 0

DETAIL 2

NOTES:

Roadway

- 1. For locations of Diaphragm Removal/Replacement and Beam Repairs and Bill of Material, see Sheets S2-18and S2-19.
- 2. All proposed beam end repair plates and bottom flange repair angles shall conform to the requirements of AASHTO M270 Grade 50. All proposed diaphragms and their connection angles shall conform to the requirements of AASHTO M270 Grade 36.
- 3. Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Diaphragm connection holes shall be 15 /6" for 3 /4" bolts. Two hardened washers shall be required at diaphragm connections.
- 4. Existing diaphragm and connection angle removal shall be paid for as Structural Steel Removal.
- 5. All proposed beam end repair plates, bottom flange repair angles and associated bolts/fasteners shall be paid for as Structural Steel Repair. All proposed diaphragms, connection angles and associated bolts/fasteners shall be paid for as Furnishing and Erecting Structural Steel.

- 6. Load carrying components designated "CVN" shall conform to the Charpy V-Notch Impact Testing Requirements, Zone 2.
- 7. Use holes in new steel as template to field drill holes in existing steel unless noted otherwise.
- 8. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the special provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures". Cost included in Structural Steel Repair.
- * Field Drill Holes in Clip Angle to match existing holes in Beam or Diaphragm
- ** Dimension may vary in the field based on existing hole locations in beam web
- *** Exist. L6x4x $\frac{3}{4}$ connection angle at Type 3 diaphragm beam repair end to be removed

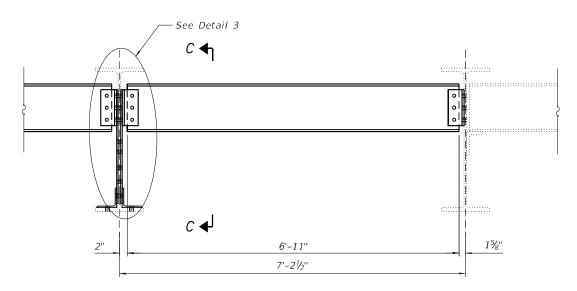


Structural Steel Removal

DESIGNED - AMS REVISED -CHECKED - RTB REVISED -REVISED -- 8/11/2021 REVISED -

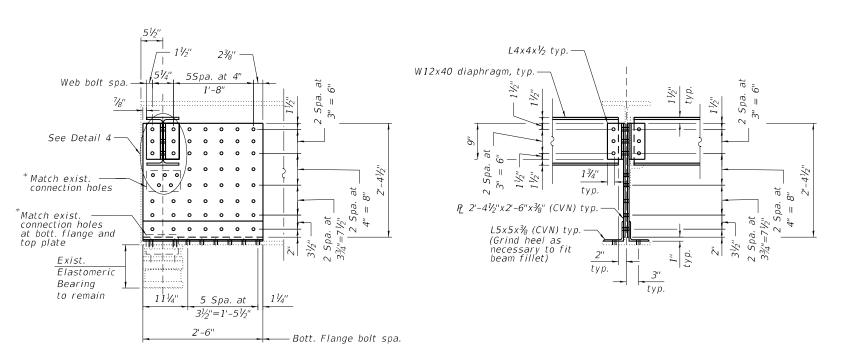
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION STRUCTURAL STEEL REPAIR DETAILS (SHEET 1 OF 2) **STRUCTURE NO. 016-0049** SHEET S2-20 OF S2-30 SHEETS

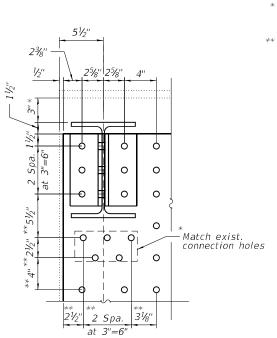
COUNTY SHEETS 57 2020-170-BR COOK 89 73 CONTRACT NO. 62M30



PROPOSED END DIAPHRAGM - TYPE 2

PROPOSED END DIAPHRAGM - TYPE 3





- * Field Drill Holes in Repair Plate and/or Angle to match existing holes in Beam or Diaphragm
- ** Dimension may vary in the field based on existing hole locations in beam web

11/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2" 21/2"

DETAIL 3 DETAIL 4 DETAIL 5

NOTES:

- 1. For diaphragm removal, notes, and Bill of Material, see Sheets S2-19 and S2-20.
- 2. Cost of field drilling shall be included in the cost of Structural Steel Repair.

SECTION C-C

BEAM END REPAIR DETAIL

3. Existing end diaphragms adjacent to diaphragms being replaced shall be temporarily supported during diaphragm replacement work. Cost included with Structural Steel Removal.

