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This structure will retain the same number 101-0140.

All new structural steel shall conform to AASHTO Classification M-270, Gr. 36 unless otherwise noted.

Reinforcement bars shall conform to the requirements of AASH10 M-31, M-42 or M-53, Grade 60.

Prior to pouring the new concrete deck, all loose rust, loose mill scale and other loose potentially detrimental foreign material shall be removed from the surfaces of the beams or girders in contact with concrete. The cost of this work will be included in the pay item covering removal of the existing concrete. All heavy rust and other tightly adhered potentially detrimental foreign matter shall also be removed from the surfaces of the beams or girders in contact with concrete. Tightly adhered paint may remain unless otherwise noted. This removal shall be accomplished by methods that will not damage the steel. The cost of this work will be paid for according to Article 109.04 of the Standard Specifications.

Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

The existing structural steel coating contains lead. The contractor should take appropriate precautions to deal with the presence of lead on this project.

Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost shall be included in the cost of "Concrete Removal".

Existing longitudinal reinforcement extending into the removed area shall be cleaned, straightened and incorporated into the new construction. Existing transverse reinforcement may be cut as shown and removed.

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Structure

LOCATION SKETCH

During construction operations, the Contractor shall provide temporary shielding from shoulder to shoulder of the roadway crossed. See Special Provisions.

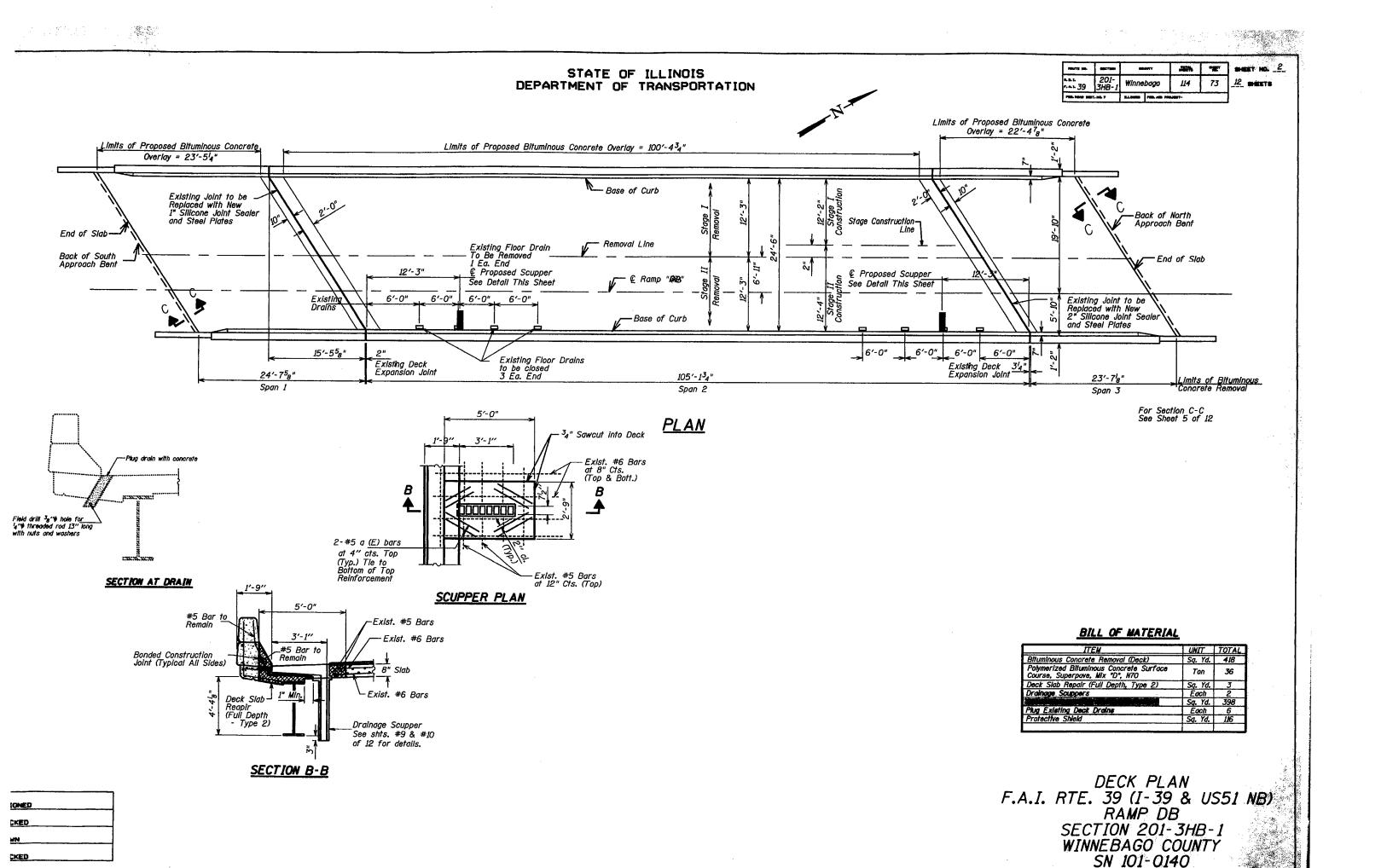
Joint openings shall be adjusted according to Article 503.10(c) of the Standard Specifications when the deck is poured at an ambient temperature other than 50 december.

