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HARDESTY & HANOVER, LLC DANIEL E. MACHAMER, P.E., S.E. DATE: 3/21/2024

DATE: 3/21/2024

SIGNATURE AND SEAL APPLY TO DRAWINGS: 1, 17

EXPIRES: 11/30/25

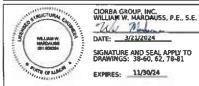


HARDESTY & HANOVER, ILC PAUL SKELTON, P.E.

DATE: 3/21/2024

SIGNATURE AND SEAL APPLY TO DRAWINGS: 83-116

EXPIRES: 11/30/25





DATE: 3/21/2024

DATE: 3/21/2024

SIGNATURE AND SEAL APPLY TO DRAWNINGS: 216, 19, 19, 25

EXPIRES: 11/20/25



OSEH, INC.

OSEH,





STATE OF ILLINOIS 06-14-2024 LETTING ITEM 107 DEPARTMENT OF TRANSPORTATION

PROPOSED HIGHWAY PLANS

FAP ROUTE 607: US 30 (CASS STREET)

OVER DES PLAINES RIVER

SECTION 2018-067-BR

PROJECT NHPP-MME4(425)

MOVABLE BRIDGE REHABILITATION

(S.N. 099-0101)

WILL COUNTY

C-91-410-20

THIS IMPROVEMENT IS LOCATED IN THE CITY OF JOLIET

INDEX OF SHEETS

SEE SHEET 2 FOR INDEX OF SHEETS

HIGHWAY STANDARDS

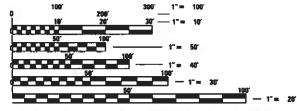
SEE SHEET 2 FOR LIST OF HIGHWAY STANDARDS

DISTRICT 1 DETAILS

SEE SHEET 2 FOR LIST OF DISTRICT 1 DETAILS

CASS STREET

ADT (2018) = 11,600 ADT (2050) = 15,100 DESIGN SPEED = 30 MPH POSTED SPEED = 30 MPH DESIGN DESIGNATION - OTHER PRINCIPAL ARTERIAL (URBAN)



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION 1-800-892-0123
OR 811

PROJECT ENGINEER: DARPIT SHAH, PE (847) 705-4755
PROJECT MANAGER: BRIAN KUTTAB, PE (847) 705-4431

CONTRACT NO. 62M79

PROJECT BEGINS STA. 6+65.00 RIOE JOLIET TOWNSHIP WILL Community Fig. 2 Will Community Fi

LOCATION MAP

GROSS LENGTH = 552.00 FT. = 0.105 MILE NET LENGTH = 552.00 FT. = 0.105 MILE



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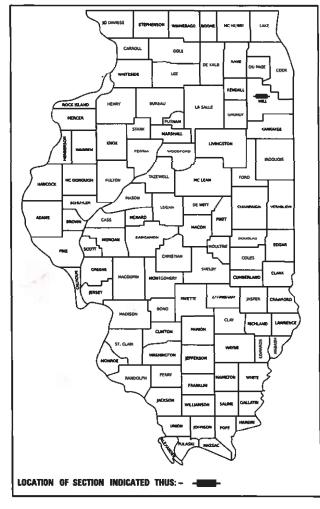
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OSEH inc.

SINGH AND CHARLENCE CONSULTANCE OF THE SECURITIES OF THE SECURITIE

D-91-607-20



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SUBMITTED

May 10, 2024

ENGINEER OF DESIGN AND ENVIRONMENT

May 10, 2024

DIRECTOR OF HIGHWAYS PROJECT IMPLEMENTATION

PRINTED BY THE AUTHORITY OF THE STATE OF ILLINOIS

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LIST OF IDOT HIGHWAY STANDARDS

000001-08	STANDARD SYMBOLS, ABBREVIATIONS AND PATTERNS
001001 - 02	AREAS OF REINFORCEMENT BARS
001006	DECIMAL OF AN INCH AND OF A FOOT
280001-07	TEMPORARY EROSION CONTROL SYSTEMS
353001-05	PCC BASE COURSE WITH HMA BINDER AND SURFACE COURSES
420401-13	PAVEMENT CONNECTOR (PCC) FOR BRIDGE APPROACH SLAB
420701-03	PAVEMENT WELDED WIRE REINFORCEMENT
515001 - 04	NAME PLATE FOR BRIDGES
602001-02	CATCH BASIN - TYPE A
602301-04	INLET - TYPE A
602401-07	PRECAST MANHOLE TYPE A - 4' DIAMETER
602402-03	PRECAST MANHOLE TYPE A - 5' DIAMETER
602416-09	PRECAST MANHOLE TYPE A - 8' DIAMETER
602601-06	PRECAST REINFORCED CONCRETE FLAT SLAB TOP
602701-02	MANHOLE STEPS
604001-05	FRAME AND LIDS TYPE 1
606001-08	CONCRETE CURB TYPE B AND COMBINATION CONCRETE CURB AND GUTTER
701427-05	LANE CLOSURE, MULTILANE, INTERMITTENT OR MOVING OPER., FOR SPEEDS <= 40 MPH
701502-09	URBAN LANE CLOSURE, 2L, 2W, WITH BIDIRECTIONAL LEFT TURN LANE
701601-09	URBAN LANE CLOSURE, MULTILANE, 1W OR 2W WITH NONTRAVERSABLE MEDIAN
701606-10	URBAN LANE CLOSURE, MULTILANE, 2W WITH MOUNTABLE MEDIAN
701701-10	URBAN LANE CLOSURE, MULTILANE INTERSECTION
701801-06	SIDEWALK, CORNER OR CROSSWALK CLOSURE
701901-09	TRAFFIC CONTROL DEVICES

IDOT DISTRICT 1 DETAILS

TC-22

BD-08	FRAMES AND LIDS ADJUSTMENT WITH MILLING
BD - 32	BUTT JOINT AND HMA TAPER DETAILS
BE-702	MISCELLANEOUS ELECTRICAL DETAILS, SHEET A - (CABLE SPLICE, POLE WIRING, TRANCH DETAIL)
TC-10	TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS
TC-11	TYPICAL APPLICATIONS RAISED REFLECTIVE PAVEMENT MARKERS (SNOW-PLOW RESISTANT)
TC-13	DISTRICT ONE TYPICAL PAVEMENT MARKINGS
TC-14	TRAFFIC CONTROL AND PROTECTION AT TURN BAYS (TO REMAIN OPEN TO TRAFFIC)
TC-21	DETOUR SIGNING FOR CLOSING STATE HIGHWAYS

ARTERIAL ROAD INFORMATION SIGN

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

US 30 OVER DES PLAINES RIVER INDEX OF SHEETS, HIGHWAY STANDARDS AND DISTRICT DETAILS						
INDEX OF SE	ILLIO, IIIC	IIIVVAI	JIANDAI	ND3 ANL	DISTRICT DETAILS	
SCALE: N.T.S.	SHEET 1	OF 1	SHEETS	STA.	TO STA.	

F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.		
607	2018-067-BR	2018-067-BR		128	2	
			CONTRACT NO. 62M7			
	ILLINOIS	FED. AI	D PROJECT			

GENERAL NOTES

- THE CONTRACTOR IS RESPONSIBLE TO OBTAIN AND COMPLY WITH ALL PERMITS REQUIRED BY APPLICABLE REGULATORY AGENCIES.
- ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE ILLINOIS DEPARTMENT OF TRANSPORTATION'S "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION" (ADOPTED JANUARY 1, 2022); THE "SUPPLEMENTAL SPECIFICATIONS AND RECURRING SPECIAL PROVISIONS" (ADOPTED JANUARY 1, 2024); THE LATEST EDITION OF THE "ILLINOIS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS, THE DETAILS" ON THE PLANS AND THE "SPECIAL PROVISIONS" INCLUDED IN THE CONTRACT DOCUMENTS. ANY REFERENCE TO STANDARDS THROUGHOUT THE PLANS OR SPECIAL PROVISIONS SHALL BE INTERPRETED AS THE LATEST STANDARD OF THE ILLINOIS DEPARTMENT OF TRANSPORTATION.
- 3. THE CONTRACTOR/DEVELOPER ASSUMES ALL RESPONSIBILITY AND LIABILITY FOR ANY ACTION RESULTING FROM THEIR WORK WITHIN THE PUBLIC RIGHT-OF-WAY.
- 4. ALL CONSTRUCTION MATERIALS WITHIN THE PUBLIC RIGHT-OF-WAY MUST BE IDOT CERTIFIED. DOCUMENTATION OF MATERIAL CERTIFICATION SHALL BE SUBMITTED PRIOR TO ENGINEER APPROVAL ALL CONSTRUCTION MATERIAL NEEDING INSPECTION SHALL BE DONE ACCORDING TO THE LATEST IDOT PROJECT AND PROCEDURES GUIDE
- 5. THE CONTRACTOR SHALL PROVIDE THE ENGINEER A LIST OF MATERIALS USED AND IDENTIFY THEIR ASSOCIATED IDOT CERTIFICATION, A COPY OF ALL MATERIAL TESTING COMPANY RESULTS, AND A WEEKLY FIELD REPORT UTILIZING THE APPROPRIATE IDOT FORM
- 6. ALL COORDINATES SHOWN ARE BASED UPON THE ILLINOIS STATE PLANE COORDINATE SYSTEM EAST ZONE, MAP COORDINATES REFLECT NAD 83 (2011 ADJUSTMENT)
- EXCEPT AS NOTED ON THE PLANS, PAVEMENT GRADES SHOWN ARE AT THE TOP OF PAVEMENT
- THE CONTRACTOR SHALL MAINTAIN THE SITE IN A CLEAN AND ORDERLY MANNER. DEBRIS AND ANY SURPLUS MATERIAL SHALL BE REMOVED AND RESTORATION SHALL PROCEED AS WORK PROCEEDS. IF THE ENGINEER SO DIRECTS, THE CONTRACTOR SHALL STOP ALL OTHER WORK AND CONCENTRATE ON CLEAN-UP AND RESTORATION. DEBRIS AND SURPLUS MATERIAL SHALL BE DISPOSED OF BY THE CONTRACTOR OFF-SITE, ANY DAMAGE CAUSED BY THE CONTRACTOR TO EXISTING PAVEMENT, CURB & GUTTER, AND SIDEWALK NOT SHOWN AS REMOVED OR WORKED ON DURING CONSTRUCTION SHALL BE REPAIRED OR REPLACED.
- AGGREGATE SUBGRADE IMPROVEMENT (CU YD) HAS BEEN PROVIDED FOR USE AT THE LOCATIONS INDICATED FOR SOILS THAT TEND TO BE UNSTABLE AND/OR UNSUITABLE. THE ACTUAL NEED FOR REMOVAL AND REPLACEMENT WITH ASI WILL BE DETERMINED IN THE FIELD AT THE TIME OF CONSTRUCTION BY THE GEOTECHNICAL ENGINEER. ALL POTENTIALLY UNSTABLE SOILS SHOULD BE TESTED WITH A STATIC OR DYNAMIC CONE PENETROMETER AND TREATED IN ACCORDANCE WITH ARTICLE 301.04 OF THE SSRBC AND IDOT SUBGRADE STABILITY MANUAL. IF UNSTABLE AND/OR UNSUITABLE SOILS ARE NOT ENCOUNTERED, THEN THE QUANTITY SHALL BE DEDUCTED, AND NO ADDITIONAL COMPENSATION WILL BE DUE TO THE CONTRACTOR.
- 10. THE AGGREGATE GRADATION FOR THE AGGREGATE SUBGRADE IMPROVEMENT 12" LOWER LIFT SHALL BE CS 1 OR RR 1".
- 11. ALL EXCAVATED MATERIAL, WHICH INCLUDES DIGGING OR GRADING OF ANY SOIL OR FILL MATERIAL, WITH THE EXCEPTION OF AGGREGATE FILLS, MUST BE INCORPORATED WITHIN THE IDOT RIGHT-OF-WAY
- 12. THE CONTRACTOR SHALL PROTECT AND CAREFULLY PRESERVE ALL SECTION OR SUBSECTION MONUMENTS, PROPERTY CORNERS, AND REFERENCE MARKERS UNTIL THE OWNER, HIS AGENT, OR AN AUTHORIZED SURVEYOR HAS WITNESSED OR OTHERWISE REFERENCED THEIR LOCATIONS.
- 13. SEEDING WILL NOT BE PERMITTED AT ANY TIME WHEN THE GROUND IS FROZEN, WET, OR IN AN UNTILLABLE CONDITION. LOCATIONS TO BE SEEDED WILL BE DETERMINED BY THE
- 14. THE CONTRACTOR WILL BE REQUIRED TO COMPLY WITH ALL STATE REGULATIONS REGARDING AIR, WATER, AND NOISE POLLUTION. THE CONTRACTOR IS PROHIBITED FROM BURNING ANY MATERIAL WITHIN OR ADJACENT TO THE IMPROVEMENT.
- 15. BEFORE BEGINNING ANY WORK, THE CONTRACTOR SHALL RETAIN AND RECORD FOR FUTURE REFERENCE, ALL EXISTING PAVEMENT MARKING LINES (AND RAISED REFLECTIVE PAVEMENT MARKERS) IN ORDER THAT THESE LOCATIONS CAN BE RE-ESTABLISHED FOR STRIPING. EXACT LOCATIONS OF ALL PAVEMENT MARKINGS SHALL BE AS DIRECTED BY THE ENGINEER.
- 16. THE CONTRACTOR SHALL CONTACT THE DISTRICT ONE TRAFFIC CONTROL SUPERVISOR FOR ARTERIALS AT KALPANA.KANNAN-HOSADURGA@ILLINOIS.GOV AT LEAST 72 HOURS IN ADVANCE OF BEGINNING WORK.
- 17. THE RESIDENT ENGINEER SHALL CONTACT ERIC CAMPOS, AREA TRAFFIC FIELD ENGINEER, VIA EMAIL AT ERIC.CAMPOS@ILLINOIS.GOV, A MINIMUM OF TWO WEEKS BEFORE INSTALLING PERMANENT PAVEMENT MARKINGS
- 18. BEFORE STARTING ANY EXCAVATION, THE CONTRACTOR SHALL CALL "JULIE" AT THE TOLL-FREE NUMBER 800-892-0123 FOR FIELD LOCATIONS OF ANY AND ALL UTILITIES AND BURIED FACILITIES. 48 HOUR NOTIFICATION IS REQUIRED.

- 19. TRANSITIONS SHALL BE USED TO MATCH PROPOSED CURB AND GUTTER AND MEDIAN ITEMS OF WORK TO EXISTING CURBS AND GUTTER AND MEDIANS IN THE FIELD. UNLESS OTHERWISE SHOWN, THE TRANSITION SHALL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PROPOSED ITEMS OF WORK SPECIFIED.
- 20. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITIES WITH UTILITY COMPANIES AND THE CITY OF JOLIET.
- 21. THE LOCATION AND ELEVATIONS OF THE UNDERGROUND UTILITIES AS SHOWN ON THE PLANS ARE NOT TO BE TAKEN AS EXACT. THE CONTRACTOR SHALL USE SPECIAL CARE WHEN CONDUCTING CONSTRUCTION OPERATIONS NEAR THEM TO PREVENT DAMAGE
- 22. THE CONTRACTOR WILL NOT BE ALLOWED TO SET UP A YARD OR FIELD OFFICE ON STATE PROPERTY WITHOUT WRITTEN PERMISSION FROM THE DEPARTMENT.
- 23. ALL DAMAGE TO EXISTING PAVEMENT MARKINGS OR RAISED REFLECTIVE PAVEMENT MARKERS OUTSIDE THE REMOVAL LINE SHOWN ON THE PLANS SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE.
- 24. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMFNSIONS AND CONDITIONS EXISTING IN THE FIELD PRIOR TO CONSTRUCTION AND ORDERING OF MATERIALS
- 25. DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS.
- 26. THE CONTRACTOR SHALL REMOVE, STORE, AND RE- ERECT ALL EXISTING SIGNS IN ACCORDANCE WITH THE STATE STANDARD SPECIFICATIONS.
- 27. PACE MUST BE NOTIFIED A MINIMUM OF TWO (2) WEEKS IN ADVANCE OF ANY ROAD CLOSURES. RICHARD WILLMAN AT TRANSPORTATION.ENGINEER@PACEBUS.COM
- 28. THE CONTRACTOR SHALL MAINTAIN CONVEYANCE OF ALL DRAINAGE FLOWS DURING CONSTRUCTION OF THIS PROJECT. WHEN EXISTING DRAINAGE FACILITIES ARE DISTURBED THE CONTRACTOR SHALL PROVIDE AND MAINTAIN TEMPORARY OUTLETS AND CONNECTIONS FOR ALL PRIVATE AND PUBLIC DRAINS, SEWERS, CULVERTS AND OTHER DRAINAGE FACILITIES. THE CONTRACTOR SHALL PROVIDE FACILITIES TO TAKE IN ALL STORM WATER WHICH WILL BE RECEIVED BY THESE DRAINS AND SEWERS AND DISCHARGE THE SAME. THE CONTRACTOR SHALL PROVIDE AND MAINTAIN A PUMPING PLAN, IF NECESSARY, AND A TEMPORARY OUTLET AND BE PREPARED AT ALL TIMES TO DISPOSE OF THE WATER RECIEVED FROM THESE TEMPORARY CONNECTIONS UNTIL SUCH TIME THAT THE PERMANENT CONNECTION WITH SEWERS ARE BUILT AND IN SERVICE THE WORK WILL BE INCLUDED IN THE COST OF THE CONTRACT
- 29. THE FEDERALLY ENDANGERED INDIANA BAT (MYOTIS SODALIS) AND THE THREATENED NORTHERN LONG-EARED BAT (MYOTIS SEPTENTRIONALIS) CAN BE FOUND THROUGHOUT ILLINOIS. MEASURES TO MINIMIZE THE POTENTIAL TAKE OF THE INDIANA BAT OR NORTHERN LONG-EARED BAT SHALL BE PERFORMED BY CLEARING TREES THREE (INCHES) AT BREAST HEIGHT OUTSIDE OF THE REPRODUCTIVE SEASON. IF TREE CLEARING IS NECESSARY, IT SHALL NOT OCCUR DURING THE APRIL 1 THRU SEPTEMBER 30TH TIME FRAME TO AVOID IMPACTING THE INDIANA AND NORTHERN LONG-EARED BATS.

COMMITMENTS

- WHILE NOT REQUIRING A PERMIT FROM THE UNITED STATES COAST GUARD (USCG). THE CONTRACTOR WILL NEED A LETTER OF AUTHORIZATION FROM THE USCG PRIOR TO PROCEEDING WITH WORK. THIS MAY INCLUDE PLAN SUBMITTAL FOR REVIEW BY THE USCG. TOWARDS THE END OF THE DESIGN PROCESS, THE PHASE II DESIGNER TO COORDINATE PLAN SUBMITTAL, IF REQUIRED, WITH THE USCG FOR THEIR OFFICIAL RESPONSE. COORDINATION BY THE DESIGNER DOES NOT RELIEVE THE CONTRACTOR FROM ADDITIONAL COORDINATION FROM THE USCG.
- 2. A U.S. ARMY CORPS OF ENGINEERS PERMIT WAS DETERMINED NOT TO BE NECESSARY FOR THE IMPROVEMENT PLANS AS SHOWN. HOWEVER, THE CONTRACTOR MUST COORDINATE WITH THE U.S. ARMY CORPS OPERATIONS MANAGER PRIOR TO CONSTRUCTION.
- 3. AN IDNR FLOODWAY PERMIT WAS DETERMINED NOT TO BE NECESSARY FOR THE IMPROVEMENT PLANS AS SHOWN. HOWEVER, THE CONTRACTOR WILL BE RESPONSIBLE FOR OBTAINING A FLOODWAY PERMIT IF ANY TEMPORARY STRUCTURE WILL BE LEFT IN PLACE THAT WOULD IMPEDE FLOWS, SUCH AS A COFFERDAM, CAUSEWAY, OR MOORING A BARGE IN PLACE.

(> CìorbaGroup Higgins Rd. Ste 600, Chicago, IL 6063

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PLOT DATE = 2/23/2024	DATE - 2/16/2024	REVISED -

US 30 OVER DES PLAINES RIVER							
GENERAL	NOTES	AND C	оммітм	//FNTS	607		
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SHEET 1	OF 1	SHEETS	STA.	TO STA.			

SCALE: N.T.S.

		SUMMARY OF QUANTITIES			80% FED 2	20% STATE
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013
	20200100	EARTH EXCAVATION	CUYD	145	145	
	20200200	ROCK EXCAVATION	CUYD	158	158	
	20201200	REMOVAL AND DISPOSAL OF UNSUITABLE MATERIAL	CUYD	40	40	
	20800150	TRENCH BACKFILL	CUYD	34	34	
	28000510	INLET FILTERS	EACH	39	39	
	30300112	AGGREGATE SUBGRADE IMPROVEMENT 12"	SQ YD	240	240	
	31101180	SUBBASE GRANULAR MATERIAL, TYPE B 2"	SQ YD	88	88	
	31200500	STABILIZED SUBBASE - HOT-MIX ASPHALT, 4"	SQ YD	167	167	
	40600290	BITUMINOUS MATERIALS (TACK COAT)	POUND	593	593	
	40600400	MIXTURE FOR CRACKS, JOINTS, AND FLANGEWAYS	TON	8	8	
	40600982	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	SQ YD	40	40	
	4000					
	40602985	HOT-MIX ASPHALT BINDER COURSE, IL-9.5, N70	TON	74	74	
	4000 4000	LIOT MIV ACRUALT CUIDEAGE COURSE II O S ANVIRON VIEW	701	7.	7.	
	40604062	HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N70	TON	74	74	

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	US 30 OVER DES PLAINES RIVER				F.A.P. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
	SUMMARY OF QUANTITIES					607	2018-067-BR		WILL	128	4
									CONTRACT	NO. 62	2M79
SCALE: N.T.S. SHEET 1 OF 11 SHEETS STA. TO STA. ILLINOIS FED. AID PROJECT					PROJECT						

FILE NAME: N:\PROJ\0021239.01\v

		SUMMARY OF QUANTITIES			80% FED	80% FED 20% STATE		
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013		
	42000060	WELDED WIRE REINFORCEMENT	SQ YD	167	167			
	42000080	PAVEMENT CONNECTOR (PCC) FOR BRIDGE APPROACH SLAB	SQ YD	167	167			
	42001300	PROTECTIVE COAT	SQYD	283	283			
	42400200	PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH	SQ FT	790	790			
	44000100	PAVEMENT REMOVAL	SQYD	234	234			
	44000500	COMBINATION CURB AND GUTTER REMOVAL	FOOT	153	153			
	44000600	SIDEWALK REMOVAL	SQFT	1,707	1,707			
	44201721	CLASS D PATCHES, TYPE III, 6 INCH	SQ YD	24	24			
	44201723	CLASS D PATCHES, TYPE IV, 6 INCH	SQ YD	37	37			
	50102400	CONCRETE REMOVAL	CUYD	15		15		
	50104720	REMOVAL OF EXISTING CONCRETE DECK	EACH	1		1		
	50157300	PROTECTIVE SHIELD	SQYD	884		884		
	50200100	STRUCTURE EXCAVATION	CUYD	85.8		85.8		

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

	US 30 OVER DES PLAINES RIVER						F.A. P. RTE.	SEC	TION	COUNTY	TOTAL	SHEET NO.
							607	2018-	067-BR	WILL	128	5
) IAIIAIWII I	01 40/	AIN I I I I I I I					CONTRACT	NO. 62	2M79
	SCALE: N.T.S. SHEET 2 OF 11 SHEETS STA. TO STA.							ILLINOIS FED. A	D PROJECT			

		SUMMARY OF QUANTITIES			80% FED	20% STATE
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013
	50300225	CONCRETE STRUCTURES	CUYD	48.9		48.9
	50300255	CONCRETE SUPERSTRUCTURE	CUYD	216.8		216.8
	50300260	BRIDGE DECK GROOVING	SQ YD	840		840
	50300300	PROTECTIVE COAT	SQ YD	1,181		1,181
	50301350	CONCRETE SUPERSTRUCTURE (APPROACH SLAB)	CUYD	143.1		143.1
	50500105	FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1		1
	50700105	TREATED TIMBER	F.B.M.	9,922		9,922
	50700305	HARDWARE	POUND	557		557
	50800205	REINFORCEMENT BARS, EPOXY COATED	POUND	126,740		126,740
	52000110	PREFORMED JOINT STRIP SEAL	FOOT	139		139
	52100010	ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	17		17
	52100520	ANCHOR BOLTS, 1"	EACH	34		34
	550A0340	STORM SEWERS, CLASS A, TYPE 2 12"	FOOT	45		45
	550A0340	STORM SEWERS, CLASS A, TYPE 2 12"	FOOT	45		

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

	US 30 OVER DES PLAINES RIVER SUMMARY OF QUANTITIES					SECT I ON	COUNTY	TOTAL	SHEET NO.
						2018-067-BR	WILL	128	6
ļ		30WWAN 0	COANTIL				CONTRACT	NO 62	2M79
	SCALE: N.T.S. SHEET 3 OF 11 SHEETS STA. TO STA.					ILLINOIS FED. A	D PROJECT		

		SUMMARY OF QUANTITIES			80% FED 2	20% STATE
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013
	55100500	STORM SEWER REMOVAL 12"	FOOT	7		7
	58700300	CONCRETE SEALER	SQFT	2,998		2,998
	59000200	EPOXY CRACK INJECTION	FOOT	363		363
	60200105	CATCH BASINS, TYPE A, 4'-DIAMETER, TYPE 1 FRAME, OPEN LID	EACH	2	2	
	60221000	MANHOLES, TYPE A, 5'-DIAMETER, TYPE 1 FRAME, OPEN LID	EACH	1	1	
	00004450	MANUALES TYPE A SURVANETED TYPE AFRAME OLOSED UR	FACIL	4	4	
	60224459	MANHOLES, TYPE A, 8'-DIAMETER, TYPE 1FRAME, CLOSED LID	EACH	1	1	
	60234200	INLETS, TYPE A, TYPE 1 FRAME, OPEN LID	EACH	2	2	
	60258200	MANHOLES TO BE RECONSTRUCTED WITH NEW TYPE 1 FRAME, CLOSED LID	EACH	1	1	
	60266600	VALVE BOXES TO BE ADJUSTED	EACH	2	2	
	60500050	REMOVING CATCH BASINS	EACH	2	2	
\vdash	60500060	REMOVING INLETS	EACH	1	1	
	60603800	COMBINATION CONCRETE CURB AND GUTTER, TYPE B-6.12	FOOT	127	127	
*	66900200	NON-SPECIAL WASTE DISPOSAL	CUYD	145	145	

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PLOT DATE = 3/25/2024	DATE - 2/16/2024	REVISED -

	US 30 OVER DES PLAINES RIVER						F.A.P. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
							607	2018-067-BR		WILL	128	7
		30	IVIIVIA	01 40/	ANTITIES					CONTRACT	NO. 62	2M79
	SCALE: N.T.S. SHEET 4 OF 11 SHEETS STA. TO STA.					ILLINOIS	FED. AID	PROJECT				

		SUMMARY OF QUANTITIES			80% FED 20% STATE		
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013	
*	66900530	SOIL DISPOSAL ANALYSIS	EACH	1	1		
*	66901001	REGULATED SUBSTANCES PRE-CONSTRUCTION PLAN	LSUM	1	1		
*	66901003	REGULATED SUBSTANCES FINAL CONSTRUCTION REPORT	L SUM	1	1		
*	66901006	REGULATED SUBSTANCES MONITORING	CAL DA	10	10		
	67100100	MOBILIZATION	LSUM	1	1		
	70107025	CHANGEABLE MESSAGE SIGN	CAL DA	180	180		
*	72000100	SIGN PANEL - TYPE 1	SQ FT	10	10		
*	78000200	THERMOPLASTIC PAVEMENT MARKING - LINE 4"	FOOT	925	925		
*	78000600	THERMOPLASTIC PAVEMENT MARKING - LINE 12"	FOOT	264	264		
*	78000650	THERMOPLASTIC PAVEMENT MARKING - LINE 24"	FOOT	22	22		
*	78009004	MODIFIED URETHANE PAVEMENT MARKING - LINE 4"	FOOT	1,013	1,013		
*	78009012	MODIFIED URETHANE PAVEMENT MARKINGS - LINE 12"	FOOT	120	120		
*	78009024	MODIFIED URETHANE PAVEMENT MARKINGS - LINE 24"	FOOT	36	36		

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PLOT DATE = 3/25/2024	DATE - 2/16/2024	REVISED -

	US 30	OVER D	ES PLA	INES RIV	ER	F.A.P. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
	SUMMARY OF QUANTITIES						607 2018-067-BR WILL			128	8
	30	IVIIVIAIII	01 40	AIVITILO					CONTRACT	NO. 62	2M79
SCALE: N.T.S.	SHEET 5	OF 11	SHEETS	STA.	TO STA.		ILLINO	IS FED. A	D PROJECT		

3300200 300220	00 RAISED REFLECTIVE PAVEMENT MARKER 00 RAISED REFLECTIVE PAVEMENT MARKER REMOVAL	EACH EACH	24 24	RDWY 0004 24 24	BRIDGE REHAB 0013
3300200	00 RAISED REFLECTIVE PAVEMENT MARKER REMOVAL CONDUIT ATTACHED TO STRUCTURE, 1" DIA., PVC COATED	EACH			
1100320	CONDUIT ATTACHED TO STRUCTURE, 1" DIA., PVC COATED		24	24	
1100320	CONDUIT ATTACHED TO STRUCTURE, 1" DIA., PVC COATED		24	24	
	CONDUIT ATTACHED TO STRUCTURE, 1" DIA., PVC COATED GALVANIZED STEEL	FOOT			
	CONDUIT ATTACHED TO STRUCTURE, 1" DIA., PVC COATED GALVANIZED STEEL	FOOT			
300220			325		325
	JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 6" X 6" X 4"	EACH	10		10
300980	JUNCTION BOX, STAINLESS STEEL, EMBEDDED IN STRUCTURE, 8" X 8" X 6"	EACH	4		4
1702110	0 ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 10	FOOT	1,007		1,007
2110001	01 LUMINAIRE, LED, ROADWAY, OUTPUT DESIGNATION A	EACH	8		8
2110021	LUMINAIRE, LED, UNDERPASS, WALLMOUNT, OUTPUT DESIGNATION D	EACH	2		2
2200606	WATERWAY OBSTRUCTION WARNING LUMINAIRE, LED, 180 DEGREE RED, PARAPET MOUNTED	EACH	4		4
2200609	WATERWAY OBSTRUCTION WARNING LUMINAIRE, LED, 360 DEGREE GREEN	EACH	4		4
200600	00 REMOVAL OF LIGHTING UNIT, NO SALVAGE	EACH	2		2
5000200	00 MAINTENANCE OF EXISTING TRAFFIC SIGNAL INSTALLATION	EACH	1		1
	15 ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 14 5C	FOOT	700		700
	0020	00200 MAINTENANCE OF EXISTING TRAFFIC SIGNAL INSTALLATION	00200 MAINTENANCE OF EXISTING TRAFFIC SIGNAL INSTALLATION EACH	00200 MAINTENANCE OF EXISTING TRAFFIC SIGNAL INSTALLATION EACH 1	00200 MAINTENANCE OF EXISTING TRAFFIC SIGNAL INSTALLATION EACH 1

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	US 30	OVER D	ES PLA	INES RIVE	ER	F.A.P. RTE	SECT	TION		COUNTY	TOTAL SHEETS	SHEET NO.
	SUMMARY OF QUANTITIES						607 2018-067-BR WIL			WILL	128	9
	30	IVIIVIAII I	01 40/	ANTITIES.						CONTRACT	NO. 62	2M79
SCALE: N.T.S.	SHEET 6	OF 11	SHEETS	STA.	TO STA.			ILLINOIS	FED. AII	D PROJECT		

		SUMMARY OF QUANTITIES			80% FED	20% STATE
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013
*	87301900	ELECTRIC CABLE IN CONDUIT, EQUIPMENT GROUNDING CONDUCTOR, NO. 6 1C	FOOT	500		500
*	87502500	TRAFFIC SIGNAL POST, GALVANIZED STEEL 16 FT.	EACH	2		2
*	88030020	SIGNAL HEAD, LED, 1-FACE, 3-SECTION, MAST-ARM MOUNTED	EACH	1		1
*	88030050	SIGNAL HEAD, LED, 1-FACE, 3-SECTION, BRACKET MOUNTED	EACH	2		2
	88200410	TRAFFIC SIGNIAL RACKRI ATE LOUVERED FORMED DI ACTIC	EACH	1		1
	88200410	TRAFFIC SIGNAL BACKPLATE, LOUVERED, FORMED PLASTIC	EACH	'		'
*	89502300	REMOVE ELECTRIC CABLE FROM CONDUIT	FOOT	1,200		1,200
*	89502375	REMOVE EXISTING TRAFFIC SIGNAL EQUIPMENT	EACH	1		1
	Z0001899	JACK AND REMOVE EXISTING BEARINGS	EACH	17		17
	Z0001904	STRUCTURAL STEEL REMOVAL	LSUM	1		1
	Z0001905	STRUCTURAL STEEL REPAIR	POUND	7,737		7,737
	Z0004552	APPROACH SLAB REMOVAL	SQ YD	237	237	
	Z0006900	BRIDGE FLOOR REMOVAL	SQ YD	804		804
	Z0007101	CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES NO. 1	LSUM	1		1

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		US 30 OVER	DES PLA	NINES RIVER		F.A. P. RTE.	SECTION	COUNTY	TOTAL	SHEET NO.
		SUMMARY OF QUANTITIES						WILL	128	10
ļ		JUINIMA	11 01 00/	ANTITIES				CONTRACT	NO. 6	2M79
	SCALE: N.T.S.	SHEET 7 OF 1	1 SHEETS	STA.	TO STA.		ILLINOIS FED.	AID PROJECT		

		SUMMARY OF QUANTITIES			80% FED	20% STATE
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013
	Z0010501	CLEANING AND PAINTING STEEL BRIDGE NO. 1	L SUM	1		1
	Z0012754	STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT	878		878
	Z0012755	STRUCTURAL REPAIR OF CONCRETE (DEPTH GREATER THAN 5 INCHES)	SQFT	554		554
	Z0013798	CONSTRUCTION LAYOUT	L SUM	1	1	
	Z0030850	TEMPORARY INFORMATION SIGNING	SQFT	55	55	
*	Z0033028	MAINTENANCE OF LIGHTING SYSTEM	CAL MO	12		12
	20033020	WAINTENANCE OF EIGHTING GTOTEM	OAL WO	12		12
	Z0043900	PREFORMED JOINT FILLER	FOOT	123		123
	Z0050600	REMOVE AND RESET ORNAMENTAL FENCE	FOOT	90	90	
	Z0056668	STORM SEWERS, TYPE 2, WATER MAIN QUALITY PIPE, 12"	FOOT	26	26	
	Z0073200	TEMPORARY SHORING AND CRIBBING	EACH	2		2
	Z0073500	TEMPORARY SUPPORT SYSTEM	L SUM	1		1
*	Z0073510	TEMPORARY TRAFFIC SIGNAL TIMING	EACH	11	11	
	X0322005	REMOVAL OF EXISTING TIMBER MATERIAL	L SUM	1		1

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						F.A.P. RTE	SECT	ION		COUNTY 5		SHEET NO.
SUMMARY OF QUANTITIES						607	2018-067-BR WILL		128	11		
	30	IVIIVIAII I	01 40/	AIN I I I I LO						CONTRACT	NO. 62	2M79
SCALE: N.T.S.			ILLINOIS	FED. AIC	D PROJECT							

			80% FED 20% STATE			
SPECIALTY	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013
	X5091742	REMOVE AND RE-ERECT EXISTING BRIDGE RAIL	FOOT	575		575
*	X0324599	ROD AND CLEAN EXISTING CONDUIT	FOOT	400		400
	X0323802	REMOVE AND REPLACE FIBERGLASS JACKET	SQFT	1,118		1,118
_	X0323926	COUNTERWEIGHT REPAIR MATERIAL	L SUM	1		1
_	X0326232	FENDER SYSTEM	L SUM	1		1
*	X0326366	ELECTRICAL EQUIPMENT REMOVAL AND SALVAGE	EACH	18		18
	X0326440	SURFACE REMOVAL, VARIABLE DEPTH (SPECIAL)	SQYD	843	843	
	X0326546	TRAFFIC GATES	EACH	4		4
	X0326556	MECHANICAL DEMOLITION	L SUM	1		1
_	X0326557	REFURBISHING OF OPERATING MACHINERY	L SUM	1		1
	X0326558	REPLACEMENT OF CENTER LOCKS	EACH	4		4
	X0327611	REMOVE AND REINSTALL BRICK PAVER	SQFT	69	69	
*	X0327739	MISCELLANEOUS ELECTRICAL WORK	L SUM	1		1

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DEPARTMENT OF TRANSPORTATION

						F.A. P. RTE.	SECT I ON	COUNTY	TOTAL	SHEET NO.		
	SUMMARY OF QUANTITIES						607	507 2018-067-BR		128	12	
Į	SUMINIANT OF GOANTITIES								CONTRACT	NO. 62	2M79	
	SCALE: N.T.S.	SHEET 9	OF 11	SHEETS	STA.	TO STA.		ILLINOIS FED. AID PROJECT				

		SUMMARY OF QUANTITIES			80% FED	20% STATE
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013
	X0327760	STEEL GRID DECK	SQ FT	7,233		7,233
	X0328035	BRIDGE BARRIER	FOOT	345		345
	X0900033	BRIDGE BALANCING	L SUM	1		1
*	X1200139	REMOVAL OF LIGHTING LUMINAIRE, SALVAGE	EACH	4		4
	X5051202	REMOVE AND RE-ERECT EXISTING STRUCTURAL STEEL	L SUM	1		1
	X5067500	BRIDGE CLEANING AND PAINTING WARRANTY	L SUM	1		1
	X5210006	LIVE LOAD BEARINGS	EACH	4		4
	X5509900	ABANDON AND FILL EXISTING STORM SEWER	FOOT	27	27	
	X6025604	PROPOSED MANHOLE/CATCH BASIN CONNECTION OVER EXISTING STORM SEWER	EACH	2	2	
	X6030310	FRAMES AND LIDS TO BE ADJUSTED (SPECIAL)	EACH	3	3	
	X6700410	ENGINEER'S FIELD OFFICE, TYPE A (SPECIAL)	CAL MO	24	24	
	X7010216	TRAFFIC CONTROL AND PROTECTION, (SPECIAL)	L SUM	1	1	
*	X8250500	LIGHTING UNIT COMPLETE, SPECIAL	EACH	4		4

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	US 30	OVER	DES PLA	AINES R	RIVER	F.A.P. RTE	SECTION
	607	2018-067-BR					
		IMMARY	01 40	~!VIIIIE	<u> </u>		
SCALE: N.T.S.	SHEET 10	OF 11	SHEETS	STA.	TO STA.		ILLINOIS FED

COUNTY TOTAL SHEET NO.
WILL 128 13 CONTRACT NO. 62M79
FED. AID PROJECT

		SUMMARY OF QUANTITIES			80% FED 2	20% STATE
SPECIALTY ITEM	CODE NO.	ITEM	UNIT	TOTAL	RDWY 0004	BRIDGE REHAB 0013
*	X8420105	REMOVAL OF WATERWAY OBSTRUCTION WARNING LUMINAIRE	EACH	8		8
*	X8420111	REMOVAL OF UNDERPASS LIGHTING UNIT, NO SALVAGE	EACH	6		6
*	X8430100	REMOVE EXISTING CONDUIT ATTACHED TO STRUCTURE	FOOT	330		330
*	X8950310	RELOCATE EXISTING SIGNAL HEAD AND POST	EACH	2		2
i	Z0076600	TRAINEES	H0URS	1500		1500
i -	Z0076604	TRAINEES - TRAINING PROGRAM GRADUATE	HOURS	1500		1500

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PLOT DATE = 3/25/2024	DATE -	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

		US 30	OVER [DES PLA	NINES RIVE	:R	F.A. P. RTE.	SECTION
	SUMMARY OF QUANTITIES							2018-067-BR
l	SCALE: N.T.S.	SHEET 11	OF 11	SHEETS	STA.	TO STA.		ILLINOIS

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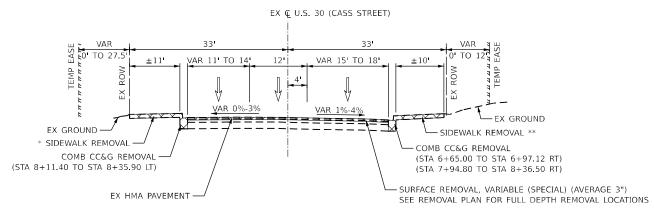
		SCHEDULE -	ROADWAY AND	SIDEWALK RE	MOVALS			
	40600982	4400100	44000500	44000600	X0326440	X0327611	Z0004552	Z0050600
LOCATION	HOT-MIX ASPHALT SURFACE REMOVAL - BUTT JOINT	PAVEMENT REMOVAL	COMBINATION CURB AND GUTTER REMOVAL	SIDEWALK REMOVAL	SURFACE REMOVAL, VARIABLE DEPTH (SPECIAL)	REMOVE AND REINSTALL BRICK PAVER	APPROACH SLAB REMOVAL	REMOVE AND RESET ORNAMENTAL FENCE
STA. 6+65 - 6+69.5	19.0							
STA. 6+69.5 - 8+11.4					767.3			
STA. 8+11.4 - 8+36.5		122.7						
STA. 8+36.5 - 8+58.5							102.2	
STA. 11+35 - 11+72							133.9	
STA. 11+72 - 11+96.2		111.1						
STA. 11+96.2 - 12+12.5					75.3			
STA. 12+12.5 - 12+17	20.5							
ISLAND - STA. 7+50, LT								
STA. 6+65 - 7+00, RT			36.6			69.0		
STA. 7+04.5 - 7+30, LT								20.0
STA. 7+25 - 7+53.8, RT								
STA. 7+80 - 8+58.5, LT			24.1	349.5				70.0
STA. 8+11.4 - 8+56.2, RT			42.6	463.5				
STA. 11+30.8 - 11+96.2, RT			24.2	534.2				
STA. 11+51.5 - 11+96.2, LT			24.7	358.9				
TOTAL	40.0	234.0	153.0	1,707.0	843.0	69.0	237.0	90.0

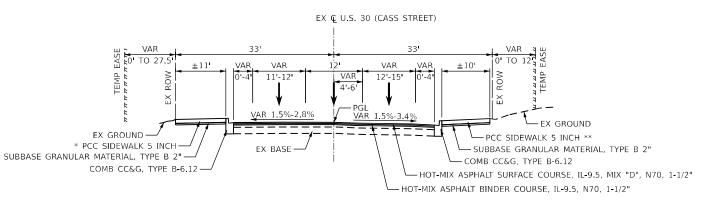
		EAF	RTHWORK SCHED	ULE		
STATION	LENGTH	EARTH EXCAVATION	NON-SPECIAL WASTE	EXCAVATION FOR EMBANKMENT ADJUSTED FOR SHRINKAGE (15%) [1]	EMBANKMENT	EARTHWORK BALANCE WASTE () OR SHORTAGE (-) [1]
		Α	В	C=(A-B)*(1-%)	D	E=C-D
	(FT)	(CU YD)	(CU YD)	(CU YD)	(CU YD)	(CU YD)
BROADWAY AV	E					
6+65.0						
7+00.0	35.0	1.7	1.7	0.0	0.0	0.0
7+50.0	50.0	0.0	0.0	0.0	0.0	0.0
8+00.0	50.0	2.4	2.4	0.0	0.0	0.0
8+11.4	11.4	15.9	15.9	0.0	0.0	0.0
8+26.4	15.0	39.3	39.3	0.0	0.0	0.0
			BRIDGE OMISSIO	N		
11+74.4						
11+96.2	21.8	57.9	57.9	0.0	0.0	0.0
12+17.0	20.8	27.3	27.3	0.0	0.0	0.0
STRUCTURAL E	EXC.					
STORM SEWER	TRENCH EXC.					
SIGNAL EXCAV	ATION					
SUB-TOTAL		144.6	144.6	0.0	0.0	0.0
PROJECT TOTA	L L	145.0	145.0	0.0	0.0	0.0

^[1] COLUMN FOR INFORMATION ONLY.