HBM
ENGINEERING GROUP, LLC

USER NAME =	DESIGNED	-	AMS	REVISED	-
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PLOT DATE =	DATE	-	8/11/2021	REVISED	-

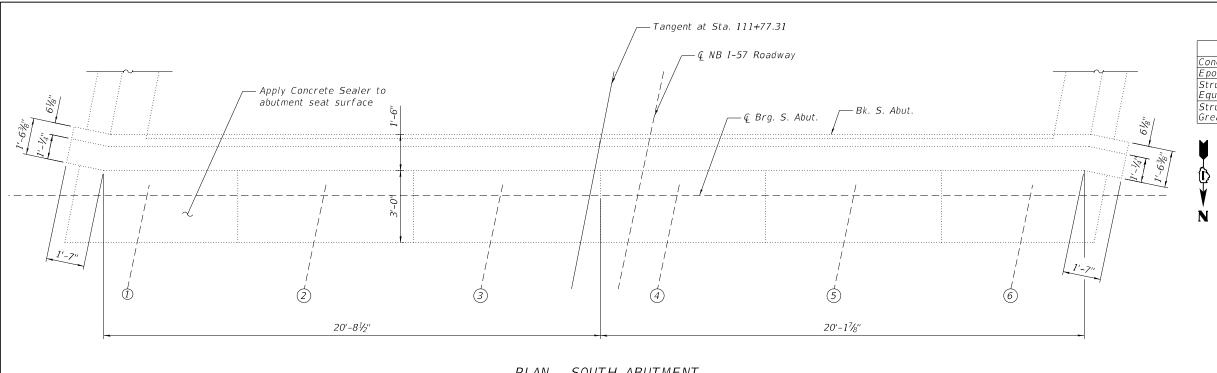
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL STEEL REPAIR DETAILS (SHEET 2 OF 2)
STRUCTURE NO. 016-0049

SHEET \$2-21 OF \$2-30 SHEETS

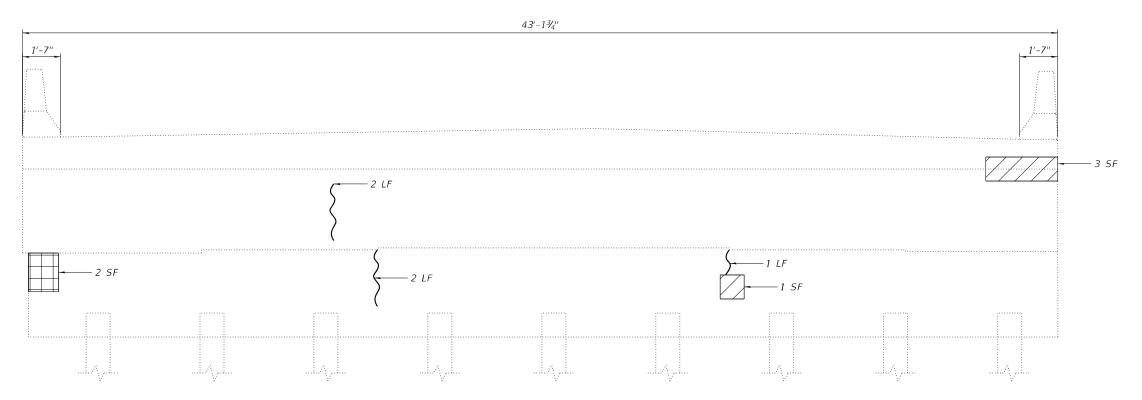
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ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	129
Epoxy Crack Injection	Foot	5
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	4
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	2

PLAN - SOUTH ABUTMENT



ELEVATION - SOUTH ABUTMENT

(Looking South)

NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

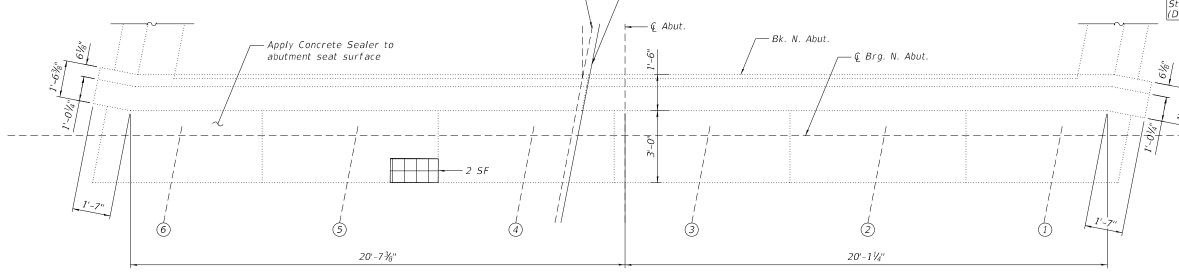
- Square Foot

- Linear Foot

USER NAME =	DESIGNED	-	JMI	REVISED -
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PLOT SCALE =	DRAWN	-	JMI	REVISED -
PLOT DATE =	DATE	-	8/11/2021	REVISED -