TOTAL BILL OF MATERIAL

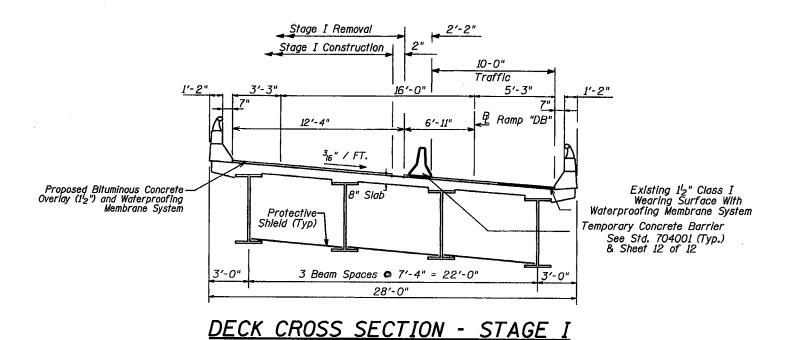
HOMEST COMMENTS

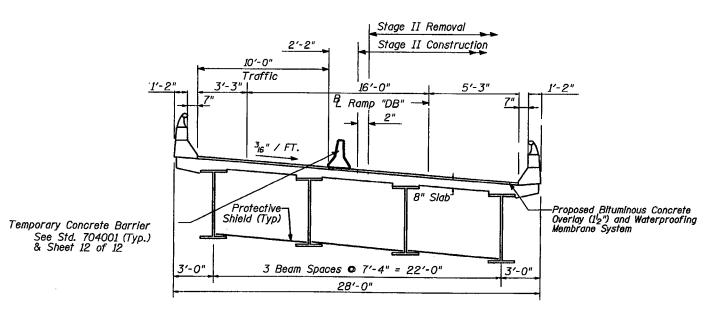
34			<u>.                                    </u>	
TEN	UNIT	SUPER	SUB	TOTAL
Concrete Removal	Cu Yd	7.4		7.4
Bituminous Concrete Removal (Deck)	Sq Yd	418		418
Concrete Superstructure	Cu Yd	7.1		7.1
Reinforcement Bars (Epoxy Coated)	Pound	1090		1090
Polymer Concrete	Cu Ft.	6.5		6.5
Silicone_Joint Segler	Foot	130		130
Deck Slab Repair (Full Depth - Type I)	Sq Yd	3		3
Deck Slab Repair (Full Depth, Type 2)	Sq Yd	10		10.
Deck Slab Repair (Partial Depth)	Sq Yd	52		52
Drainage Scupper	Each	2		2
Plug Exising Deck Drains	Each	6		6
Polymerized Bituminous Congrete Surface Course, Superpave, Mix "D", N70	Ton	36		36
Protective Shield	Sq. Yd.	116		115
Bar Splicers	Each	20		20
Furnishing & Erecting Structural Steel	Pound	3786		3786
	Sq. Yd.	398		398
Jack & Remove Existing Bearings	Each	2		2

GENERAL PLAN AND ELEVATION
F.A.I. RTE. 39 (I-39 & US51 NB)
RAMP DB
SECTION 201-3HB-1
WINNEBAGO COUNTY
SN 101-0140



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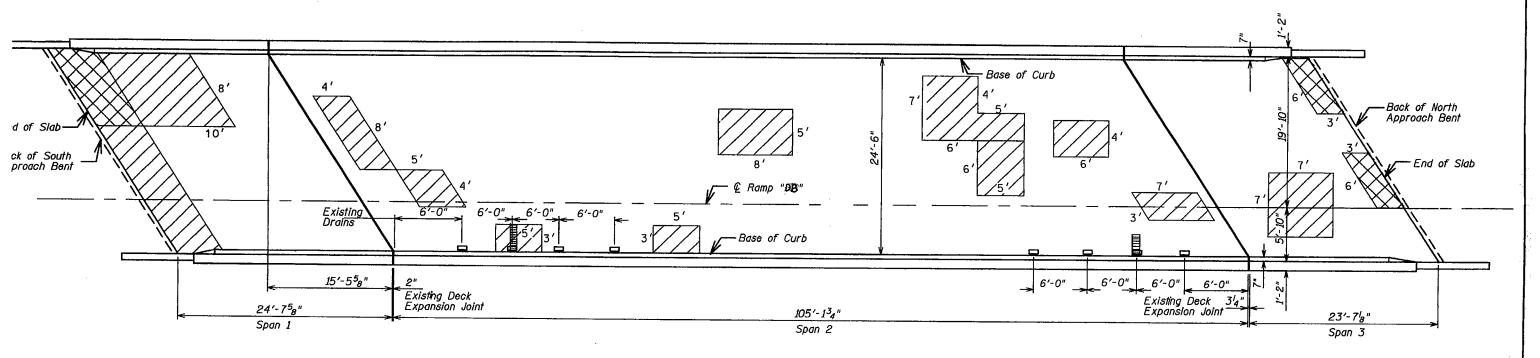


DECK CROSS SECTION - STAGE II

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DECK CROSS SECTIONS
F.A.I. RTE. 39 (I-39 & US51 NB)
RAMP DB
SECTION 201-3HB-1
WINNEBAGO COUNTY
SN 101-0140

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### PLAN





Deck Slab Repair (Partial)



Deck Slab Repair (Full Depth)

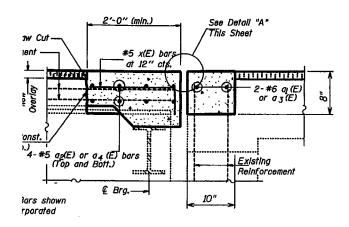
#### BILL OF MATERIAL

. Yd.	52
. Yd.	3
. Yd.	7
L	7. Yd. 7. Yd.