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	US 30 OVER DES PLAINES RIVER SCHEDULE OF QUANTITIES				F.A.P. RTE			COUNTY	TOTAL SHEETS	SHEET NO.			
					607	2018-067-BR		WILL	128	15			
	SUILDOLL OF COMMITTIES									CONTRACT	NO. 62	2M79	
	SCALE: N.T.S.	SHEET 1	OF	1 SHEETS	STA.	TO STA.			ILLINOIS	FED. AI	D PROJECT		





EXISTING U.S. 30 (CASS STREET)

* STA 8+11.40 TO STA 8+58.50 LT

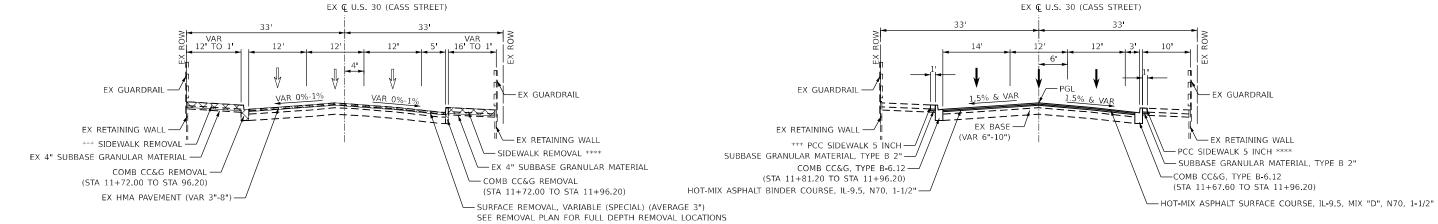
** STA 6+65.00 TO STA 6+97.12 RT STA 7+94.80 TO STA 8+56.20 RT STA 6+65.00 TO STA 8+36.46 (BRIDGE OMISSION: STA 8+36.46 TO STA 11+72.00) LOOKING EAST

PROPOSED U.S. 30 (CASS STREET)

STA 6+65.00 TO STA 8+26.40 (BRIDGE OMISSION: STA 8+26.40 TO STA 11+74.40) LOOKING EAST

* STA 8+11.40 TO STA 8+44.40 LT

** STA 6+65.00 TO STA 6+97.12 RT STA 7+94.80 TO STA 8+42.60 RT



EXISTING U.S. 30 (CASS STREET)

*** STA 11+51.50 TO STA 11+96.20 LT

**** STA 11+30.80 TO STA 11+96.20 RT

STA 11+72.00 TO STA 12+17.00 LOOKING EAST

PROPOSED U.S. 30 (CASS STREET)

STA 11+74.40 TO STA 12+17.00 LOOKING EAST

*** STA 11+81.20 TO STA 11+96.20 LT

**** STA 11+67.60 TO STA 11+96.20 RT

	MIXTURE TYPE	PERCENT AIR VOIDS @ NDES	QMP			
	PAVEMENT RESURFACING					
	HOT-MIX ASPHALT SURFACE COURSE, IL-9.5, MIX "D", N70; 1.5"	4% @ 70 GYR.	QC/QA			
	HOT-MIX ASPHALT BINDER COURSE, IL-9.5, N70; 1.5"	4% @ 70 GYR.	QC/QA			
	PATCHING					
	CLASS D PATCH (HMA BINDER COURSE, IL-19.0); 6"	4% @ 70 GYR.	QC/QA			
	STABILIZED SUBBASE					
	STABILIZED SUBBASE - HOT-MIX ASPHALT, 4" (HMA BINDER COURSE, IL-19.0)	3% @ 50 GYR.	QC/QA			
QMP DESIGNATION: QUALITY CONTROL/QUALITY ASSURANCE (QC/QA); QUALITY CONTROL FOR PERFORMANCE (QCP); PAY FOR PERFORMANCE (PFP)						

LEGEND REMOVAL

- 1. THE UNIT WEIGHT USED TO CALCULATE ALL HMA SURFACE MIXTURE QUANTITIES IS 112 LBS/SQ YD/IN.
- 2. THE "AC TYPE" FOR POLYMERIZED HMA MIXES SHALL BE "SBS/SBR PG 76-22" AND FOR NON POLYMERIZED HMA THE "AC TYPE" SHALL BE "PG 64-22" UNLESS MODIFIED BY RECLAIMED MATERIALS SPECIFICATIONS.
- 3. CONTRACTOR SHALL MILL BEFORE PATCHING.

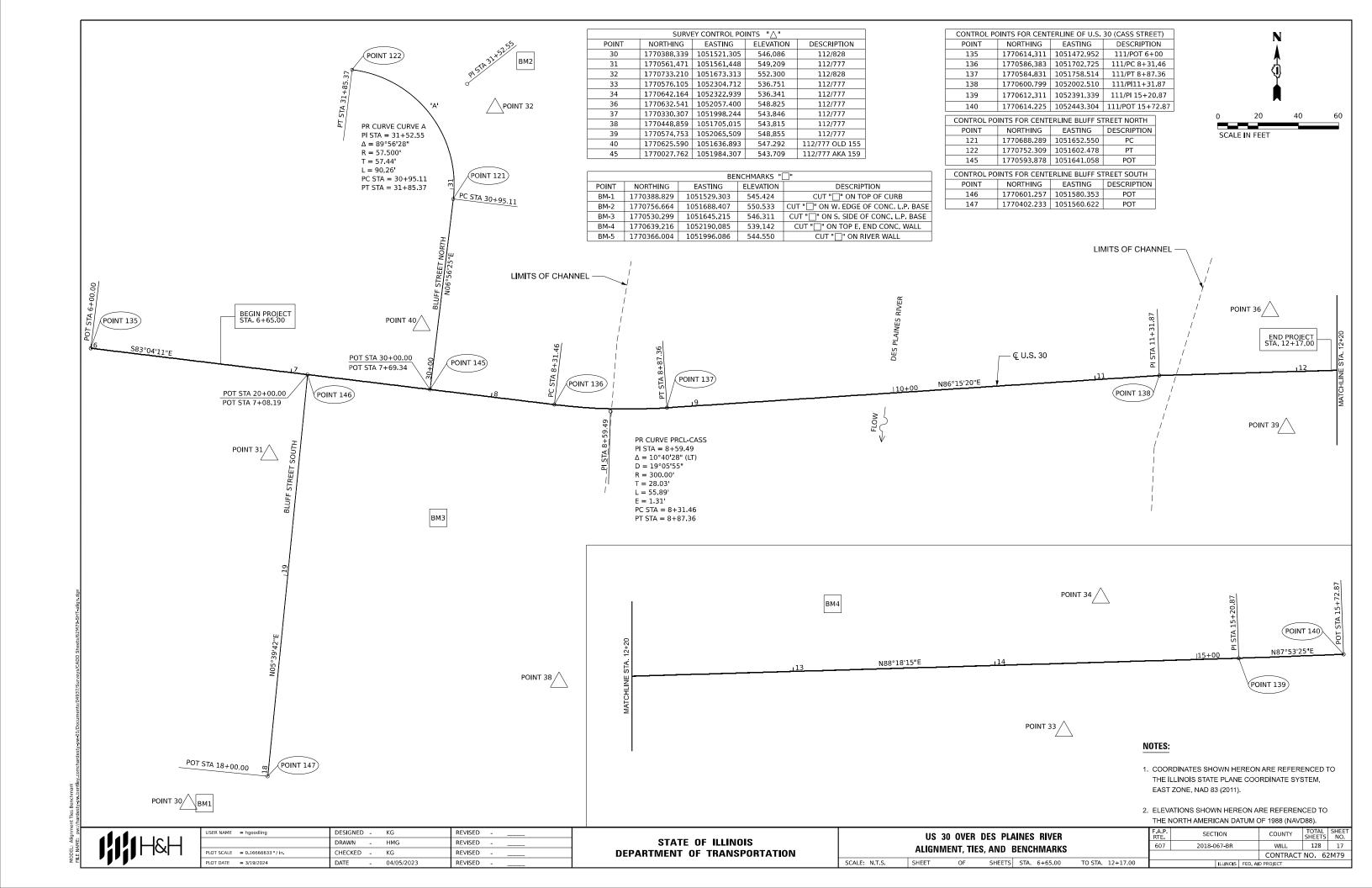
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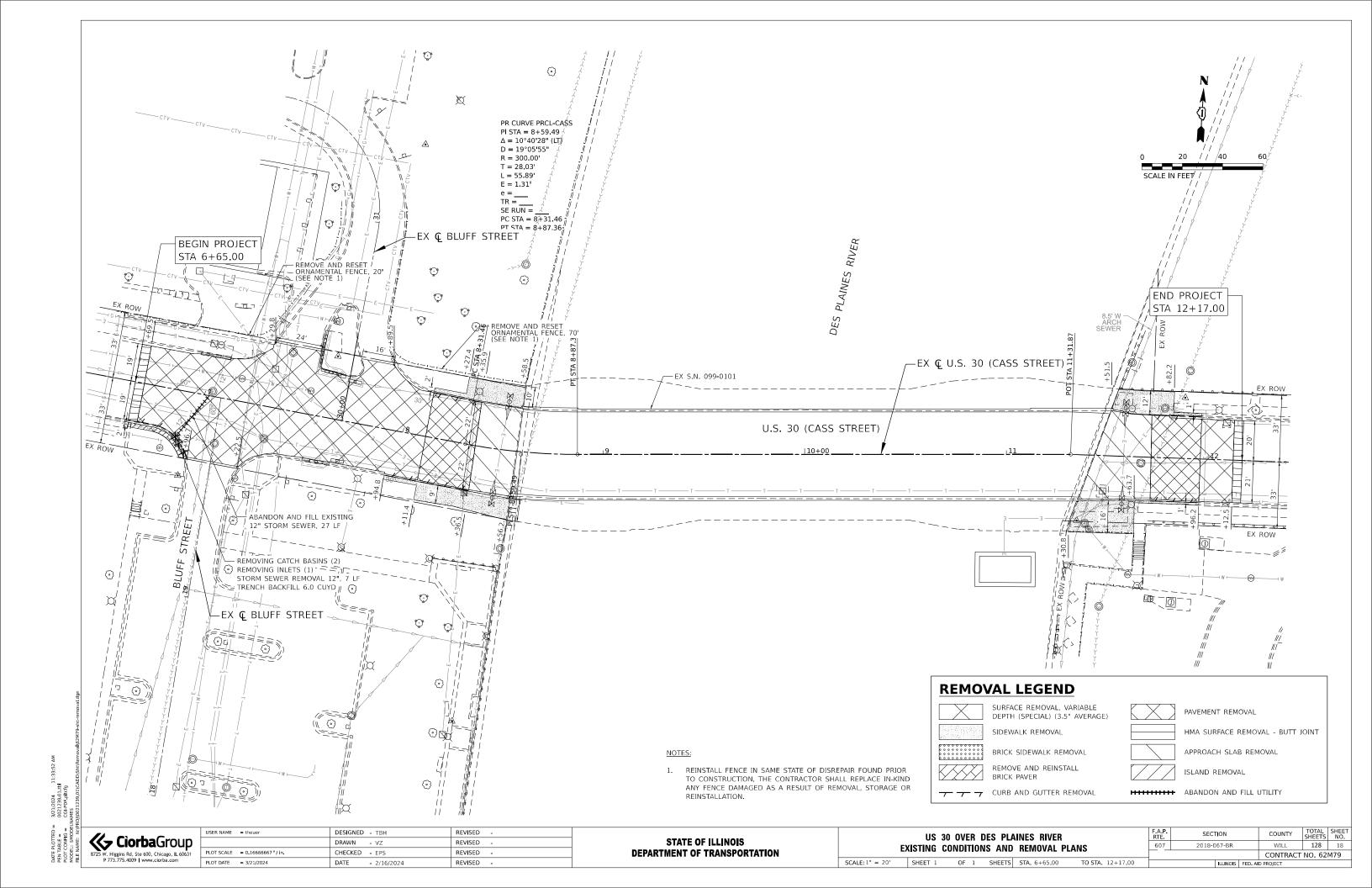
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	DRAWN - VZ	REVISED -
PLOT SCALE = 20.000 / in.	CHECKED - EPS	REVISED -
PLOT DATE = 2/23/2024	DATE - 2/16/2024	REVISED -

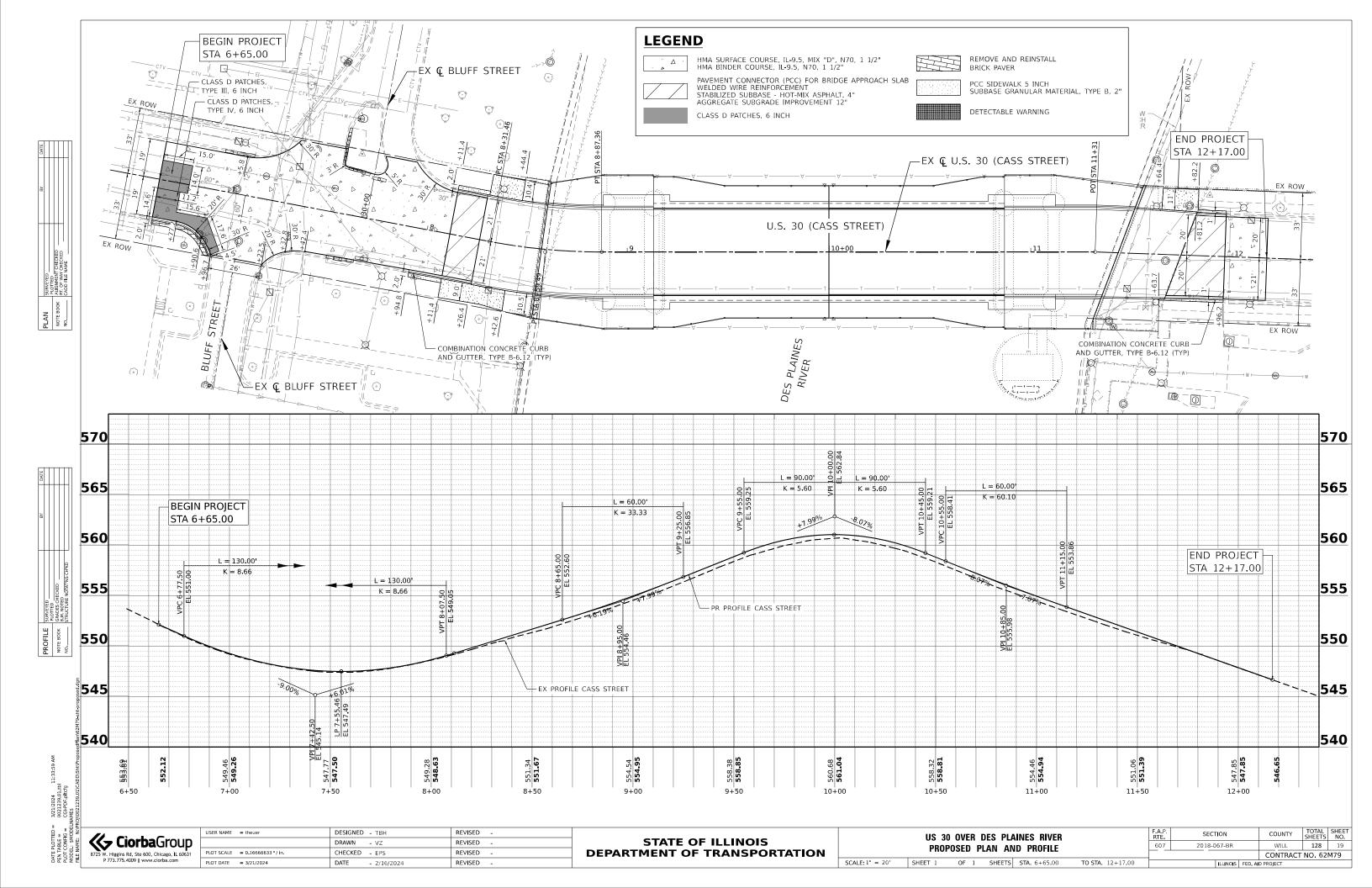
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DEPARTMENT OF TRANSPORTATION				

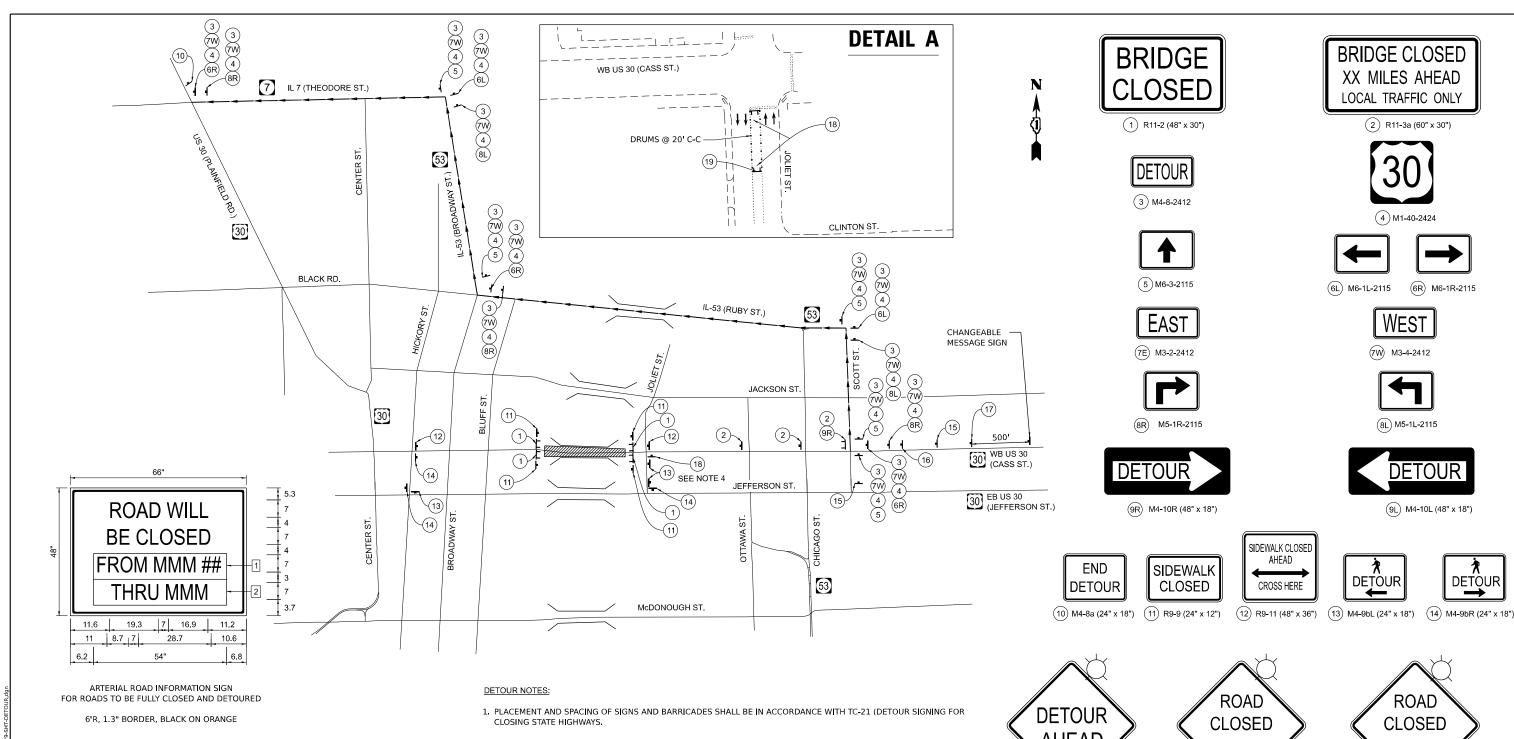
SCALE: N.T.S.

US 30 OVER DES PLAINES RIVER TYPICAL SECTIONS			F.A.P. RTE				COUNTY	TOTAL SHEETS	SHEET NO.		
			607	7 2018-067-BR		WILL	128	16			
TIFICAL SECTIONS								CONTRACT	NO. 6	2M79	
SHEET 1	OF 1	SHEETS	STA.	TO STA.	ILLINOIS FED. AID PROJECT						







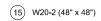


ARTERIAL ROAD INFORMATION SIGN NOTES:

- 1. SIGN SHALL BE PAID FOR AS TEMPORARY INFORMATION SIGNING. ONE SIGN ASSEMBLY EQUALS 27.3 SQ FT.
- 2. OVERLAY PANELS SHALL BE "HIGHWAY C" FONT
- 3. OVERLAY PANEL TO CONTAIN STARTING DATE OF FULL CLOSURE AND DETOUR IMPLEMENTATION
- 4. OVERLAY PANEL 2 TO CONTAIN ENDING MONTH OF FULL CLOSURE AND DETOUR, OMIT THE DATE ON PANEL; 5. RESIDENTIAL ACCESS FOR TRAFFIC EASTBOUND U.S. 30 BETWEEN HICKORY ST. AND BLUFF ST. WILL BE MAINTAINED
- 5. ERECT SIGN ASSEMBLY (POST-MOUNTED) WITH PANELS 1 AND 2 IN PLACE ON ROAD TO BE CLOSED IN EACH DIRECTION NEAR POINT OF CLOSURE OR WITHIN SECTION TO BE FULLY CLOSED TWO (2) WEEKS PRIOR TO START DATE OF FULL CLOSURE. REMOVE ASSEMBLY AFTER CLOSURE.

- 2. THE COST OF SUPPLYING, ERECTING AND MAINTAINING BARRICADES, WARNING LIGHTS AND SIGNS SHALL BE INCLUDED IN THE CONTRACT COST OF TRAFFIC CONTROL AND PROTECTION (SPECIAL).
- 3. THE CONTRACTOR SHALL DEVELOP AND MAINTAIN APPROPRIATE SIGNAL TIMINGS FOR THE FOLLOWING INTERSECTIONS: IL ROUTE 7 / PLAINFIELD RD, IL ROUTE 7 / BROADWAY ST., AND IL 53 (RUBY ST.) / IL 53 (BROADWAY ST.), OR AS DIRECTED BY
- 4. CLOSE OFF LEFT TURN LANE FROM JOLIET ST. TO WESTBOUND U.S. 30 UTILIZING IDOT STANDARD 701701. PLACE "NO LEFT TURN" SIGN IN LEFT TURN LANE. SEE DETAIL A FOR MORE INFORMATION.
- UTILIZING IDOT STANDARD 701427.
- 6. A MINIMUM OF 14 DAYS IN ADVANCE OF THE BRIDGE CLOSURE ON WB US 30, THE CONTRACTOR SHALL PLACE ONE PORTABLE CHANGEABLE MESSAGE SIGN AT THE BEGINNING OF THE PROJECT ALONG US 30 AS DIRECTED AND AT A LOCATION DESIGNATED BY THE ENGINEER TO INFORM MOTORISTS OF THE UPCOMING CLOSURE. THE MESSAGE SHALL BE APPROVED BY THE ENGINEER. THIS WORK IS TO BE PAID FOR PER CONTRACT UNIT PRICE FOR CHANGEABLE MESSAGE SIGN.
- 7. LOCAL ACCESS SHALL BE PROVIDED AT ALL TIMES TO RESIDENTS AND BUSINESSES.
- 8. THE CONTRACTOR SHALL NOT OBSTRUCT ANY EXISTING SIGN WITH THE PLACEMENT OF DETOUR SIGNAGE.







(16) W20-3 (48" x 48")



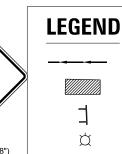
(17) W20-3 (48" x 48")





TO STA.

(19) W20-5L (48" x 48")



LEGEND	-
	DETOUR ROUTE
	WORK ZONE
コ	TYPE III BARRICADE

AMBER FLASHING LIGHT

OSEH inc. 401.5 CARLTON AVE SUITE 201
WHEATON, IL 601.87
WHEATON, IL 601.87

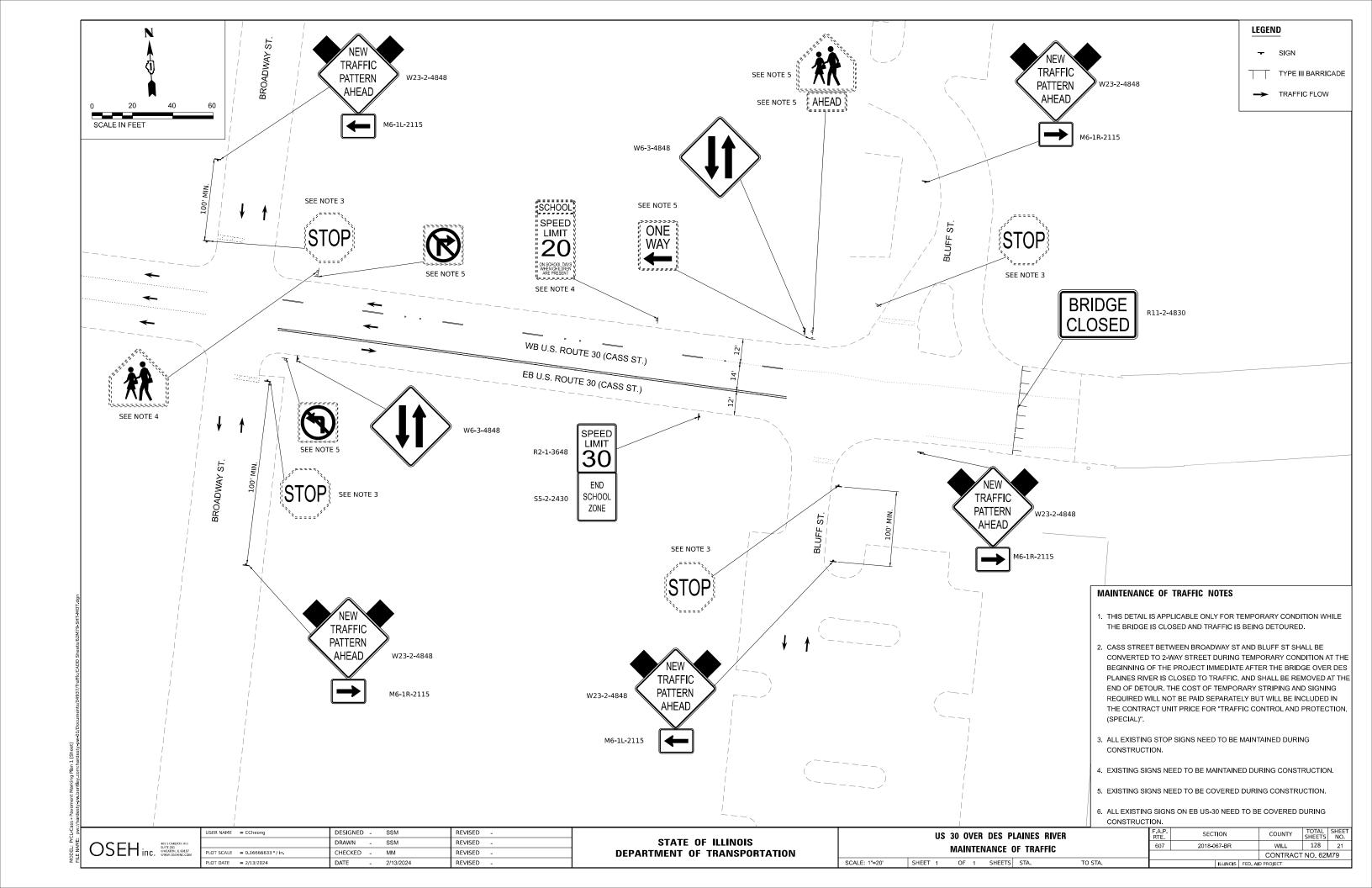
DESIGNED -REVISED DRAWN -SSM REVISED CHECKED -REVISED PLOT DATE = 2/13/2024 DATE REVISED

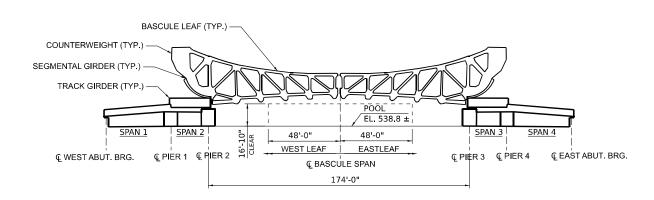
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** US 30 OVER DES PLAINES RIVER **DETOUR ROUTE** SHEETS STA.

SCALE: N.T.S

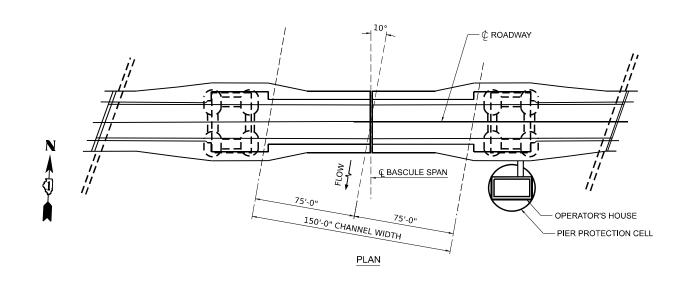
SHEET

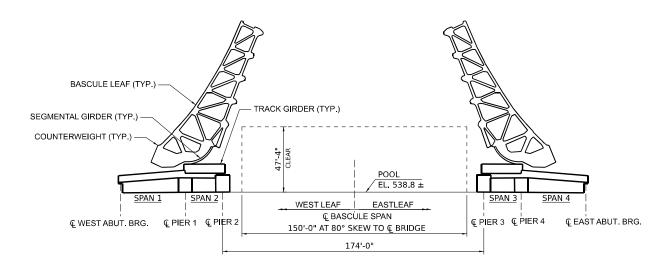
SECTION COUNTY 128 20 2018-067-BR WILL CONTRACT NO. 62M79



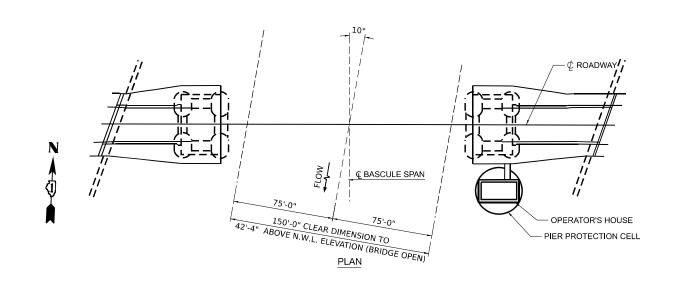


POSITION 1 (FULL CLOSED)





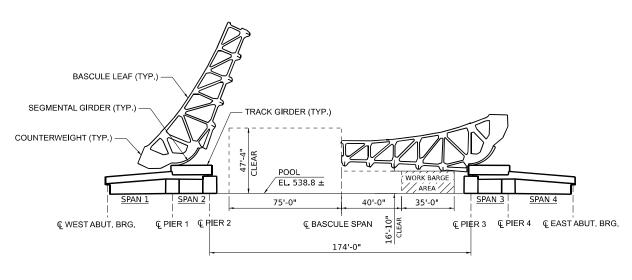
POSITION 2



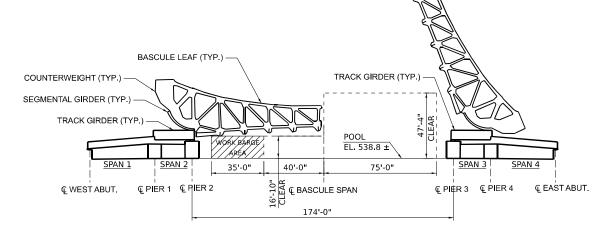
		U
OSEH inc.	401 S CARLTON AVE SUITE 201 WHEATON, IL 60187 WWW.OSEHINC.COM	P
		ь

USER NAME = CCheong	DESIGNED - SSM	REVISED
	DRAWN - SSM	REVISED
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PLOT DATE = 2/13/2024	DATE - 2/13/2024	REVISED

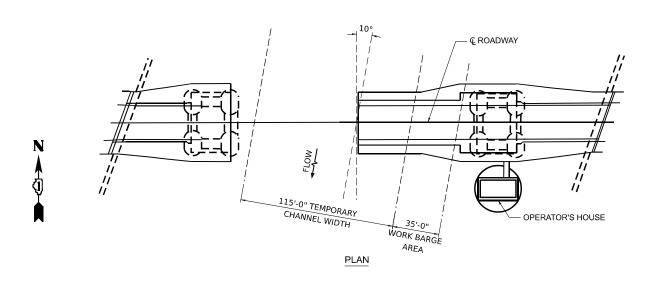
		F.A.P RTE. SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
WATER NAVIGATION PLAN – CONSTRUCTION STAGING I	607	2018-067-BR		WILL	128	22
WATER WAVIGATION TEAM - CONSTRUCTION STAGENG T				CONTRACT	NO. 62	v 179
SCALE: N.T.S SHEET 1 OF 2 SHEETS STA TO STA		ILLINO	IS FED.	AID PROJECT		

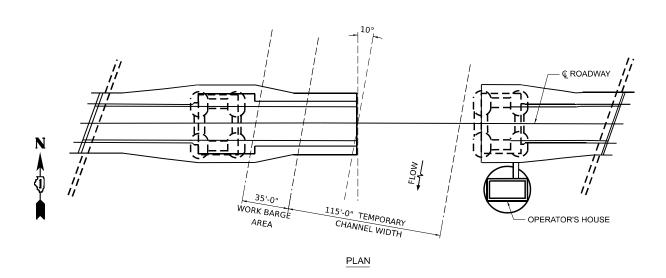


POSITION 3



POSITION 4





NOTES:

- 1. WHEN THE LEAVES OF THE BASCULE SPAN ARE INOPERABLE IN THE FULL OPEN POSITION, THE LEAVES 6. THE CONTRACTOR MAY MAINTAIN THE BRIDGE IN POSITION 3 OR POSITION 4 FOR A FEW HOURS MUST BE SECURED. THE CONTRACTOR SHALL SUBMIT THE METHOD AND DETAILS FOR SECURING THE LEAVES IN FULL OPEN POSITION TO THE ENGINEER FOR REVIEW.
- 2. THE LEAVES SHALL BE SECURED IN THE FULL OPEN POSITION DURING REHABILITATION OF THE STRUCTURAL, MECHANICAL AND ELECTRICAL SYSTEMS AND WHEN LEAF IS IN AN IMBALANCED CONDITION.
- THE NEW UNTESTED MACHINERY AND ELECTRICAL COMPONENTS OF THE DRIVE SYSTEM SHALL NOT BE USED TO LOWER THE LEAVES WITHOUT AN INDEPENDENT SUPPORT/LOWERING SYSTEM, THE CONTRACTOR SHALL SUBMIT THE METHOD OF INDEPENDENT SUPPORT/LOWERING SYSTEM TO THE ENGINEER FOR REVIEW. CONTRACTOR SHALL PROVIDE ALL NECESSARY BLOCKING TO FULLY OPEN
- WHEN NOT SECURED IN THE OPEN POSITION THE CONTRACTOR SHALL MAINTAIN BALANCE OF THE BASCULE LEAVES DURING CONSTRUCTION BY SECURING TEMPORARY BALLAST IN APPROPRIATE LOCATIONS SUCH THAT THE IMBALANCE IS NOT MORE THAN DESCRIBED IN THE SPECIAL PROVISION AND THE DRIVE SYSTEM IS NOT OVERSTRESSED.
- SEE MECHANICAL DRAWINGS FOR MACHINERY WORK. SEE ELECTRICAL DRAWINGS FOR ELECTRICAL WORK. SEE STRUCTURAL DRAWINGS FOR REPAIR AND REPLACEMENT WORK WHICH INCLUDES STEEL DECK GRATING REPLACEMENT, SIDEWALK REPLACEMENT, STEEL REPAIRS/REPLACEMENT AND

- DURING DAYLIGHT CONSTRUCTION SHIFT UPON APPROVAL BY THE U.S. COAST GUARD, MAXIMUM ENCROACHMENT INTO THE CHANNEL BY WORK BARGE IS 35 FEET.
- 7. DURING EVENING AND NIGHT AND ANYTIME CONSTRUCTION HAS CEASED, BOTH LEAVES MUST BE LEFT IN POSITION 2 (FULL OPEN) WITH WORK BARGE REMOVED FROM THE CHANNEL.
- 8. THE ROADWAY WILL BE CLOSED TO TRAFFIC DURING REHABILITATION.
- FOR CLARITY, OPERATOR'S HOUSE IS NOT SHOWN IN ELEVATIONS.