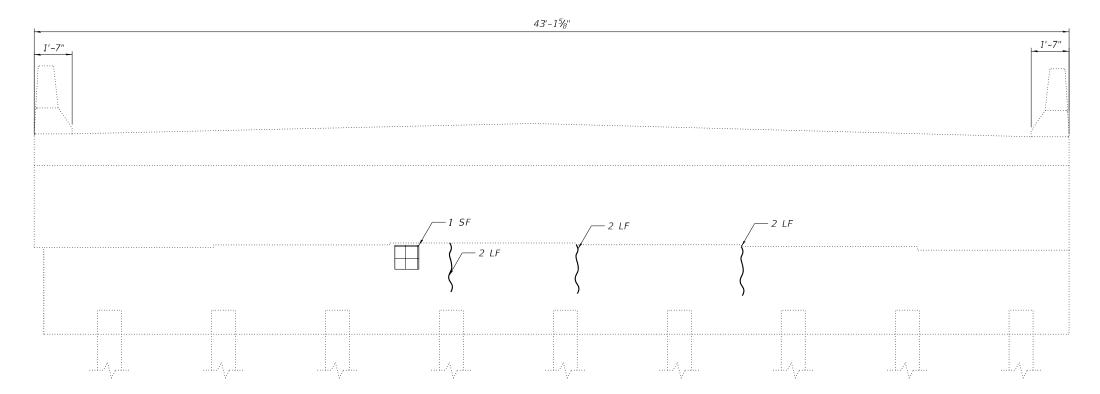
ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	129
Epoxy Crack Injection	Foot	6
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	3



€ NB I-57 Roadway -

PLAN - NORTH ABUTMENT

-Tangent at Sta. 111+77.31



ELEVATION - NORTH ABUTMENT

(Looking North)

NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

.E	GE	ND	

Structural Repair of Concrete
(Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

SF - Square Foot

- Linear Foot

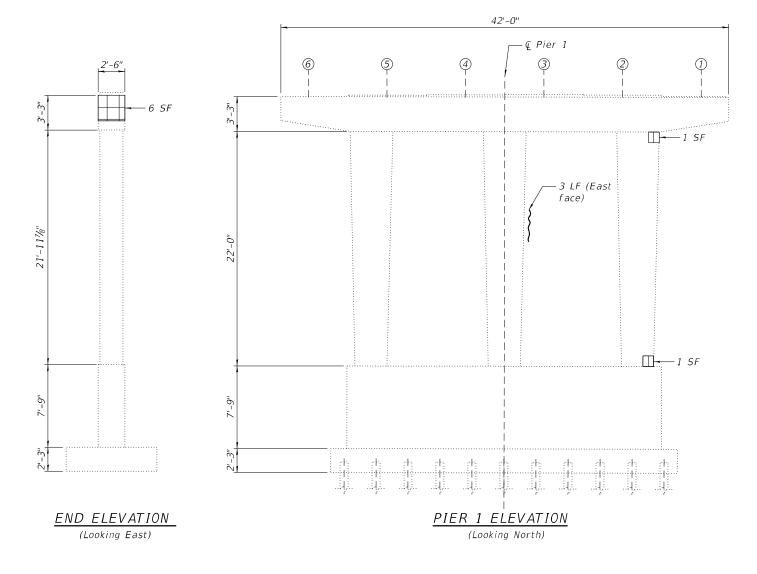
HBM ENGINEERING GROUP, LLC

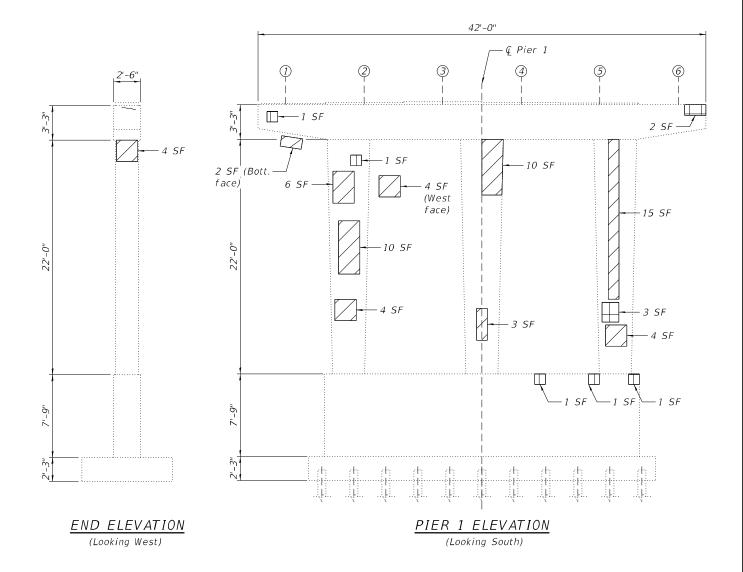
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT REPAIRS STRUCTURE NO. 016-0049

ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	3
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	62
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	18





NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

SF - Square Foot - Linear Foot

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Т	ENGINEERING GROUP, LLC

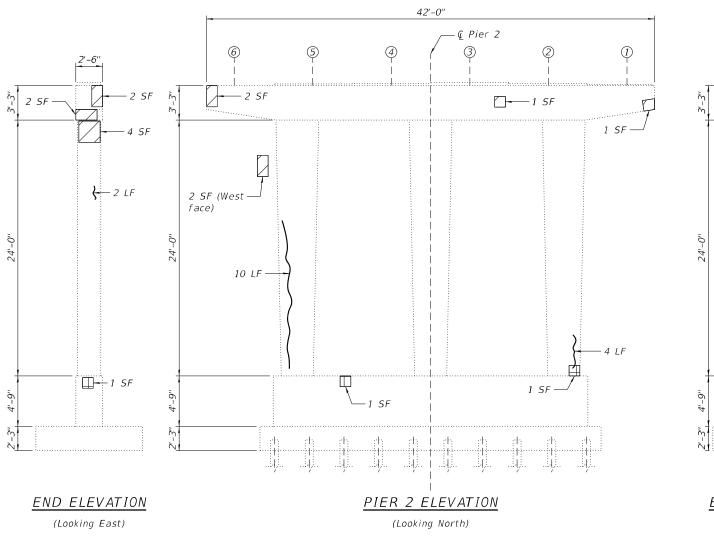
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PLOT DATE =	DATE - 8/11/2021	REVISED -

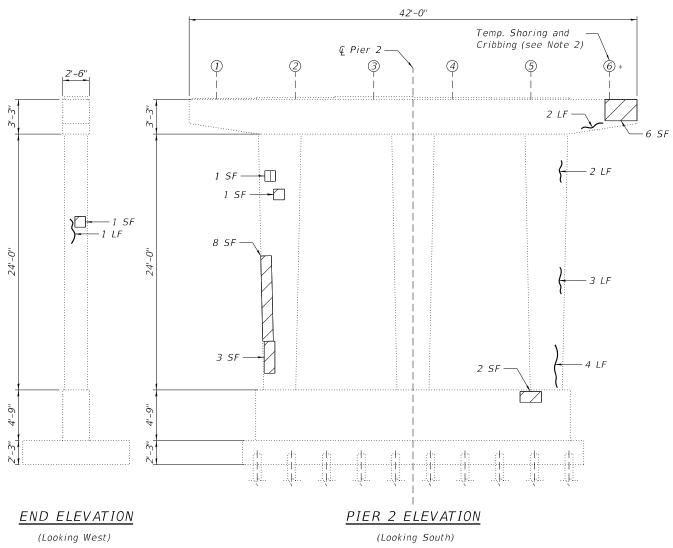
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7	2020-170-BR		соок	89	77
			CONTRA	ACT NO.	62M30
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SHEET S2-24 OF S2-30 SHEETS

ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	28
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	35
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	4
Temporary Shoring And Cribbing	Each	1





<u>NOTES:</u>

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

SUMMARY OF REACTIONS PIER 2					
R DL	(k)	89.5			
R LL	(k)	50.0			
R IM	(k)	13.0			
R Total	(k)	152.5			

<u>LEGEND</u>



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)



Epoxy Crack Injection (Width > 0.06")

SF - Square Foot

F – Linear Foot

- Temporary Shoring and Cribbing



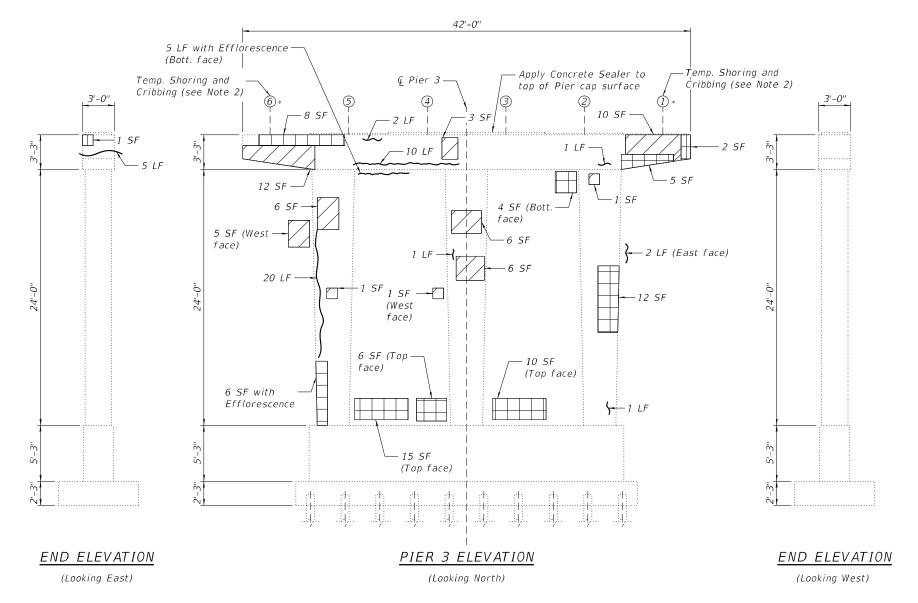
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PLOT DATE =	DATE	-	8/11/2021	REVISED -