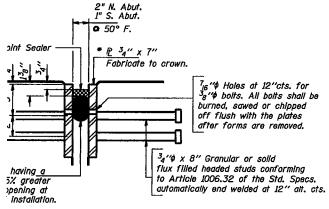
The plan quantities shown for Deck Slab Repair (Partial and Full Depth) are estimated quantities. The actual locations and quantity of Deck Slab Repair shallbe determined by the resident engineer in the field after removal of the existing wearing surface. Actual repair locations shall be shown on the as-built plans.

DECK REPAIR PLAN
F.A.I. RTE. 39 (I-39 & US51 NB)
RAMP DB
SECTION 201-3HB-1
WINNEBAGO COUNTY

Deck Survey : 01/10/00



#### JOINT RECONSTRUCTION



#### DETAIL "A"

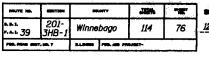
sh in segments of 20 ft. maximum i. Maximum space between installed ants shall be  $^36$ ". Seal space with ne Sealant suitable for Structural

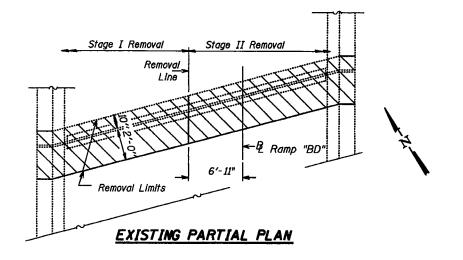
fabrication all surfaces of the plates shall be given one shop of paint specified for Structural No field painting required.

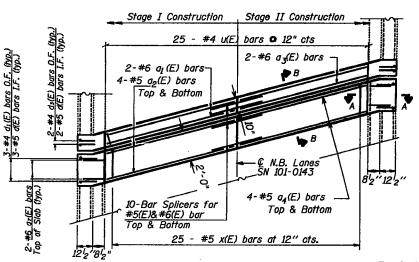
#### BILL OF MATERIAL

No.	Size	Leng	gth	,	Shape
16	#5	2'-9	"	-	
	#6			_	
	#5			_	
	#6			-	
	#5	14'-5	5"	-	
8	#6			1	
20	#5				
					_
					<i>60</i> )
	#4				
	#4				
50	#5	2	5"		
Item					
			Pou	ınd	1090
	)		Cu.	Yd.	7.1
emoval			Cu.	Yd.	7.4
and Erecting	Structural St	eel	Pou	ınd	3026
nt Sealer			Fo	ot	130
5			Ea	ch	20
crete			Cu.	Ft.	6.5
	16 4 16 4 16 8 20 12 8 50 50 Item ent Bars, Epouperstructure emoval and Erecting at Sealer	16 #5 4 #6 16 #5 4 #6 16 #5 4 #6 16 #5 8 #6 20 #5 12 #4 50 #4 50 #5 Item ent Bars, Epoxy Coated uperstructure emoval and Erecting Structural State Sealer	16 #5 2'-9 4 #6 14'-2 16 #5 14'-2 16 #5 14'-2 4 #6 14'-5 16 #5 14'-2 8 #6 #7-0 20 #5 4'-3 12 #4 4'-6 8 #4 6'-0 50 #4 1'-7 50 #5 2'-5 Item ent Bars, Epoxy Coated uperstructure emoval and Erecting Structural Steel of Sealer	16 #5 2'-9" 4 #6 14'-2" 16 #5 14'-2" 4 #6 14'-5" 4 #6 14'-5" 16 #5 14'-5" 8 #6 4'-0" 20 #5 4'-3" 12 #4 4'-6" 8 #4 6'-0" 50 #4 1'-7" 50 #5 2'-5" Item ent Bars, Epoxy Coated uperstructure emoval and Erecting Structural Steel Pount Sealer Examples Each	16 #5 2'-9"