REVISED -

REVISED -

REVISED

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SSM

MM

2/13/2024

DATE

10. WORK REQUIRED TO COMPLY WITH ALL COAST GUARD CLOSURE AND NAVIGATION REQUIREMENTS SHALL BE INCLUDED IN TOTAL CONTRACT.

COUNTERWEIGHT REPAIR	RS,	
	USER NAME = CCheong	DESIGNED -
CLU 401 S CARLTON AVE	i .	DRAWN -
Jうロロ inc. WHEATON, IL 60187		CHECKED -

PLOT DATE = 2/13/2024

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

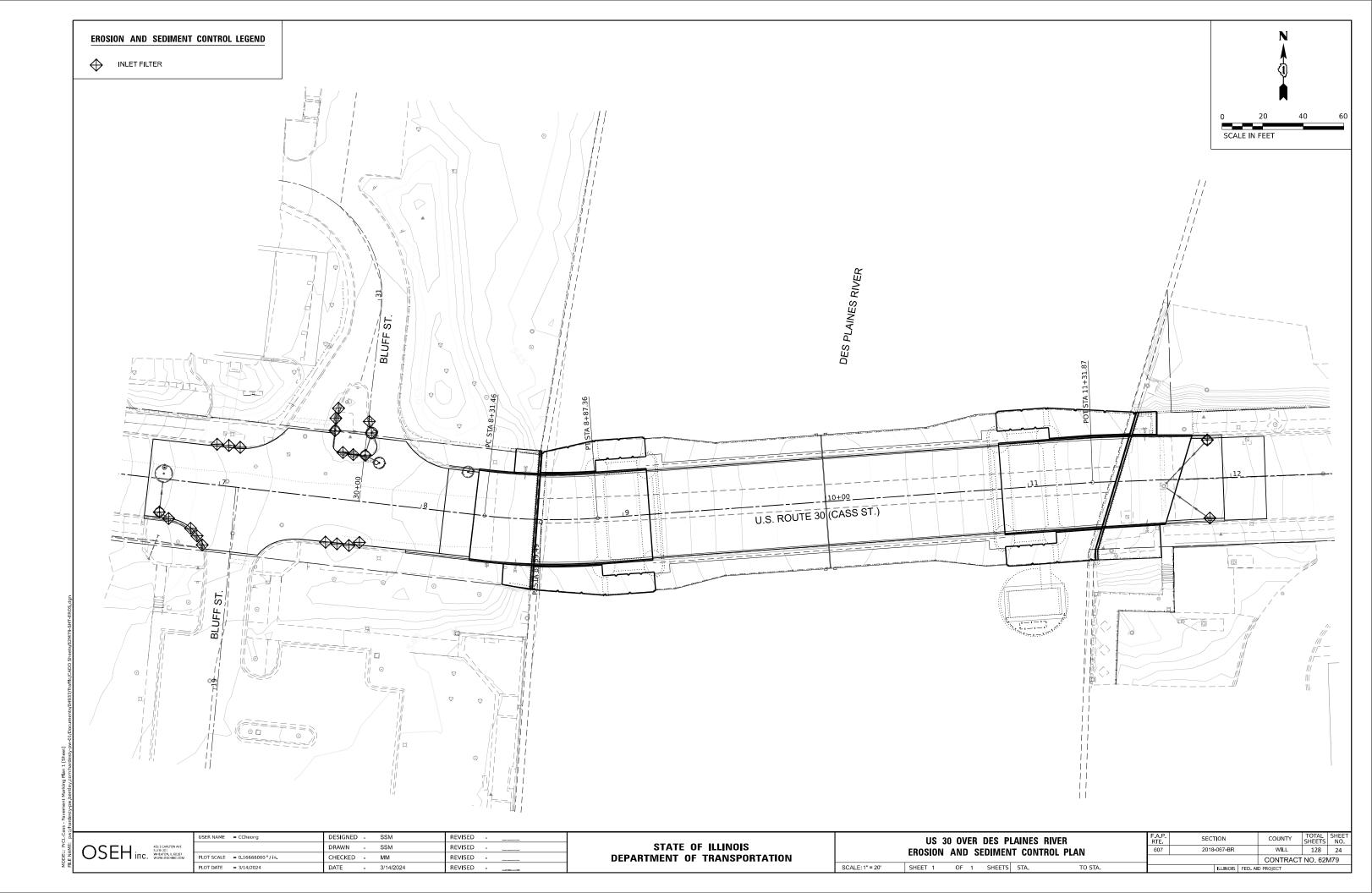
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WATER	11/11/10/11/10			101110011011	OIMOING				
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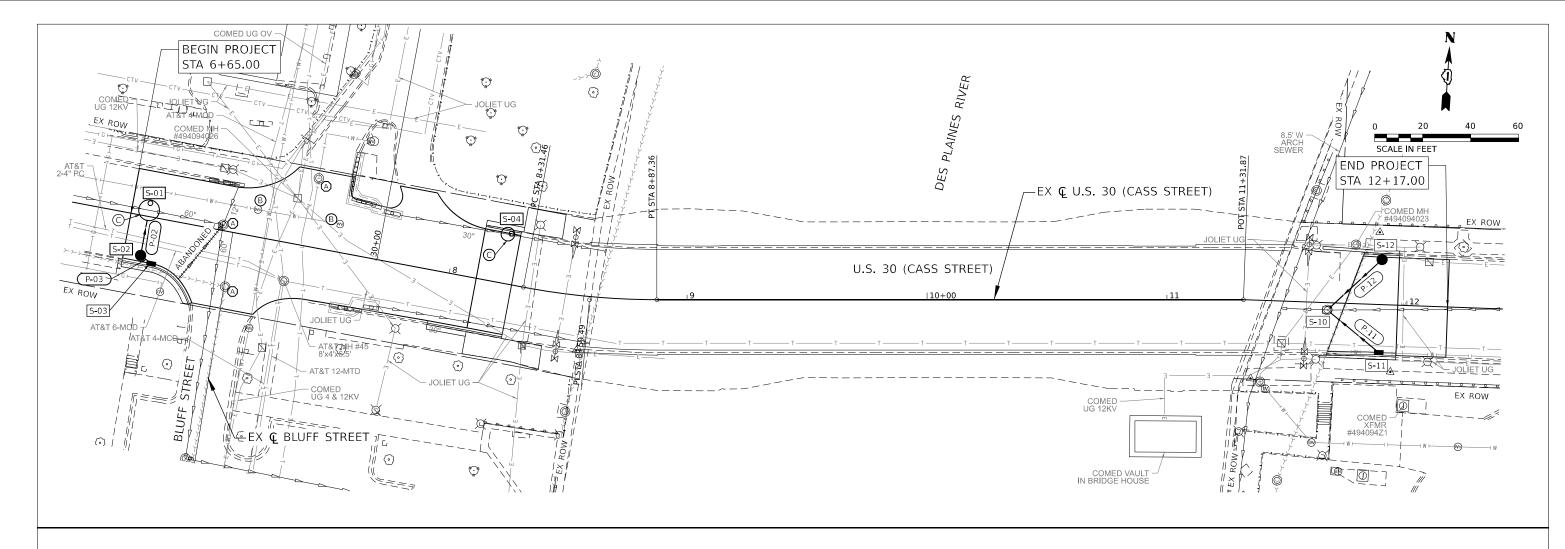
COUNTY

WILL

CONTRACT NO. 62M79

128 23





MANHOLE, TYPE A, 8'-DIAMETER, TYPE 1 FRAME, CLOSED LID CENTER OF LID STA 6+72.17, 5.6' LT RIM ELEV = 551.38 EX W 60" INV = 541.90 EX E 60" INV = 541.90 S 12" INV = 546.05

CATCH BASIN, TYPE A, 4'-DIAMETER, TYPE 1 FRAME, OPEN LID CENTER OF LID STA 6+72 17, 17 5 RT RIM ELEV = 551.13N 12" INV = 546.15 E 12" INV = 546.15 (SEE NOTE 2)

INLET, TYPE A, TYPE 1 FRAME, OPEN LID CENTER OF LID STA 6+77.15, 19.0' RT RIM ELEV = 550.69W 12" INV = 546.17

MANHOLE, TYPE A, 5'-DIAMETER, TYPE 1 FRAME, OPEN LID CENTER OF LID STA 8+22.43, 21.0' LT RIM ELEV = 549.52EX W 30" INV = 539.80 EX E 30" INV = 539.70

MANHOLES TO BE RECONSTRUCTED WITH NEW TYPE 1 FRAME, CLOSED LID CENTER OF STRUCTURE RIM = 550.10NE 12" INV = 544.10 SE 12" INV = 544.10 RECONSTRUCT MANHOLE TO TWO FEET BELOW THE PROPOSED PIPE INVERT ELEVATION

INLET, TYPE A, TYPE 1 FRAME, OPEN LID CENTER OF LID STA 11+89.00, 20.0' RT RIM ELEV = 548.38N 12" INV = 544.23

CATCH BASIN, TYPE A, 4'-DIAMETER, TYPE 1 FRAME, OPEN LID CENTER OF LID STA 11+89.00, 20.0' LT RIM ELEV = 548.43 S 12" INV = 544.24

STORM SEWER, CLASS A, TYPE 2 12" DIAMETER, 14 LF @ 0.65% TBF = 5.4 CY

STORM SEWER, CLASS A, TYPE 2 12" DIAMETER, 3 LF @ 0.65% TBF = 1.0 CY

STORM SEWER, TYPE 2, WATER MAIN QUALITY PIPE 12" DIAMETER, 26 LF @ 0.50% TBF = 10.0 CY

STORM SEWER, CLASS A, TYPE 2 12" DIAMETER, 28 LF @ 0.50%TBF = 10.7 CY

1) ALL EXISTING PIPE SIZES AND INVERT ELEVATIONS CONNECTING TO PROPOSED STRUCTURES SHALL BE CONFIRMED BY THE CONTRACTOR PRIOR TO ORDERING PRECAST STRUCTURES.

2) CENTER OF S-02 OPEN LID IS INTENTIONALLY OFFSET 1.5 FEET FROM EDGE OF PAVEMENT.

DRAINAGE LEGEND

—

 EXISTING STORM SEWER PROPOSED STORM SEWER

EXISTING STORM STRUCTURES

PROPOSED STORM STRUCTURES

FRAMES AND LIDS TO BE ADJSUTED, SPECIAL lacktriangle(SEE IDOT DISTRICT ONE DETAIL BD-08)

VALVE BOXES TO BE ADJSUTED

PROPOSED MANHOLE CONNECTION OVER EXISTING STORM SEWER



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PLOT DATE = 2/23/2024	DATE - 2/16/2024	REVISED -

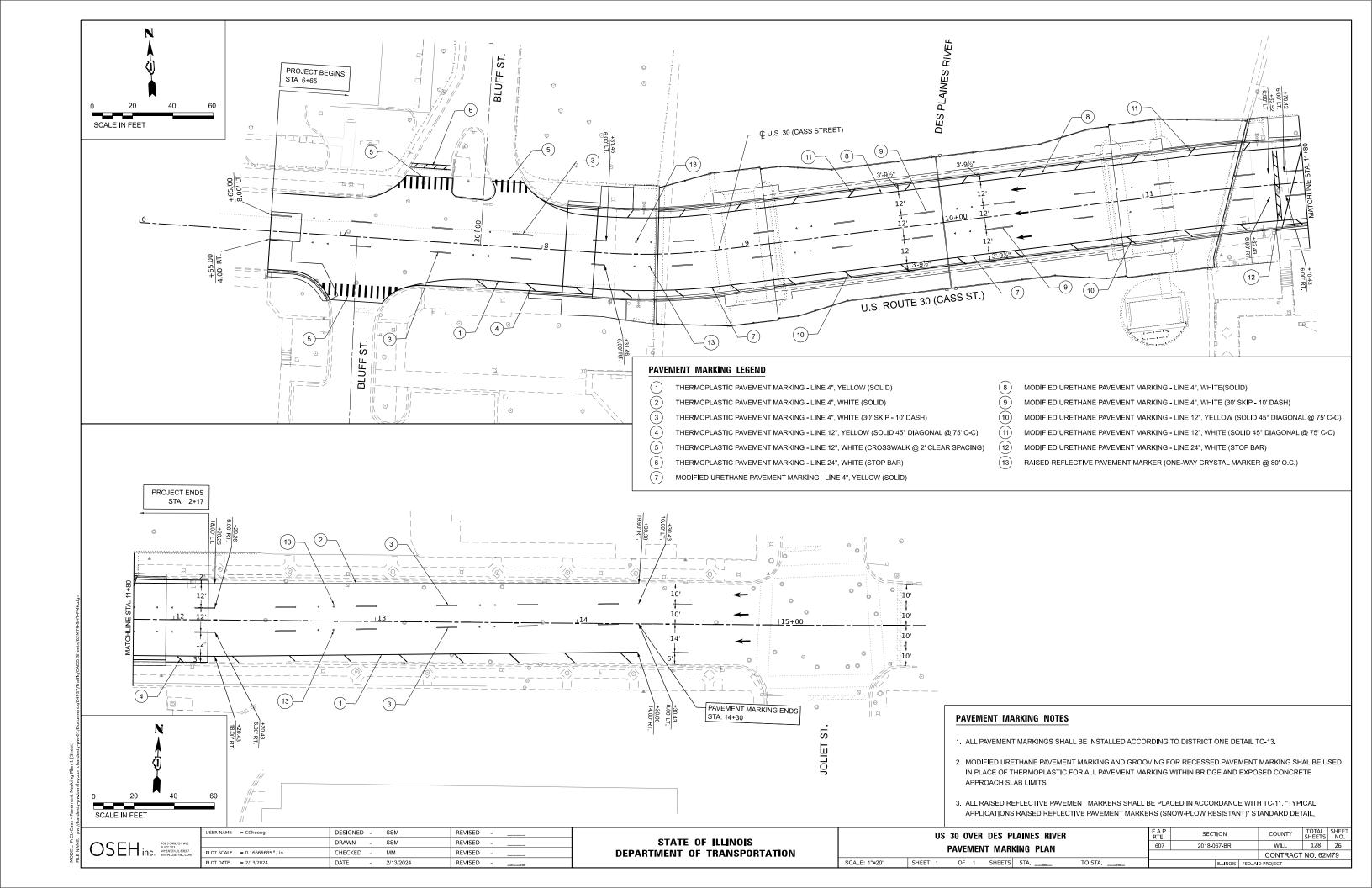
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

US 30	OVER	DES	PLAINE	S RIVER	
PROI	POSED	DR#	INAGE	PLAN	

F.A.P. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
607	2018-067-BR		WILL	128	25
			CONTRACT	NO. 62	M79
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SCALE: 1" = 20' SHEET 1 OF 1 SHEETS STA. 6+65.00

TO STA. 12+17.00



US 30 (CASS STREET) AT DES PLAINES RIVER BRIDGE LOOKING WEST



REMOVAL NOTES:

THE FOLLOWING ITEMS SHALL BE REMOVED BY THE CONTRACTOR AND SHALL BE DISPOSED OF BY THEM OUTSIDE THE RIGHT-OF-WAY AT THEIR EXPENSE. THIS WORK SHALL BE PAID FOR UNDER PAY ITEM "REMOVE EXISTING TRAFFIC SIGNAL EQUIPMENT". THE SALVAGE VALUE OF THE REMOVED EQUIPMENT SHALL BE REFLECTED IN THE CONTRACT BID PRICE.

SIGN PANEL TRAFFIC SIGNAL POST 3-SECTION SIGNAL HEAD TRAFFIC SIGNAL BACKPLATE EACH EACH EACH

NOTES:

- EXISTING CABLES BETWEEN THE BRIDGE HOUSE AND EXISTING SIGNAL POSTS SHALL BE REMOVED. EXISTING CONDUITS ARE TO REMAIN IN PLACE. SEE TRAFFIC SIGNAL INSTALLATION SHEET (TS-02) FOR MORE DETAILS.THIS WORK SHALL BE PAID FOR UNDER PAY ITEM "REMOVE ELECTRIC CABLE FROM CONDUIT".
- EXISTING TRAFFIC SIGNAL EQUIPMENT ON THE WEST SIDE OF THE BRIDGE IS TO REMAIN.

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USER NAME = mgarvida	DESIGNED - EEC	REVISED -
	DRAWN - EEC	REVISED -
PLOT SCALE = 100.0000 / in.	CHECKED - GG	REVISED -
PLOT DATE = 2/10/2024	DATE - 2/8/2024	REVISED -

US RTE 30 (C	ASS ST/W	ESTERN	AVE) A	T DES	PLAINES RIVER BRIDGE	F.A.P. RTE	SECTION
,					10VAL PLAN	607	2018-067-BR
SCALE:	SHEET 1	OF 2	SHEETS	STA.	TO STA.		ILLINOIS F

TS-01

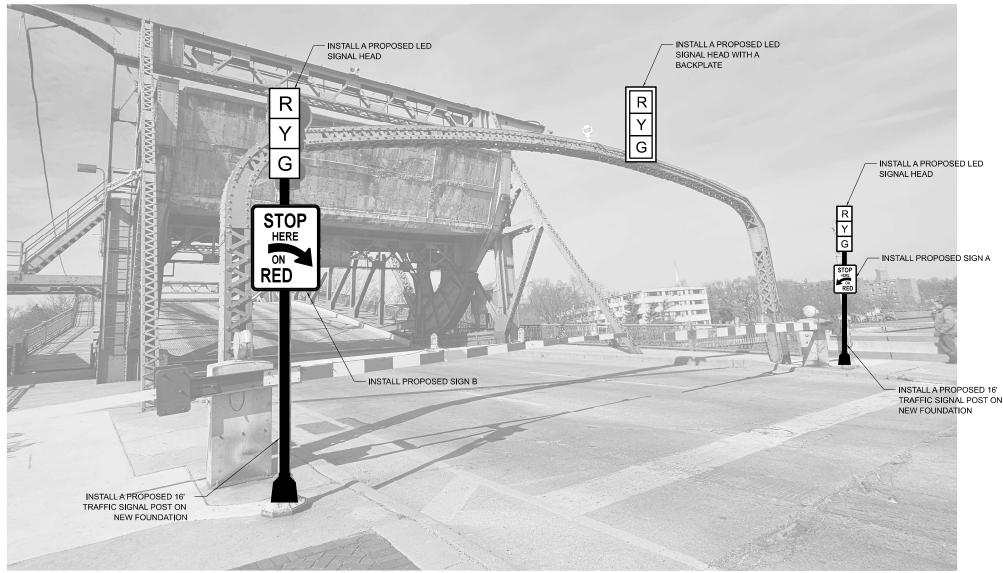
128 27

CONTRACT NO. 62M79

COUNTY

WILL

US 30 (CASS STREET) AT DES PLAINES RIVER BRIDGE LOOKING WEST



NOTES:

- 1. THE CONTRACTOR SHALL INSTALL NEW SIGNAL POST FOUNDATIONS AND CONDUIT ELBOWS FOR THE TRAFFIC SIGNAL POSTS. THE EXISTING CONDUITS FROM THE BRIDGE HOUSE TO THE EXISTING TRAFFIC SIGNAL POSTS SHALL REMAIN IN PLACE AND SHALL BE INTERCEPTED. THESE WORKS SHALL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE COST OF "MISCELLANEOUS ELECTRICAL WORK" PAY ITEM.
- 2. THE EXISTING CABLES FOR THE TRAFFIC SIGNALS SHALL BE REMOVED, AND NEW CABLES SHALL BE INSTALLED.
- 3. SEE SHEE S-19 STRUCTURE PLANS FOR THE SIGNAL POST FOUNDATION DETAIL.
- 4. THE TWO EXISTING TRAFFIC SIGNAL POST AND TWO 3-SECTION SIGNAL HEADS LOCATED NORTHWEST AND SOUTHWEST QUADRANT OF THE WEST BRIDGE APPROACH SHALL BE REMOVED AND RELOCATED ONTO NEW FOUNDATION. PRIOR TO THE REMOVAL OF THE SIGNAL POST FOUNDATION, THE EXISTING ELECTRIC CABLES FOR THE 3-SECTION TRAFFIC SIGNAL HEAD SHALL BE PULLED BACK TO THE NEAREST EXISTING HANDHOLE OR JUNCTION BOX AND RE-USED UPON INSTALLATION OF NEW TRAFFIC SIGNAL POST AND FOUNDATION. REUSE THE EXISTING CONDUITS, REPLACING ONLY THE SEGMENTS THAT INCUR DAMAGE DURING THE REMOVAL AND INSTALLATION OF THE NEW FOUNDATION. THIS WORK SHAL BE PAID FOR UNDER PAY ITEM "RELOCATE EXISTING SIGNAL HEAD AND POST." SEE SHEET S-17 FOR MORE DETAILS.

SIGN DETAILS

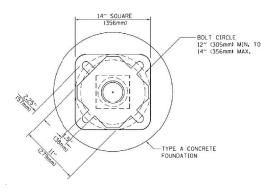




SIGN A R10-6a (24"X30") 1 REQUIRED

R10-6aR (24"X30") 1 REQUIRED

SIGNAL POST BOLT PATTERN DETAIL



SCHEDULE OF QUANTITIES

ITEM	ITEM DESCRIPTION	UNITS	TOTAL QTY
72000100	SIGN PANEL - TYPE 1	SQ FT	10
85000200	MAINTENANCE OF EXISTING TRAFFIC SIGNAL INSTALLATION	EACH	1
87301245	ELECTRIC CABLE IN CONDUIT, SIGNAL NO. 14 5C	FOOT	700
87301900	ELECTRIC CABLE IN CONDUIT, EQUIPMENT GROUNDING CONDUCTOR, NO. 6 1C	FOOT	500
87502500	TRAFFIC SIGNAL POST, GALVANIZED STEEL 16 FT.	EACH	2
88030020	SIGNAL HEAD, LED, 1-FACE, 3-SECTION, MAST-ARM MOUNTED	EACH	1
88030050	SIGNAL HEAD, LED, 1-FACE, 3-SECTION, BRACKET MOUNTED	EACH	2
88200410	TRAFFIC SIGNAL BACKPLATE, LOUVERED, FORMED PLASTIC	EACH	1
89502300	REMOVE ELECTRIC CABLE FROM CONDUIT	FOOT	1,200
89502375	REMOVE EXISTING TRAFFIC SIGNAL EQUIPMENT	EACH	1
X8950310	RELOCATE EXISTING SIGNAL HEAD AND POST	EACH	2
X0324599	ROD AND CLEAN EXISTING CONDUIT	FOOT	400

TS-02



USER NAME = mgarvida	DESIGNED - EEC	REVISED -
	DRAWN - EEC	REVISED -
PLOT SCALE = 100.0000 / in.	CHECKED - GG	REVISED -
PLOT DATE = 2/10/2024	DATE - 2/8/2024	REVISED -

LIGH	ITING AND ELECTRICAL LEGEND
SYMBOL	DESCRIPTION
○— [[]	EXISTING LIGHTING UNIT TO REMAIN (AT VILLAGE OF JOLIET)
- R	IDOT EXISTING LUMINAIRE TO BE REMOVED
-α	PROPOSED IDOT LIGHTING UNIT MOUNTED TO BRIDGE STRUCTURE, 4'-10.5" OVERALL POLE HEIGHT (20' MOUNTING HEIGHT), 3'-8" DAVIT ARM, LED LUMINAIRE, HORIZONTAL MOUNT, OUTPUT DESIGNATION A (120 VAC) U.N.O.
₩ R/G	IDOT PROPOSED NAVIGATION LIGHTING UNIT 360 DEGREE RED AND GREEN (120 VAC)
$\overline{\mathbb{Q}}_{\mathbb{R}}$	IDOT PROPOSED NAVIGATION LIGHTING UNIT 180 DEGREE RED (120 VAC)
○ R	IDOT EXISTING UNDERPASS LUMINAIRE TO BE REMOVED, NO SALVAGE
O	IDOT PROPOSED LUMINAIRE, LED, UNDERPASS, WALLMOUNT, OUTPUT DESIGNATION D
J _R	EXISTING JUNCTION BOX TO BE REMOVED
JE	EXISTING JUNCTION BOX TO BE REMAIN
•	PROPOSED JUNCTION BOX, 6"X6"X4", STAINLESS STEEL
CE	EXISTING TERMINAL CABINET TO REMAIN
——— E ——	EXISTING CONDUIT ATTACHED TO STRUCTURE TO BE REMOVED
——Е—	EXISTING CONDUIT ATTACHED TO STRUCTURE TO REMAIN
A	EXISTING AERIAL CABLE TO REMAIN
	ELECTRIC CABLE IN CONDUIT ATTACHED TO STRUCTURE, SIZE AND TYPE AS NOTED

	ABBREVIATIONS
ABBREVIATION	DESCRIPTION
AC	ALTERNATING CURRENT
A/C	AERIAL CABLE
ATS	ATTACHED TO STRUCTURE
B.O.C. CB	BACK OF CURB CIRCUIT BREAKER
CKT	CIRCUIT BREAKER CIRCUIT
CM	CENTIMETER
COMED	COMMONWEALTH EDISON COMPANY
CP	CONTROL PANEL
CT	CURRENT TRANSFORMER
DA DC	DAVIT ARM DIRECT CURRENT
DIA	DIAMETER
DP	DISTRIBUTION PANEL
E	EXISTING UNIT TO REMAIN
EX.	EXISTING
ECA	ELECTRIC CABLE ASSEMBLY
EIS E.O.P.	EMBEDDED IN STRUCTURE EDGE OF PAVEMENT
F.O.C.	FACE OF CURB
FT	FEET OR FOOT
FU	FUSE
GND	GROUND
HID	HIGH INTENSITY DISCHARGE
JB KVA	JUNCTION BOX KILOVOLT-AMPERE
KW	KILOWATTS
LED	LIGHT EMITTING DIODE
LP	LIGHT POLE
М	METER
MA	MAST ARM
MC MM	MULTI-CONDUCTOR MILLIMETER
M.H.	MOUNTING HEIGHT
MW	MESSENGER WIRE
NESC	NATIONAL ELECRIC SAFETY CODE
NO.#	NUMBER
N.T.S. P	NOT TO SCALE PROPOSED
PB	PUSH BUTTON
PNL	PANEL
PVC	POLYVINYL CHLORIDE
PVCC RGC	PVC COATED RIGID GALVANIZED CONDUIT
PT R	POTENTIAL TRANSFORMER EXISTING UNIT TO BE REMOVED
K	(OWNER SALVAGED U.N.O.)
RL	RELOCATED LIGHTING UNIT
RR	EXISTING UNIT TO BE REMOVED AND
	REINSTALLED
RECP RGC	RECEPTACLE RIGID GALVANIZED CONDUIT
SEL SW	SELECTOR SWITCH
SPARE	SPARE
SPACE	SPACE
SS	STAINLESS STEEL
STA	STATION
T/F UD	TOP OF FOUNDATION UNIT DUCT
U.N.O.	UNLESS NOTED OTHERWISE
UGC, GS	UNDERGROUND CONDUCT, GALVANIZED STEEL
VAC	VOLTS, ALTERNATING CURRENT
W	WATTS
WP	WOOD POLE
XFMR HPS	TRANSFORMER HIGH PRESSURE SODIUM
LPS	LOW PRESSURE SODIUM
LTFM	LIQUID TIGHT FLEXIBLE METALLIC

INDEX OF DRAWINGS

DRAWING NO.	TITLE
LT-01	LEGEND, ABBREVIATIONS, GENERAL NOTES, AND INDEX OF DRAWINGS
LT-02	LIGHTING REMOVAL PLAN
LT-03	PROPOSED LIGHTING PLAN
LT-04	PROPOSED NAVIGATION LIGHTING AND REMOVAL PLAN
LT-05	NAVIGATION LIGHTING DETAILS
LT-06	IDOT STANDARD DRAWING

SCALE:

GENERAL NOTES

THE ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE LATEST CODES, STANDARDS AND THE IDOT STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION ADOPTED JANUARY 1, 2022, AND SUPPLEMENTAL SPECIFICATIONS AND RECURRING SPECIAL PROVISIONS.

IDOT-D1 STANDARDS:

STANDARD NO. TITLE

BE-702 MISC. ELECTRICAL DETAILS SHEET A

LIGHTING SCHEDULE OF QUANTITIES

ITEM	UNIT	TOT. QUANTITY
CONDUIT ATTACHED TO STRUCTURE, 1" DIA., PVC COATED GALVANIZED STEEL	FOOT	325
JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 6" X 6" X 4"	EACH	6
ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 1/C NO. 10	FOOT	1,007
LUMINAIRE, LED, UNDERPASS, WALLMOUNT, OUTPUT DESIGNATION D	EACH	2
WATERWAY OBSTRUCTION WARNING LUMINAIRE, LED, 180 DEGREE RED, PARAPET MOUNTED	EACH	4
WATERWAY OBSTRUCTION WARNING LUMINAIRE, LED, 360 DEGREE GREEN	EACH	4
REMOVAL OF LIGHTING UNIT, NO SALVAGE	EACH	2
REMOVAL OF LIGHTING LUMINAIRE, SALVAGE	EACH	4
LIGHTING UNIT COMPLETE, SPECIAL	EACH	4
REMOVAL OF WATERWAY OBSTRUCTION WARNING LUMINAIRE	EACH	8
REMOVAL OF UNDERPASS LIGHTING UNIT, NO SALVAGE	EACH	6
REMOVE EXISTING CONDUIT ATTACHED TO STRUCTURE	FOOT	330
MAINTENANCE OF LIGHTING SYSTEM	CAL MO	12
LUMINAIRE, LED, ROADWAY, OUTPUT DESIGNATION A	EACH	8

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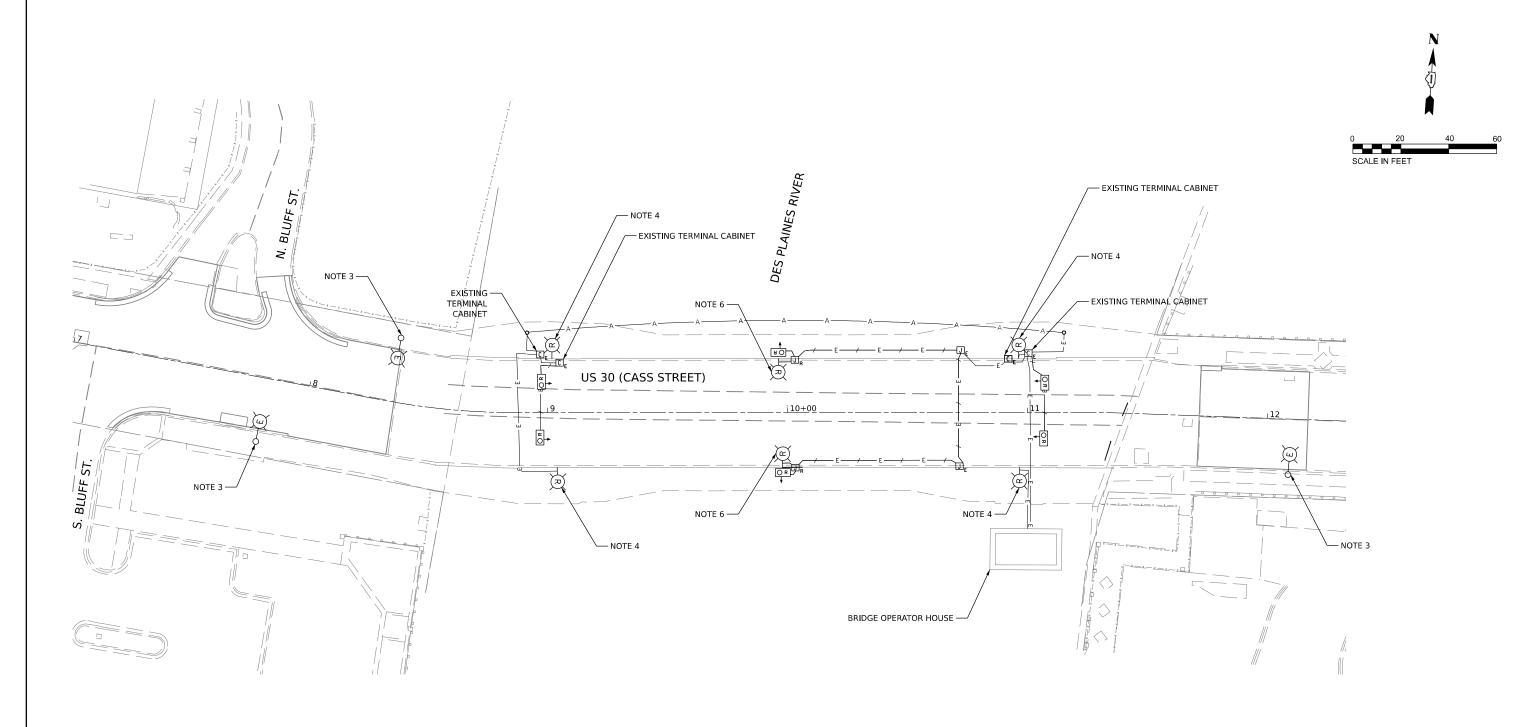
USER NAME = mgarvida	DESIGNED - VN	REVISED -
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PLOT SCALE = 100.0000 / in.	CHECKED - MG/RP	REVISED -
PLOT DATE = 2/10/2024	DATE - 2/8/2024	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

LEGEND, ABBREVIATIONS, GENERAL NOTES, AND INDEX OF DRAWINGS							
	SHEET	1	OF	6	SHEETS	STA.	TO STA.

SECTION		COUNTY	TOTAL SHEETS	SHE
2018-067-BR		WILL	128	29
		CONTRACT	NO. 62	M79
ILLINOIS	FED. A	D PROJECT		

LT-01



NOTES

- 1. REFER TO SHEET LT-01 FOR ELECTRICAL LIGHTING GENERAL NOTES AND LEGEND.
- 2. EXISTING LIGHTING SYSTEM TO REMAIN OPERATIONAL UNTIL PROPOSED LIGHTING SYSTEM IS INSTALLED AND OPERATIONAL.
- 3. EXISTING LIGHTING UNIT TO REMAIN,
- 4. EXISTING MOUNTING BRACKETS, CONDUIT, AND CABLES TO REMAIN IN PLACE. LUMINAIRES TO BE REMOVED AND REPLACED.
- 5. POWER FOR THE IDOT LIGHTING SYSTEM IS SUPPLIED FROM PANEL NLP INSIDE BRIDGE OPERATOR HOUSE.
- 6. EXISTING LUMINAIRE SUPPORT TO BE REMOVED. THIS WORK SHALL BE INCLUDED IN THE PAY ITEM "REMOVAL OF LIGHTING UNIT, NO SALVAGE".

LT-02



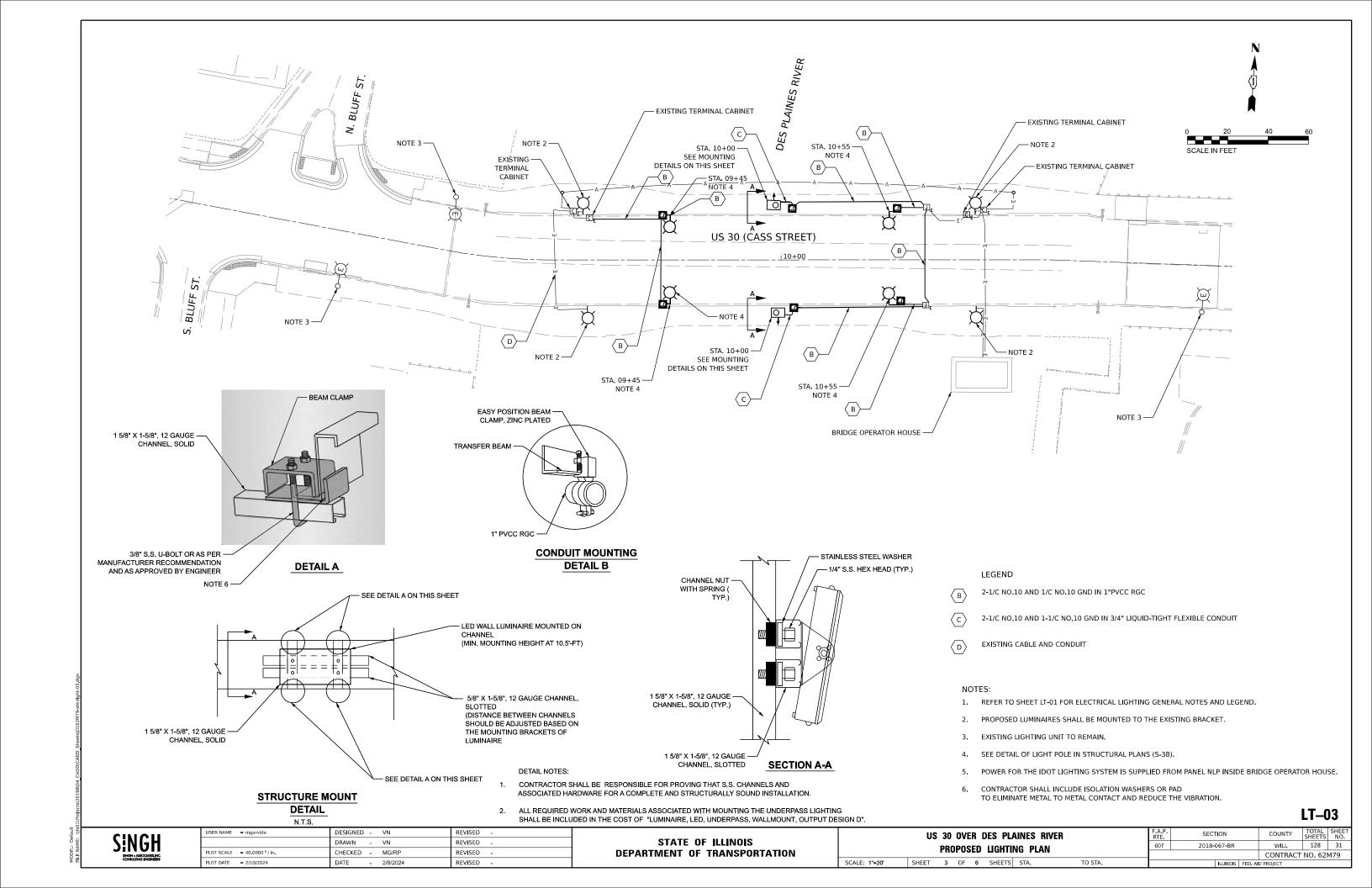
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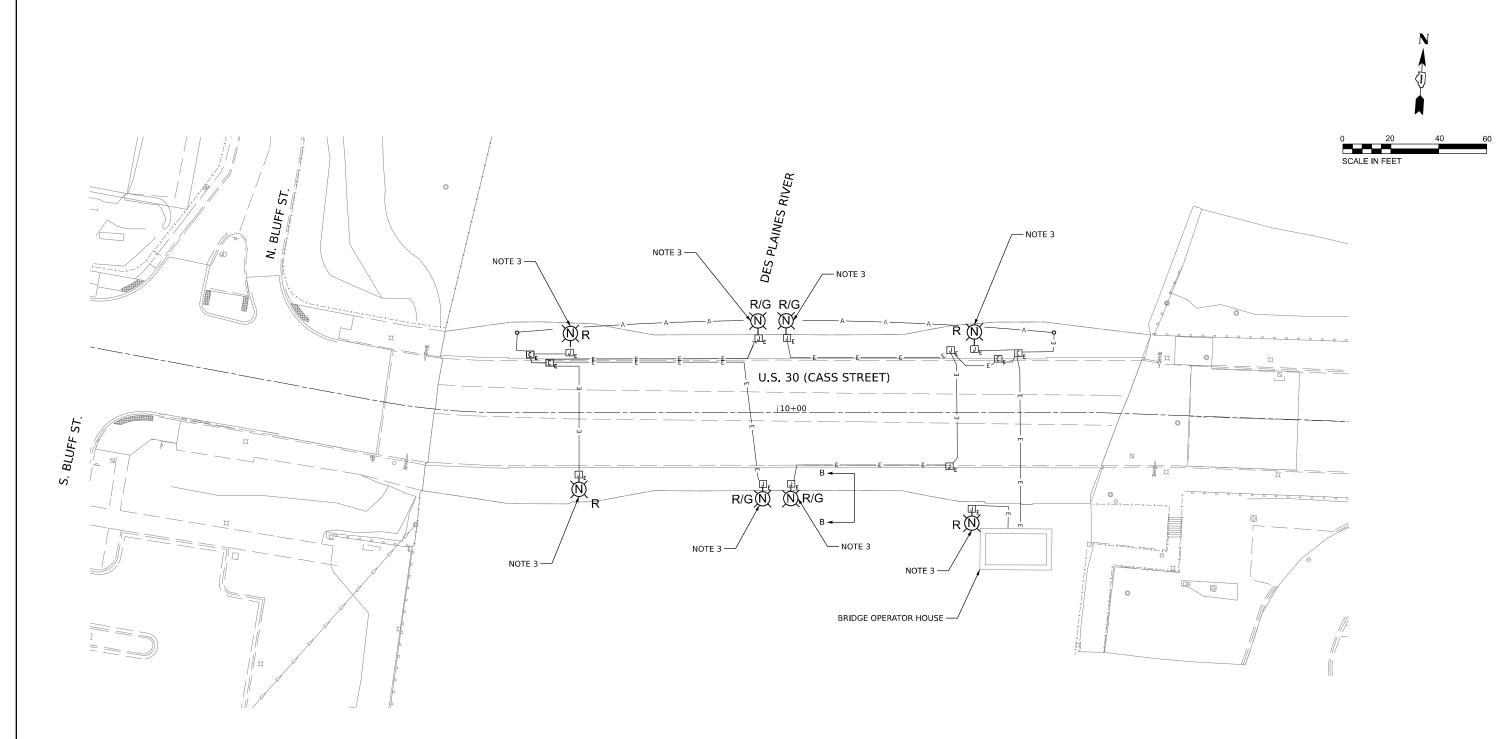
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

	US	30	OVE	RI	DES PL	AINES	RIVER
		LIG	ITH	٧G	REMOV	AL PLA	AN
SCALE: 1"=20'	SHEET	2	OF	6	SHEETS	STA.	TO ST

A.P. SECTION COUNTY TOTAL SHEETS NO.
07 2018-067-BR WILL 128 30

CONTRACT NO. 62M79





NOTES:

- 1. SEE SHEET LT-01 FOR LEGEND, ABBREVIATIONS, AND GENERAL NOTES.
- 2. EXISTING NAVIGATION LIGHTING SHALL REMAIN OPERATIONAL DURING CONSTRUCTION. ALL NAVIGATION LUMINAIRES FOUND TO BE NON-OPERATIONAL SHALL BE REPLACED WITH NEW LUMINAIRES PRIOR TO CONSTRUCTION. LUMINAIRES SHALL BE ACCORDING TO ARTICLE 1067.07 OF THE STANDARD SPECIFICATIONS. THE COST OF THIS WORK SHALL BE INCLUDED IN THE "MAINTENANCE OF LIGHTING SYSTEM" PAY ITEM.
- 3. THE PROPOSED NAVIGATION LIGHTING LUMINAIRES ARE TO BE INSTALLED IN THE SAME LOCATION AS EXISTING NAVIGATION LUMINAIRES.
- 4. POWER FOR THE NAVIGATIONAL LIGHTING SYSTEM IS SUPPLIED FROM PANEL NLP INSIDE BRIDGE OPERATOR HOUSE.
- 5. SEE SECTION B-B ON SHEET LT-05.