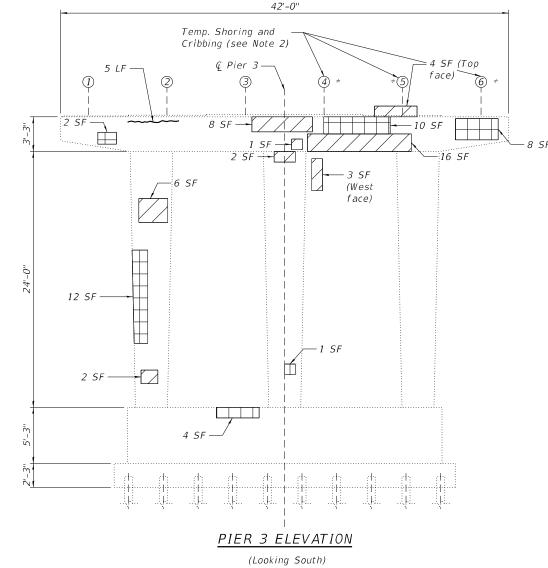
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2020-170-BR			соок	89	78
			CONTRA	CT NO.	62M3
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ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	126
Epoxy Crack Injection	Foot	52
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	93
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	106
Temporary Shoring And Cribbing	Each	5





NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

SUMMARY OF REACTIONS						
		Pier 3, Span 3	Pier 3, Span 4			
R DL	(k)	27.4	38.5			
R LL	(k)	43.6	45.77			
R IM	(k)	11.7	11.05			
R Total	(k)	82.7	95.32			
•						





Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

- Square Foot

- Linear Foot

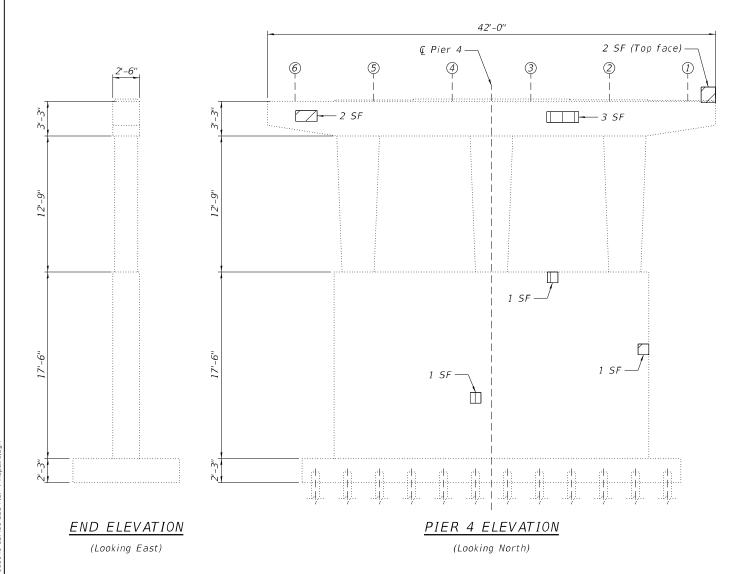
- Temporary Shoring And Cribbing

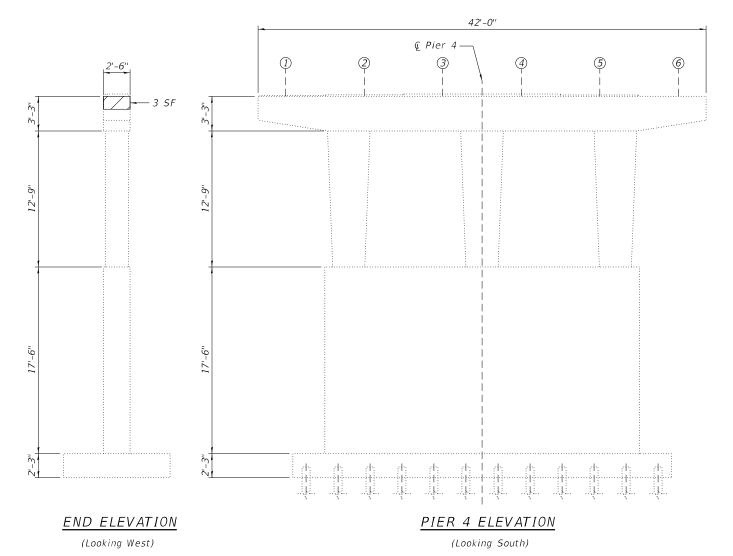


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PLOT SCALE =	DRAWN	-	JMI	REVISED	-
PLOT DATE =	DATE	-	8/11/2021	REVISED	-

LI.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
7 2020-170-BR		соок	89	79	
			CONTRA	ACT NO.	62M30
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ITEM	UNIT	QUANTITY
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	8
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	5





NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)