Reinforcement bars designated (F) shall

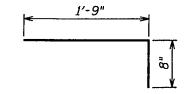






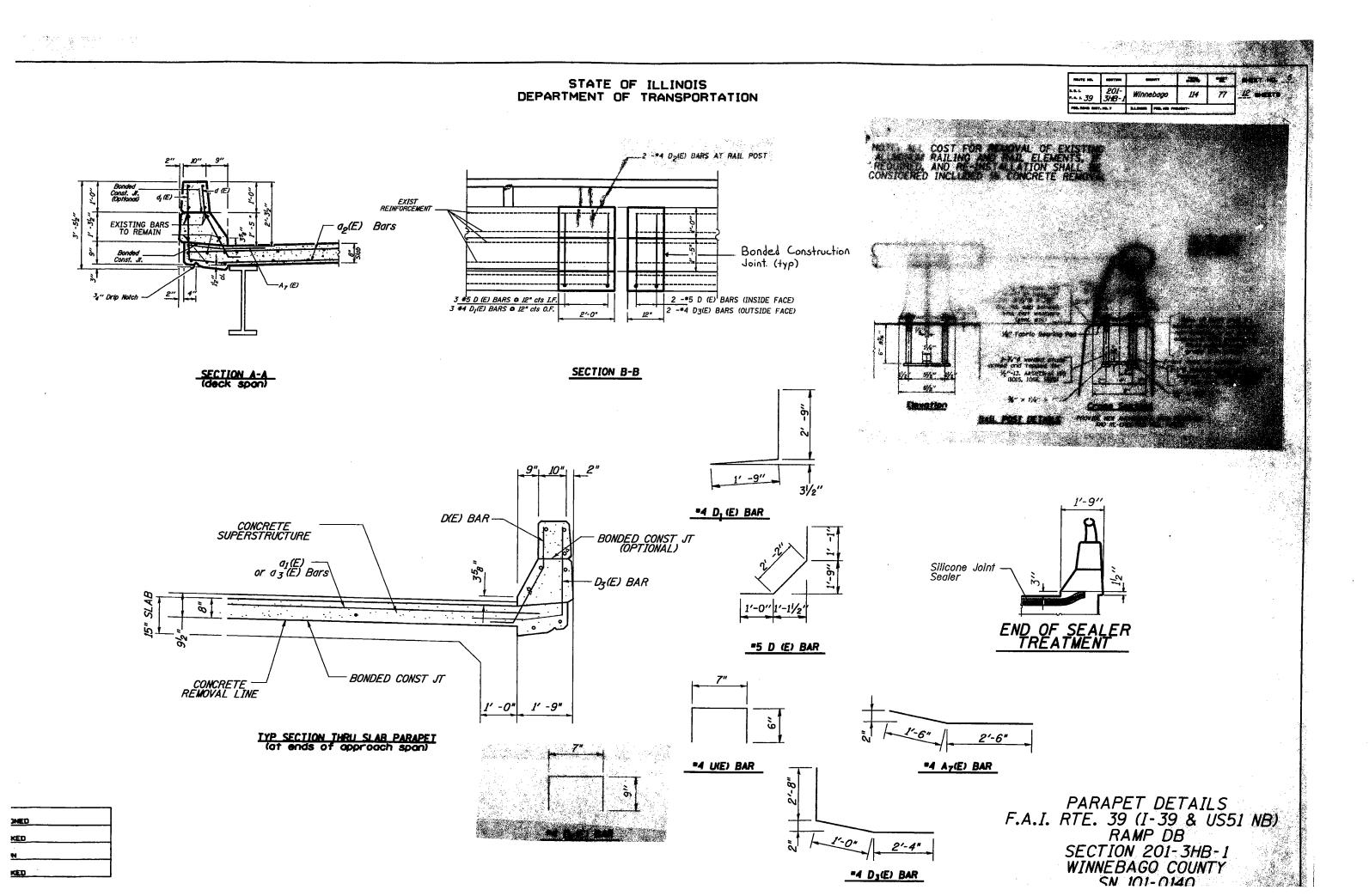
PROPOSED PARTIAL PLAN

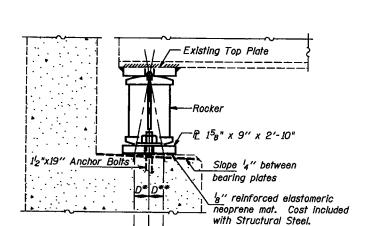
For Section A-A and Section B-B See Next Sheet



BAR x(E)

JOINT REPLACEMENT DETAILS F.A.I. RTE. 39 (I-39 & US51 NB) RAMP DB SECTION 201-3HB-1 WINNEBAGO COUNTY SN 101-0140





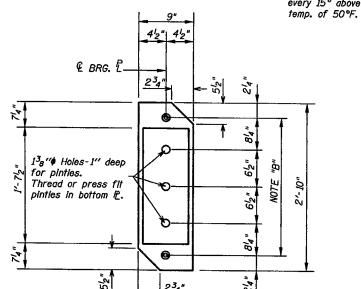
#### SECTION AT ABUTMENT

● 50°F.~~

\*D=18"/100 ft. of exp. for every 15° below the normal temp. of 50°F.

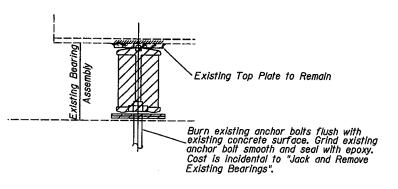
steel erection at various temperatures.

> \*\*D=18"/100 ft. of exp. for every 15° above the normal

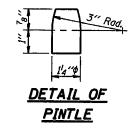


<u> BEARING - PLAN AT</u> ABUTMENT

#### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION



#### BEARING REMOVAL



Note:
-Diaphragm removal and replacement may be required to facilitate drilling holes. Cost shall be included with furnishing and erecting structural

-New bearings and anchor bolts are included with furnishing and erecting structural steel.

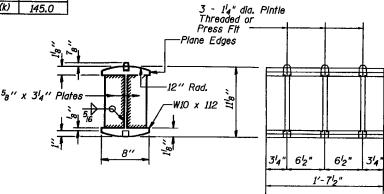
-Min. Jack Capacity = 110 Tons

Prior to ordering any material, the contractor shall verify in the field all bearing heights and shim thickness dimensions.

#### GIRDER REACTIONS

- ULTIPLE	TILAU	1 10/13	
R Q	(k)	79.6	
R L	(k)	53.6	_
IMP.	(k)	11.8	
R (Total)	(k)	145.0	
			_