LT-04



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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

US 30 OVER DES PLAINES RIVER								
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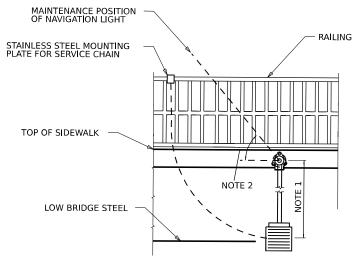
A.P. SECTION COUNTY TOTAL SHEET'S NO.
07 2018-067-BR WILL 128 32

CONTRACT NO. 62M79



RED AND GREEN CHANNEL CENTER NAVIGATION LIGHT

NOT TO SCALE

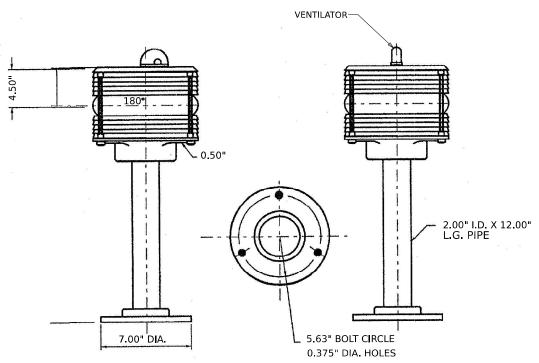


NOTES:

- LENGTH AS NEEDED FOR NAVIGATION LIGHT TO REACH LOW BRIDGE STEEL.
- ANGLE AS NEEDED FOR NAVIGATION LIGHT TO REACH MAINTANANCE POSITION ABOVE THE RAILING.

SECTION B-B - CHANNEL MARKER NAVIGATION LIGHT

NOT TO SCALE



CHANNEL MARGIN NOT TO SCALE

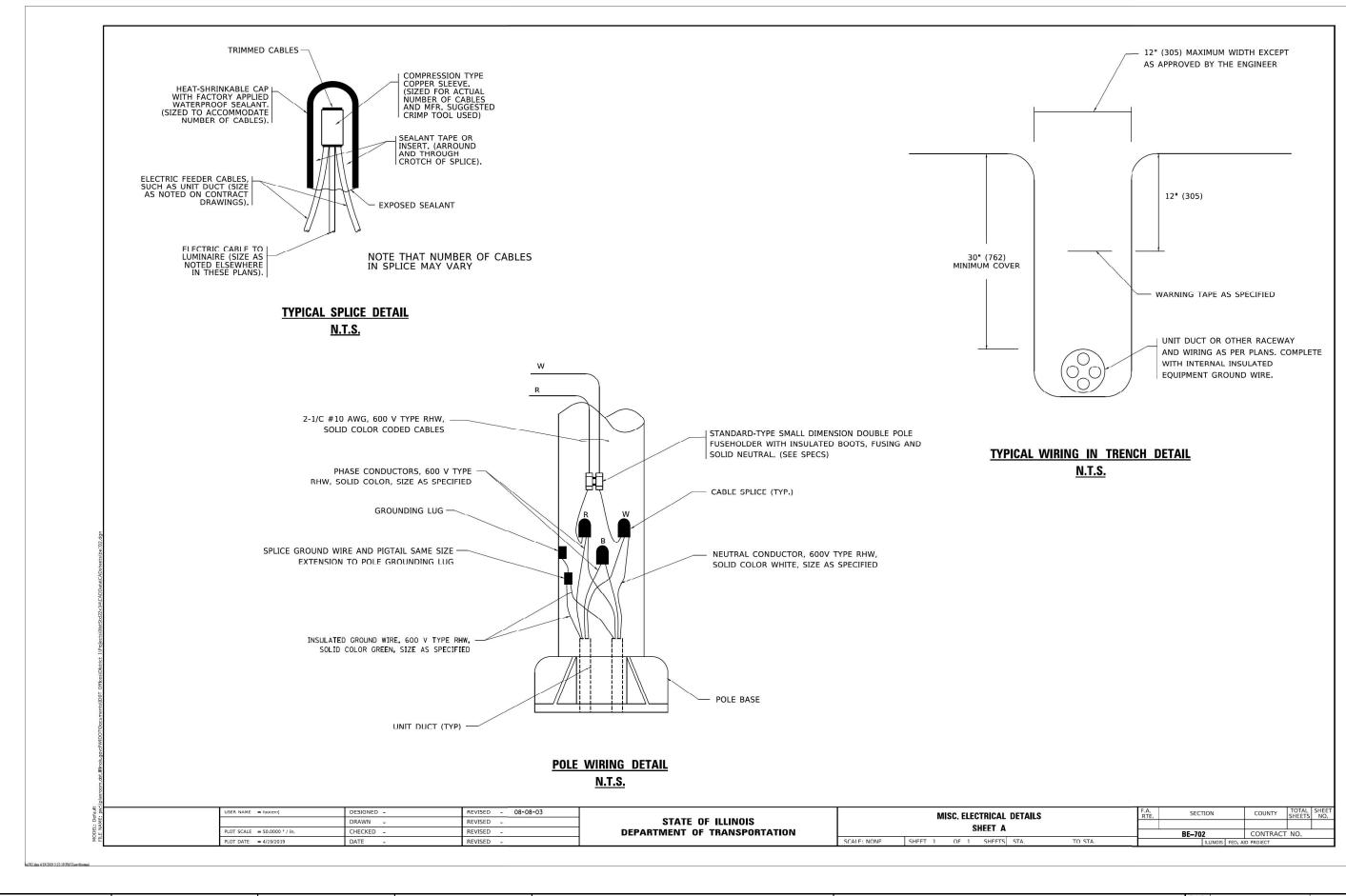
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LT-05



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ΤE	SECT	TION	COUNTY	SHEETS	NC	
307	2018-0	WILL	128	33		
			CONTRACT	NO. 62	M79	
		ILLINOIS FED. AID PROJECT				



LT-06

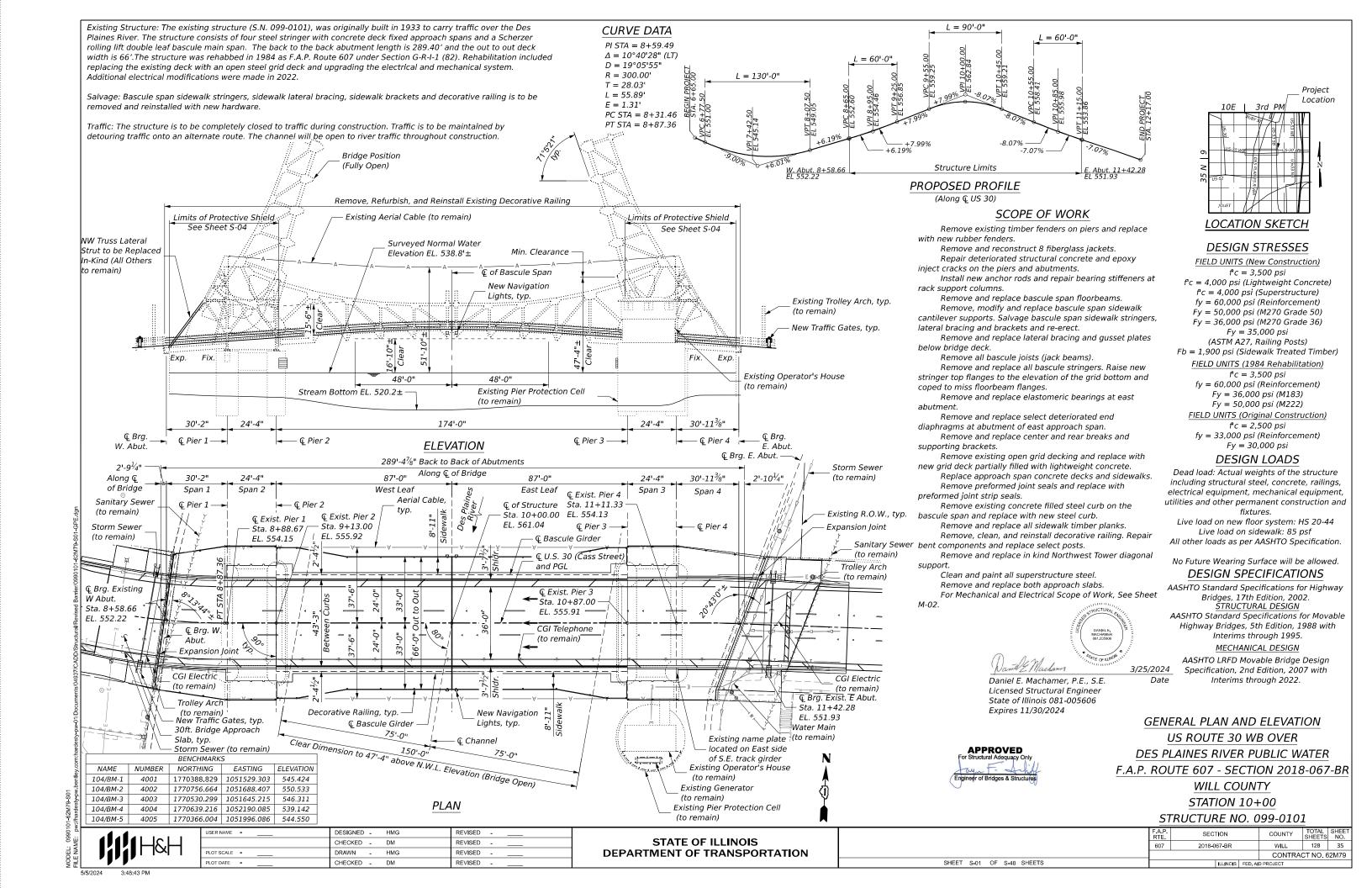
SINGH - ASSOCIATES, INC.

USER NAME = mgarvida	DESIGNED - VN	REVISED -
	DRAWN - VN	REVISED -
PLOT SCALE = 2 / in.	CHECKED - MG/RP	REVISED -
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SCALE:

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IDOT STANDARD DRAWING									
	SHEET	6	OF	6	SHEETS	STA.	TO STA.		



GENERAL NOTES:

- Fasteners shall be ASTM F3125 Grade A325 Type 1, mechanically galvanized bolts in painted areas. Bolts 7/8 in. diameter, holes 15/16 in. diameter, unless otherwise noted.
- Calculated weight of Structural Steel AASHTO M270, Grade 50 = 431,610 lbs. Calculated weight of Structural Steel AASHTO M270, Grade 36 = 1,290 lbs.
- All structural steel shall be AASHTO M270, Grade 50, unless noted otherwise.
- No field welding is permitted except as specified in the contract documents.
- Reinforcement bars designated (E) shall be epoxy coated.
- Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be paid for according to Article 109.04 of the Standard Specifications.
- As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding 1/4 in. deep shall be identified and reported to the Bureau of Bridges & Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard
- Plan dimensions and details relative to existing structure have been taken from existing plans and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering materials. Such variations shall not be cause for additional compensation for a change in the scope of work. However, the Contractor will be paid for the quantity actually furnished at the unit price bid for work.
- Members or pieces not specifically marked for removal or replacement are to remain in place.
- All removal work shall be performed with care so that materials which are to remain in place or to be reused will not be damaged. If the Contractor damages any materials that are to remain in place, the damaged materials shall be repaired or replaced in a manner satisfactory to the Engineer at the expense of the Contractor. Special attention shall be made to protecting new and existing machinery throughout construction.
- 11. Where called for on the plans, existing structural steel, which is to remain in place shall be modified by drilling, sawing or a combination of both. Flame cutting of members, which are to remain in place, will not be allowed.
- 12. Existing rivets to be removed shall have the heads removed by mechanical methods. Flame cutting for the purpose of removing existing rivets will not be
- 13. All new holes shall be drilled, not burned and reamed. Field reaming of bolt holes in plates shall only be allowed with the approval of the Engineer. The cost of field reaming shall be included in the cost of Furnishing and Erecting Structural Steel.
- Where existing structural steel to remain has been cut or new holes have been drilled, the edges shall be dressed to a smooth, uniform surface with no notches or gouges.
- 15. The cost of field welding and field drilling of new or existing structural members, as noted in the plans shall be included in the cost of Furnishing and Erecting Structural Steel or Structural Steel Repair as appropriate.
- 16. It is the Contractor's responsibility to take measurement in the field of the existing structure wherever new steel is to replace or to connect into existing material prior to ordering or fabricating any new steel. The Contractor shall be responsible for the proper fitting and assembly of all parts of this work. The Contractor's shop drawings shall indicate which dimensions were obtained by actual field measurements.

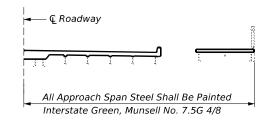
- 17. The main load carrying member components subject to tensile stress shall conform to Charpy-V-Notch Impact Energy Requirement, Zone 2. These components include all tension flanges and webs for floorbeams and stringers, center and rear break plates, and all members denoted "CVN" in the plans.
- 18. The Contractor shall submit a detailed plan of proposed construction procedures and sequences for the proposed work on the bascule span to the Engineer for review and approval prior to commencing this work. Approval shall not relieve the Contractor of any responsibility for the stability of the bridge during the removal and replacement operations. Temporary support structures required to ensure stability of the bridge during construction shall be paid as Temporary Support System and Temporary Shoring and Cribbing.
- 19. Work on the structure shall be done in such a manner that the closure of the bridge to river traffic (periods where the bridge is restricted to the down position) is prohibited. River traffic is to be maintained at all times. Work that requires the bridge to be immobilized shall be done with the bridge restricted to the up position. The Contractor shall obtain coast guard approval for any work that may interfere with navigational operations of the navigable waterway. See Sheets 22 and 23 for navigational clearance requirements. A work plan shall be prepared by the Contractor, reviewed and approved by the Engineer and be submitted by the Engineer to the Coast Guard at the address listed below for approval:

Bridge Administrator US Coast Guard Eighth Coast Guard District 1222 Spruce Street St. Louis, MO 63103-2832

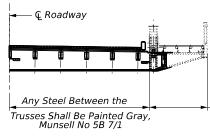
- 20. The Contractor shall take all necessary measures to ensure that no debris falls into the Des Plaines River or endangers or interferes with river traffic beneath the bridge. If any debris falls into the waterway, the Contractor shall remove it from the river to the engineer's satisfaction and at no additional cost. The cost of this work shall be considered included in the contract.
- 21. The Contractor shall obtain a construction permit from the Illinois Department of Natural Resources (IDNR), Office of Water Resources for any temporary construction activity placed in the water except cofferdams. This shall include the placement of material for run-arounds, causeways, etc. Any permit application by the Contractor shall refer to the IDNR 3704 Floodway Construction permit number allowing permanent construction as shown in the contract plans.
- When the leaves of the bascule span are inoperable in fully open position, the leaves must be secured. The Contractor shall submit the method and details, for securing the leaves in fully open position, to the Engineer for approval, the cost of this work shall be considered incidental to the Contract.
- 23. The Contractor shall obtain all necessary permits from the State of Illinois prior to commencing construction. The costs shall be included in the cost of the contract.

GENERAL NOTES FOR PAINT:

- The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except, where otherwise noted. The final finish coat of the bascule span floorbeams shall be Interstate Green, Munsell NO. 7.5G 4/8, from the interior face of the truss to the connection with the sidewalk brackets. The color of the final finish coat for all other bascule span interior steel surfaces shall be Gray, Munsell No 5B 7/1.
- Cleaning and painting of the existing structural steel shall be as specified in the Special Provision for "Cleaning and Painting Existing Steel Structures". All existing structural steel shall be cleaned per Near White Blast Cleaning - SSPC -SP10. The color of the final finish coat for the approach span steel, sidewalk brackets, decorative railing and truss, shall be Interstate Green, Munsell No.
- Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection according to Special Provision "Cleaning and Painting Contact Surface areas of Existing Steel Structures". All contact surfaces on new and existing steel, including connection bolts, nut or washer contact areas are to be free of scale, burrs, dirt, other foreign materials, oil previously applied paint, lacquer or other coatings that would prevent solid seating of connected parts. Cost included with Structural Steel Repair.
- 4. The existing structural steel coating contains lead. The Contractor should take appropriate precautions to deal with the presence of lead on this project.
- The Contractor shall submit calculations and details demonstrating the structural integrity of the bridge is maintained under the additional imposed loads of the containment system. Acceptance by the Engineer shall not relieve the Contractor of their ultimate responsibility for controlling paint debris from escaping the work zone. See Special Provisions.
- A minimum of four air monitors will be required to monitor abrasive blasting operations at this site. See Special Provisions for "Containment and Disposal of Lead Paint Cleaning Residues."
- The containment shall be dropped in the event of sustained winds of 40 MPH or greater and all materials and equipment secured.
- Contractor is to protect all mechanical and electrical assemblies from all cleaning, blasting, painting or other foreign material. Protection procedure and sketches shall be submitted to the Engineer for review and approval. Coordinate all painting work with all related mechanical and electrical work.



PAINT DETAILS (At Approach Spans, Symmetrical about & Roadway)



Any Steel Outside of the Trusses Including the Trusses Shall be Painted Interstate Green, Munsell No. 7.5G 4/8.

PAINT DETAILS

(At Bascule Span, Symmetrical about & Roadway)

REFERENCE DRAWINGS KEY

Contract No Sheet No.	Reference
1345 - XX	Original 1931 Design Plans
630 - XX	Original 1931 Shop Drawings
38306 - XX	1984 Bridge Rehabilitation Plans (Section G-R-I-1)

607



USER NAME =	DESIGNED - CEB	REVISED -
	CHECKED - HMG	REVISED -
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PLOT DATE =	CHECKED - HMG	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

GENERAL NOTES STRUCTURE NO. 099-0101 SHEET S-02 OF S-48 SHEETS

)	SECTION		COUNTY	TOTAL SHEETS	SHE
	2018-067-BR		WILL	128	36
CONTRACT NO. 62M					
	III INOIS EED AID PROJECT				

INDEX O	F SHEETS:		
Structural		Electrical	
S-01	General Plan & Elevation	E-01	Electrical Notes and Scope of Work
S-02	General Notes	E-02	Traffic Gate - Details
S-03	Index of Drawings and Total Bill of Material	E-03	Single Line Diagram I
S-04	Approach Spans - Deck Removal	E-04	Single Line Diagran II
S-05	West Approach Spans - Top of Slab Elevations	E-05	3 Line Schematic Diagram I
S-06	West Approach Spans - Top of Slab Elevations Tables I	E-06	3 Line Schematic Diagram II
S-07	West Approach Spans - Top of Slab Elevations Tables II	E-07	3 Line Schematic Diagram III
S-08	East Approach Spans - Top of Slab Elevations	E-08	Nearside North Oncoming Traffic Gate Schematic Diagram
S-09	East Approach Spans - Top of Slab Elevations Tables I	E-09	Nearside South Offgoing Traffic Gate Schematic Diagram
S-10	East Approach Spans - Top of Slab Elevations Tables II	E-10	Farside North Offgoing Traffic Gate Schematic Diagram
S-11	West Approach Slab - Top of Slab Elevations	E-11	Farside South Oncoming Traffic Gate Schematic Diagram
S-12	East Approach Slab - Top of Slab Elevations	E-12	Nearside Motor Brakes Schematic Diagram
S-13	West Approach Spans - Deck Plan	E-13	Farside Motor Brakes Schematic Diagram
S-14	West Approach Spans - Deck Sections	E-14	Nearside Machinery Brakes Schematic Diagram
S-15	East Approach Spans - Deck Plan	E-15	Farside Machinery Brakes Schematic Diagram
S-16	East Approach Spans - Deck Sections	E-16	Nearside Limit Switch Schematic Diagram
S-17	West Approach Slab - Plan	E-17	Farside Limit Switch Schematic Diagram
S-18	West Approach Slab - Details	E-18	Drive Schematic I
S-19	East Approach Slab - Plan	E-19	Drive Schematic II
S-20	East Approach Slab - Details	E-20	Drive Schematic III
S-21	Approach Spans - Preformed Joint Strip Seal	E-21	Drive Schematic IV
S-22	Approach Spans - Framing Plan		
S-23	Approach Spans - Miscellaneous Steel Details I		
S-24	Approach Spans - Miscellaneous Steel Details II		
S-25	Approach Spans - Miscellaneous Steel Details III		
S-26	Approach Spans - Miscellaneous Steel Details IV		
S-26A	Approach Spans - Miscellaneous Steel Details V		
S-27	Approach Span - Rack Frame Column Repair Details		
S-28	Approach Spans - Bearing Replacement		
S-29	Bascule Span - Existing Plan Removal & Repairs		
S-30	Bascule Span - Framing & Deck Plan		
S-31	Bascule Span - Deck Sections & Details		
S-32	Bascule Span - Sidewalk Details		
S-33	Bascule Span - Stringer Details		
S-34	Bascule Span - Floorbeam Details I		
S-35	Bascule Span - Floorbeam Details II		
S-36	Bascule Span - Lateral Bracing Details		
S-37	Bascule Span - Miscellaneous Details I		
S-38	Bascule Span - Miscellaneous Details II		
S-39	Bascule Span - Miscellaneous Details III		
S-40	Bascule Span - Center Break Details		
S-41	Bascule Span - Rear Break Details		
S-42	Steel Barrier		
S-43	Counterweight Repairs		
$C \Lambda \Lambda$	Wast Abutments Denairs		

TOTAL BILL OF MATERIALS

DESCRIPTION	UNIT	SUPER	SUB	TOTAL
CONCRETE REMOVAL	CU YD	1.2	13.8	15
REMOVAL OF EXISTING CONCRETE DECK	EACH	1		1
PROTECTIVE SHIELD	SQ YD	884		884
STRUCTURE EXCAVATION	CU YD		85.8	85.8
CONCRETE STRUCTURES	CU YD		48.9	48.9
CONCRETE SUPERSTRUCTURE	CU YD	207.7	9.1	216.8
BRIDGE DECK GROOVING	SQ YD	840		840
PROTECTIVE COAT	SQ YD	1,181		1,181
CONCRETE SUPERSTRUCTURE (APPROACH SLAB)	CU YD	143.1		143.1
FURNISHING AND ERECTING STRUCTURAL STEEL	L SUM	1		1
TREATED TIMBER	F.B.M.	9,922		9,922
HARDWARE	POUND	557		557
REINFORCEMENT BARS, EPOXY COATED	POUND	121,410	2,120	123,530
PREFORMED JOINT STRIP SEAL	FOOT	139		139
ELASTOMERIC BEARING ASSEMBLY, TYPE I	EACH	17		17
ANCHOR BOLTS, 1"	EACH		34	34
CONCRETE SEALER	SQ FT		2,998	2,998
EPOXY CRACK INJECTION	FOOT		363	363
REMOVAL OF EXISTING TIMBER MATERIAL	L SUM	0.7	0.3	1
REMOVE AND RE-ERECT EXISTING BRIDGE RAIL	FOOT	575		575
REMOVE AND REPLACE FIBERGLASS JACKET	SQ FT		1,118	1,118
COUNTERWEIGHT REPAIR MATERIALS	L SUM	1		1
FENDER SYSTEM	L SUM	1		1
STEEL GRID DECK	SQ FT	7,233		7,233
BRIDGE BARRIER	FOOT	345		345
REMOVE AND RE-ERECT EXISTING STRUCTURAL STEEL	L SUM	1		1
BRIDGE CLEANING AND PAINTING WARRANTY	L. SUM	1		1
JACK AND REMOVE EXISTING BEARINGS	EACH		17	17
STRUCTURAL STEEL REMOVAL	L SUM	1		1
STRUCTURAL STEEL REPAIR	POUND	7,737		7,737
BRIDGE FLOOR REMOVAL	SQ YD	804		804
CONTAINMENT AND DISPOSAL OF LEAD PAINT CLEANING RESIDUES NO. 1	L SUM	1		1
CLEANING AND PAINTING STEEL BRIDGE NO. 1	L SUM	1		1
STRUCTURAL REPAIR OF CONCRETE (DEPTH EQUAL TO OR LESS THAN 5 INCHES)	SQ FT		878	878
STRUCTURAL REPAIR OF CONCRETE (DEPTH GREATER THAN 5 INCHES)	SQ FT		554	554
PREFORMED JOINT FILLER	FOOT	123		123
TEMPORARY SHORING AND CRIBBING	EACH		2	2
TEMPORARY SUPPORT SYSTEM	L SUM	1		1

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	CHECKED - HMG	REVISED
PLOT SCALE =	DRAWN - CEB	REVISED
PLOT DATE =	CHECKED - HMG	REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

INDEX OF DRAWINGS AND TOTAL BILL OF MATERIAL	F.A.P. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 099-0101		2018-067-BR	WILL	128	37
			CONTRA	CT NO. 6	2M79
SHEET S-03 OF S-48 SHEETS		ILLINOIS FED. AI	D PROJECT		

S-44

S-45 S-46 S-47

5-48

M-01 M-02 M-03

M-04 M-05 M-06

M-07

M-08 M-09

M-10

M-11 M-12 M-13

Mechanical

West Abutments Repairs East Abutments Repairs Pier 1 & Pier 2 Repairs

Pier 3 & Pier 4 Repairs

General Machinery Notes Mech./Elec. General Plan & Elevation

Machinery Room Demolitions Plan Machinery Room Layout Plan Open Gearing Sections

Bearing Assembly Sections

Bearing A Assembly and Details

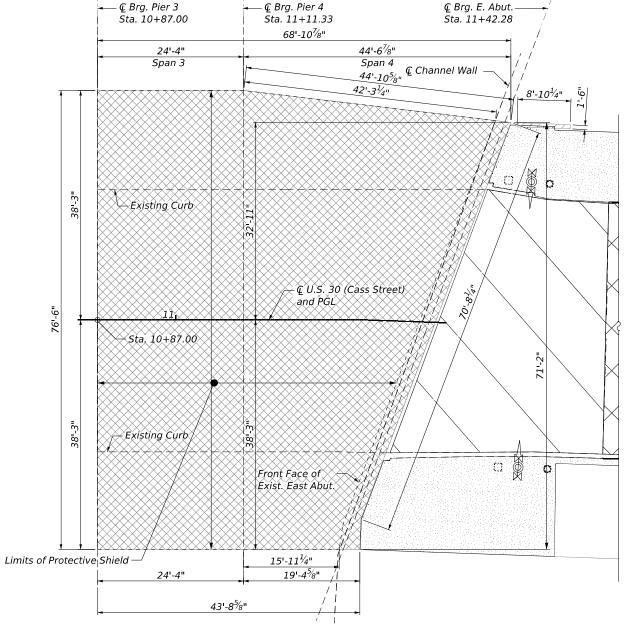
Machinery Support and Rotary Cam Limit Switch Details Machinery Tables, and Center Lock Detail

Counterweight Balance Tables and Balance Block Detail

Open Gearing Details Shafting Details

Bearing Details

Fender Protection



WEST APPROACH SPANS PLAN

The Contractor shall exercise extreme care during removal of abutment backwall and concrete deck to prevent damage to conduits. Any damage to the existing conduits to remain in place shall be repaired at the Contractor's expense.

For existing approach slab, sidewalk, curb and gutter, and pavement removal limits, quantities, and hatch legend, see Roadway Plans.

The removal of existing ornamantal fence attached to the west and east wingwalls/retaining walls is included in "Remove and Re-erect Existing Bridge Rail". Approach slab removal is included in "Approach Slab Removal", see Roadway

The removal of deck joints shall be included in the cost of "Removal of Existing Concrete Deck".

The removal of approach slab sidewalk over retaining wall shall be included in the cost of "Concrete Removal", see plan view and legend.

EAST APPROACH SPANS PLAN

LEGEND

Removal of Existing Concrete Deck

Removal of Existing Sidewalk over Retaining Wall paid as "Concrete Removal".

Removal and Repalcement of Existing Traffic Gate, See Electrical Plans

Existing Trolley Support to Remain

Removal of Existing Traffic Signal Q Pole and Post to be Replaced or Reinstalled, See TS Plans

BILL OF MATERIAL

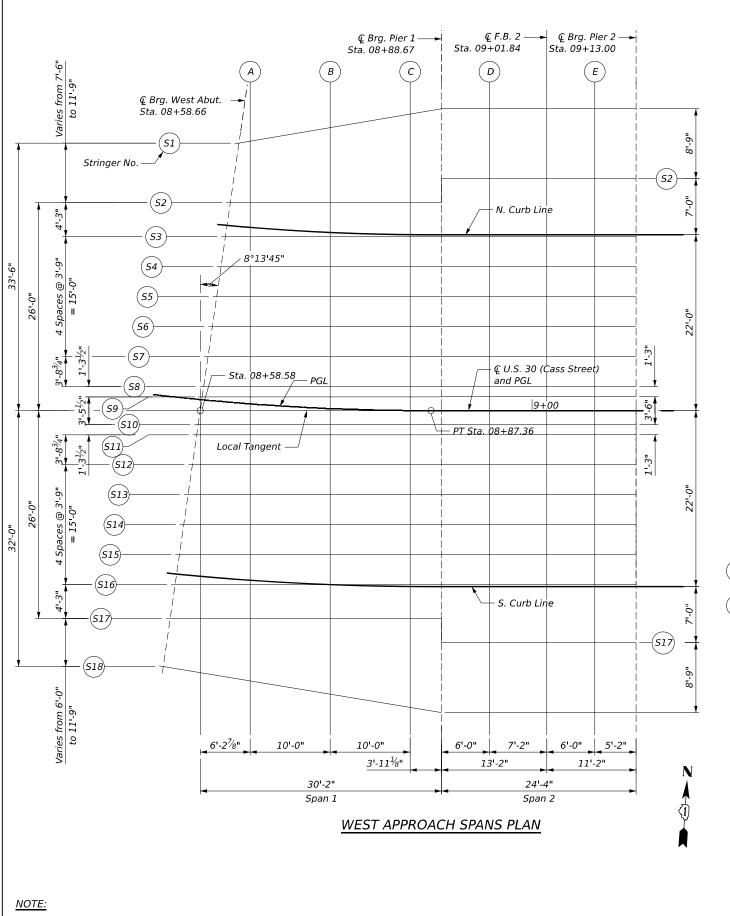
Item	Unit	Quantity
Removal of Existing Concrete Deck	Each	1
Protective Shield	Sq Yd	884
Concrete Removal	Cu Yd	1.2

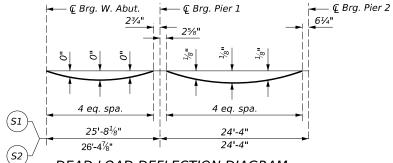
CìorbaGroup 5 W. Higgins Rd, Ste 600, Chicago, IL 60631 P 773.775.4009 | www.ciorba.com

DESIGNED - CP, SIK REVISED -CHECKED - WM REVISED -REVISED CHECKED -REVISED .

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** APPROACH SPANS - DECK REMOVAL **STRUCTURE NO. 099-0101** SHEET S-04 OF S-48 SHEETS

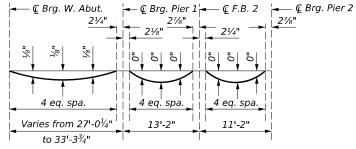
TOTAL SHEET NO. 128 38 SECTION COUNTY 607 2018-067-BR WILL CONTRACT NO. 62M79





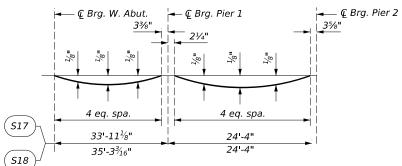
DEAD LOAD DEFLECTION DIAGRAM STRINGERS S1 & S2

(Includes weight of concrete only.)



DEAD LOAD DEFLECTION DIAGRAM STRINGERS S3 THRU S16

(Includes weight of concrete only.)



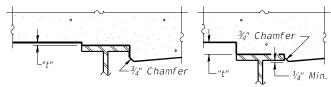
DEAD LOAD DEFLECTION DIAGRAM

STRINGERS S17 & S18

(Includes weight of concrete only.)

Note:

The deflections provided in the Dead Load Deflection Diagrams, shown above, are not to be used in the field if the Engineer is working from the theoretical grade elevations adjusted for dead load deflections as shown on this sheet, Sheet S-06 and Sheet S-07.



At Minimum Fillet

At Maximum Fillet

To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals shown in the tables on this sheet, Sheet S-06 & Sheet S-07. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown in the tables on this sheet, Sheet S-06 & Sheet S-07, minus the deck thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS

STRINGER S1

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+57.29	31.63 Lt.	552.31	552.31
CL Brg. W. Abut	08+60.40	32.38 Lt.	552.51	552.51
A	08+64.74	32.76 Lt.	552.77	552.77
В	08+74.74	35.09 Lt.	553.41	553.41
С	08+84.74	37.07 Lt.	554.07	554.07
CL Brg. Pier 1	08+88.67	37.75 Lt.	554.34	554.34
D	08+94.67	37.75 Lt.	554.76	554.77
CL F.B. 2	09+01.83	37.75 Lt.	555.27	555.28
E	09+07.83	37.75 Lt.	555.71	555.72
CL Brg. Pier 2	09+13.00	37.75 Lt.	556.10	556.10
			1	

STRINGER S2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+56.95	24.58 Lt.	552.29	552.29
CL Brg. W. Abut	08+59.96	24.85 Lt.	552.48	552.48
_				
Α	08+62.65	25.07 Lt.	552.65	552.64
В	08+73.55	25.71 Lt.	553.33	553.33
С	08+84.49	25.99 Lt.	554.05	554.05
CL Brg. Pier 1	08+88.67	26.00 Lt.	554.34	554.34
CL Brg. Pier 1	08+88.67	29.00 Lt.	554.34	554.34
D	08+94.67	29.00 Lt.	554.76	554.77
CL F.B. 2	09+01.83	29.00 Lt.	555.27	555.28
Е	09+07.83	29.00 Lt.	555.71	555.72
CL Brg. Pier 2	09+13.00	29.00 Lt.	556.10	556.10

NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+56.83	22.00 Lt.	551.76	551.76
CL Brg. W. Abut	08+59.79	22.00 Lt.	551.95	551.95
_				
Α	08+62.92	22.00 Lt.	552.14	552.14
В	08+71.74	22.00 Lt.	552.69	552.70
С	08+84.53	22.00 Lt.	553.54	553.54
CL Brg. Pier 1	08+88.67	22.00 Lt.	553.82	553.82
D	08+94.67	22.00 Lt.	554.24	554.24
CL F.B. 2	09+01.83	22.00 Lt.	554.75	554.75
Ε	09+07.83	22.00 Lt.	555.19	555.19
CL Brg. Pier 2	09+13.00	22.00 Lt.	555.58	555.58

Stringer designation number shown on plans varies from reference 38306.