SF - Square Foot

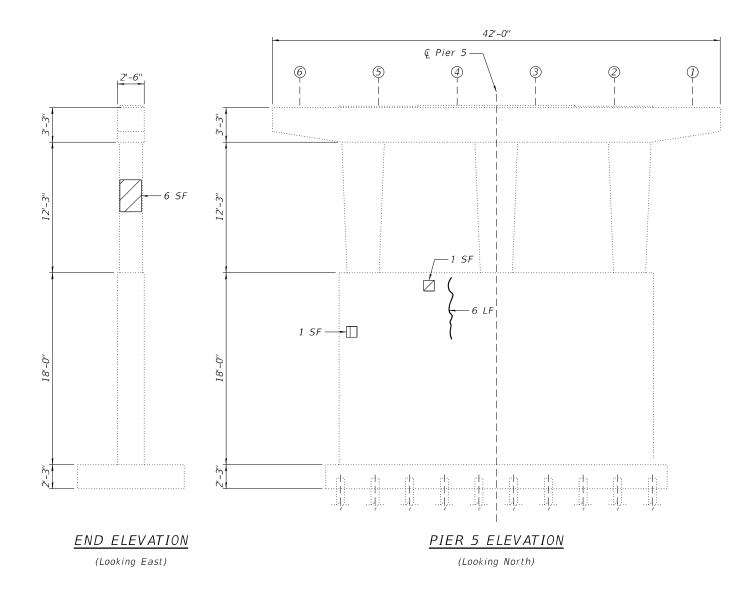


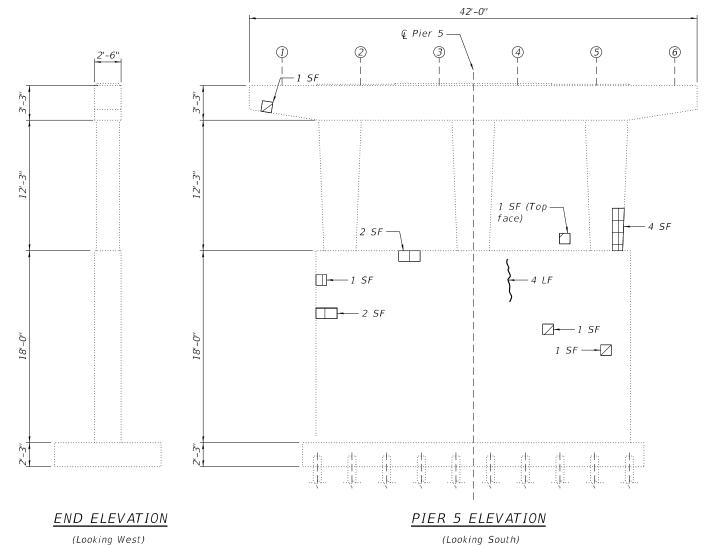
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F.A.I RTE	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
57	2020-170-BR	соок	89	80
		CONTRA	ACT NO.	62M3
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ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	10
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	11
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	10





NOTES:

Quantities and limits shown are estimated for bidding purposes only. The
actual areas to be repaired, and the type(s) of repairs to be used, will be
determined by the Engineer in the field at the time of construction.