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DETAIL OF ROCKER

#### BEARING REPLACEMENT NOTES

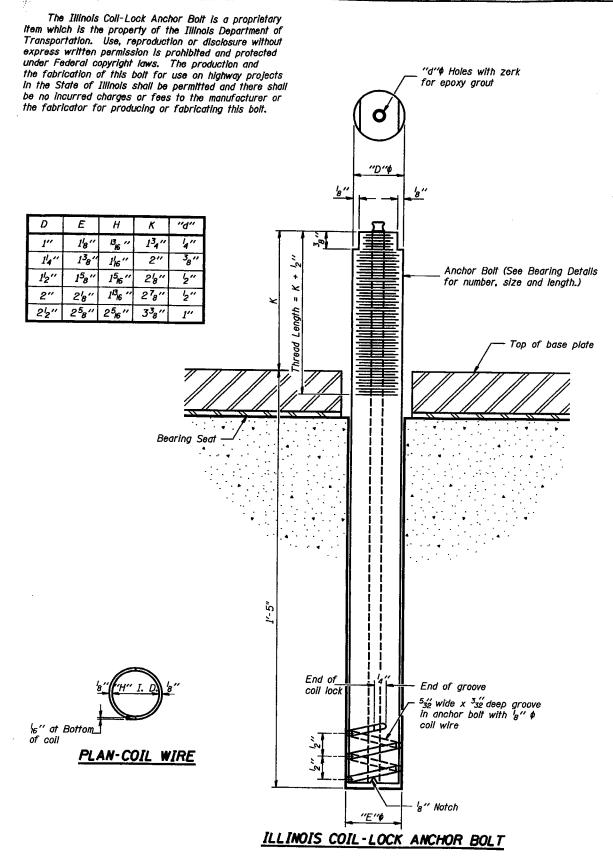
Bearing Removal and Replacement Schedule: SN 101-0140:

Note "B": 2" Dia. Holes for 1½" Dia. x 19" Anchor Rods (See Sheet <u>B</u> of 11 for Anchor Bolt Detalls). Anchor bolt assemblies shall be hot dipped galvanized and no field painting is required.

#### BILL OF MATERIAL - 1 BRIDGE

ITEM	UNIT	TOTAL
Jack and Remove Existing Bearings	Each	2
Furnishing and Erecting Structural Steel	Pound	760

ABUTMENT REPAIR PLAN F.A.I. RTE. 39 (I-39 & US51 NB) RAMP DB SECTION 201-3HB-1 WINNEBAGO COUNTY SN 101-0140



### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

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12 SHEETS

### MATERIALS FOR ILLINOIS COIL-LOCK ANCHOR BOLT

The anchor bolt shall be fabricated from cold drawn or hot finished seamless carbon steel mechanical tubing conforming to ASTM A 519, Grade 1026, CW and supplied with hexagonal nuts and cut washers.

The coil wire shall be made of any suitable soft steel wire.

The finished anchor bolt shall be cleaned of rust and other foreign materials and wrapped or packaged to prevent contamination until they are installed. The epoxy grout shall be a two-component, epoxy resin bonding system conforming to ASTM C 881, Type I, Grade 1 and of a Class suitable for the temperature at installation.

## INSTALLATION PROCEDURE for the ILLINOIS COIL-LOCK ANCHOR BOLT

- 1. With the coil wire in place, the bolt shall be inserted into the hole and turned clockwise to a snug fit in the hole. Nut and washer shall be placed on the bolt. The nut shall be tensioned until the steel base plates are held securely to the concrete bearing seat.
- 2. Epoxy grout shall be pumped through the zerk fitting with a pressure gun. Pumping shall continue until the epoxy overflows the hole around the bolt shank. After pumping is discontinued, excess epoxy shall be immediately wiped off.

#### ALTERNATE ANCHOR BOLTS

The Contractor may use, at his option, the capsule or the adhesive cartridge type anchor rods that have been previously tested and given a prior approval by the Department. The Contractor shall install these anchor rods in pre-drilled holes according to the manufacturer's recommendations and procedures.

The capsule or the adhesive cartridge type anchor rods shall be a two part system composed of:

- 1. A threaded rod stud with nut and washer of the type specified.
- A sealed glass capsule or a sealed glass adhesive cartridge containing premeasured amounts of the adhesive chemical.