≥		
<u>م</u>		US
ME	<<> Ciorba Group	
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=	P 773,775,4009 www.ciorba.com	ы

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)		CHECKED - WM	REVISED -
1	PLOT SCALE =	DRAWN - SBA	REVISED -
	PLOT DATE =	CHECKED - WM	REVISED -

F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
607	2018-067-BR	WILL	128	39	
			CONTRAC	CT NO. 6	2M79
	ILLINOIS	FED. A	D PROJECT		

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+56.76	20.29 Lt.	551.79	551.79
CL Brg. W. Abut	08+59.71	20.56 Lt.	551.96	551.96
A	08+63.03	20.83 Lt.	552.17	552.17
В	08+73.76	21.47 Lt.	552.83	552.84
С	08+84.54	21.74 Lt.	553.54	553.55
CL Brg. Pier 1	08+88.67	21.75 Lt.	553.82	553.82
D	08+94.67	21.75 Lt.	554.24	554.24
CL F.B. 2	09+01.83	21.75 Lt.	554.75	554.75
E	09+07.83	21.75 Lt.	555.20	555.20
CL Brg. Pier 2	09+13.00	21.75 Lt.	555.59	555.59

STRINGER S6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
Bk. W. Abut	08+56.27	8.94 Lt.	551.93	551.93	
CL Brg. W. Abut	08+59.12	9.21 Lt.	552.10	552.10	
Α	08+63.97	9.61 Lt.	552.39	552.39	
В	08+74.29	10.23 Lt.	553.03	553.05	
С	08+84.64	10.49 Lt.	553.72	553.72	
CL Brg. Pier 1	08+88.67	10.50 Lt.	553.99	553.99	
-					
D	08+94.67	10.50 Lt.	554.41	554.41	
CL F.B. 2	09+01.83	10.50 Lt.	554.92	554.92	
Е	09+07.83	10.50 Lt.	555.37	555.37	
CL Brg. Pier 2	09+13.00	10.50 Lt.	555.75	555.75	
-					

STRINGER S9

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.92	0.10 Lt.	552.04	552.04
CL Brg. W. Abut	08+58.68	0.36 Lt.	552.20	552.20
Α	08+64.65	0.90 Lt.	552.56	552.57
В	08+74.67	1.48 Lt.	553.19	553.20
С	08+84.72	1.74 Lt.	553.85	553.86
CL Brg. Pier 1	08+88.67	1.73 Lt.	554.12	554.12
D	08+94.67	1.73 Lt.	554.54	554.54
CL F.B. 2	09+01.83	1.73 Lt.	555.05	555.05
Е	09+07.83	1.73 Lt.	555.50	555.50
CL Brg. Pier 2	09+13.00	1.73 Lt.	555.89	555.89
			•	

STRINGER S4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+56.15	16.47 Lt.	551.81	551.81
CL Brg. W. Abut	08+59.51	16.78 Lt.	552.01	552.01
A	08+63.35	17.09 Lt.	552.24	552.24
В	08+73.94	17.72 Lt.	552.90	552.91
С	08+84.57	17.99 Lt.	553.60	553.60
CL Brg. Pier 1	08+88.67	18.00 Lt.	553.88	553.88
D	08+94.67	18.00 Lt.	554.30	554.30
CL F.B. 2	09+01.83	18.00 Lt.	554.81	554.81
E	09+07.83	18.00 Lt.	555.25	555.25
CL Brg. Pier 2	09+13.00	18.00 Lt.	555.64	555.64

STRINGER S7

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+56.12	5.16 Lt.	551.97	551.97
CL Brg. W. Abut	08+58.92	5.43 Lt.	552.14	552.14
Α	08+64.26	5.88 Lt.	552.47	552.47
В	08+74.46	6.48 Lt.	553.10	553.11
С	08+84.68	6.74 Lt.	553.77	553.78
CL Brg. Pier 1	08+88.67	6.75 Lt.	554.05	554.05
D	08+94.67	6.75 Lt.	554.47	554.47
CL F.B. 2	09+01.83	6.75 Lt.	554.98	554.98
E	09+07.83	6.75 Lt.	555.42	555.42
CL Brg. Pier 2	09+13.00	6.75 Lt.	555.81	555.81

CL U.S. 30 (CASS AVE)

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.91	0.00 Rt.	552.04	552.04
CL Brg. W. Abut	08+58.66	0.00 Rt.	552.21	552.21
_				
A	08+64.72	0.00 Rt.	552.58	552.59
В	08+74.74	0.00 Rt.	553.22	553.23
С	08+84.74	0.00 Rt.	553.88	553.88
CL Brg. Pier 1	08+88.67	0 Rt.	554.15	554.15
D	08+94.67	0 Rt.	554.57	554.57
CL F.B. 2	09+01.83	0 Rt.	555.08	555.08
E	09+07.83	0 Rt.	555.52	555.52
CL Brg. Pier 2	09+13.00	0 Rt.	555.91	555.91
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STRINGER S5

Location	Station	Offset Theoretical Grade Elevations Adju.		Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+56.43	12.73 Lt.	551.88	551.88
CL Brg. W. Abut	08+59.31	13.00 Lt.	552.05	552.05
A	08+63.66	13.35 Lt.	552.32	552.32
В	08+74.12	13.97 Lt.	552.97	552.98
С	08+84.61	14.24 Lt.	553.66	553.66
CL Brg. Pier 1	08+88.67	14.25 Lt.	553.93	553.93
D	08+94.67	14.25 Lt.	554.35	554.35
CL F.B. 2	09+01.83	14.25 Lt.	554.87	554.87
E	09+07.83	14.25 Lt.	555.31	555.31
CL Brg. Pier 2	09+13.00	14.25 Lt.	555.70	555.70

STRINGER S8

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection			
Bk. W. Abut	08+55.97	1.36 Lt.	552.02	552.02			
CL Brg. W. Abut	08+58.74	1.65 Lt.	552.19	552.19			
Α	08+64.55	2.14 Lt.	552.54	552.54			
В	08+74.62	2.75 Lt.	553.17	553.18			
С	08+84.71	2.99 Lt.	553.83	553.84			
CL Brg. Pier 1	08+88.67	3.02 Lt.	554.10	554.10			
D	08+94.67	3.02 Lt.	554.52	554.52			
CL F.B. 2	09+01.83	3.02 Lt.	555.04	555.04			
E	09+07.83	3.02 Lt.	555.48	555.48			
CL Brg. Pier 2	09+13.00	3.02 Lt.	555.87	555.87			

STRINGER S10

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.78	3.43 Rt.	551.98	551.98
CL Brg. W. Abut	08+58.51	3.13 Rt.	552.15	552.15
Α	08+64.91	2.59 Rt.	552.56	552.56
В	08+74.82	2.01 Rt.	553.19	553.20
С	08+84.75	1.76 Rt.	553.85	553.86
CL Brg. Pier 1	08+88.67	1.73 Rt.	554.12	554.12
D	08+94.67	1.73 Rt.	554.54	554.54
CL F.B. 2	09+01.83	1.73 Rt.	555.05	555.05
E	09+07.83	1.73 Rt.	555.50	555.50
CL Brg. Pier 2	09+13.00	1.73 Rt.	555.89	555.89

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US	SER NAME =	DESIGNED -	RA	REVISED	-
		CHECKED -	WM	REVISED	-
PL	OT SCALE =	DRAWN -	SBA	REVISED	-
PL	OT DATE =	CHECKED -	WM	REVISED	-

WEST APPROACH SPANS - TOP OF SLAB ELEVATIONS TABLES I	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 099-0101		2018-067-BR	WILL	128	40
31K0010KE NO: 033-0101			CONTRA	CT NO. 6	2M79
SHEET S-06 OF S-48 SHEETS		ILLINOIS FED AL	D PROJECT		-

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.73	4.69 Rt.	551.96	551.96
CL Brg. W. Abut	08+58.45	4.42 Rt.	552.13	552.13
A	08+65.00	3.86 Rt.	552.54	552.55
В	08+74.87	3.28 Rt.	553.18	553.19
С	08+84.77	3.01 Rt.	553.84	553.84
CL Brg. Pier 1	08+88.67	3.02 Rt.	554.10	554.10
D	08+94.67	3.02 Rt.	554.52	554.52
CL F.B. 2	09+01.83	3.02 Rt.	555.04	555.04
E	09+07.83	3.02 Rt.	555.48	555.48
CL Brg. Pier 2	09+13.00	3.02 Rt.	555.87	555.87

STRINGER S14

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.32	16.05 Rt.	551.76	551.76
CL Brg. W. Abut	08+57.94	15.77 Rt.	551.93	551.93
Α	08+65.80	15.06 Rt.	552.42	552.43
В	08+75.32	14.50 Rt.	553.04	553.05
С	08+84.86	14.26 Rt.	553.67	553.68
CL Brg. Pier 1	08+88.67	14.25 Rt.	553.93	553.93
D	08+94.67	14.25 Rt.	554.35	554.35
CL F.B. 2	09+01.83	14.25 Rt.	554.87	554.87
Е	09+07.83	14.25 Rt.	555.31	555.31
CL Brg. Pier 2	09+13.00	14.25 Rt.	555.70	555.70

STRINGER S16

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.06	23.63 Rt.	552.18	552.18
CL Brg. W. Abut	08+57.62	23.33 Rt.	552.33	552.33
Α	08+66.30	22.54 Rt.	552.35	552.36
В	08+75.60	22.00 Rt.	552.94	552.95
С	08+84.92	21.76 Rt.	553.57	553.57
CL Brg. Pier 1	08+88.67	21.75 Rt.	553.82	553.82
D	08+94.67	21.75 Rt.	554.24	554.24
CL F.B. 2	09+01.83	21.75 Rt.	554.75	554.75
E	09+07.83	21.75 Rt.	555.20	555.20
CL Brg. Pier 2	09+13.00	21.75 Rt.	555.59	555.59
	Bk. W. Abut CL Brg. W. Abut A B C C CL Brg. Pier 1 D CL F.B. 2 E	Bk. W. Abut 08+55.06 CL Brg. W. Abut 08+57.62 A 08+66.30 B 08+75.60 C 08+84.92 CL Brg. Pier 1 08+88.67 D 08+94.67 CL F.B. 2 09+01.83 E 09+07.83	Bk. W. Abut CL Brg. W. Abut 08+55.06 23.63 Rt. 23.33 Rt. A 08+66.30 22.54 Rt. 8 08+75.60 22.00 Rt. C 08+84.92 21.76 Rt. CL Brg. Pier 1 08+88.67 21.75 Rt. D 08+94.67 21.75 Rt. CL F.B. 2 09+01.83 21.75 Rt. E 09+07.83 21.75 Rt.	Bk. W. Abut CL Brg. W. Abut 08+55.06 08+57.62 23.63 Rt. 23.33 Rt. 552.18 552.33 A B 08+66.30 08+75.60 C 08+84.92 22.54 Rt. 22.00 Rt. 552.94 21.76 Rt. 552.94 553.57 CL Brg. Pier 1 08+88.67 21.75 Rt. 21.75 Rt. 553.82 554.24 554.75 554.75 609+07.83 CL F.B. 2 09+07.83 09+07.83 21.75 Rt. 555.20 555.20

STRINGER S12

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.59	8.48 Rt.	551.89	551.89
CL Brg. W. Abut	08+58.27	8.20 Rt.	552.06	552.06
Α	08+65.28	7.58 Rt.	552.50	552.51
В	08+75.02	7.01 Rt.	553.13	553.14
С	08+84.80	6.76 Rt.	553.78	553.79
CL Brg. Pier 1	08+88.67	6.75 Rt.	554.05	554.05
D	08+94.67	6.75 Rt.	554.47	554.47
CL F.B. 2	09+01.83	6.75 Rt.	554.98	554.98
E	09+07.83	6.75 Rt.	555.42	555.42
CL Brg. Pier 2	09+13.00	6.75 Rt.	555.81	555.81

STRINGER S15

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.19	19.83 Rt.	551.70	551.70
CL Brg. W. Abut	08+57.77	19.55 Rt.	551.86	551.86
A	08+66.06	18.8 Rt.	552.38	552.39
В	08+75.46	18.25 Rt.	552.99	553.00
C	08+84.89	18.01 Rt.	553.62	553.62
CL Brg. Pier 1	08+88.67	18.00 Rt.	553.88	553.88
D	08+94.67	18.00 Rt.	554.30	554.30
CL F.B. 2	09+01.83	18.00 Rt.	554.81	554.81
E	09+07.83	18.00 Rt.	555.25	555.25
CL Brg. Pier 2	09+13.00	18.00 Rt.	555.64	555.64

STRINGER S17

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+54.92	27.92 Rt.	552.17	552.17
CL Brg. W. Abut	08+57.44	27.63 Rt.	552.32	552.32
Α	08+66.58	26.53 Rt.	552.89	552.89
В	08+75.75	26.24 Rt.	553.47	553.48
С	08+84.95	26.01 Rt.	554.08	554.09
CL Brg. Pier 1	08+88.67	26.00 Rt.	554.34	554.34
CL Brg. Pier 1	08+88.67	29.00 Rt.	554.34	554.34
_				
D	08+94.67	29.00 Rt.	554.76	554.77
CL F.B. 2	09+01.83	29.00 Rt.	555.27	555.28
Е	09+07.83	29.00 Rt.	555.71	555.72
CL Brg. Pier 2	09+13.00	29.00 Rt.	556.10	556.10
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STRINGER S13

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection		
Bk. W. Abut	08+55.45	12.26 Rt.	551.82	551.82		
CL Brg. W. Abut	08+58.10	11.98 Rt.	551.99	551.99		
A	08+65.54	11.32 Rt.	552.46	552.47		
В	08+75.17	10.76 Rt.	553.08	553.09		
С	08+84.83	10.51 Rt.	553.73	<i>553.73</i>		
CL Brg. Pier 1	08+88.67	10.50 Rt.	553.99	553.99		
D	08+94.67	10.50 Rt.	554.41	554.41		
CL F.B. 2	09+01.83	10.50 Rt.	554.92	554.92		
E	09+07.83	10.50 Rt.	555.37	555.36		
CL Brg. Pier 2	09+13.00	10.50 Rt.	555.75	<i>555.75</i>		

SOUTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+55.12	22.00 Rt.	551.66	551.66
CL Brg. W. Abut	08+57.67	22.00 Rt.	551.82	551.82
A	08+66.27	22.00 Rt.	552.35	552.35
В	08+75.60	22.00 Rt.	552.94	552.95
С	08+84.92	22.00 Rt.	553.56	<i>553.57</i>
CL Brg. Pier 1	08+88.67	22.00 Rt.	553.82	553.82
D	08+94.67	22.00 Rt.	554.24	554.24
CL F.B. 2	09+01.83	22.00 Rt.	554.75	554.75
E	09+07.83	22.00 Rt.	555.19	555.19
CL Brg. Pier 2	09+13.00	22.00 Rt.	555.58	555.58

STRINGER S18

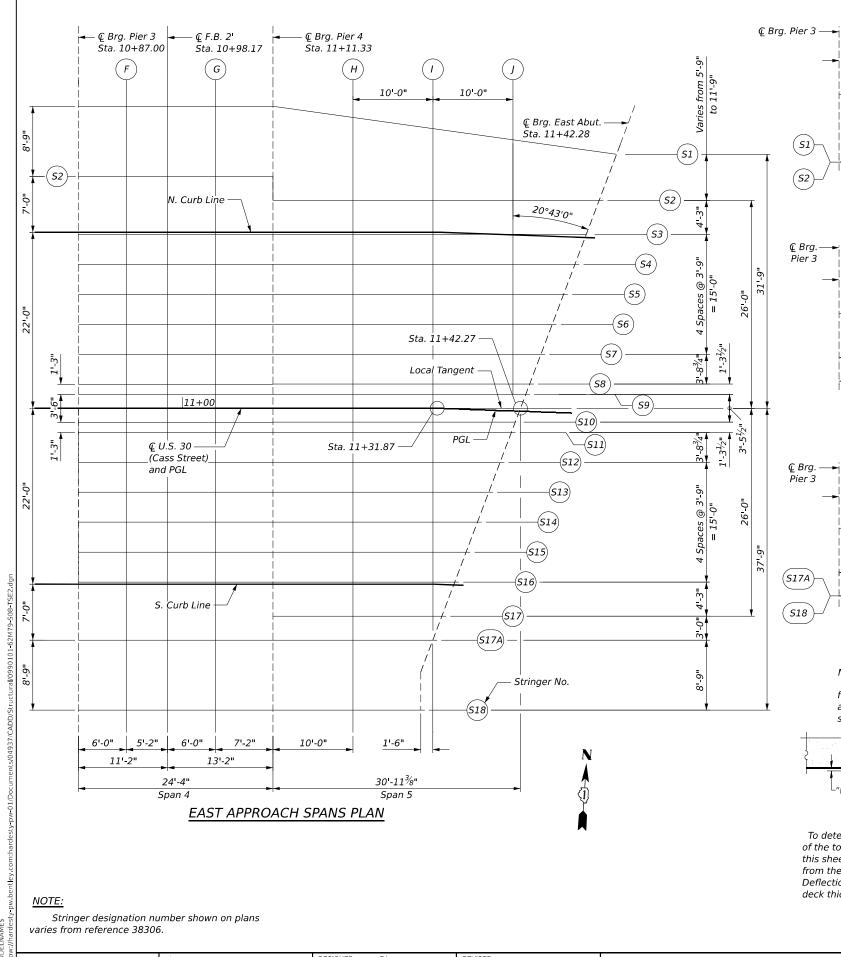
Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
Bk. W. Abut	08+54.74	33.52 Rt.	552.16	552.16
CL Brg. W. Abut	08+57.20	33.69 Rt.	552.31	552.31
A	08+67.06	34.56 Rt.	552.92	552.92
В	08+76.08	35.69 Rt.	553.49	553.50
C	08+85.03	37.11 Rt.	554.09	554.09
CL Brg. Pier 1	08+88.67	37.75 Rt.	554.34	554.34
D	08+94.67	37.75 Rt.	554.76	554.77
CL F.B. 2	09+01.83	37.75 Rt.	555.27	555.28
E	09+07.83	37.75 Rt.	555.71	555.72
CL Brg. Pier 2	09+13.00	37.75 Rt.	556.10	556.10
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US	SER NAME =	DESIGNED -	RA	REVISED	-
		CHECKED -	WM	REVISED	-
PL	OT SCALE =	DRAWN -	SBA	REVISED	-
PL	OT DATE =	CHECKED -	WM	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

WEST APPROACH SPANS - TOP OF SLAB ELEVATIONS TABLES II	F.A.P. RTE	SECTION
STRUCTURE NO. 099-0101	607	2018-067-BR
31100101L 110: 033-0101		
CHEET CAR OF CHEETO		



Location Station Offset Theoretical Grade Elevations Adjusted For Dead Load Deflection CL Brg. Pier 3 10+87.00 37.75 Lt. 556.10 556.10 F CL F.B. 2' 10+98.17 37.75 Lt. 555.27 37.75 Lt. 555.27 555.28 G 11+04.17 37.75 Lt. 554.84 554.84 554.84 554.84 CL Brg. Pier 4 11+11.33 37.75 Lt. 552.90 552.91 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 552.27 552.28 CL Brg. E Abut Bk. E Abut 11+53.14 Bk. E Abut 11+55.86 32.25 Lt. 551.16 551.16 32.25 Lt. 551.16 551.16 551.16					
F 10+93.00 37.75 Lt. 555.66 555.67 CL F.B. 2' 10+98.17 37.75 Lt. 555.27 555.28 11+04.17 37.75 Lt. 554.84 554.84 CL Brg. Pier 4 11+11.33 37.75 Lt. 554.32 554.32 H 11+21.33 36.35 Lt. 553.60 553.61 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	Location	Station	Offset		Grade Elevations Adjusted For Dead Load
F 10+93.00 37.75 Lt. 555.66 555.67 CL F.B. 2' 10+98.17 37.75 Lt. 555.27 555.28 11+04.17 37.75 Lt. 554.84 554.84 CL Brg. Pier 4 11+11.33 37.75 Lt. 554.32 554.32 H 11+21.33 36.35 Lt. 553.60 553.61 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	CL Bra. Pier 3	10+87.00	37.75 Lt.	556.10	556.10
CL F.B. 2' 10+98.17 37.75 Lt. 555.27 555.28 G 11+04.17 37.75 Lt. 554.84 554.84 CL Brg. Pier 4 11+11.33 37.75 Lt. 554.32 554.32 H 11+21.33 36.35 Lt. 553.60 553.61 I 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35					
G 11+04.17 37.75 Lt. 554.84 554.84 CL Brg. Pier 4 11+11.33 37.75 Lt. 554.32 554.32 H 11+21.33 36.35 Lt. 553.60 553.61 I 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	F	10+93.00	37.75 Lt.	555.66	555.67
G 11+04.17 37.75 Lt. 554.84 554.84 CL Brg. Pier 4 11+11.33 37.75 Lt. 554.32 554.32 H 11+21.33 36.35 Lt. 553.60 553.61 I 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	CLEB 2'	10+98 17	37 75 l t	555 27	555 28
CL Brg. Pier 4 11+11.33 37.75 Lt. 554.32 554.32 H 11+21.33 36.35 Lt. 553.60 553.61 I 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35					
H 11+21.33 36.35 Lt. 553.60 553.61 I 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	G	11+04.17	37./5 Lt.	554.84	554.84
I 11+31.33 34.96 Lt. 552.90 552.91 J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	CL Brg. Pier 4	11+11.33	37.75 Lt.	554.32	554.32
J 11+40.13 33.88 Lt. 552.27 552.28 CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	Н	11+21.33	36.35 Lt.	553.60	553.61
CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	1	11+31.33	34.96 Lt.	552.90	552.91
CL Brg. E Abut 11+53.14 32.53 Lt. 551.35 551.35	1	11+40.13	33.88.1 <i>t</i>	552 27	552 28
	,	11170.13	33.00 Et.	332.27	332.20
Bk. E Abut 11+55.86 32.25 Lt. 551.16 551.16	CL Brg. E Abut	11+53.14	32.53 Lt.	551.35	551.35
	Bk. E Abut	11+55.86	32.25 Lt.	551.16	551.16

STRINGER S2

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	29.00 Lt.	556.10	556.10
F	10+93.00	29.00 Lt.	555.66	555.67
CL F.B. 2'	10+98.17	29.00 Lt.	555.27	555.28
G	11+04.17	29.00 Lt.	554.84	554.84
CL Brg. Pier 4	11+11.33	29.00 Lt.	554.32	554.32
CL Brg. Pier 4	11+11.33	26.00 Lt.	554.32	554.32
Н	11+21.33	26.00 Lt.	553.60	553.61
1	11+31.33	26.00 Lt.	552.90	552.91
J	11+40.40	26.32 Lt.	552.26	552.27
CL Brg. E Abut	11+51.17	26.71 Lt.	551.49	551.49
Bk. E Abut	11+54.02	26.81 Lt.	551.29	551.29

STRINGERS S17/S17A & S18 (Includes weight of concrete only.)

DEAD LOAD DEFLECTION DIAGRAM

⊈ Brg. Pier 4 —►

DEAD LOAD DEFLECTION DIAGRAM

STRINGERS S1 & S2

(Includes weight of concrete only.)

4 eq. spa.

⊈ Brg. Pier 4 —►

4 eq. spa.

DEAD LOAD DEFLECTION DIAGRAM

STRINGERS S3 THRU S16

(Includes weight of concrete only.)

4 eq. spa.

24'-4"

4 eq. spa.

11'-2"

← Brg. E. Abut.

→

4 eq. spa.

43'-41/2"

40'-93/8"

€ Brg. E. Abut. -

Varies from 22'-8¹¹/₁₆"

to 39'-2¹/₁₆"

© Brg. E. Abut. —►

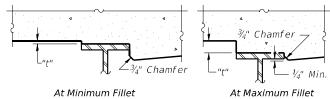
4 eq. spa.

21'-13/8" 18'-5¾" (517/

S17A

(518)

Note: The above deflections are not to be used in the field if the Engineer is working from the grade elevations adjusted for dead load deflections as shown on this sheet, Sheets S-09 and S-10.



At Minimum Fillet

To determine "t": After all structural steel has been erected, elevations of the top flanges of the beams shall be taken at intervals as shown on this sheet, Sheets S-09 and Sheets S-10. These elevations subtracted from the "Theoretical Grade Elevations Adjusted for Dead Load Deflection" shown on this sheet, Sheet S-09 and Sheet S-10, minus the deck thickness, equals the fillet heights "t" above top flange of beams.

FILLET HEIGHTS

NORTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	22.00 Lt.	555.58	555.58
F	10+93.00	22.00 Lt.	555.14	555.14
CL F.B. 2'	10+98.17	22.00 Lt.	554.75	<i>554.75</i>
G	11+04.17	22.00 Lt.	554.32	554.31
CL Brg. Pier 4	11+11.33	22.00 Lt.	553.80	553.80
Н	11+21.33	22.00 Lt.	553.08	553.09
1	11+31.33	22.00 Lt.	552.38	552.39
l j	11+40.55	22.00 Lt.	551.72	551.73
CL Brg. E Abut	11+49.58	22.00 Lt.	551.09	551.09
Bk. E Abut	11+52.40	22.00 Lt.	550.89	550.89
	•	•	•	

		USER NAME =
ME	<<> Liorba Group	
Ν	8725 W. Higgins Rd, Ste 600, Chicago, IL 60631	PLOT SCALE =
FIE	P 773.775.4009 www.ciorba.com	PLOT DATE =

	USER NAME =	DESIGNED - RA	REVISED -
D		CHECKED - WM	REVISED -
631	PLOT SCALE =	DRAWN - SBA	REVISED -
	PLOT DATE =	CHECKED - WM	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** EAST APPROACH SPANS - TOP OF SLAB ELEVATIONS **STRUCTURE NO. 099-0101** SHEET S-08 OF S-48 SHEETS

SECTION SHEETS NO. 607 2018-067-BR WILL CONTRACT NO. 62M79

3/29/2024 11:41:03 AM

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	21.75 Lt.	555.59	555.59
F	10+93.00	21.75 Lt.	555.14	555.14
CL F.B. 2	10+98.17	21.75 Lt.	554.76	554.76
G	11+04.17	21.75 Lt.	554.32	554.32
CL Brg. Pier 4	11+11.33	21.75 Lt.	553.80	553.80
Н	11+21.33	21.75 Lt.	553.09	553.09
1	11+31.33	21.75 Lt.	552.38	552.39
J	11+40.55	22.07 Lt.	551.72	551.73
CL Brg. E Abut	11+49.72	22.41 Lt.	551.08	551.08
Bk. E Abut	11+52.57	22.50 Lt.	550.87	550.87

STRINGER S6

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	10.50 Lt.	555.76	555.76
-				
F	10+93.00	10.50 Lt.	555.31	555.31
CL F.B. 2	10+98.17	10.50 Lt.	554.93	554.93
G	11+04.17	10.50 Lt.	554.49	554.49
CL Brg. Pier 4	11+11.33	10.50 Lt.	553.97	553.97
Н	11+21.33	10.50 Lt.	553.25	553.26
1	11+31.33	10.50 Lt.	552.55	552.56
1	11+40.95	10.83 Lt.	551.86	551.87
Í				
CL Brg. E Abut	11+45.87	11.01 Lt.	551.51	551.51
Bk. E Abut	11+48.72	11.11 Lt.	551.31	551.31

STRINGER S9

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	1.73 Lt.	555.89	555.89
F	10+93.00	1.73 Lt.	555.44	555.44
CL F.B. 2	10+98.17	1.73 Lt.	555.06	555.06
G	11+04.17	1.73 Lt.	554.62	554.62
CL Brg. Pier 4	11+11.33	1.73 Lt.	554.10	554.10
н	11+21.33	1.73 Lt.	553.39	553.39
1	11+31.33	1.73 Lt.	552.68	552.69
]	11+41.26	2.07 Lt.	551.97	551.98
CL Brg. E Abut	11+42.87	2.12 Lt.	551.86	551.86
Bk. E Abut	11+45.72	2.23 Lt.	551.65	551.65

STRINGER S4

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	18.00 Lt.	555.64	555.64
F	10+93.00	18.00 Lt.	555.20	555.20
CL F.B. 2	10+98.17	18.00 Lt.	554.81	554.81
G	11+04.17	18.00 Lt.	554.38	554.37
CL Brg. Pier 4	11+11.33	18.00 Lt.	553.86	553.86
Н	11+21.33	18.00 Lt.	553.14	553.15
1	11+31.33	18.00 Lt.	552.44	552.45
J	11+40.68	18.32 Lt.	551.77	551.78
CL Brg. E Abut	11+48.43	18.60 Lt.	551.22	551.22
Bk. E Abut	11+51.28	18.71 Lt.	551.01	551.01

STRINGER S7

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	6.75 Lt.	555.81	555.81
F	10+93.00	6.75 Lt.	555.36	555.36
CL F.B. 2	10+98.17	6.75 Lt.	554.98	554.98
G	11+04.17	6.75 Lt.	554.54	554.54
CL Brg. Pier 4	11+11.33	6.75 Lt.	554.03	554.03
Н	11+21.33	6.75 Lt.	553.31	553.32
/	11+31.33	6.75 Lt.	552.60	552.61
J	11+41.09	7.08 Lt.	551.91	551.91
CL Brg. E Abut	11+44.58	7.21 Lt.	551.66	551.66
Bk. E Abut	11+47.43	7.31 Lt.	551.46	551.46

CL U.S. 30 (CASS AVE)

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	0.00	555.91	555.91
_				
F	10+93.00	0.00	555.47	<i>555.47</i>
CL F.B. 2	10+98.17	0.00	555.08	555.08
G	11+04.17	0.00	554.65	554.64
CL Brg. Pier 4	11+11.33	0.00	554.13	554.13
Н	11+21.33	0.00	553.41	553.42
1	11+31.33	0.00	552.71	552.72
CL Brg. E Abut	11+42.15	0.00	551.94	551.94
Bk. E Abut	11+44.96	0.00	551.74	551.74

STRINGER S5

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	14.25 Lt.	555.70	555.70
F	10+93.00	14.25 Lt.	555.25	555.25
CL F.B. 2	10+98.17	14.25 Lt.	554.87	554.87
G	11+04.17	14.25 Lt.	554.43	554.43
CL Brg. Pier 4	11+11.33	14.25 Lt.	553.92	553.92
Н	11+21.33	14.25 Lt.	553.20	553.21
1	11+31.33	14.25 Lt.	552.49	552.50
J	11+40.82	14.58 Lt.	551.82	551.82
CL Brg. E Abut	11+47.15	14.80 Lt.	551.36	551.36
Bk. E Abut	11+50.00	14.91 Lt.	551.16	551.16

STRINGER S8

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	3.02 Lt.	555.87	555.87
F	10+93.00	3.02 Lt.	555.42	555.42
CL F.B. 2	10+98.17	3.02 Lt.	555.04	555.04
G	11+04.17	3.02 Lt.	554.60	554.60
CL Brg. Pier 4	11+11.33	3.02 Lt.	554.09	554.09
Н	11+21.33	3.02 Lt.	553.37	553.37
/	11+31.33	3.02 Lt.	552.66	552.67
J	11+41.22	3.36 Lt.	551.96	551.96
CL Brg. E Abut	11+43.31	3.43 Lt.	551.81	551.81
Bk. E Abut	11+46.16	3.53 Lt.	551.60	551.60

STRINGER S10

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted Fo Dead Load Deflection
CL Brg. Pier 3	10+87.00	1.73 Rt.	555.89	555.89
F	10+93.00	1.73 Rt.	555.44	555.44
CL F.B. 2	10+98.17	1.73 Rt.	555.06	555.06
G	11+04.17	1.73 Rt.	554.62	554.62
CL Brg. Pier 4	11+11.33	1.73 Rt.	554.10	554.10
н	11+21.33	1.73 Rt.	553.39	553.39
1 1	11+31.33	1.73 Rt.	552.68	552.69
CL Brg. E Abut	11+41.68	1.38 Rt.	551.95	551.95
Bk. E Abut	11+44.53	1.28 Rt.	551.75	551.75
G CL Brg. Pier 4 H I CL Brg. E Abut	11+04.17 11+11.33 11+21.33 11+31.33 11+41.68	1.73 Rt. 1.73 Rt. 1.73 Rt. 1.73 Rt. 1.38 Rt.	554.62 554.10 553.39 552.68 551.95	554.6 554.1 553.3 552.6 551.9

CìorbaGroup

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	USER NAME =	DESIGNED - RA	REVISED -
)		CHECKED - WM	REVISED -
1	PLOT SCALE =	DRAWN - SBA	REVISED -
	PLOT DATE =	CHECKED - WM	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

EAST APPROACH SPANS - TOP OF SLAB ELEVATIONS TABLES I STRUCTURE NO. 099-0101

SHEET S-09 OF S-48 SHEETS

FA.P. SECTION COUNTY TOTAL SHEETS NO.
607 2018-067-BR WILL 128 43

CONTRACT NO. 62M79

Location	Station	Offset Theoretical Grade Elevations		Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	3.02 Rt.	555.87	555.87
F	10+93.00	3.02 Rt.	555.42	555.42
CL F.B. 2	10+98.17	3.02 Rt.	555.04	555.04
G	11+04.17	3.02 Rt.	554.60	554.60
Cl Bra Bior 4	11+11.33	3.02 Rt.	EE 4.00	FF 4 00
CL Brg. Pier 4	11+11.33	3.02 KL	554.09	554.09
Н	11+21.33	3.02 Rt.	553.37	553.37
1	11+31.33	3.02 Rt.	552.66	552.67
CL Brg. E Abut	11+41.25	2.69 Rt.	551.96	551.96
Bk. E Abut	11+44.09	2.59 Rt.	551.76	551.76

Location	Station	Offset Theoretical Grade Ele Elevations Adju		Theoretical Grade Elevations Adjusted For Dead Load Deflection	
CL Brg. Pier 3	10+87.00	14.25 Rt.	555.70	555.70	
F	10+93.00	14.25 Rt.	555.25	555.25	
CL F.B. 2	10+98.17	14.25 Rt.	554.87	554.87	
G	11+04.17	14.25 Rt.	554.43	554.43	
CL Brg. Pier 4	11+11.33	14.25 Rt.	553.92	553.92	
Н	11+21.33	14.25 Rt.	553.20	553.21	
1	11+31.33	14.25 Rt.	552.49	552.50	
CL Brg. E Abut	11+37.40	14.06 Rt.	552.07	552.07	
Bk. E Abut	11+40.25	13.96 Rt.	551.87	551.87	

STRINGER S14

SOUTH CURB LINE

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection	
CL Brg. Pier 3	10+87.00	22.00 Rt.	555.58	555.58	
F	10+93.00	22.00 Rt.	555.14	555.14	
CL F.B. 2	10+98.17	22.00 Rt.	554.75	554.75	
G	11+04.17	22.00 Rt.	554.32	554.31	
CL Brg. Pier 4	11+11.33	22.00 Rt.	553.80	553.80	
Н	11+21.33	22.00 Rt.	553.08	553.09	
1	11+31.33	22.00 Rt.	552.38	552.39	
CL Brg. E Abut	11+34.72	22.00 Rt.	552.14	552.14	
Bk. E Abut	11+37.53	22.00 Rt.	551.94	551.94	

STRINGER S12

Location	Station	Offset Theoretical Grade Elevations		Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	6.75 Rt.	555.81	555.81
F CL F.B. 2 G CL Bra. Pier 4	10+93.00 10+98.17 11+04.17	6.75 Rt. 6.75 Rt. 6.75 Rt. 6.75 Rt.	555.36 554.98 554.54 554.03	555.36 554.98 554.54
H H	11+21.33	6.75 Rt.	553.31	553.32
'	11+31.33	6.75 Rt.	552.60	552.61
CL Brg. E Abut Bk. E Abut	11+39.96 11+42.81	6.46 Rt. 6.36 Rt.	552.00 551.80	552.00 551.80

STRINGER S15

Location	Station	Offset Theoretical Grade Elevations		Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	18.00 Rt.	555.64	555.64
F	10+93.00	18.00 Rt.	555.20	555.20
CL F.B. 2	10+98.17	18.00 Rt.	554.81	554.81
G	11+04.17	18.00 Rt.	554.38	554.37
CL Brg. Pier 4	11+11.33	18.00 Rt.	553.86	553.86
Н	11+21.33	18.00 Rt.	553.14	553.15
1	11+31.33	18.00 Rt.	552.44	552.45
CL Brg. E Abut	11+36.11	17.86 Rt.	552.10	552.10
Bk. E Abut	11+38.96	17.76 Rt.	551.90	551.90

STRINGER S17

Location	Station	Offset Theoretical Grade El Elevations Adj		Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 4	11+11.33	26.00 Rt.	554.32	554.32
Н	11+21.33	26.00 Rt.	553.60	553.61
1	11+31.33	26.00 Rt.	552.90	552.90
CL Brg. E Abut	11+33.38	25.96 Rt.	552.75	552.75
Bk. E Abut	11+36.23	25.86 Rt.	552.55	552.55

STRINGER S13

Location	Station	Offset Theoretical Grade Elevations Ac		Theoretical Grade Elevations Adjusted For Dead Load Deflection	
CL Brg. Pier 3	10+87.00	10.50 Rt.	555.76	555.76	
F	10+93.00	10.50 Rt.	555.31	555.31	
CL F.B. 2	10+98.17	10.50 Rt.	554.93	554.93	
G	11+04.17	10.50 Rt.	554.49	554.49	
CL Brg. Pier 4	11+11.33	10.50 Rt.	553.97	553.97	
Н	11+21.33	10.50 Rt.	553.25	553.26	
1	11+31.33	10.50 Rt.	552.55	552.56	
CL Brg. E Abut	11+38.68	10.26 Rt.	552.03	552.03	
Bk. E Abut	11+41.53	10.16 Rt.	551.83	551.83	

STRINGER S16

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	21.75 Rt.	555.59	555.59
F	10+93.00	21.75 Rt.	555.14	555.14
CL F.B. 2	10+98.17	21.75 Rt.	554.76	554.76
G	11+04.17	21.75 Rt.	554.32	554.32
CL Brg. Pier 4	11+11.33	21.75 Rt.	553.80	553.80
H	11+21.33	21.75 Rt.	553.09	553.09
	11+31.33	21.75 Rt.	552.38	552.39
CL Brg. E Abut	11+34.83	21.66 Rt.	552.13	552.13
Bk. E Abut	11+37.68	21.56 Rt.	551.93	551.93

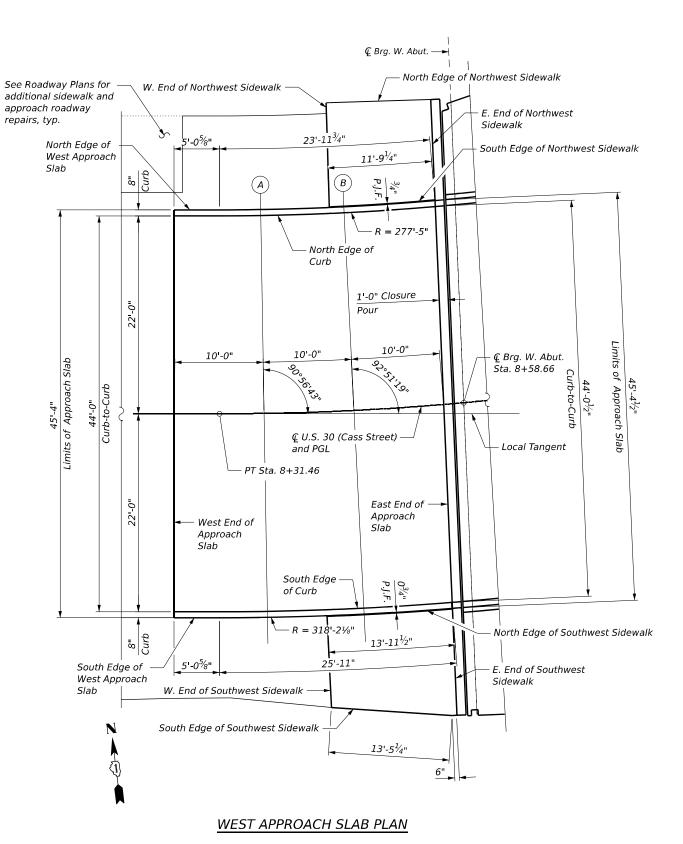
STRINGER S17A

Location	Station	Offset	Offset Theoretical Grade Elevations		
CL Brg. Pier 3	10+87.00	29.00 Rt.	556.10	556.10	
F CL F.B. 2 G	10+93.00 10+98.17 11+04.17	29.00 Rt. 29.00 Rt. 29.00 Rt.	555.66 555.27 554.84	555.66 555.29 554.85	
CL Brg. Pier 4	11+11.33	29.00 Rt.	554.32	554.32	
CL Brg. Pier 4	11+11.33	29.00 Rt.	554.32	554.32	
Н	11+21.33	29.00 Rt.	553.60	553.61	
CL Brg. E Abut	11+31.33	29.00 Rt.	552.90	552.90	
Bk. E Abut	11+35.20	28.90 Rt.	552.62	552.62	

STRINGER S18

Location	Station	Offset	Theoretical Grade Elevations	Theoretical Grade Elevations Adjusted For Dead Load Deflection
CL Brg. Pier 3	10+87.00	37.75 Rt.	556.10	556.10
F	10+93.00	37.75 Rt.	555.66	555.66
CL F.B. 2	10+98.17	37.75 Rt.	555.27	555.29
G	11+04.17	37.75 Rt.	554.84	554.85
CL Brg. Pier 4	11+11.33	37.75 Rt.	554.32	554.32
Н	11+21.33	37.75 Rt.	553.60	553.61
CL Brg. E Abut	11+29.81	37.75 Rt.	553.00	553.00
Bk. E Abut	11+34.01	37.70 Rt.	552.71	552.71

EAST APPROACH SPANS - TOP OF SLAB ELEVATIONS TABLES II	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 099-0101	607	2018-067-BR	WILL	128	44
21KOCIOKE NO. 033-0101			CONTRA	CT NO. 6	32M79
CHEET CAO OF CAO CHEETC					



DRAWN -

CHECKED - WM

REVISED

REVISED -

NORTH EDGE OF NORTHWEST SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
W. End of N.W. Sidewalk	08+48.98	34.23 Lt.	551.09
E. End of N.W. Sidewalk	08+57.96	33.89 Lt.	551.66

SOUTH EDGE OF NORTHWEST SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
W. End of N.W. Sidewalk	08+48.64	22.73 Lt.	551.25
E. End of N.W. Sidewalk	08+57.41	22.27 Lt.	551.80

NORTH EDGE OF WEST APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. App. Slab	08+26.41	22.67 Lt.	549.87
A	08+36.41	22.67 Lt.	550.49
В	08+46.41	22.67 Lt.	551.11
E. End of W. App. Slab	08+57.41	22.67 Lt.	551.79

NORTH EDGE OF CURB

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. App. Slab	08+26.41	22.00 Lt.	549.88
A	08+36.41	22.00 Lt.	550.50
В	08+46.41	22.00 Lt.	551.12
E. End of W. App. Slab	08+57.38	22.00 Lt.	551.80

CL U.S. 30 (CASS STREET)

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. App. Slab	08+26.41	0.00	550.21
A	08+36.41	0.00	550.83
В	08+46.41	0.00	551.45
E. End of W. App. Slab	08+56.41	0.00	552.07

SOUTH EDGE OF CURB

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. App. Slab	08+26.41	22.00 Rt.	549.88
Α	08+36.41	22.00 Rt.	550.50
В	08+46.41	22.00 Rt.	551.12
E. End of W. App. Slab	08+55.58	22.00 Rt.	551.69

SOUTH EDGE OF WEST APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevations
W. End of W. App. Slab	08+26.41	22.67 Rt.	549.87
Α	08+36.41	22.67 Rt.	550.49
В	08+46.41	22.67 Rt.	551.11
E. End of W. App. Slab	08+55.56	22.67 Rt.	551.68

NORTH EDGE OF SOUTHWEST SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
W. End of S.W. Sidewalk	08+45.94	22.73 Rt.	551.08
E. End of S.W. Sidewalk	08+55.56	22.73 Rt.	551.67

SOUTH EDGE OF SOUTHWEST SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
N. End of S.W. Sidewalk	08+45.94	33.73 Rt.	550.91
E. End of S.W. Sidewalk	08+55.16	34.55 Rt.	551.47

For proposed approach slab, sidewalk, curb and gutter, and pavement limits, quantities, and hatch legend, see Roadway Plans. Traffic gates and traffic signals not shown for clarity.