<u>LEGEND</u>

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

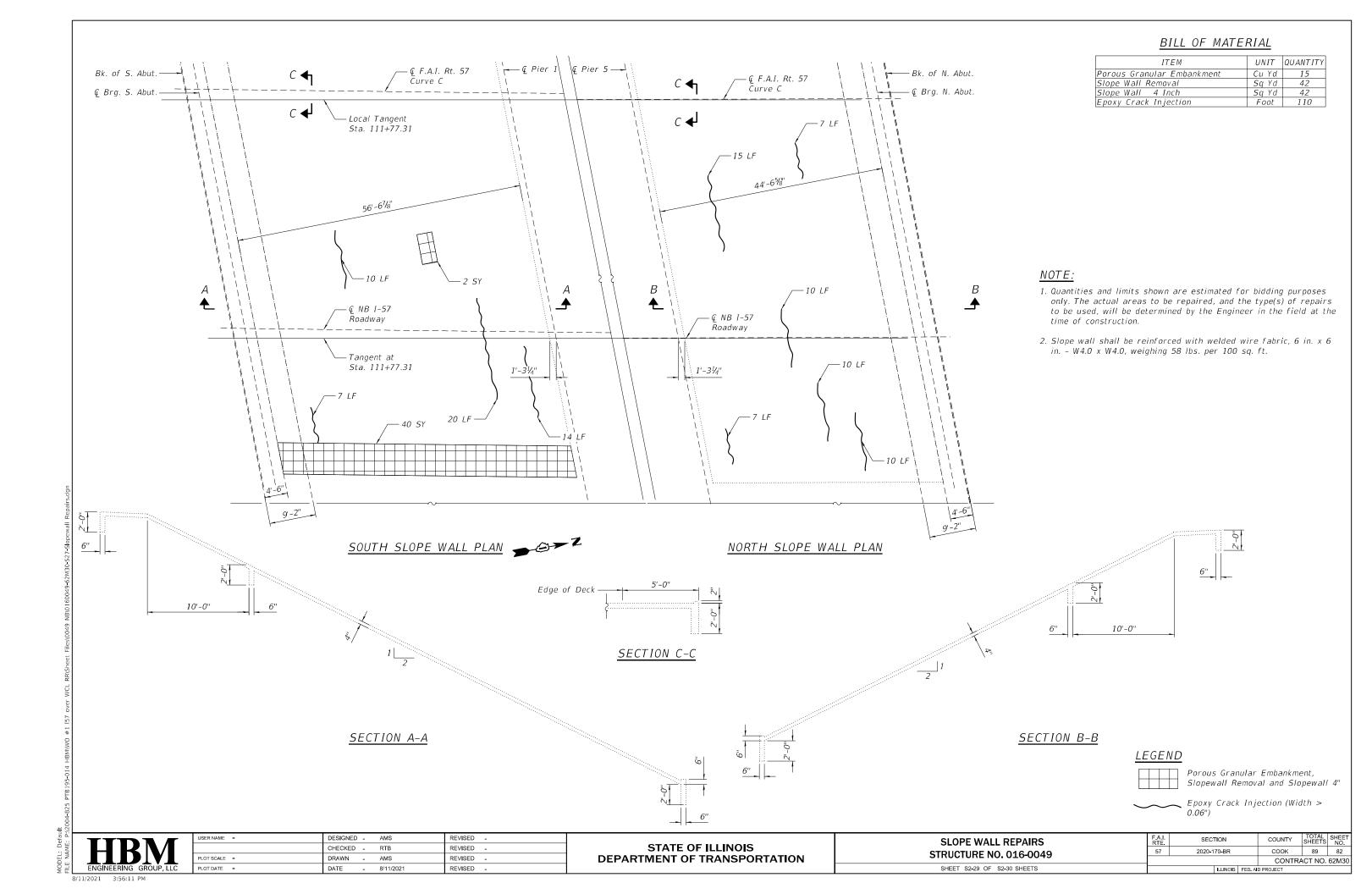
- Square Foot

- Linear Foot



USER NAME =	DESIGNED - JMI	REVISED -
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PLOT SCALE =	DRAWN - JMI	REVISED -
PLOT DATE =	DATE - 8/11/2021	REVISED -

F.A.I. RTE	SECTION			COUNTY	TOTAL SHEETS	SHEE NO.
57	57 2020-170-BR			соок	89	81
			CONTRACT NO. 62M3			
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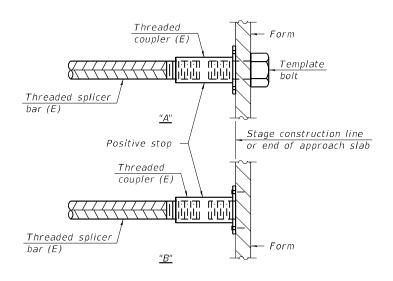
STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

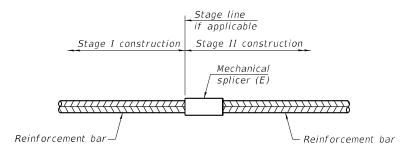
Location	Bar No. assemblies size required		Minimum lap length		
South Abutment Deck	#5	1 1	3'-6"		
South Abutment Bk. Wall	#6	6	4'-0"		
North Abutment Deck	#5	1 1	3'-6"		
North Abutment Bk. Wall	#6	6	4'-0"		
Pier 3	#5	22	3'-6"		



INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required		

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum $60\ ksi$ yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

HBM ENGINEERING GROUP LLC 1-1-2020

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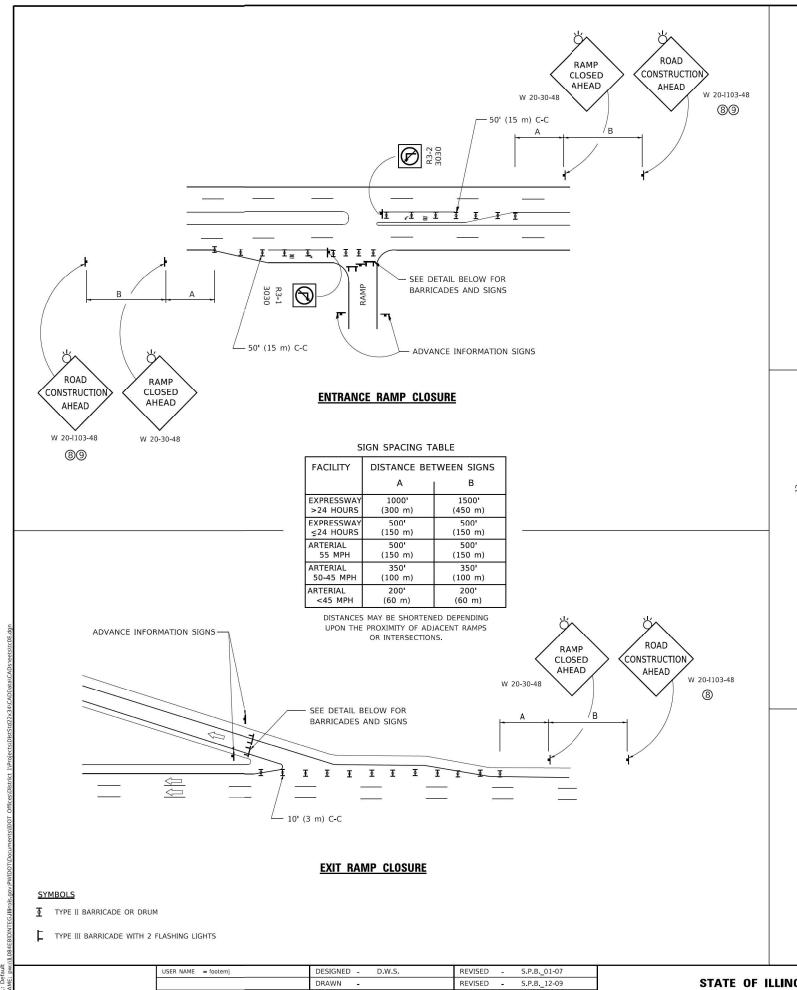
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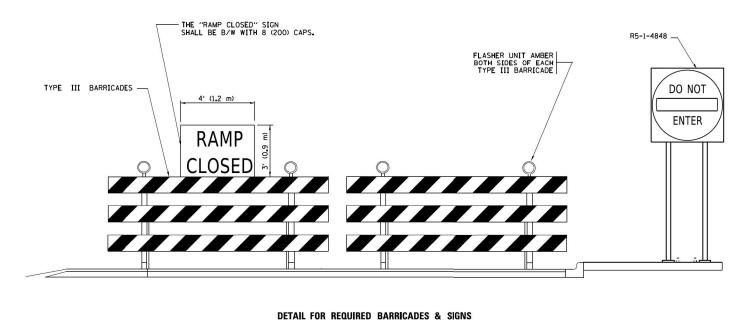
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS
STRUCTURE NO. 016-0049