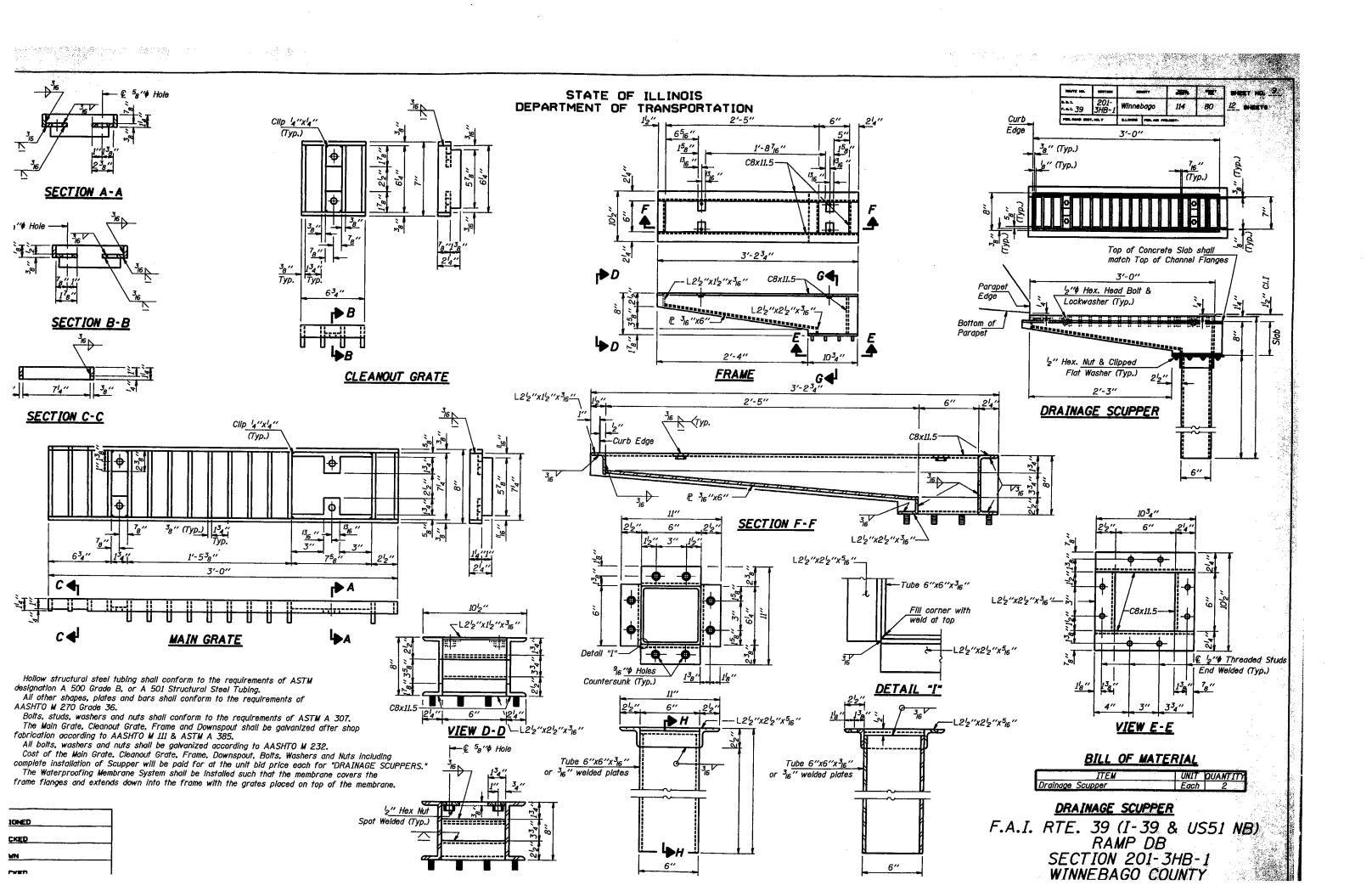
Location	Туре
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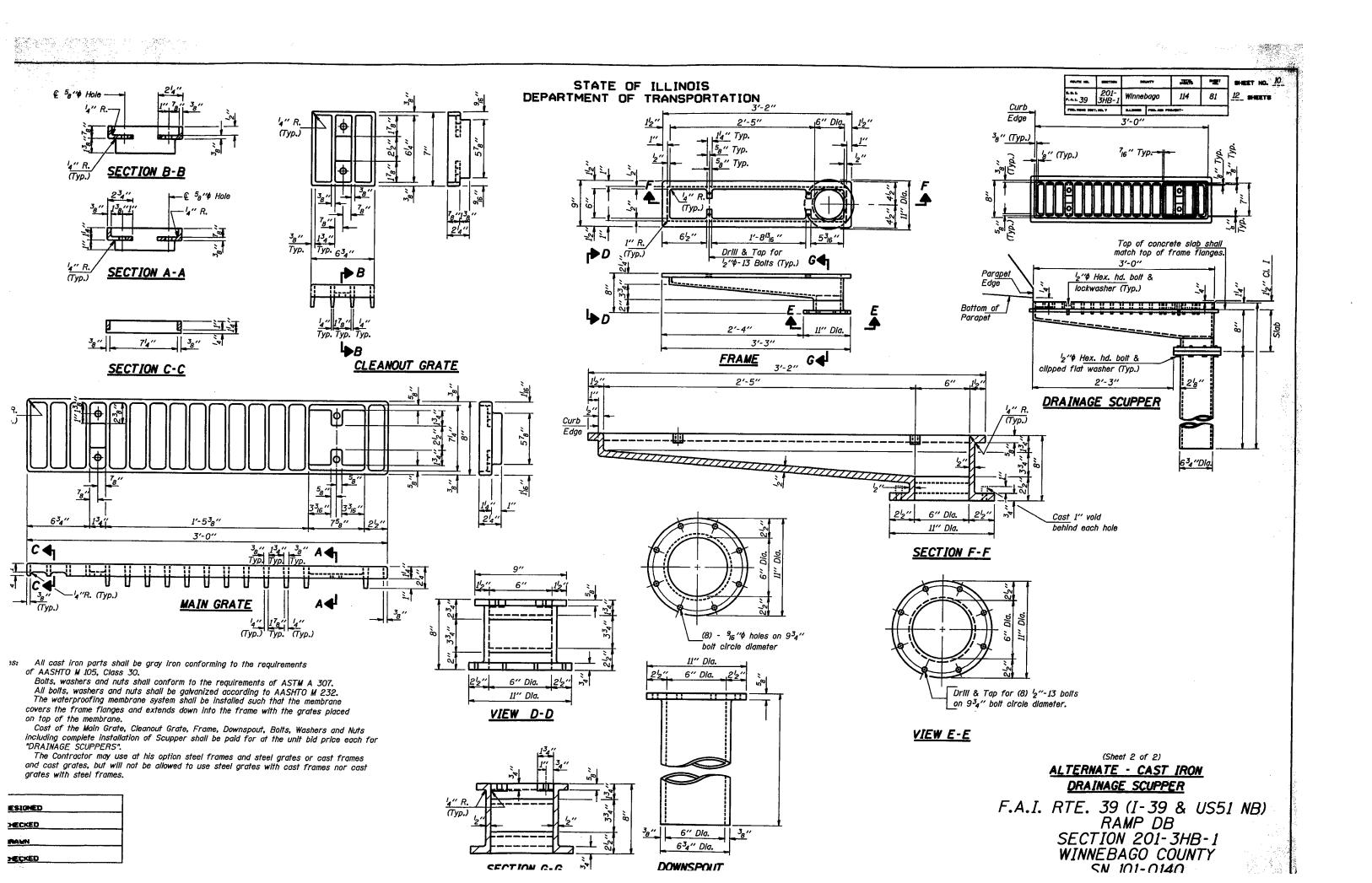
ASTM F 1554 Grade 105. ASTM A 449 and AASHTO M 314 Grade 105 anchor bolts may be substituted for the anchor bolts shown above.