SECTION

2018-067-BR

128 45

CONTRACT NO. 62M79

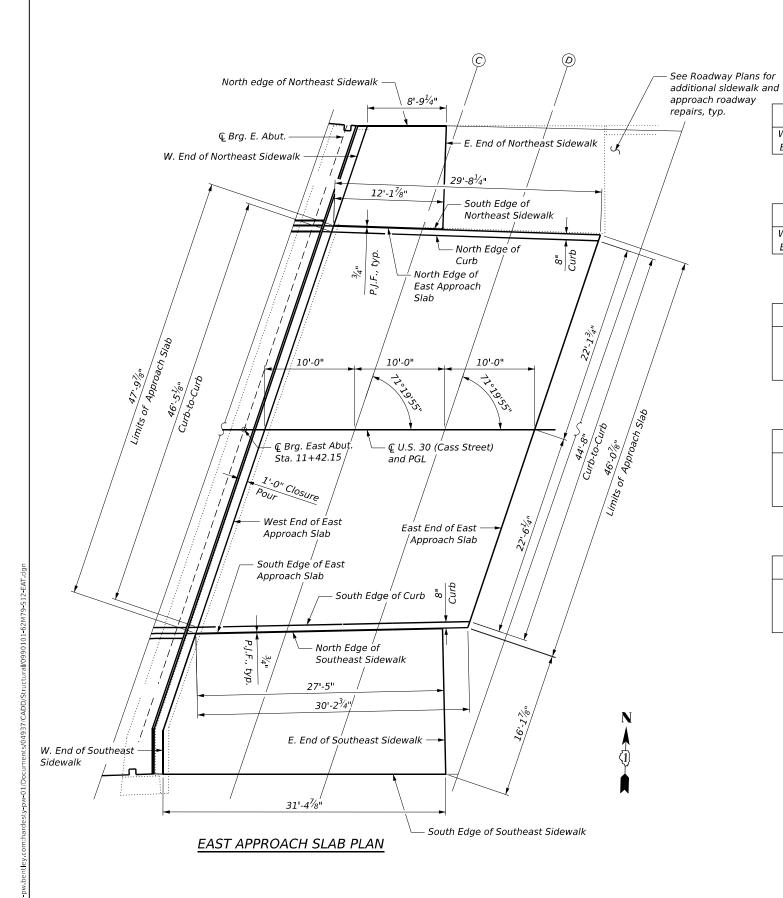
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SHEET S-11 OF S-48 SHEETS

DESIGNED - RA REVISED WEST APPROACH SLAB - TOP OF SLAB ELEVATIONS **STATE OF ILLINOIS** CHECKED - WM REVISED -607 **STRUCTURE NO. 099-0101**

DEPARTMENT OF TRANSPORTATION

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NORTH EDGE OF NORTHEAST SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
W. End of N.E. Sidewalk	11+55.84	33.76 Lt.	550.47
E. End of N.E. Sidewalk	11+64.61	33.76 Lt.	549.85

SOUTH EDGE OF NORTHEAST SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
W. End of N.E. Sidewalk	11+52.10	22.69 Lt.	550.90
E. End of N.E. Sidewalk	11+64.23	22.28 Lt.	550.05

NORTH EDGE OF EAST APPROACH SLAB

Locati	on	Station	Offset	Theoretical Grade Elevations
W. End of E.	App. Slab	11+52.08	22.63 Lt.	550.90
С		11+61.97	22.30 Lt.	550.20
D		11+71.86	21.97 Lt.	549.51
E. End of E. A	App. Slab	11+81.75	21.64 Lt.	548.82

NORTH EDGE OF CURB

Location	Station	Offset	Theoretical Grade Elevations
W. End of E. App. Slab	11+51.86	21.97 Lt.	550.92
С	11+61.75	21.64 Lt.	550.23
D	11+71.64	21.31 Lt.	549.54
E. End of E. App. Slab	11+81.53	20.98 Lt.	548.84

CL U.S. 30 (CASS STREET)

Location	Station	Offset	Theoretical Grade Elevations
W. End of E. App. Slab	11+44.44	0.00	551.78
С	11+54.44	0.00	551.07
D	11+64.44	0.00	550.36
E. End of E. App. Slab	11+74.44	0.00	549.66

SOUTH EDGE OF CURB

Location	Station	Offset	Theoretical Grade Elevations
W. End of E. App. Slab	11+37.01	21.98 Rt.	551.97
С	11+47.09	21.77 Rt.	551.26
D	11+57.16	21.55 Rt.	550.56
E. End of E. App. Slab	11+67.23	21.34 Rt.	549.85

SOUTH EDGE OF EAST APPROACH SLAB

Location	Station	Offset	Theoretical Grade Elevations
W. End of E. App. Slab	11+36.78	22.65 Rt.	551.98
С	11+46.86	22.44 Rt.	551.27
D	11+56.93	22.22 Rt.	550.56
E. End of E. App. Slab	11+67.00	22.01 Rt.	549.85

NORTH EDGE OF SOUTHEAST SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
W. End of S.E. Sidewalk	11+36.76	22.71 Rt.	551.98
С	11+46.84	22.50 Rt.	551.27
D	11+56.91	22.29 Rt.	550.56
E. End of S.E. Sidewalk	11+64.19	22.13 Rt.	550.05

SOUTH EDGE OF SOUTHEAST SIDEWALK

Location	Station	Offset	Theoretical Grade Elevations
W. End of S.E. Sidewalk	11+33.13	38.30 Rt.	552.00
С	11+41.50	38.30 Rt.	551.41
D	11+51.50	38.30 Rt.	550.70
E. End of S.E. Sidewalk	11+64.54	38.30 Rt.	549.78

votes:

For proposed approach slab, sidewalk, curb and gutter, and pavement limits, quantities, and hatch legend, see Roadway Plans. Traffic gates and traffic signals not shown for clarity.

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

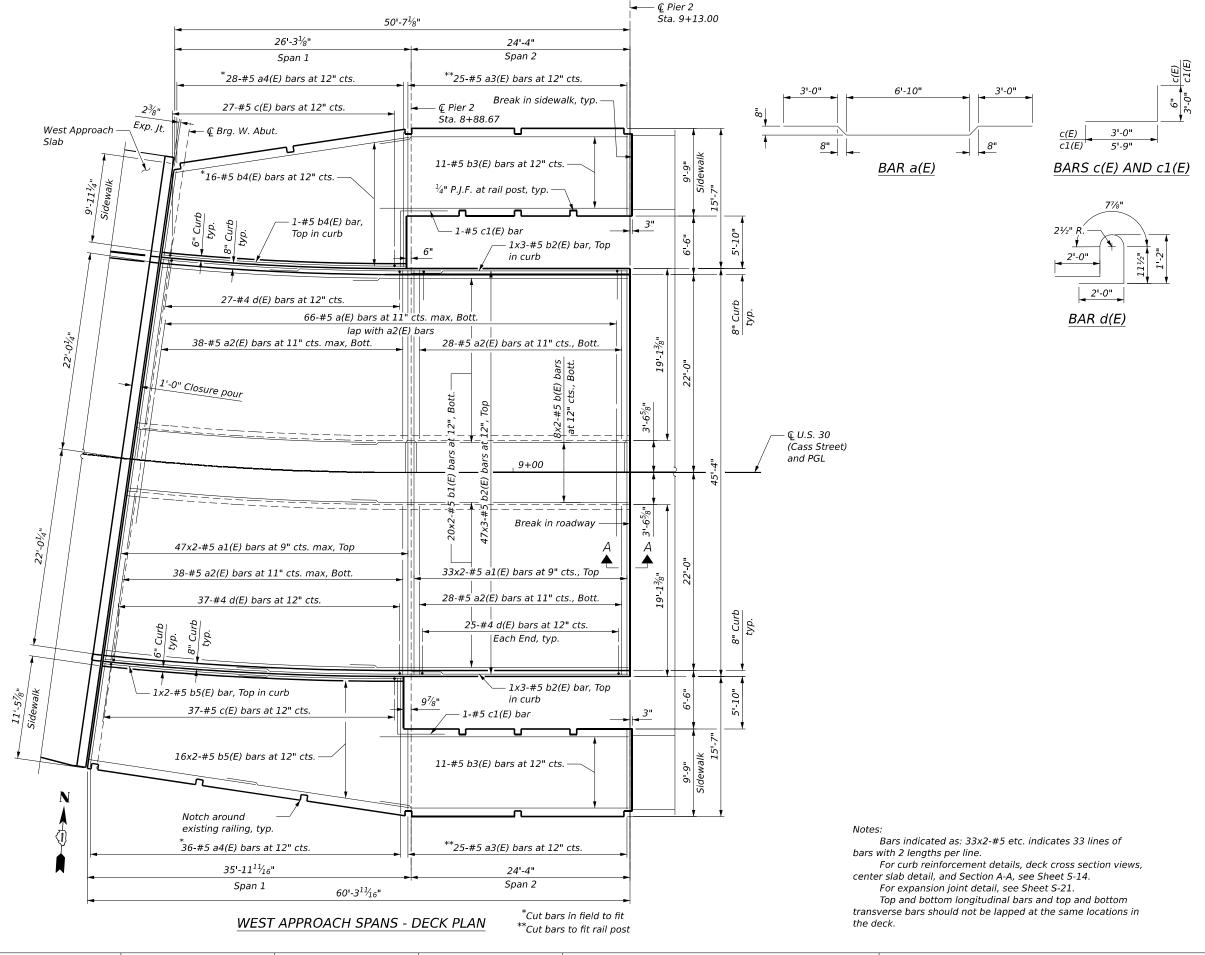
EAST APPROACH SLAB - TOP OF SLAB ELEVATIONS
STRUCTURE NO. 099-0101

SHEET S-12 OF S-48 SHEETS

 F.A.P. RTE.
 SECTION
 COUNTY SHEETS NO.
 SHEETS NO.

 607
 2018-067-BR
 WILL
 128
 46

 CONTRACT NO. 62M79



BILL OF MATERIAL

No.	Size	Length	Shape
66	#5	14'-10''	~
160	#5	24'-3"	
132	#5	19'-0''	
50	#5	9'-5''	
64	#5	15'-2''	
16	#5	29'-7''	
80	#5	30'-11''	
147	#5		
22	#5	28'-5"	
17	#5	27'-0''	
34	#5	19'-10''	
64	#5		
2	#5	8'-9''	
114	#4	6'-7''	5
e Superstr	ucture	Cu Yd	101.9
eck Groov	∕ing	Sq Yd	271
		Sq Yd	442
ement Ba	rs, Epoxy	Pound	18,200
			= -,
ed Joint Fi	ller	Foot	61
		Foot	112
	66 160 132 50 64 16 80 147 22 17 34 64 2 114 es Superstr eeck Groot ve Coat ement Ba	66 #5 160 #5 132 #5 50 #5 64 #5 16 #5 80 #5 147 #5 22 #5 17 #5 34 #5 64 #5 2 #5 114 #4 E Superstructure eleck Grooving re Coat ement Bars, Epoxy ed Joint Filler And Re-Erect	66 #5 14'-10" 160 #5 24'-3" 132 #5 19'-0" 50 #5 9'-5" 64 #5 15'-2" 16 #5 29'-7" 80 #5 30'-11" 147 #5 21'-10" 22 #5 28'-5" 34 #5 19'-10" 64 #5 3'-6" 2 #5 8'-9" 114 #4 6'-7" e Superstructure

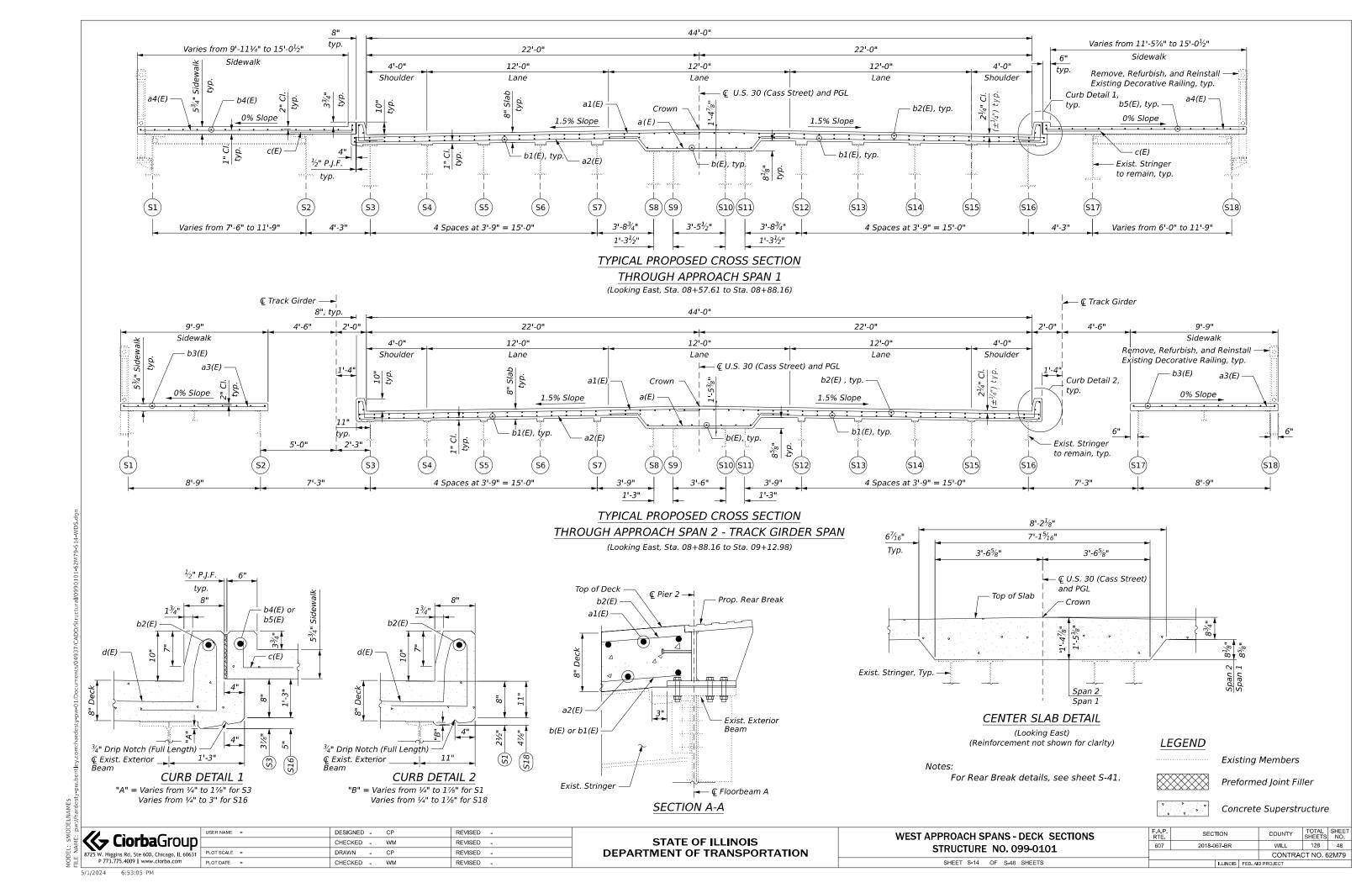
MIN. BAR LAP

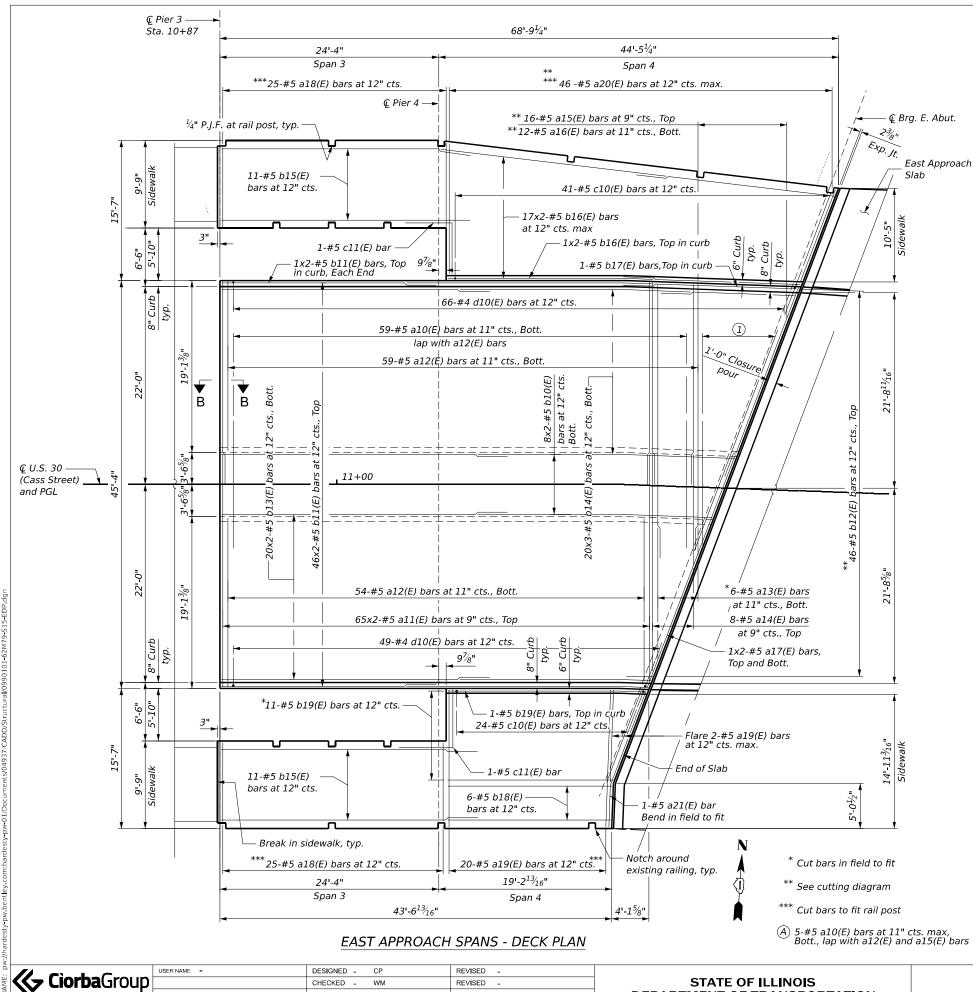
#4 Bar = 2'-5"

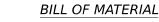
#5 Bar = 3'-6"

DESIGNED - CP REVISED -SECTION COUNTY WEST APPROACH SPANS - DECK PLAN STATE OF ILLINOIS CHECKED - WM REVISED -WILL 128 47 607 2018-067-BR **STRUCTURE NO. 099-0101** DRAWN - CP REVISED -**DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 62M79 SHEET S-13 OF S-48 SHEETS CHECKED - WM REVISED -

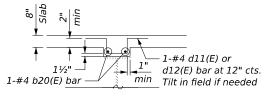
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Bar	No.	Size	Length	Shape
a10(E)	64	#5	14'-10''	~
a11(E)	130	#5	24'-3''	
a12(E)	113	#5	19'-0''	
a13(E)	6	#5	18'-10''	
a14(E)	8	#5	24'-10''	
a15(E)	8	#5	34'-6''	
a16(E)	6	#5	22'-4''	
a17(É)	4	#5	31'-5''	
a18(E)	50	#5	9'-5''	
a19(E)	22	#5	15'-3''	
a20(E)	23	#5	26'-7''	
a21(E)	1	#5	15'-10''	
1.10(E)	1.0	<i>".</i> F	201 411	
b10(E)	16	#5	30'-4''	
b11(E)	96	#5	25'-5"	
b12(E)	23	#5	24'-2''	
b13(E)	40	#5	29'-0''	
b14(E)	60	#5	23'-10"	
b15(E)	22	#5	28'-9''	
b16(E)	36	#5	23'-8''	
b17(E)	1	#5	20'-6''	
b18(E)	6	#5	19'-0''	
b19(E)	12	#5	22'-2''	
b20(E)	8	#4	24'-0''	
c10(E)	65	#5	3'-6''	1
c11(E)	2	#5	8'-9"	
C11(E)		#3	0 -9	
d10(E)	115	#4	6'-7''	
d11(E)	46	#5	5'-0''	<u> </u>
d12(E)	45	#4	4'-6''	
Concrete	Superstr	ucture	Cu Yd	105.8
	eck Groo		Sq Yd	275
Protectiv		virig	Sq Yd	445
		rs, Epoxy	•	
Coated	ентент Ба	is, Epuxy	Pound	18,460
Preforme	ed Joint Fi	ller	Foot	62
	And Re-E Bridge Ra		Foot	113



FILLET REINFORCEMENT

Additional reinforcement is required for fillet heights in excess of 6" as shown above. Bars d11(E) and d12(E) are detailed in the bar list. However, the placement of the bars is not shown in the plans. d11(E) bars are to be placed above stringer S17. d12(E) bars are to be placed above stringer \$17A.The Contractor shall place the bars b20(E) in accordance with the above detail.

MIN. BAR LAP

#4 Bar = 2'-5" #5 Bar = 3'-6"

d11(E)

Bars indicated as: 65x2-#5 etc. indicates 65 lines of bars with 2 lengths per line.

For curb reinforcement details, deck cross section views and Section B-B, see Sheet S-16.

For expansion joint detail, see Sheet S-21.

Top and bottom longitudinal bars and top and bottom transverse bars should not be lapped at the same locations in the deck

EAST APPROACH SPANS - DECK PLAN **STRUCTURE NO. 099-0101** SHEET S-15 OF S-48 SHEETS

6'-10"

BAR a10(E)

BARS a15(E), a16(E) and a20(E);

BARS b12(E)

Order bars full length. Cut as shown and use remainder of bars in opposite end of roadway

a15(E) 31'-0" 3'-6" 34'-6" a16(E) 18'-10" 3'-6" 22'-4" a20(E) 15'-3" 11'-4" 26'-7'

b12(E) 20'-8' 3'-6" 24'-2"

1.0-18

Bar A B

3'-0"

5'-9"

BARS c10(E) AND c11(E)

2'-0"

BAR d10(E)

1'-0" 1'-0"

BARS d11(E) AND d12(E)

c10(E) c11(E)

2½" F

2'-0"

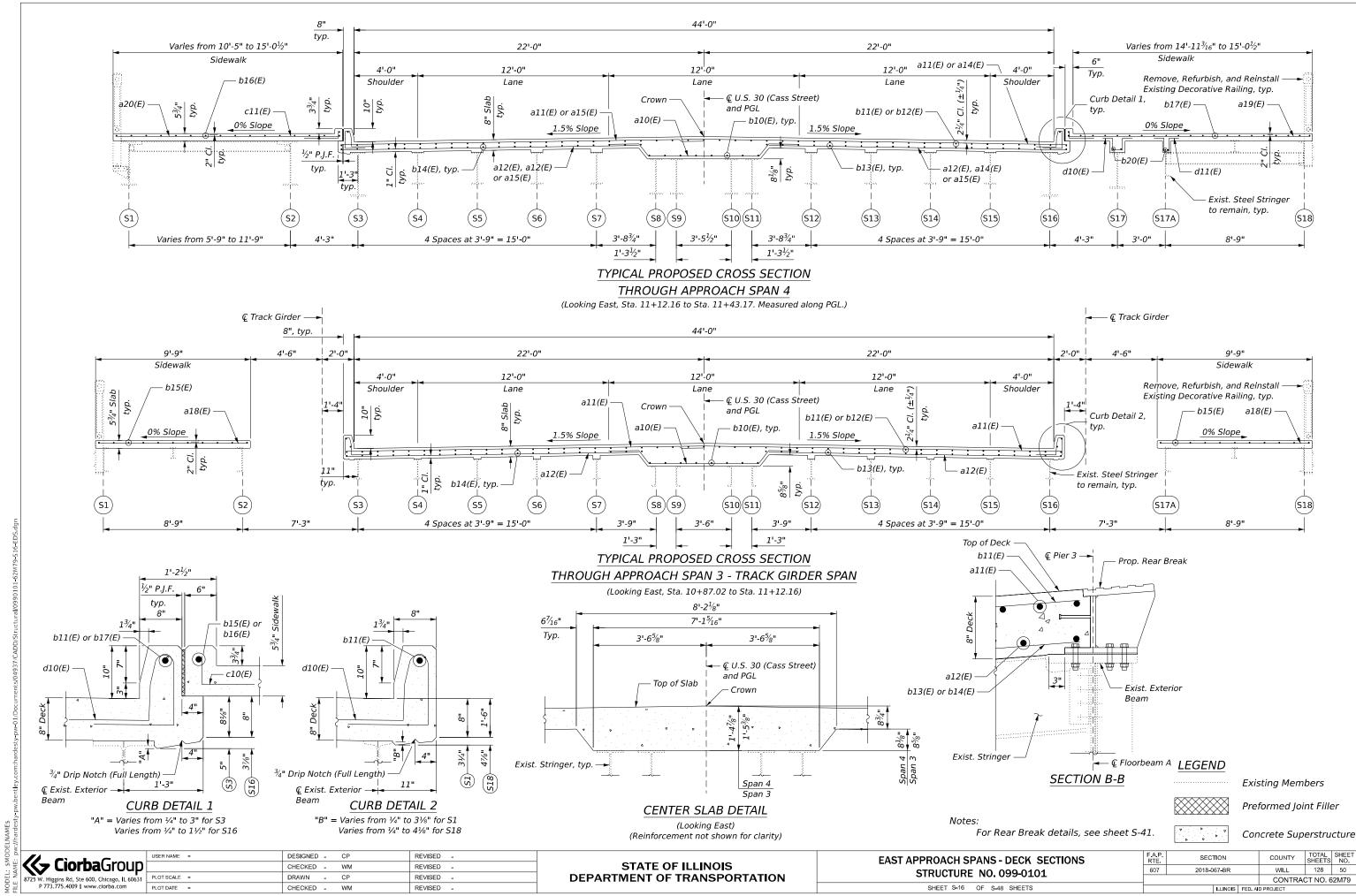
8"

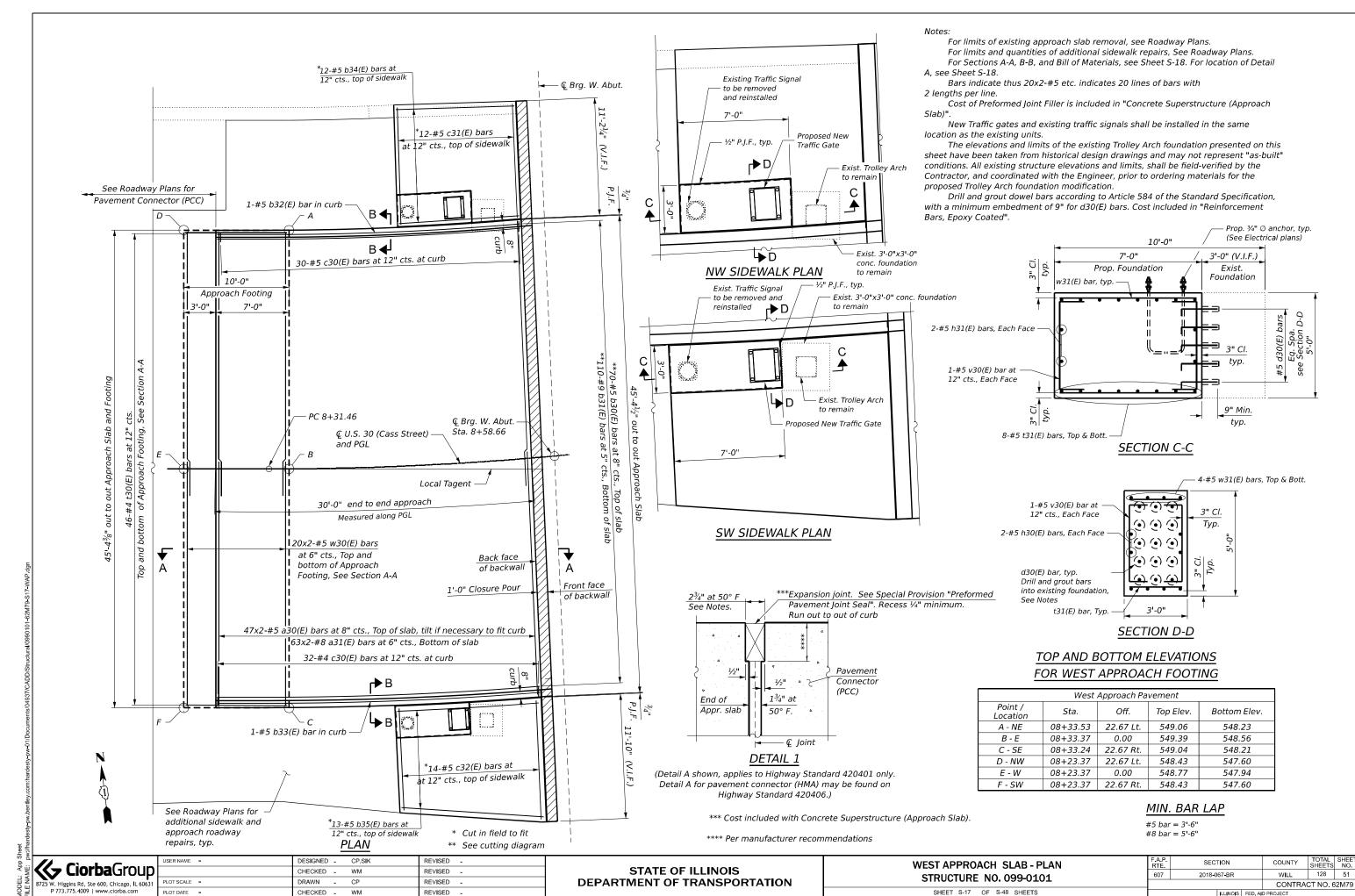
SECTION COUNTY 607 2018-067-BR WILL 128 49 CONTRACT NO. 62M79

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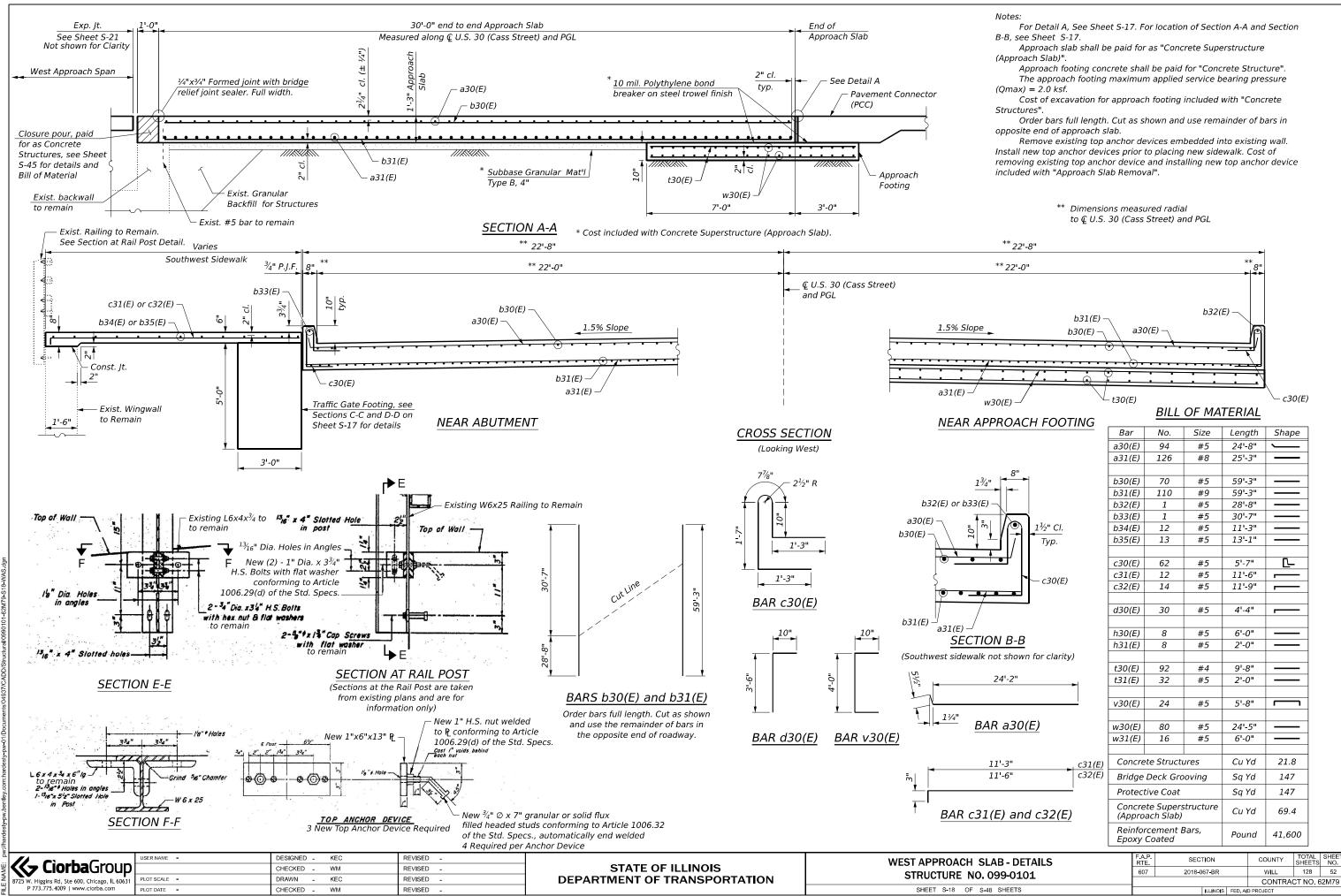
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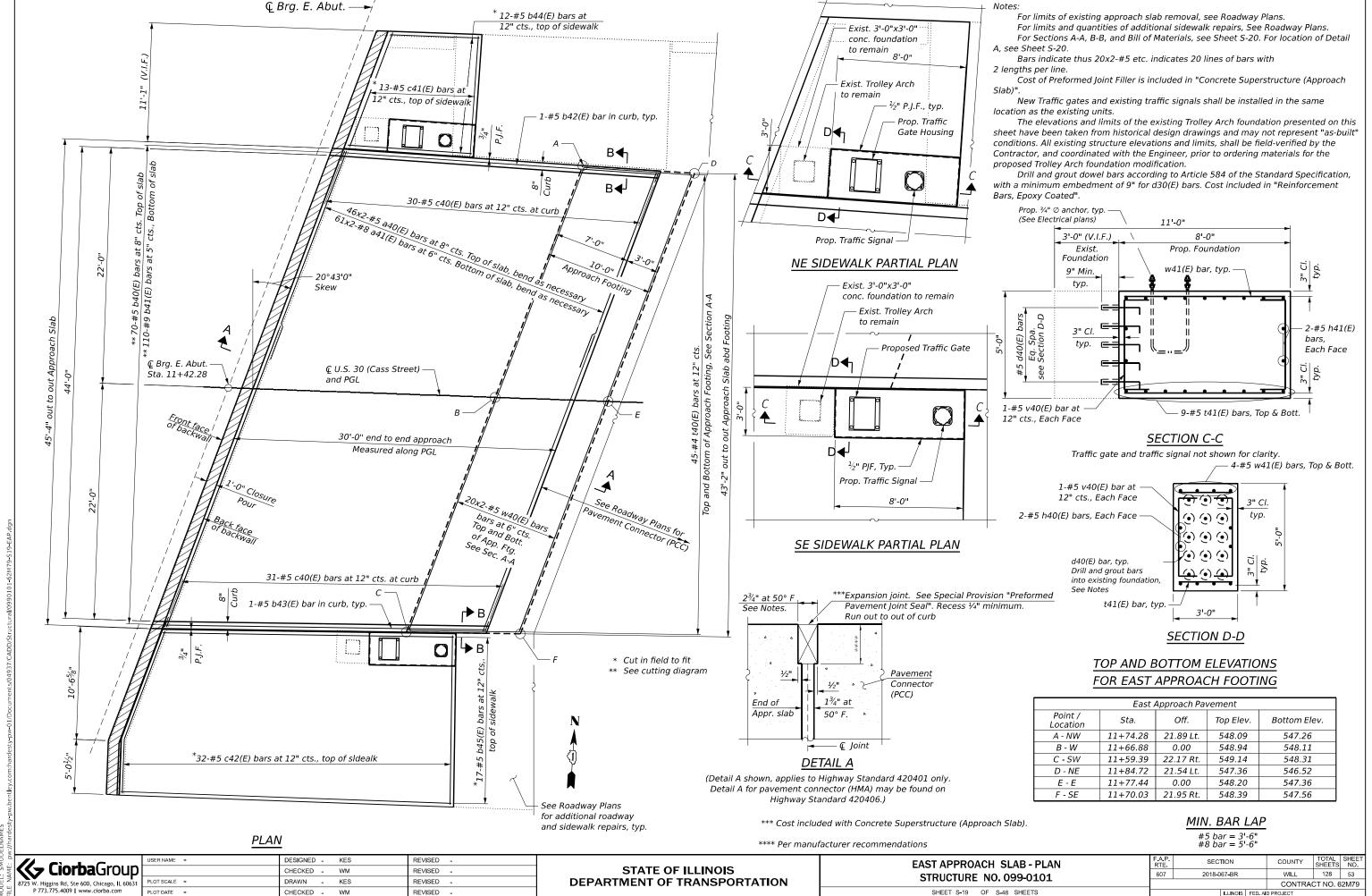
DEPARTMENT OF TRANSPORTATION



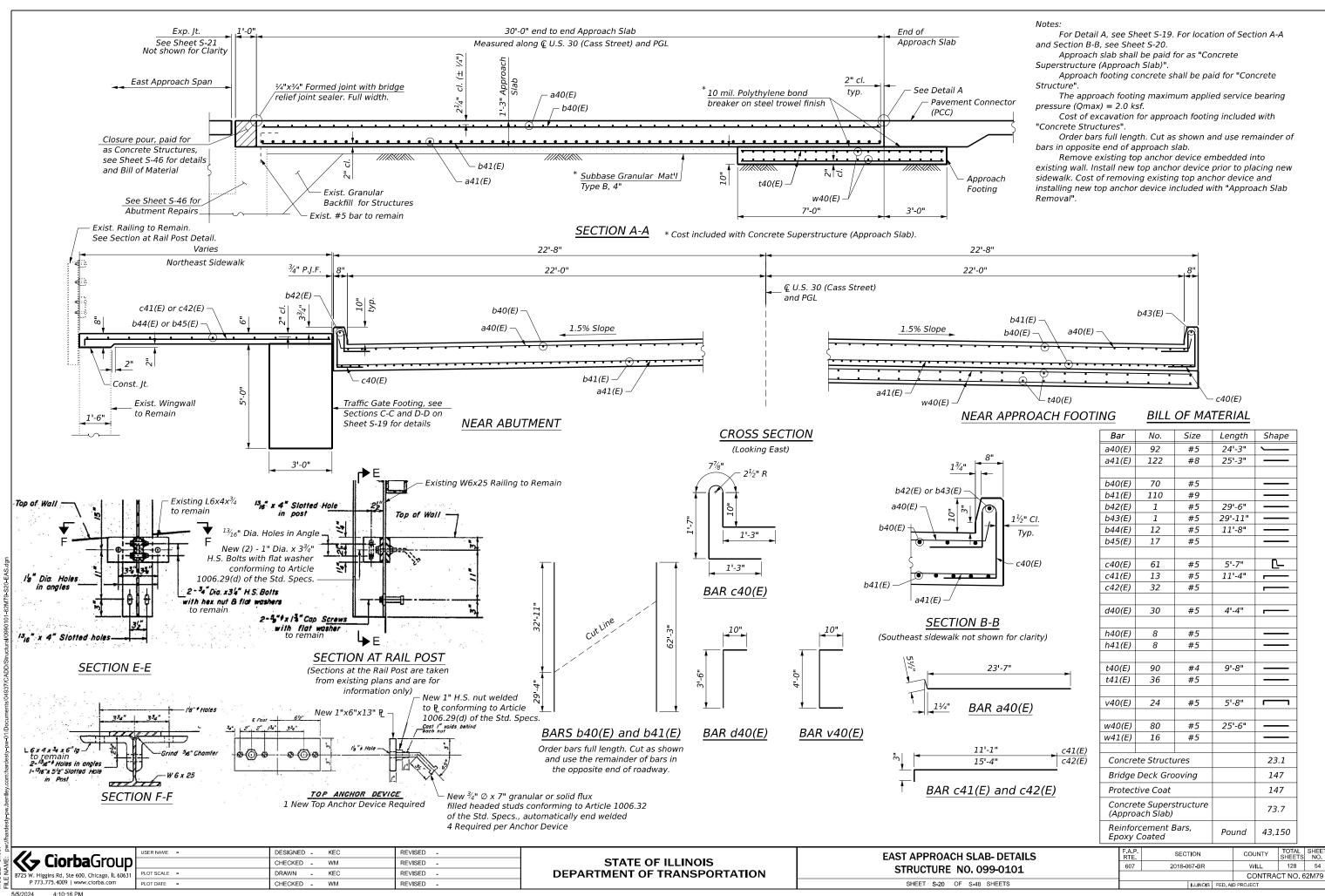


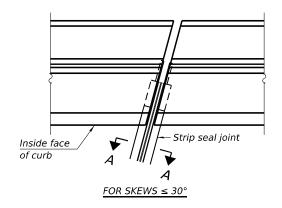
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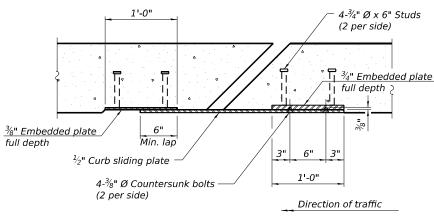


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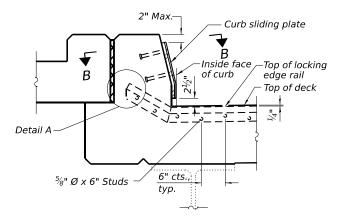




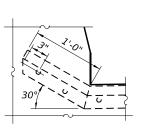
PLAN AT CURB



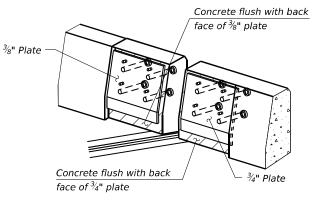
SECTION B-B



SECTION AT CURB



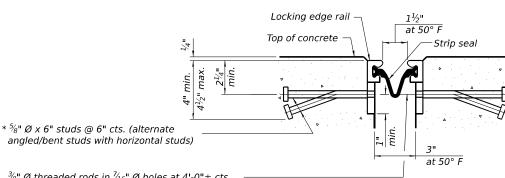
DETAIL A



TRIMETRIC VIEW (Showing embedded plates only)

Locking edge rail at 50° F Top of concrete Strip seal 2³/₈" at 50°

SHOWING ROLLED RAIL JOINT



 $\frac{3}{8}$ " Ø threaded rods in $\frac{7}{16}$ " Ø holes at 4'-0" ± cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

SECTION A-A

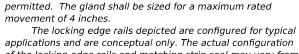
* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.