SHEET S2-30 OF S2-30 SHEETS

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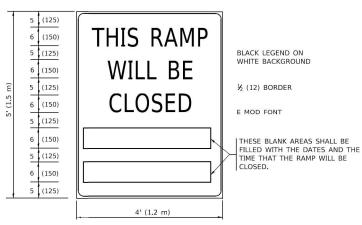
RAMP CLOSURE ADVANCE INFORMATION SIGN



RAMP CLOSURE ADVANCE WARNING SIGN

BLACK LEGEND ON ORANGE BACKGROUND MOUNTED DIAGONALLY E MOD FONT 1 (25) BORDER

THESE SIGNS ARE REQUIRED ON ALL THE EXIT GUIDE SIGNS FOR EXIT RAMPS THAT WILL BE CLOSED FOR MORE THAN FOUR (4) CONSECUTIVE DAYS



THESE SIGNS ARE REQUIRED ON BOTH SIDES OF THE RAMP, MINIMUM OF 1 WEEK IN ADVANCE OF THE CLOSURE.

THESE SIGNS SHALL BE FABRICATED AND PAID FOR ACCORDING TO THE TEMPORARY INFORMATION SIGNING SPECIAL PROVISION

GENERAL NOTES:

- CONES MAY BE SUBSTITUTED FOR DRUMS OR TYPE II 1 BARRICADES DURING DAY OPERATIONS. CONES SHALL BE A MINIMUM OF 28 (700) HIGH.
- 2 VERTICAL BARRICADES SHALL NOT BE USED FOR RAMP CLOSURES.
- 3 A FLAGGER SHALL BE POSITIONED AT EACH CLOSED RAMP THAT IS OPEN TO CONSTRUCTION VEHICLES, PRECEEDED BY A W20-7 FLAGGER WARNING SIGN.
- ALL ROUTE MARKERS AND TRAILBLAZER ASSEMBLIES WHICH DIRECT MOTORISTS TO A CLOSED ENTRANCE RAMP SHALL BE COVERED WHEN THE RAMP IS CLOSED FOR MORE THAN FOUR (4) DAYS.
- 5 THE SIGNING AND BARRICADING WHICH IS REQUIRED BY THIS DETAIL SHALL BE INCLUDED IN THE COST OF TRAFFIC CONTROL AND PROTECTION (EXPRESSWAYS).

SCALE: NONE

- 6 AUTHORIZATION FROM THE DISTRICT'S BUREAU OF TRAFFIC IS REQUIRED FOR ALL RAMP CLOSURES.
- THE RAMP CLOSURE ADVANCE INFORMATION SIGNS SHALL BE ERECTED IF THE CLOSURE TIME EXCEEDS TWENTY-FOUR (24) HOURS. ADDITIONAL ADVANCE WARNING SIGNS ON EXIT GUIDE SIGNING WILL BE REQUIRED FOR EXIT RAMP CLOSURES THAT EXCEED FOUR (4) DAYS IN LENGTH
- (8) ROAD CONSTRUCTION AHEAD SIGNS MAY BE OMITTED WHEN THIS DETAIL IS USED IN CONJUNCTION WITH OTHER TRAFFIC CONTROL THAT ALREADY INCLUDES A ROAD CONSTRUCTION AHEAD SIGN.
- ARTERIAL ROAD CONSTRUCTION AHEAD SIGNS SHALL BE INSTALLED ON THE LEFT SIDE OF TRAFFIC IF THE MEDIAN IS MORE THAN 10 FT WIDE.

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

USER NAME = footemj	DESIGNED - D.W.S.	REVISED	-	S.P.B01-07
	DRAWN -	REVISED	•	S.P.B12-09
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

ENTRANCE_AND_EXIT_RAMP **CLOSURE DETAILS** SHEET 1 OF 1 SHEETS STA

TO STA

SECTION COUNTY 2020-170-BR COOK 89 84 TC-08 CONTRACT NO. 62M30