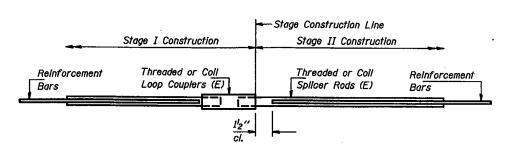
#### GENERAL NOTES

Holes in the masonry for anchor boits shall be drilled through the base plates to the diameter and depth shown or according to the manufacturer's recommendation after beams or girders have been erected and adjusted. Prior to setting the boits, the holes shall be dry and all dust and loose particles shall be removed by the use of compressed air or vacuuming. The anchor boits, furnished and installed and including the epoxy grout or capsules shall not be paid for separately but shall be included in the unit bid price for "Furnishing and Erecting Structural Steel".

ANCHOR BOLT DETAILS FOR BEARINGS F.A.I. RTE. 39 (I-39 & US51 NB) RAMP DB SECTION 201-3HB-1 WINNEBAGO COUNTY SN 101-0140







#### SPLICER DETAIL

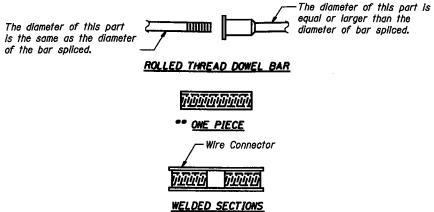
Bar Siz <del>e</del>	No. Assemblies Required	Location
#6	2	SO. ABUT.
#5	8	SO. ABUT.
#6	2	NO. ABUT.
#5	8	NO. ABUT.

(E): Indicates epoxy coating.

-	Bridge Deck	Approach Slab
Reinforcement Bars	Threaded or Coil Loop Couplers (E)	Threaded or Coll Splicer Rods (E)
	4'-0"	6'-0''

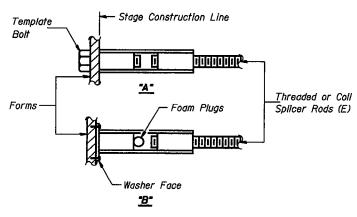
#### INTEGRAL ABUTMENT BAR SPLICER ASSEMBLY DETAIL FOR #5 BAR

Min.	Capacity	= 23.0	kips -	tension	,
Min.	Pull-out	Strength	= 9.2	kips -	tension
No	Required	-			



#### BAR SPLICER ASSEMBLY ALTERNATIVES

\*\* Heavy Hex Nuts conforming to ASTM A 563, Grade C, D or DH may be used.



#### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nalling to wood forms or cementing to steel forms. (E): Indicates epoxy coating.

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Bar splicer assemblies shall be of an approved type and shall develop in tension at least 125 percent of the yield strength of the lapped reinforcement bars.

Splicer rods shall be of minimum 60 ksi yield strength, threaded or coiled full length. All reinforcement bars shall be lapped and tied to the splicer rods or dowel bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars.

Other systems of similar design may be submitted to the Engineer for approval. Approval shall be based on certified test results from an approved testing laboratory that the proposed bar splicer assembly satisfies the following requirements:

Minimum Capacity (Tension in kips) = 1.25 x fy x  $A_t$ 

Minimum \*\*Pull-out Strength = 1.25 x fs<sub>allow</sub> x A<sub>t</sub>

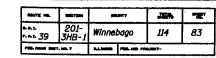
Where fy = Yield strength of lapped reinforcement bars in ksi.

fs<sub>allow</sub>= Allowable tensile stress in lapped reinforcement bars in ksi (Service Load) A<sub>t</sub> = Tensile stress area of lapped reinforcement bars. \* = 28 day concrete

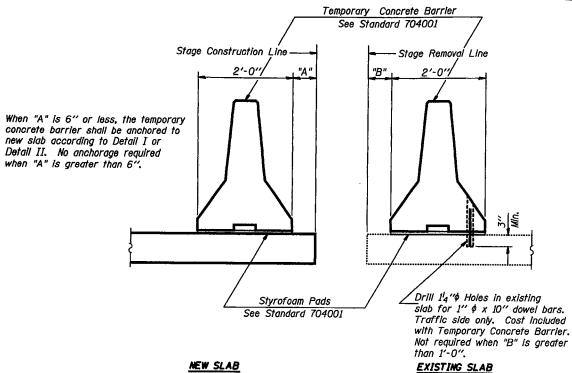
	BAR SPLIC	ER ASSEMBLI	ES			
Bar Size to be Spliced		Strength Requirements				
	Splicer Rod or Dowel Bar Length	Min. Capacity kips - tension	Min. Pull-Out Strength kips - tension			
#5	2'-0"	23.0	9.2			
#6	2'-7"	33.1	13.3			
#7	3′-5″	45.1	18.0			
#8	4'-6"	58.9	23.6			

Bar splicer assemblies shall be according to Section 508 of the Standard Specifications, except as noted. The furnishing and installation of bar splicer assemblies will be measured and paid for at the contract unit price each for "BAR SPLICERS."

