

Alternate Direction of Horizontal Diagonal Bracing for Each Bay in

Planes of Upper and Lower Chords

Upper Chord

Bracing, typ.

Structure

Number

1C0991055R251.

	Station	Design Truss Type	Cantilever Length (L)	Elev. A	Dim, D	Ds	Total Sign Area	]
,56	277+45	III-C-A	40'-0"	595.86	14'-6"	12'-6"	312.5 sq. ft.	
	Tru	iss Type A			Maximum			
	$\frac{I}{II}$	<u>с а</u>	<u></u>					
	III	-C-A	400 Sq		40		6 U-	per Chord
	15'-0'' max.	Махі	30 p.s.f. d imum Sign (See Table imum Leng	Area e)	10 p		30^-0^' Max.	
	Τ		rs shown d	are basis	TNG DIA	GRAM T. Standar	Botton Base	
		equire spe						

(1) After adjustments to level truss and insure adequate vertical clearance, all top and leveling nuts shall be tightened against the base plate with a minimum torque of 200 lb.-ft. Stainless steel mesh shall then be placed around the perimeter of the base plate. Secure to base plate with stainless steel banding.

## Note:

Trusses shall be shipped individually with adequate provision to prevent detrimental motion during transport. This may require ropes between horizontals and diagonals or energy dissipating (elastic) ties to the vehicle. The contractor is responsible for maintaining the configuration and protection of the trusses.

## TOTAL BILL OF MATERIAL

ІТЕМ	UNIT	TÖTAL
OVERHEAD SIGN STRUCTURE CANTILEVER TYPE I C A	Foot	
OVERHEAD SIGN STRUCTURE CANTILEVER TYPE II C A	Foot	
OVERHEAD SIGN STRUCTURE CANTILEVER TYPE III-C-A	Foot	40'-0"
OVERHEAD SIGN STRUCTURE WALKWAY, TYPE A	Foot	25'-5"
DRILLED SHAFT CONCRETE FOUNDATIONS	Cu. Yds.	12.6
ROCK EXCAVATION	Cu. Yds.	8,3

DESIGN: AASHTO Standar Luminaires and Traffic Sign

CONSTRUCTION: Current (a Specifications for Road and Provisions. ("Standard Spi

LOADING: 90 M.P.H. WIND

WALKWAY LOADING: Dead

DESIGN STRESSES: Field Units f'c = 3,500 p.s.l. fy = 60,000 p.s.i. (reinforce

WELDING: All welds to be accordance with current AW and the Standard Specificia

MATERIALS: Aluminum Allo ASTM A53 Grade B with a minimum yield of 46,000 p.s.i. If A500 pipe is substi diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W\*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer. The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO MI64 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to AASHTO M314 Gr. 55 with a minimum Charpy V-Notch (CVN) energy of 15 lb.-ft. at 10° F.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final ground line at each foundation shall be cleaned and coated with Bridge Seat Sealer in accordance with the Standard Specifications.

REINFORCEMENT BARS. Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

				CONTRA	CT NO	6047
	F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE
	55	2005-062	I	WILL	72	38
	STA.		1	O STA.	_	I
	FED. RC	AD DIST. NO.	ILLIN	DIS FED. AID	PROJECT	-
GENERAL NOTES	·					
d Specifications for Structural Suppol nals. ("AASHTO Specifications")	rts foi	r Highway	Sig	NS,		
t time of letting) Illinois Department   Bridge Construction, Supplemental S, ecifications")						
VELOCITY						
load plus 500 lbs. concentrated live	load.					
ement)						
continuous unless otherwise shown. IS D1.1 and D1.2 Structural Welding C tions.						
oys as shown throughout plans. All S minimum yield of 35,000 p.s.i., or As	500 Gi	rade B or	Ċ	with a		

\* If M270 Gr. 50W (M222) steel is proposed, chemistry for plate to be used shall first be approved by the Engineer as suitable for

galvanizing and welding.

SHT. S-12 OF 27 REVISIONS NAME DATE	FAI ROU CANTILEVE PLAN	RTMENT OF TRANSPORTATION TE 55 (1-80 TO U.S. 30) SIGNING WILL COUNTY R SIGN STRUCTURES & ELEVATION TRUSS & STEEL POST
	SCALE:	DRAWN BY MDB
	DATE 05/19/06	CHECKED BY MJK
	TENG	TENG & ASSOCIATES, INC. ENGINEERS/ARCHITECTS/PLANNERS CHICAGO, ILLINOIS