LOCKING EDGE RAILS

penetration is verified by mock-up.



Notes:

applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not

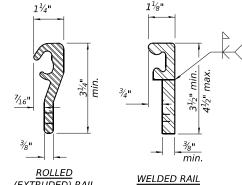
The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

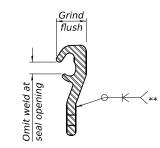
The Maximum space between locking edge rail segments shall be and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of curb sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.



** Back gouge not required if complete joint



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	139

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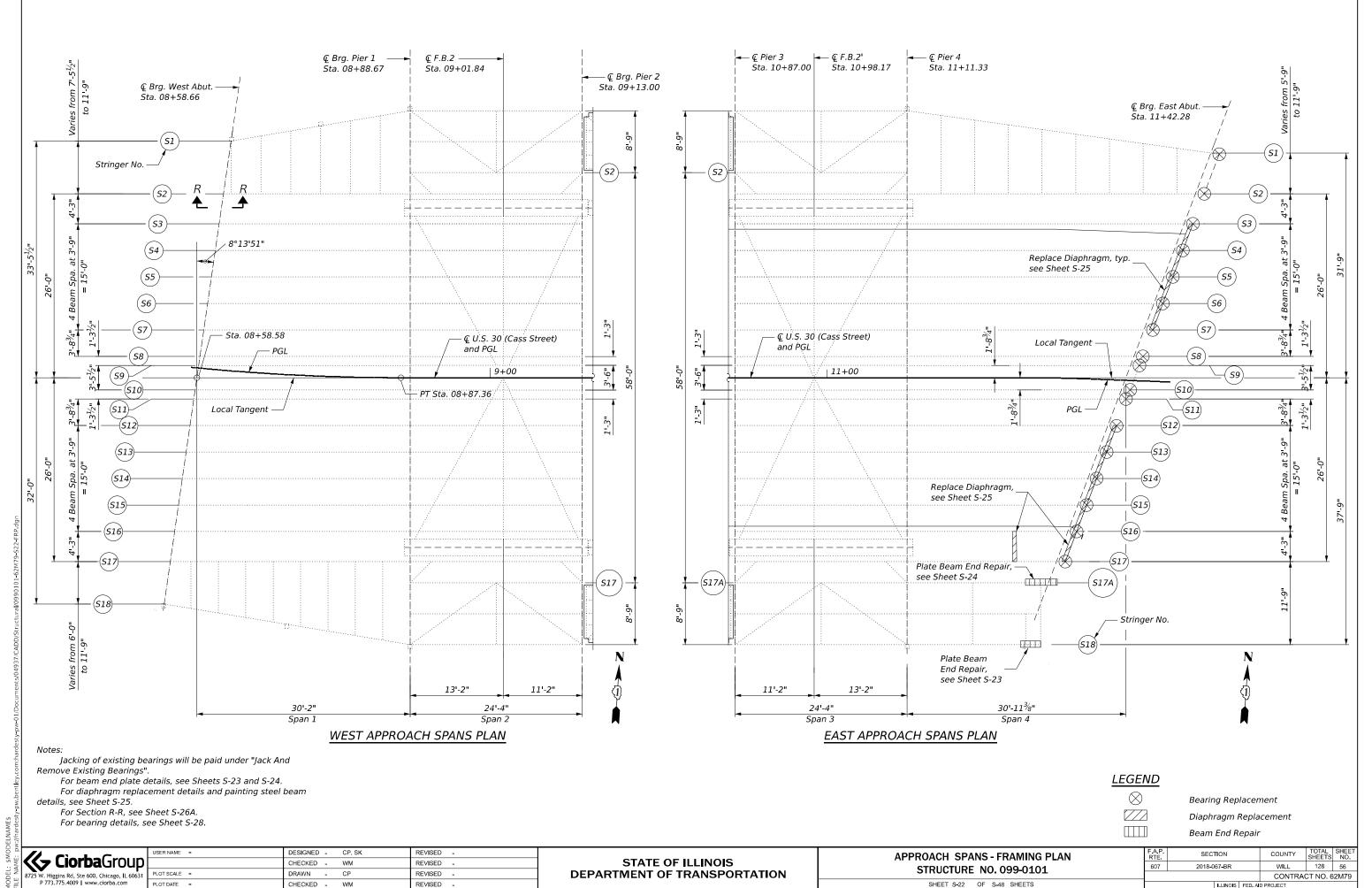
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1	PLOT SCALE =	DRAWN	-	SBA	REVISED	-
	PLOT DATE =	CHECKED	-	WM	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SHOWING WELDED RAIL JOINT

APPROACH SPANS - PREFORMED JOINT STRIP SEAL **STRUCTURE NO. 099-0101** SHEET S-21 OF S-48 SHEETS

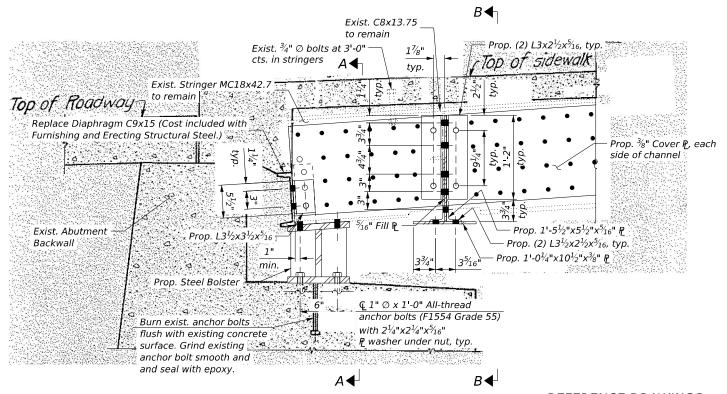
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			CONTRAC	CT NO. 6	2M79
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EAST APPROACH SPAN - S18 BEAM END REPAIR DETAILS

(Looking North)



BEAM END ELEVATION

(Looking South)

* REFERENCE DRAWINGS:

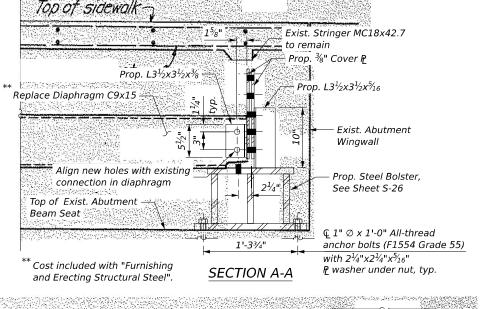
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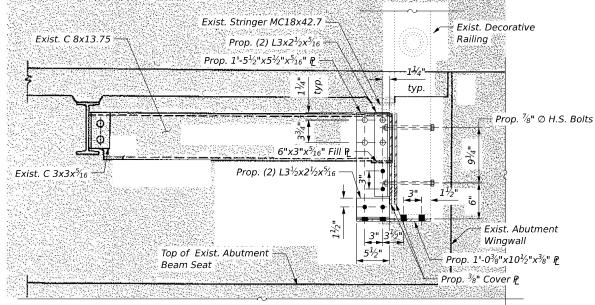
Approach Details

1345-22 Approach Stringers 630-54

* For Reference Drawings Key, see Sheet S-02.

original drawings for this bridge. Details shown are provided to illustrate the work to be performed.





SECTION B-B

APPROACH SPANS - MISCELLANEOUS STEEL DETAILS I

STRUCTURE NO. 099-0101

SHEET S-23 OF S-48 SHEETS

Notes:

Bolster height shall be field verified prior to fabrication of bolsters. Heights of existing bearings shall be measured and recorded before jacking stringers and installing bolsters.

All bolsters shall consist of structural steel plates.

All structural steel shall conform to the requirements of AASHTO M270 Grade 50.

Anchor bolts shall be ASTM F1554 all-thread (or an Engineerapproved alternate material) of the grade(s) and diameter(s) specified. The corresponding grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

Cost of all structural steel including anchor bolts/bolts shall be included in "Structural Steel Repair"

Fasteners shall be ASTM A325 Type I, Mechanically Galvanized bolts. Bolts connecting new steel members shall be $\frac{7}{8}$ " \emptyset with $^{15}\!\!/_{16}$ " \emptyset open holes, unless noted otherwise.

Cost of field drilling shall be included in "Structural Steel

For bolster details, see Sheet S-26.

BOLT LEGEND

- New Fastener in Existing or Field Drilled Hole
- New Fastener in Shop Drilled Hole new member to be used for field drilling connections in existing member
- Existing Fastener to Remain

SUGGESTED WORK PLAN

- Remove East approach span deck and connections to existing diaphragms for Stringer S18.
- Install temporary shoring and cribbing.
- Clean Stringer S18 and install web plates with H.S. bolts.
- Install angles, plates and reconnect diaphragms to Stringer S18.
- Install bolster, install anchor bolts and connect bolster to bottom flange of Stringer S18.

BILL OF MATERIAL

Structural Steel Repair Poun	TOTAL
	536
Temporary Shoring And Cribbing Each	1

Note: Portions of these drawings are extracted from

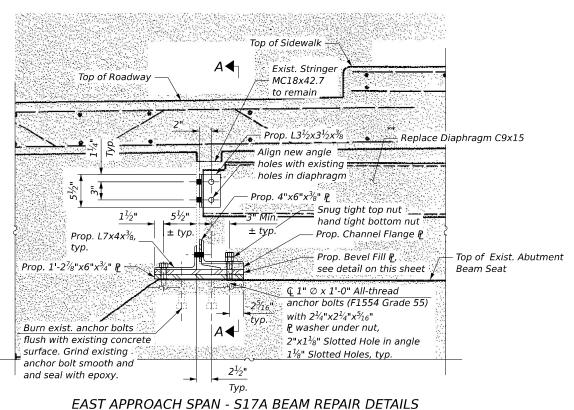
For stringer S18 service dead load reaction due to steel only, see table Exist. Beams Reactions on Sheet S-28.

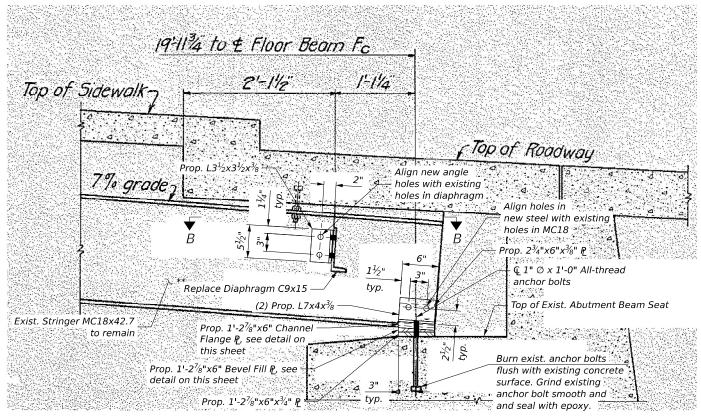
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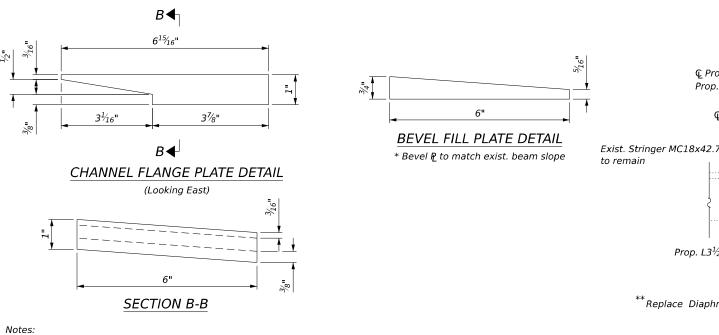
SECTION A-A

Measured along

SECTION B-B

Angle Widths

±1'-11/4"



REFERENCE DRAWINGS:

Approach Details

1345-22

Note: Portions of these drawings are extracted from original drawings for this bridge. Details shown are

provided to illustrate the work to be performed.

For Reference Drawings Key, see Sheet S-02.

BOLT LEGEND

- New Fastener in Existing or Field Drilled Hole
- New Fastener in New Hole
- Existing Fastener to Remain

SUGGESTED WORK PLAN

- Remove East approach span deck, bearing plate and existing connections to diaphragm for Stringer S17A.
- Install temporary shoring and cribbing.
- Clean Stringer S17A and install angles and plates.
- Reconnect diaphragm to Stringer S17A.
- Remove temporary shoring and cribbing.

BILL OF MATERIAL

ITEM	UNIT	TOTAL
Structural Steel Repair	Pound	79
Temporary Shoring And Cribbing	Each	1

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

Prop. $L3^{1/2}x3^{1/2}x^{3/8}$

** Replace Diaphragm C9x15

Cost included in "Furnishing and

Erecting Structural Steel".

€ Prop. 7/8" Ø H.S. Bolts in Prop. $2"x1\frac{1}{8}"$ Slotted Hole, typ.

© Exist. Slotted Hole, typ.

APPROACH SPANS - MISCELLANEOUS STEEL DETAILS II **STRUCTURE NO. 099-0101** SHEET S-24 OF S-48 SHEETS

Prop. $L7x4x^{3/8}$

Prop. 4"x6"x3/8" P

P Widths

Prop. 1'-2 $\frac{7}{8}$ "x6" Channel Flange $\frac{1}{8}$,

Angle Widths

see detail on this sheet

- Prop. L7 $x4x^{3/8}$

SECTION COUNTY 607 2018-067-BR WILL 128 CONTRACT NO. 62M79

Bearing heights shall be field verified.

Heights of existing bearings shall be measured and recorded before jacking stringers and installing structural steel. All structural steel shall conform to the requirements of AASHTO M270 Grade 50.

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554. Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications.

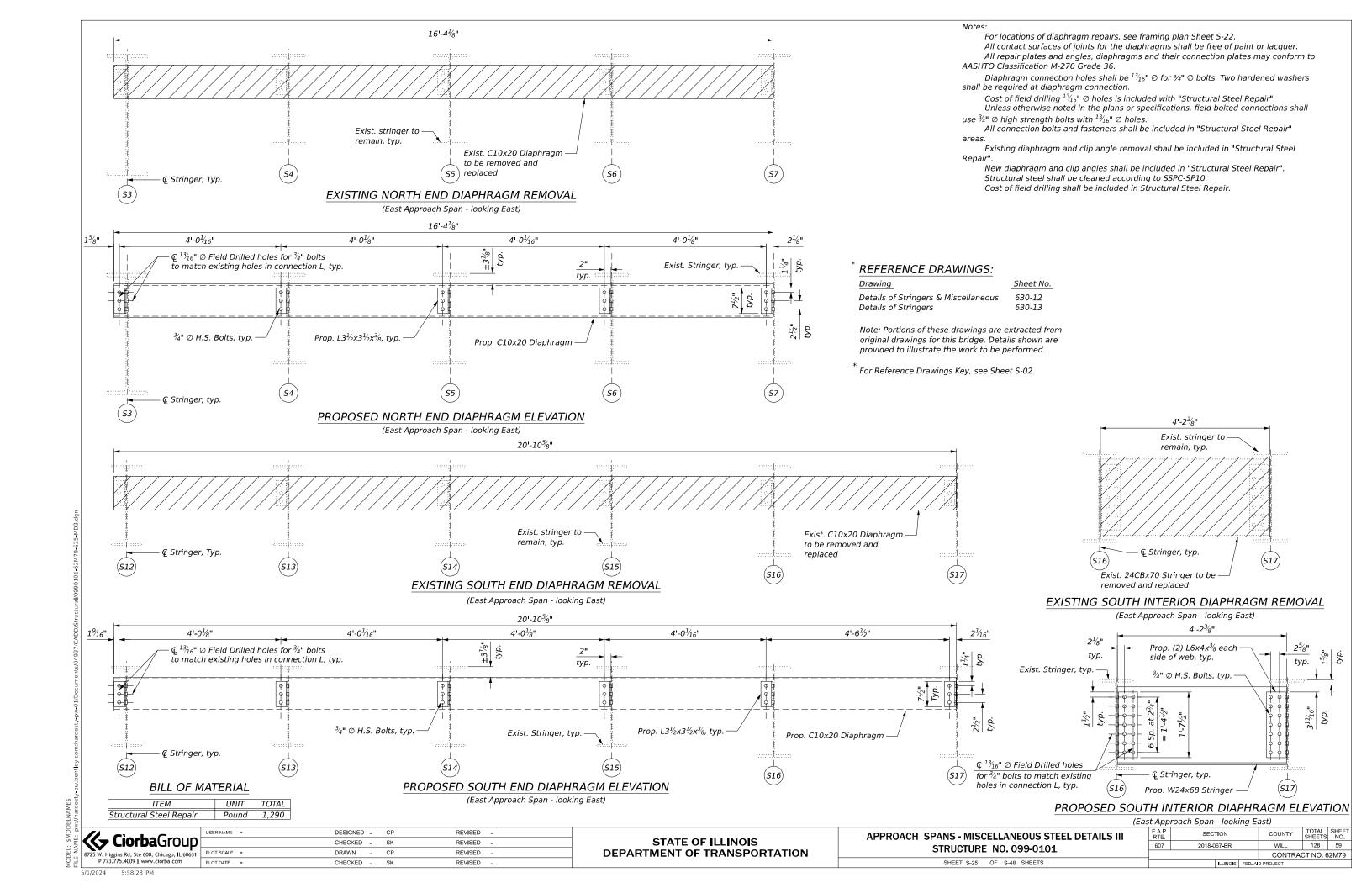
Fasteners shall be ASTM A325 Type I, Mechanically Galvanized bolts. Bolts connecting new steel members shall be ½" ⊘ with $1" \oslash open holes$, unless noted otherwise.

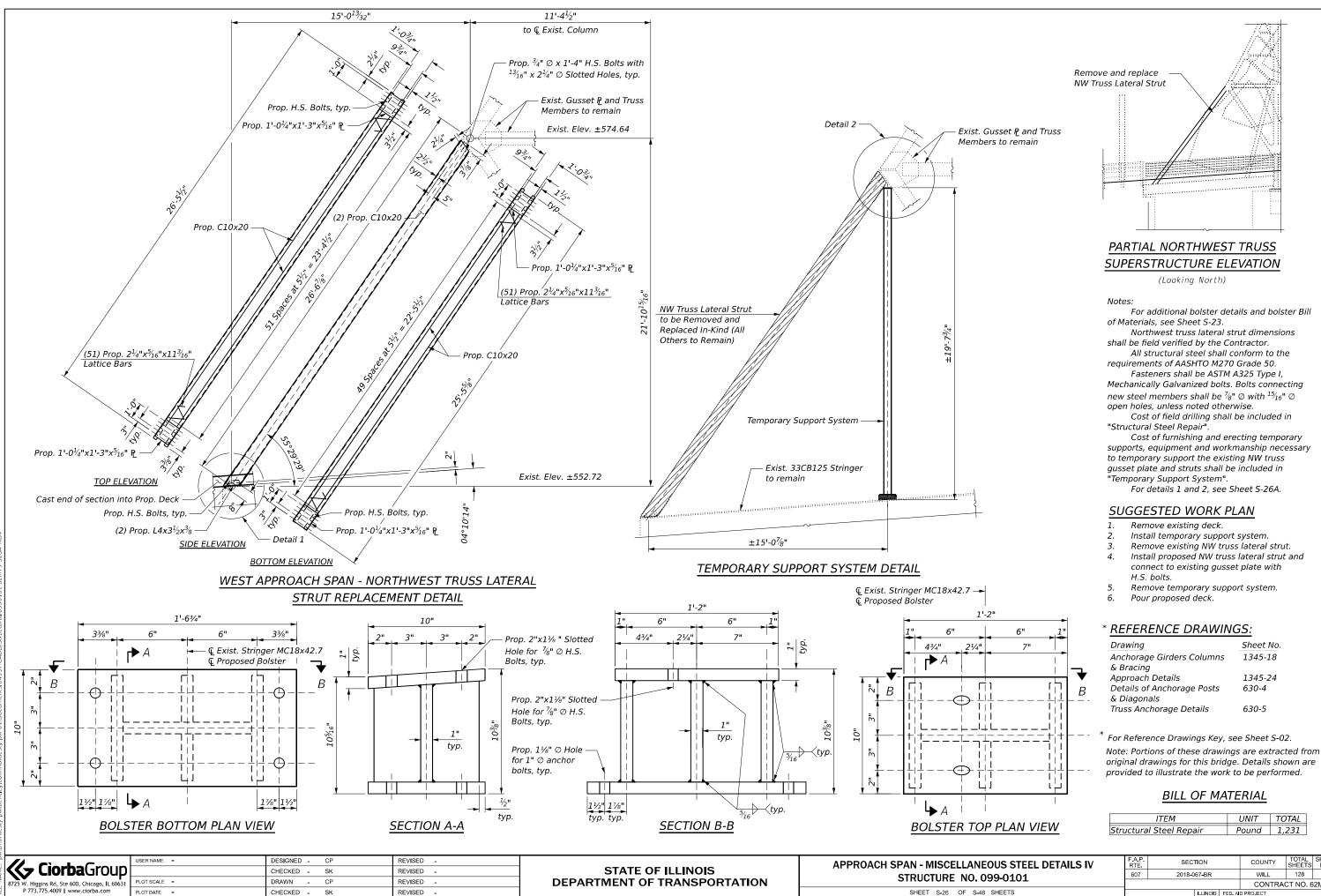
Cost of field drilling shall be included in "Structural Steel Repair".

For stringer 17A service load reacion due to steel only, see table Exist. Beams Reactions on Sheet S-28.

DESIGNED - CP REVISED **Cìorba**Group CHECKED - SK REVISED -DRAWN REVISED Higgins Rd, Ste 600, Chicago, IL 60631 CHECKED -REVISED

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1345-18

1345-24

630-4

630-5

Pound 1,231

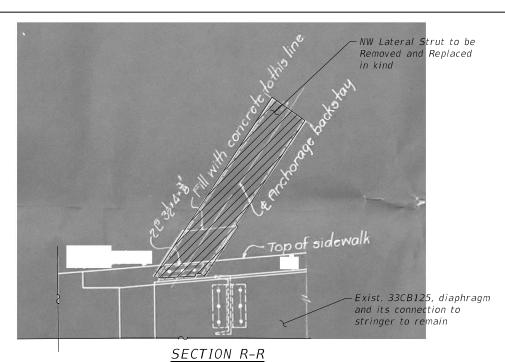
WILL

128 60

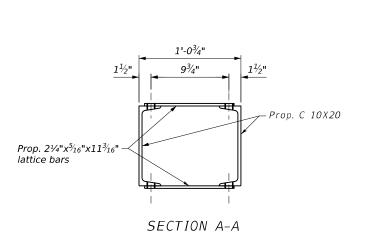
CONTRACT NO. 62M79

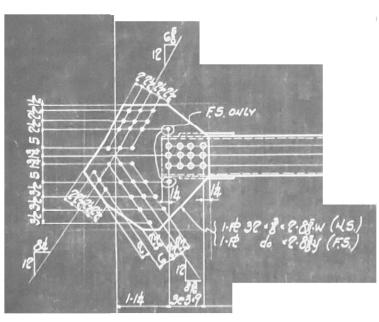
SECTION

Connection of NW Truss Lateral Strut to Exist. Stringer 33CB125 at the West Approach Span 1 (For information only. All members and fasteners to be replaced.)



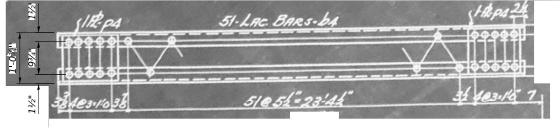
(See West Approach Span framing plan on sheet S-22)





DETAIL 2

Connection of NW Truss Lateral Strut to Exist. Horizontal Strut (For information only. All members and fasteners to remain except the fasteners connecting the existing gusset plate with the new NW Lateral Strut.)



TOP OF NW TRUSS LATERAL STRUT LATTICE BARS

(For information only. All members and fasteners to be replaced.)



BOTTOM OF NW TRUSS LATERAL STRUT LATTICE BARS

(For information only. All members and fasteners to be replaced.)

Anchorage Girders Columns 1345-18 & Bracing 1345-24

* REFERENCE DRAWINGS:

Approach Details Details of Anchorage Posts

Drawing

& Diagonals

Truss Anchorage Details 630-5

For notes and additional details, see Sheet S-26.

* For Reference Drawings Key, see Sheet S-02. Note: Portions of these drawings are extracted from original drawings for this bridge. Details shown are provided to illustrate the work to be performed.

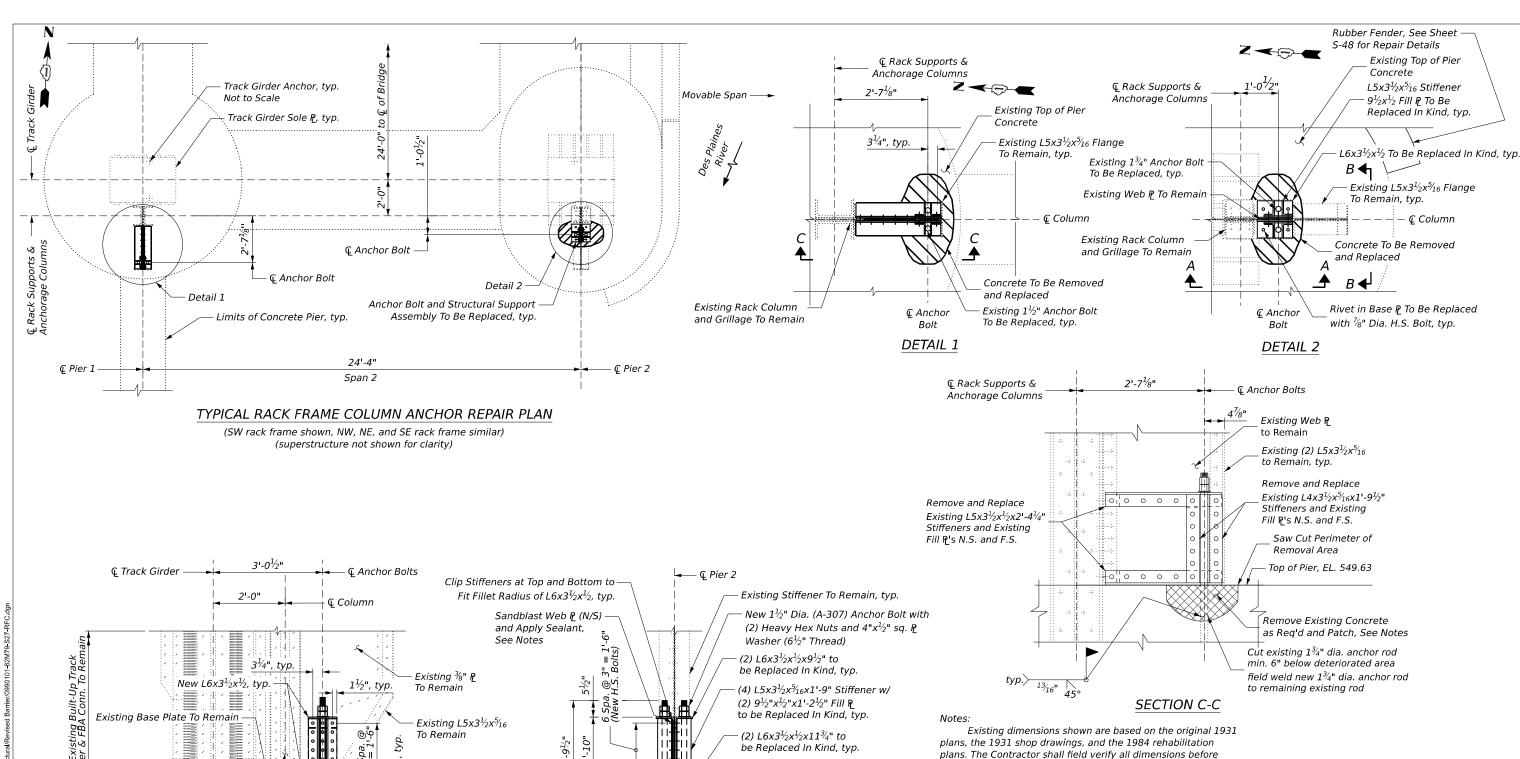
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** APPROACH SPAN - MISCELLANEOUS STEEL DETAILS V **STRUCTURE NO. 099-0101** SHEET S-26A OF S-48 SHEETS

SECTION 2018-067-BR WILL 128 60A CONTRACT NO. 62M79

630-4



be Replaced In Kind, typ. Existing Base P Top of Pier Concrete To Remain El. 549.63 - Existing $1\frac{1}{2}$ " Dia. Anchor Bolts Above Cut To Be Replaced Existing Grillage Remove Existing Concrete to Remain, typ. as Req'd and Patch. See Notes. Concrete To Be Removed and Cut Existing 11/2" Dia. Anchor Existing Track Girder -Repaired. Bolts Min. 6" Below Pedestal To Remain typ. _______45° Deteriorated Bolt Section, typ. Cut Existing $1\frac{1}{2}$ " Dia. Anchor Existing Grillage To Remain See Notes. Bolts Min. 6" Below $3\frac{1}{4}$ ", typ. Existing L5x3 $\frac{1}{2}$ x $\frac{5}{16}$ Stiffeners 1'-0" Deteriorated Bolt Section, typ.

SECTION B-B

SECTION A-A (looking east, opposite hand similar)

(looking north, opposite hand similar) (track girder and front rack column not shown for clarity)

LEGEND

- O New Fastener in Existing or Field Drilled Hole
- New Fastener in New Hole · Existing Fastener to Remain
- Existing Member to Remain
 - New Member

BILL OF MATERIAL

high-strength bolts (A325 or equivalent) in the existing ¹⁵/₁₆" Item Unit Total Structural Steel Repair Pound 1,502

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See Notes.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION APPROACH SPAN - RACK FRAME COLUMN REPAIR DETAILS **STRUCTURE NO. 099-0101** SHEET S-27 OF S-48 SHEETS

beginning fabrication and installation to confirm proper fit up

according to "Structural Repair of Concrete", but shall be

rack frame at the west bridge approach of Pier 2; however,

repairs are typical for all rack frame columns on Pier 1(N&S),

hole locations specified for bolt replacement. Structural steel

replacement of all corresponding $\frac{7}{8}$ " Dia. rivets with $\frac{7}{8}$ " Dia.

specified for replacement shall include the removal and

All connections on this sheet are primary.

steel shall be paid for under "Structural Steel Repair".

considered included in "Structural Steel Repair".

Pier 2 (N), Pier 3 (N&S), and Pier 4 (N&S).

holes, unless otherwise specified.

Concrete is to be removed and repaired where shown

The removal and replacement of specified structural

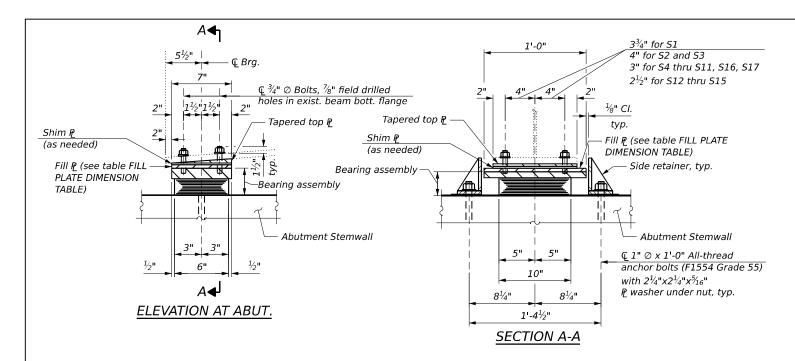
Repair details are shown at the east column of the south

Contractor to field verify all existing rivet sizes and bolt

with new components.

SECTION COUNTY 128 61 2018-067-BR WILL CONTRACT NO. 62M79

and $\frac{1}{2}$ " & $\frac{5}{16}$ " Fill Pe's To Remain



Notes:

Side retainers and stainless steel plates shall be included in the cost of "Elastomeric Bearing Assembly, Type I". Anchor bolts and side retainers at all supports shall be installed as each member is erected unless an equivalent temporary means of lateral restraint is used.

Existing seat elevations are based on the field survey conducted by American Surveying & Engineering, P.C. in 2019 and by Claassen, White & Associates, P.C. in 2022. The elevations from these surveys do not match elevations shown in reference drawing 38306-Sheet No.17. Contractor shall conduct in field surveys to determine the top of beam seat elevations, confirm the shown bearing thickness and submit results to the Engineer for review prior to ordering mat erial for bearing assembly and bearing plates. Heights of existing bearings shall be measured and recorded before jacking stringers. The actual amount raised shall be recorded at each bearing. The new bearing seat elevations shall be checked by measuring the height between the seat and bottom of stringer at the centerline of bearing and comparing it with the proposed bearing height plus raised dimension. If additional shim plates are needed they shall be included in "Jack And Remove Existing Bearings".

See Special provision for Jack and Remove Existing Bearings.

Anchor bolts shall be ASTM F1554 all-thread (or an Engineer-approved alternate material) of the grade(s) and diameter(s) specified. The corresponding specified grade of AASHTO M314 anchor bolts may be used in lieu of ASTM F1554.

Drilled and set anchor bolts shall be installed according to Article 521.06 of the Standard Specifications. The structural steel plates of the Bearing Assembly shall conform to the requirements of AASHTO M270 Grade 50.

Two $\frac{1}{8}$ " adjusting shims shall be provided for each bearing in addition to all other plates or shims and placed as shown on bearing details.

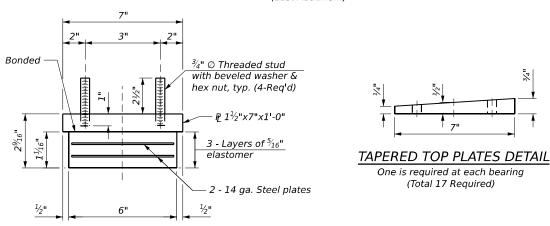
Cost of fill plates, tapered fill plates and field drilling included in "Furnishing and Erecting Structural Steel". Top plate to be installed where taper is parallel to the existing beam slope.

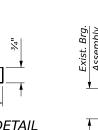
Jacking and removal of existing bearings shall not take place until existing deck has been removed. Contractor shall submit their Jacking and Removal of Existing Bearings plan for review and approval.

The new deck shall not be poured until the new bearings are in place.