DESIGNED CHECKED RSD-1 4-30-99 BAR SPLICER ASSEMBLY DETAILS F.A.I. RTE. 39 (I-39 & US51 NB) RAMP DB SECTION 201-3HB-1 WINNEBAGO COUNTY SN 101-0140



SHEET NO. 12 12 SHEETS



#### **MOTES**

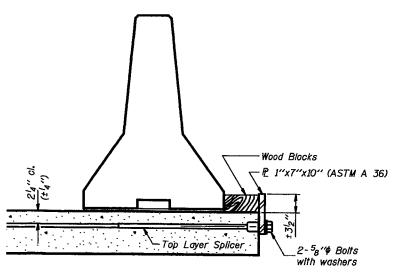
Detail I - With Bar Splicer or Couplers: Connect one (1) 1"x7"x10" steel 12 to the top layer of couplers with 2-58'4 bolts screwed to coupler at approximate & of each 10'-0'' barrier panel.

Detail II - With Extended Reinforcement Bars:

Connect one (1) 1"x7"x10" steel £ to the concrete slab with 2-5<sub>8</sub>"\$ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate € of each 10'-0" barrier panel.

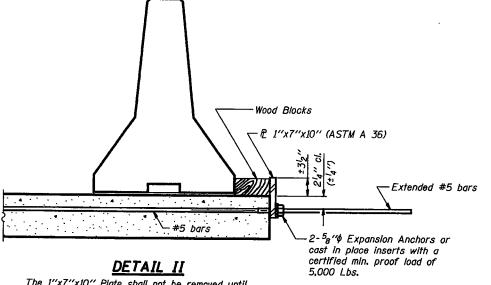
Cost of anchorage is included with Temporary Concrete Barrier.

#### SECTIONS THRU SLAB

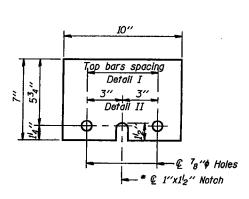


#### DETAIL I

The 1"x7"x10" Plate shall not be removed until Stage II Construction forms and reinforcement bars are in place.



The I"x7"x10" Plate shall not be removed until Stage II Construction forms and all reinforcement bars are in place and the concrete is ready to be

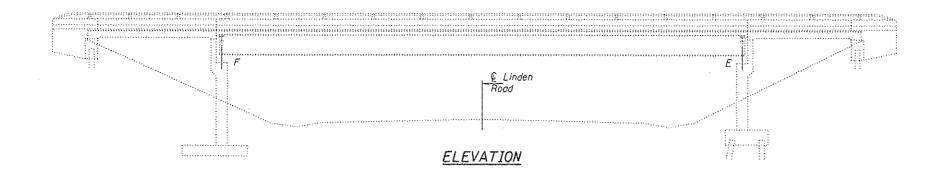


£ 1"x7"x10" \* Required only with Detail II

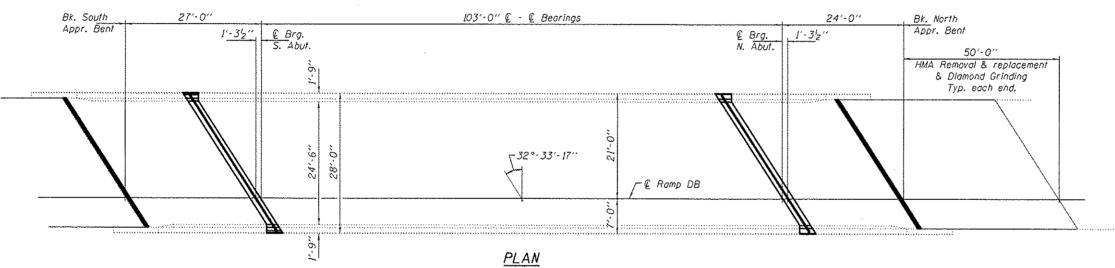
#### TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION

F.A.I. RTE. 39 (I-39 & US51 NB) RAMP DB SECTION 201-3HB-1 WINNEBAGO COUNTY

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DATE NOVEMBER 19. 2015
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

# PLAN AND ELEVATION FAI 39 (RAMP DB) OVER LINDEN ROAD SN 101-0140 SHEET NO. 1 OF 4 SHEETS

#### TOTAL BILL OF MATERIAL

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work. Reinforcement bars designated (E) shall be epoxy coated. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from

the surfaces in contact with concrete. Tightly adhered paint may remain unless

otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the

Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar

Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the deck is poured at an ambient temperature other than 50° F.

The new deck surface area shall have its final finish timed according to Article 420.09(e)(1) of the Standard Specifications. Cost included with Concrete

splicer or anchorage system. Cost included with Concrete Removal.

Work to be completed under road closure.

existing concrete.

Superstructure.

	TUTAL BILL OF MATER	<u> YIAL</u>	
	ITEM	UNIT	OUANTITY
	Concrete Removal	Cu. Yd.	4,0
	Concrete Superstructure	Cu. Yd.	4.0
	Reinforcement Bars, Epoxy Coated	Pound	240
	Preformed Joint Strip Seal	Foot	61
	Polymerized Hot-Mix Asphalt Surface Course. Mix "D", N90	Tons	53
*	Deck Slab Repair (Partial)	Sq. Ft.	387
×	Deck Slab Repair (Full Depth, Type I)	Sq. Ft.	52
*	Deck Slab Repair (Full Depth, Type II)	Sq. Ft.	77
	Profile Diamond Grinding of Concrete Pavement	Sq. Yd.	267
	Hot-Mix Asphalt Surface Removal (Deck)	Sq. Yd.	403.4
	Waterproofing Membrane System, Special	Sq. Yd.	403.4
	Silicone Joint Sealer	Foot	61
	Polymer Concrete	Cu. Ft.	6.0

\* Quantities are estimated. Actual locations and sizes to be determined