TYPE I ELASTOMERIC EXP. BRG.

(East Abutment)





Exist. beam to remain Existing Fill Plate to be removed using the air-arc method and grind smooth all weld material remaining on the bottom flange

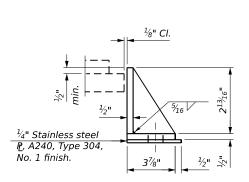
lack and Remove Existing Bearing per special provisions "Jack and Remove Existing Bearings." Jack capacity shall be a minimum of 6 tons per girder for steel beam wt. only

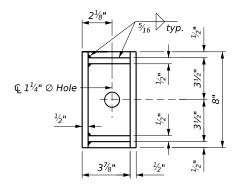
EXISTING BEARING REMOVAL DETAIL

(East Abutment) (17 Total)

BEARING ASSEMBLY

Shim plates shall not be placed under bearing assembly.





SIDE RETAINER

Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

EXIST. BEAM REACTIONS

(Includes single stringer steel dead load only)

	MEMBER	DL (KIP)
	51	6.4
*	<i>S2</i>	7.4
	53	7.1
	S4	4.0
	S5	3.8
	<i>S6</i>	3.7
	<i>S</i> 7	3.5
	58	3.4
	59	3.3
	S10	3.2
	511	3.2
	512	2.3
	513	2.2
	<i>S</i> 14	2.1
	S15	2.0
	<i>516</i>	2.6
	<i>517</i>	2.4
	S17A	1.3
	518	1.2

* Governing Service Dead Load Reaction

FILL PLATE DIMENSION TABLE

The fill plates in the table below are in addition to the tapered top plates (V.I.F.)

idition to the tapered top plates (v.i.r.)			
MEMBER	PROP. BRG. SEAT ELEV.	FILL PLATE THICKNESS	
<i>S</i> 1	547.67	-	
52	547.56	1/8"	
<i>S3</i>	547.74	3/8"	
54	547.74	1"	
<i>S5</i>	547.74	2 ³ / ₁₆ "	
<i>S</i> 6	547.74	33/8"	
<i>S7</i>	547.74	4%6"	
<i>S</i> 8	547.74	7/8"	
59	547.74	11/4"	
<i>S</i> 10	547.74	23/8"	
<i>S</i> 11	547.74	23/4"	
<i>S</i> 12	548.72	1/4"	
<i>513</i>	548.72	1½"	
<i>S</i> 14	548.72	25/8"	
<i>S</i> 15	548.72	37/8"	
<i>S</i> 16	548.72	1%16"	
S17	548.72	2 ⁷ /8"	
S17	548.72	27/8"	

BILL OF MATERIAL

ITEM	UNIT	TOTAL
lastomeric Bearing Assembly, Type I	Each	17
nchor Bolts, 1"	Each	34
ack And Remove Existing Bearings	Each	17

* REFERENCE DRAWINGS:

DRAWING Sheet No. Details of Anchorages 1345-2 38306-17 Bearing Details

Note: Portions of these drawings are extracted from original drawings for this bridge. Details shown are provided to illustrate the work to be performed.

* For Reference Drawings Key, see Sheet S-02.

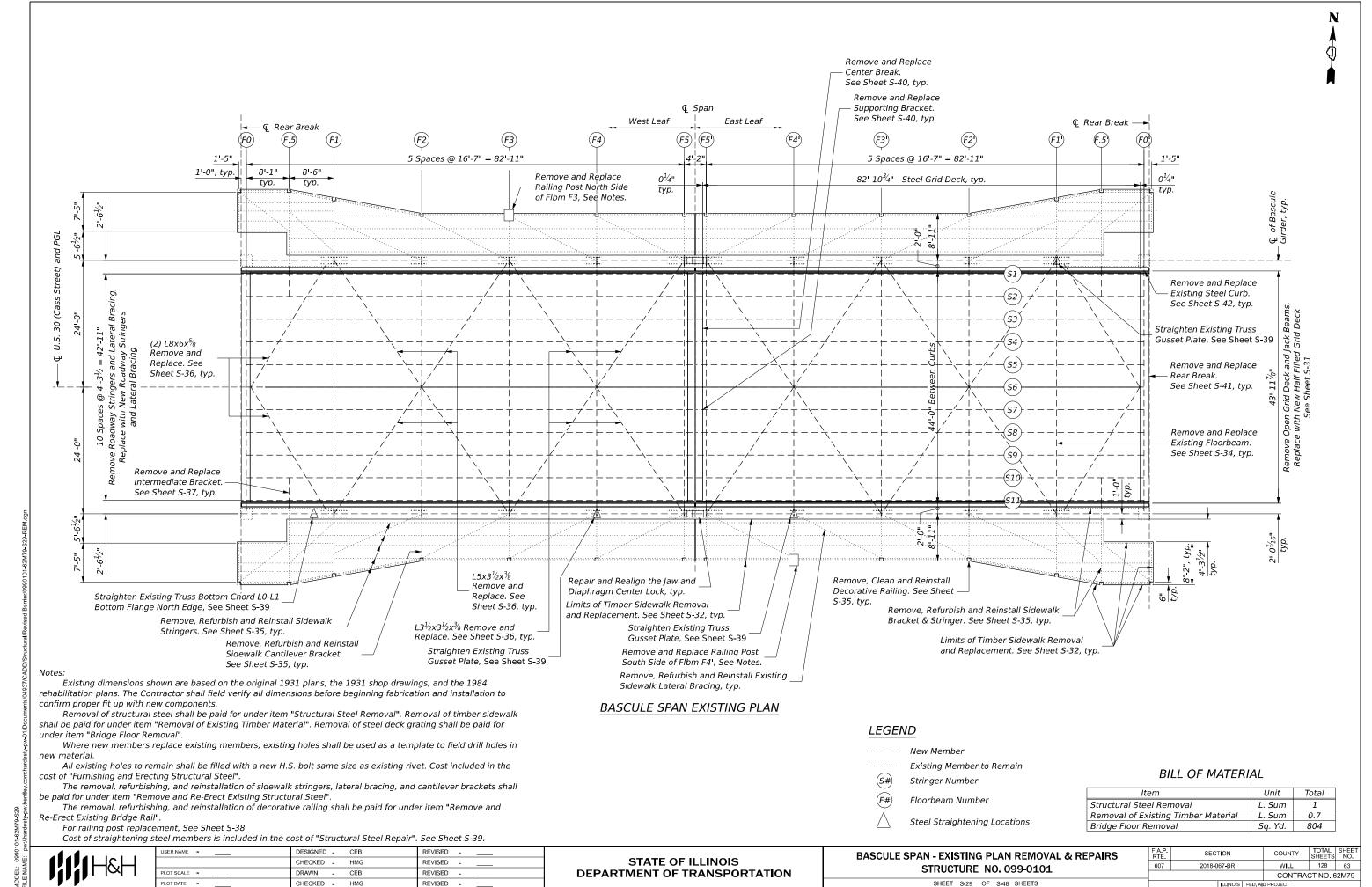


Jack And Remove Existing Bearing

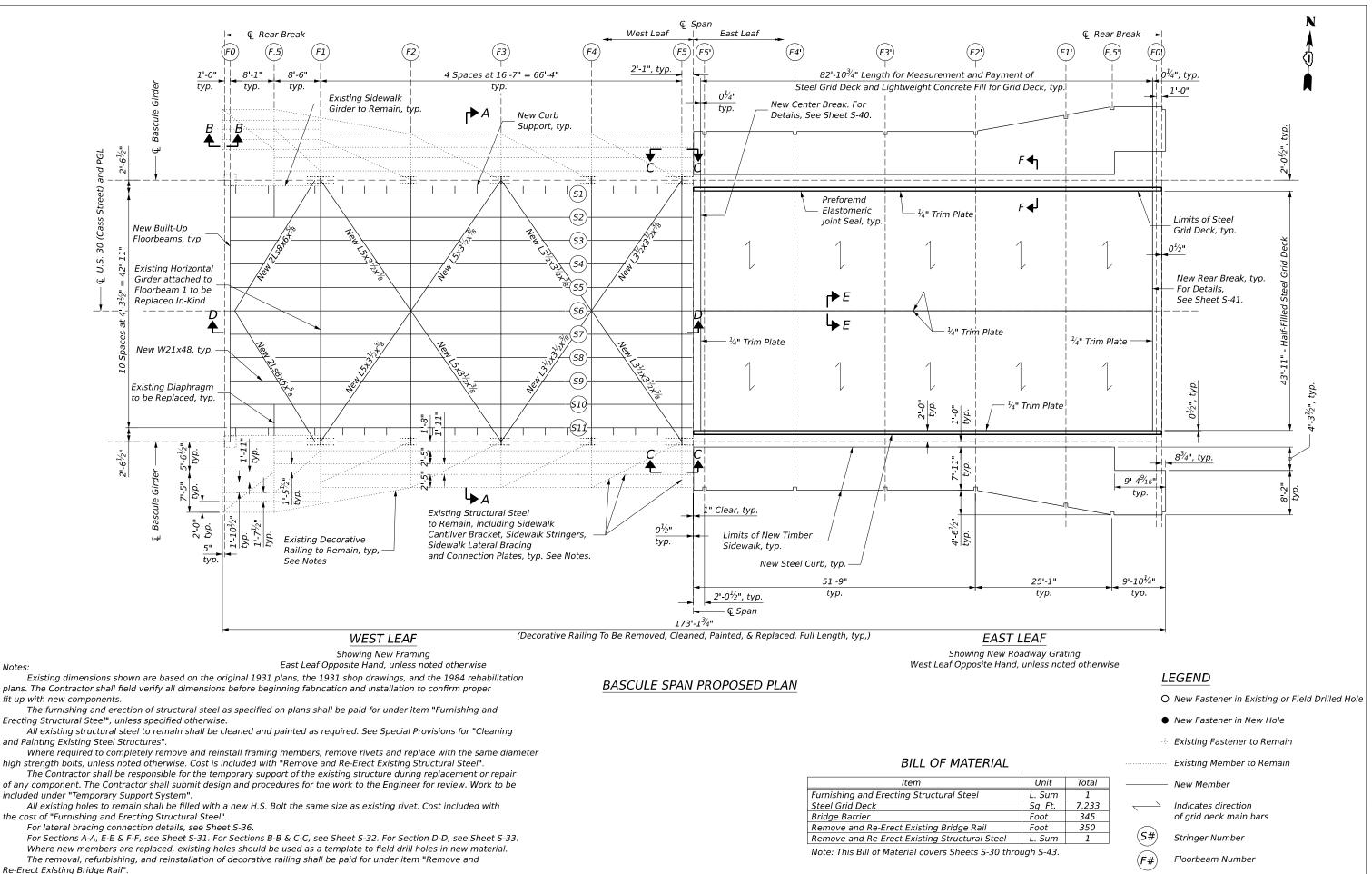


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APPROACH SPANS - BEARING REPLACEMENT	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 099-0101		2018-067-BR	WILL	128	62
			CONTRA	CT NO. 6	32M79
SHEET S-28 OF S-48 SHEETS		ILLINOID FED A	ID DDO IECT		



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DESIGNED - CEB

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

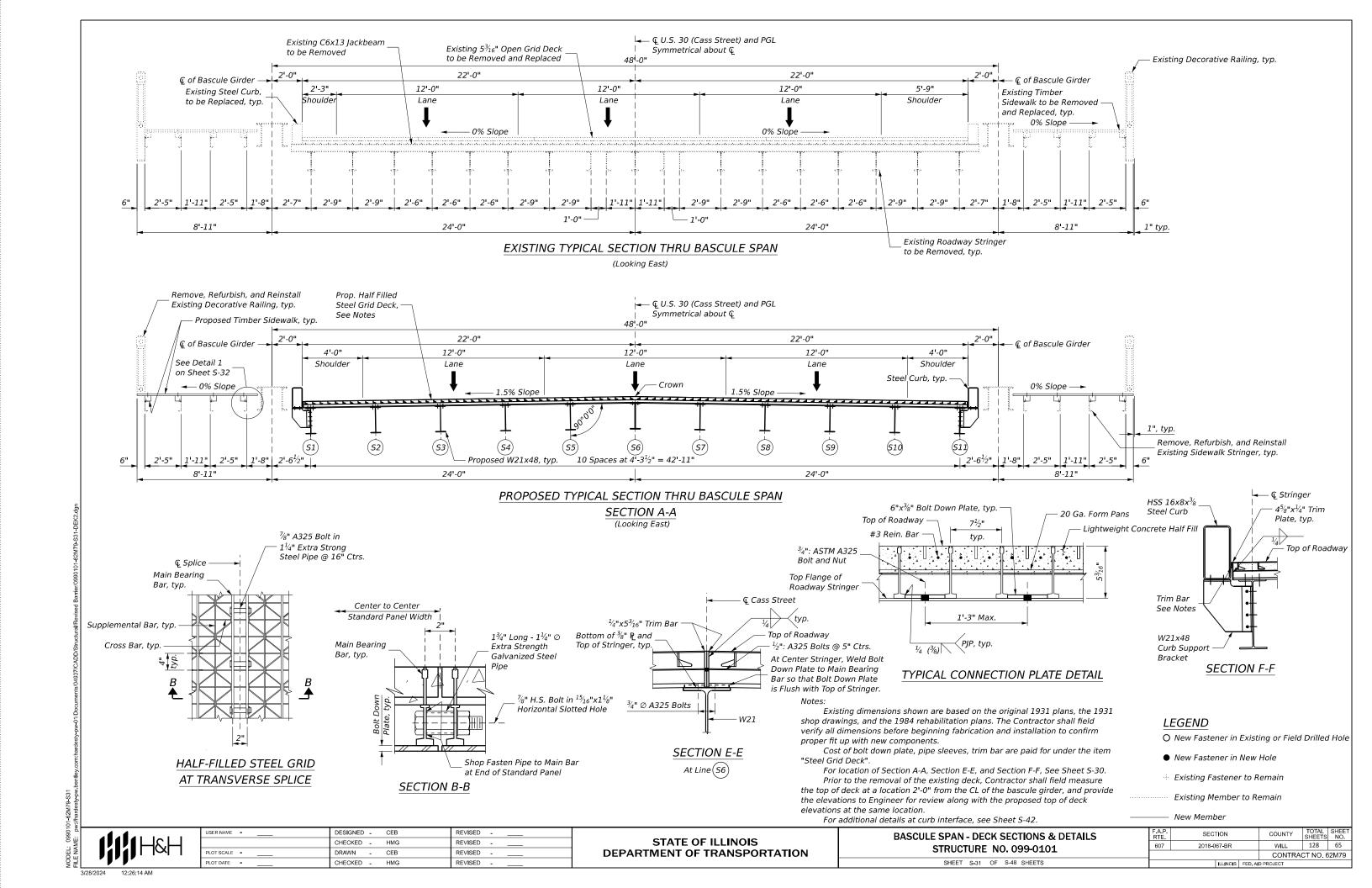
BASCULE SPAN - FRAMING & DECK PLAN STRUCTURE NO. 099-0101

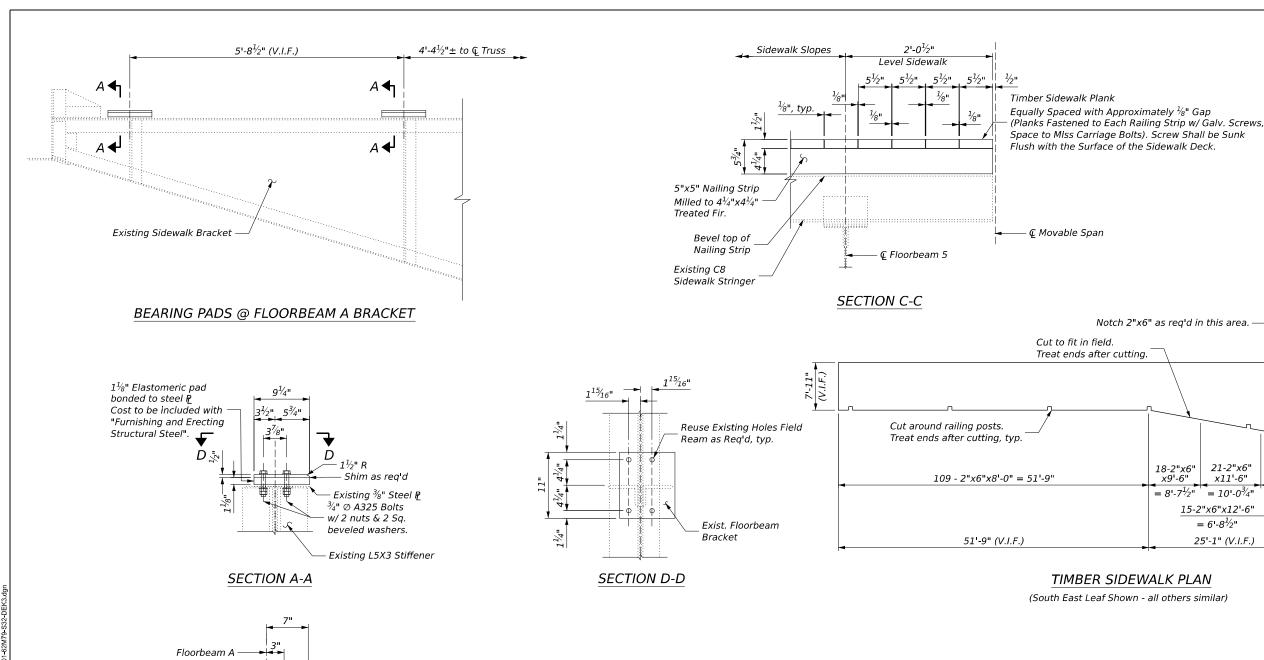
SHEET S-30 OF S-48 SHEETS

 A.P. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS NO.
 SHEETS NO.

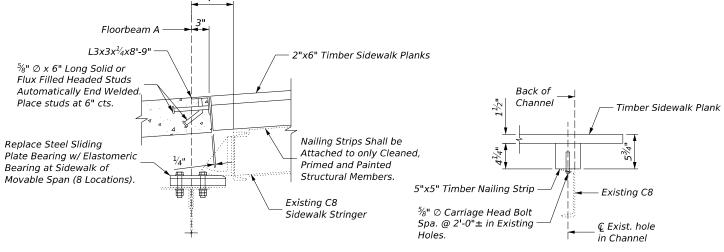
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 CONTRACT NO. 62M79





DETAIL 1



SECTION B-B SECTION THRU SIDEWALK BEARING Notes:

All lengths of 2"x6" given are suggested nominal lengths. Contractor may fabricate sidewalk with planks cut from any nominal length the Contractor chooses, so long as each plank used in sidewalk runs the full width of sidewalk.

Secure nailing strip to stringers with $\frac{5}{8}$ " \oslash carriage head bolts at 2'-0" \pm spacing using existing hole in channels. Beveled washers are required on channels. Bolts, screws, washers and their installation will be paid for under the item, "Hardware". Treated fir will be paid for under the item, "Treated Timber".

The material for the timber sidewalks shall be treated fir and shall have Fb=1,900 psi (min.).

Existing dimensions shown are based on the original 1931 plans, the 1931 shop drawings, and the 1984 rehabilitation plans. The Contractor shall field verify all dimensions before beginning fabrication and installation to confirm proper fit up with the new components.

For location of Section B-B and Section C-C, see Sheet S-30. For location of Detail 1, see Sheet S-31.

*REFERENCE DRAWINGS:

21-2"x6"

= 9'-6¹/₂"

9'-10¹/₄" (V.I.F.)

Drawing Sheet No.
Details of Struts & Brackets 630-7
*For Reference Drawings Key, see Sheet S-02.

Cut to fit in field.

Treat ends after cutting.

LEGEND

O New Fastener in Existing or Field Drilled Hole

New Fastener in New Hole

Existing Fastener to Remain

Existing Member to Remain

New Member

BILL OF MATERIAL

Treated Timber FBM	Total
	9,922
Hardware Pound	557



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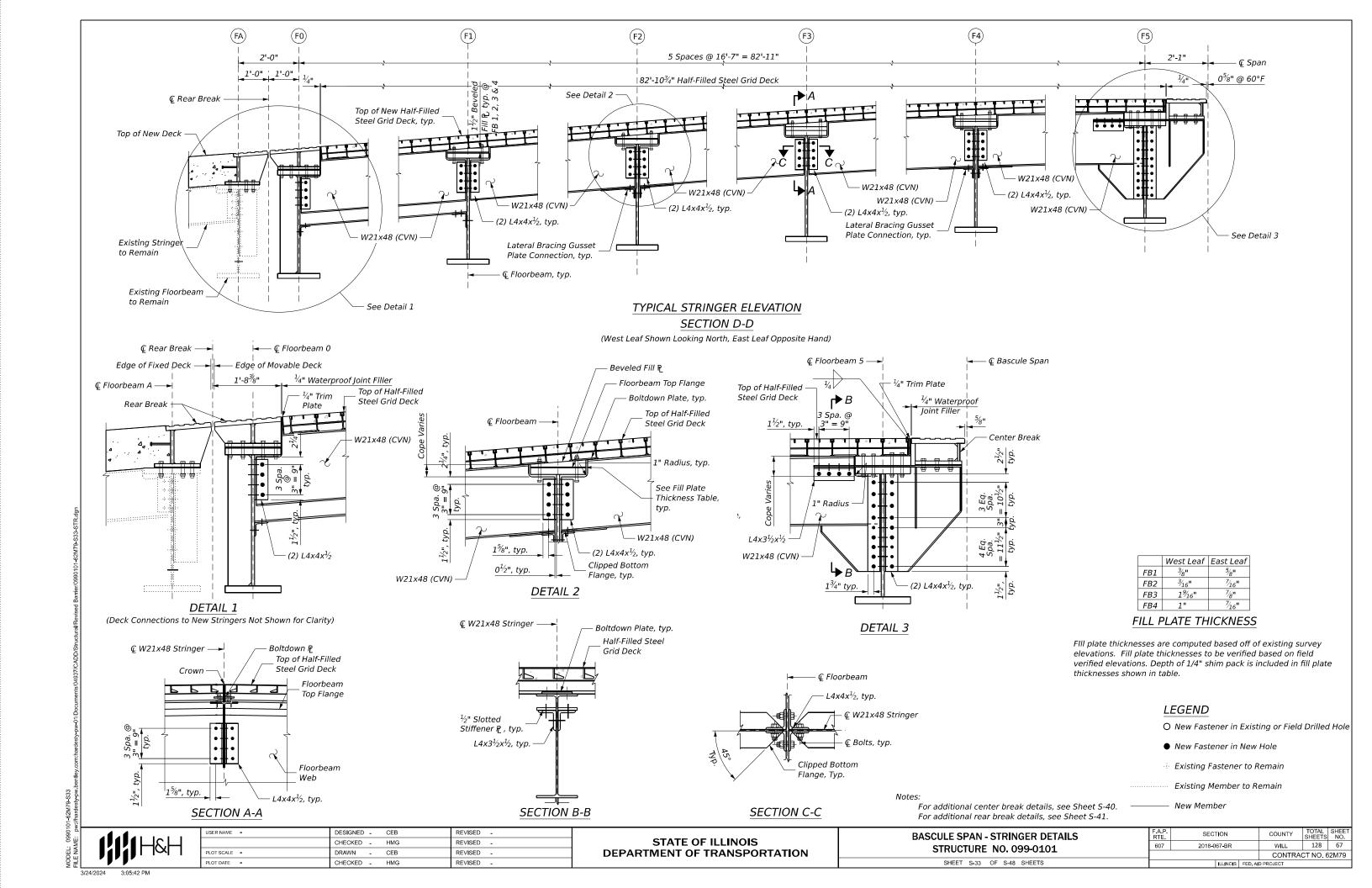
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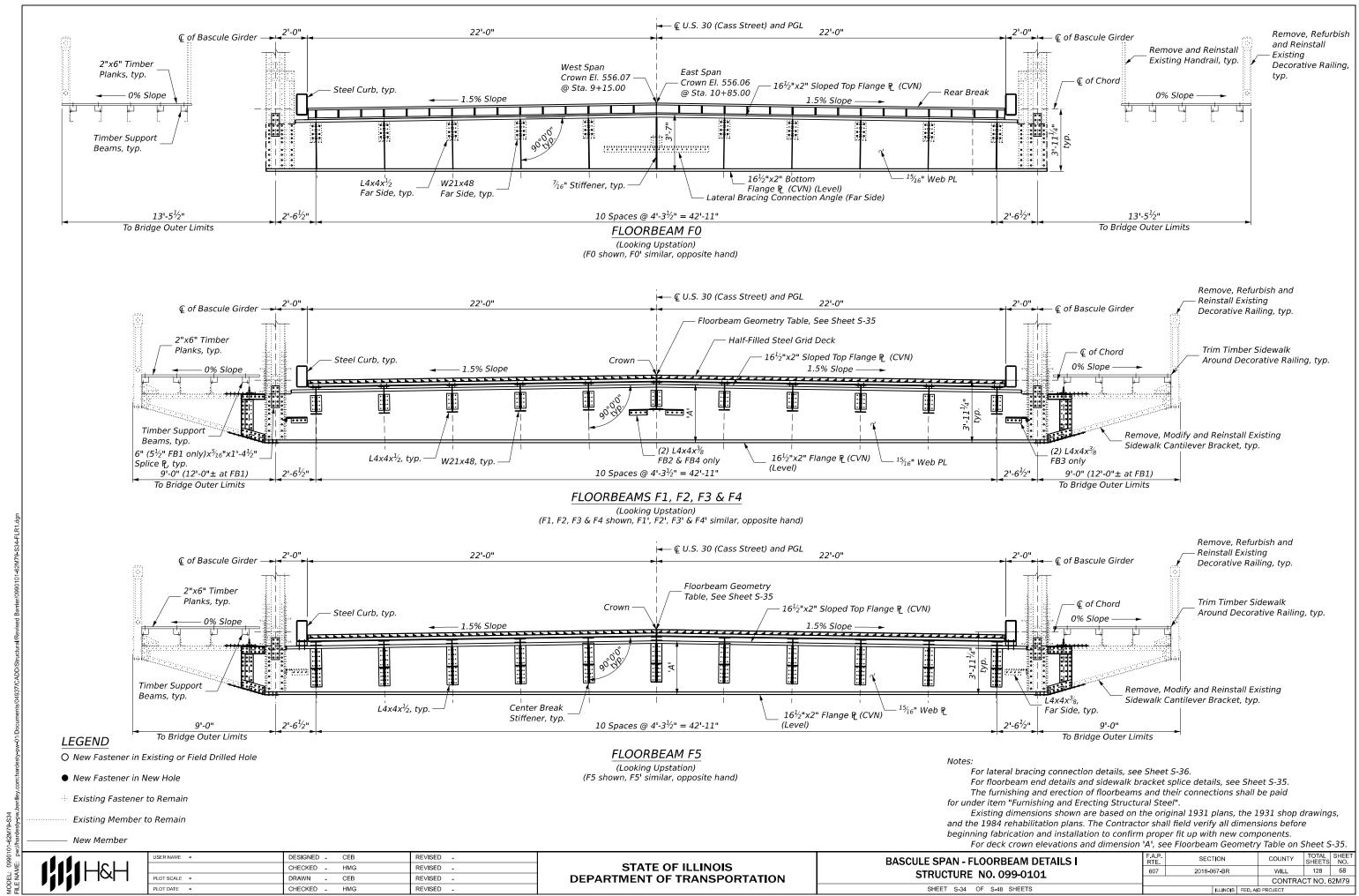
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BASCULE SPAN - SIDEWALK DETAILS STRUCTURE NO. 099-0101 A.P. SECTION COUNTY TOTAL SHEETS NO.

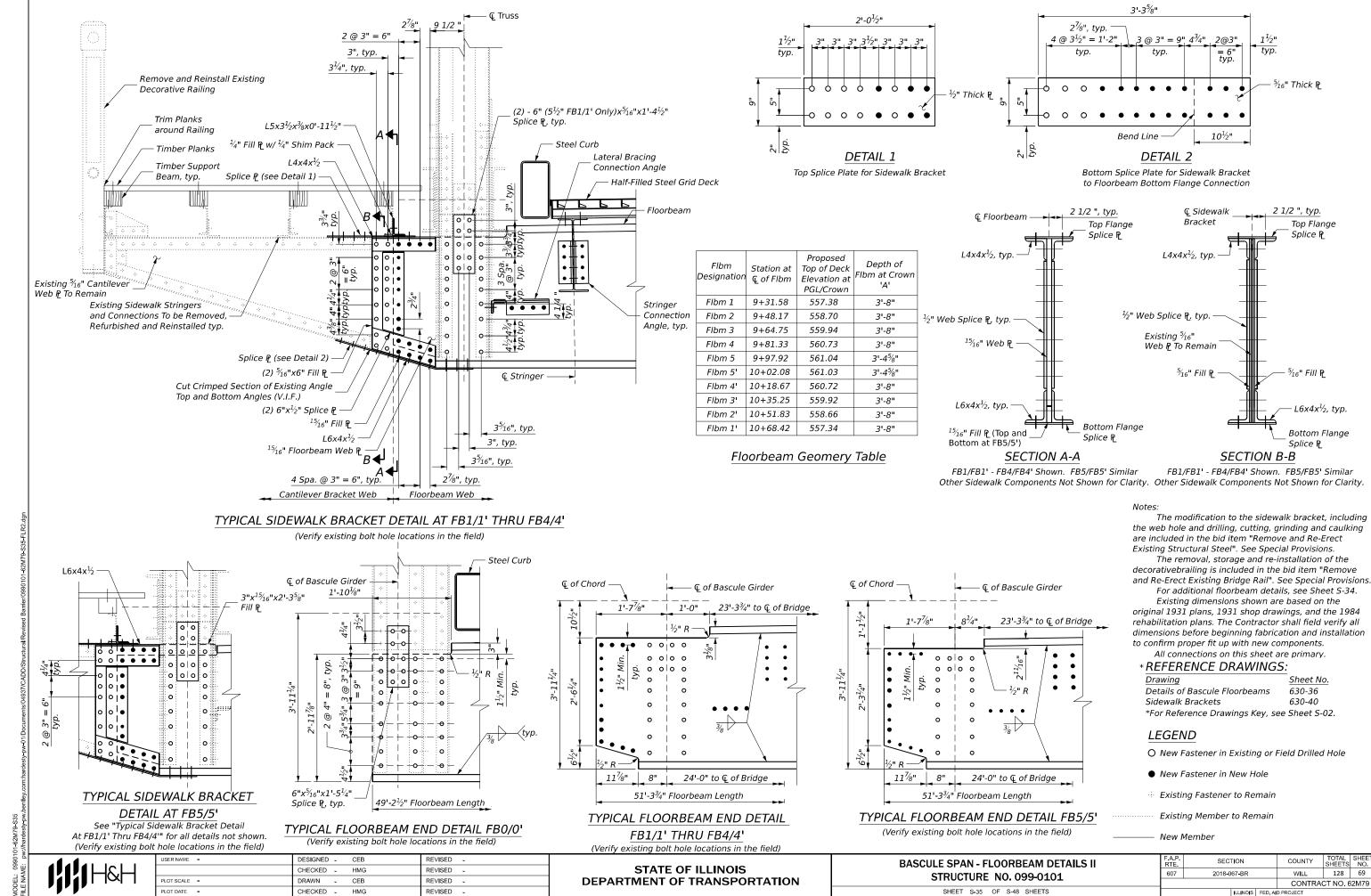
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CONTRACT NO. 62M79

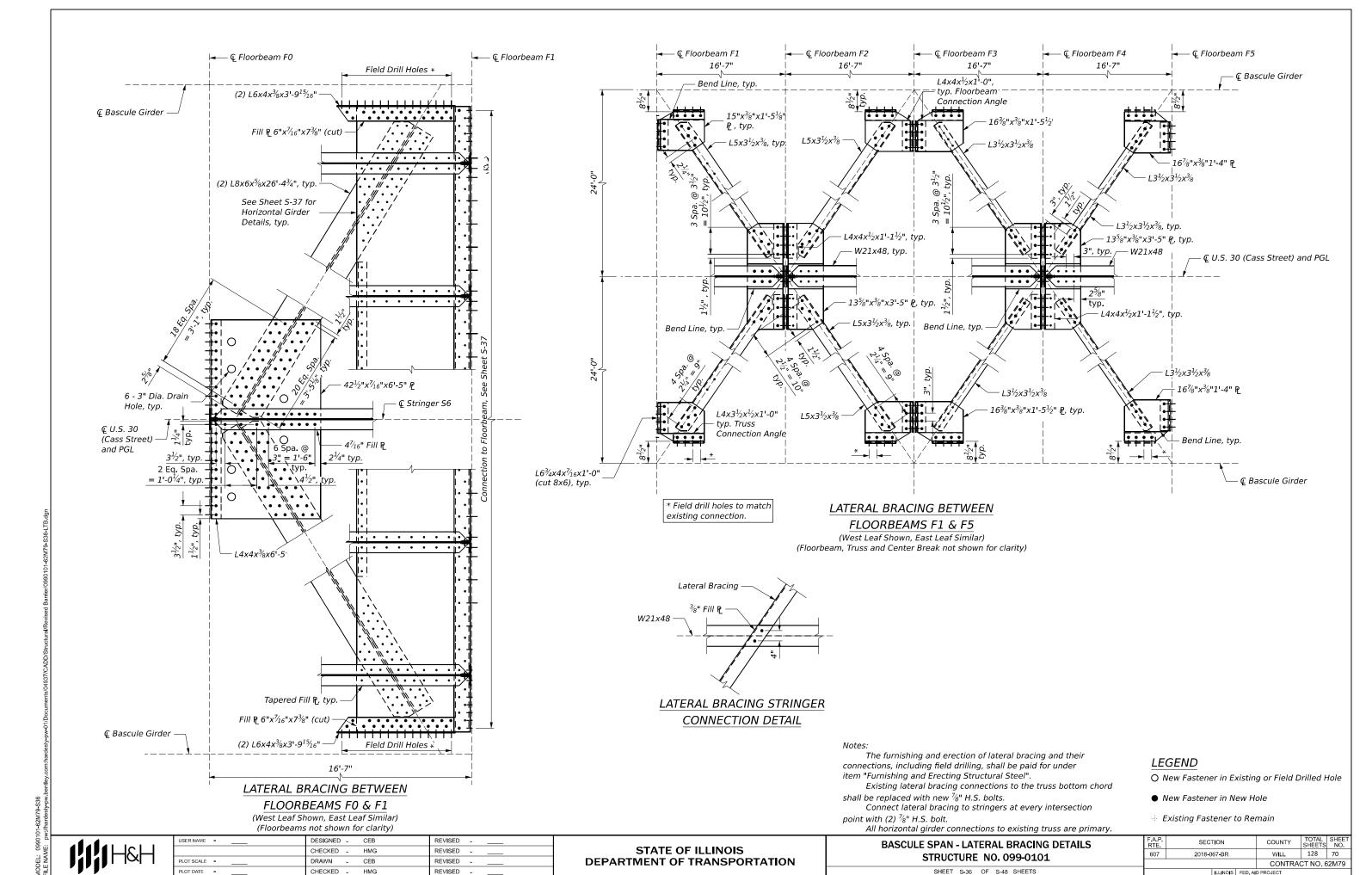




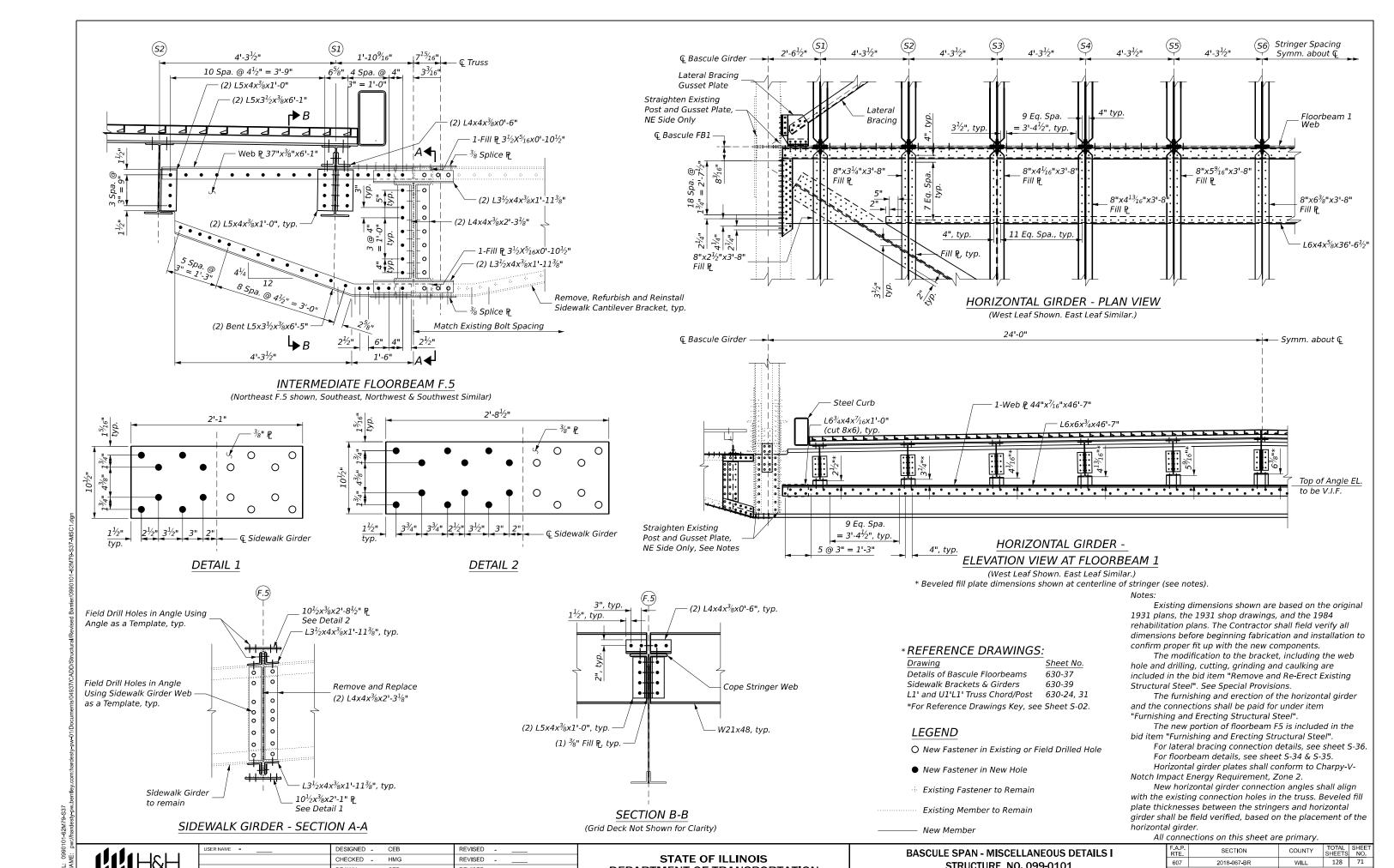
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STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

607

STRUCTURE NO. 099-0101

SHEET S-37 OF S-48 SHEETS

2018-067-BR

WILL

CONTRACT NO. 62M79

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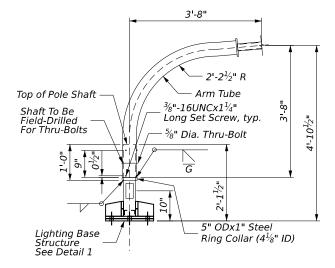
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$1\frac{1}{2}$ Dia. Anchor Hole, typ. To be Field Drilled. 4" O.D. Pipe Shaft 2" Dia. Opening in Base 🗗 ¾" Base ₽ ¾" Stiffener ₱, typ. $16"x4"x^{3/4}"$ Spacer P, typ.

© Segmental Girder Web Girder Web 2" Shear Backup Blocks, Seamental Girder See Notes, typ. Web ₽, typ. $1\frac{1}{8}$ " Dia. H.S. Bolt, to Existing Stiffener, typ. Match Existing Rivet Hole, typ. Existing 2" Web P, typ. Existing Connection to Shear Backup Blocks, typ. Install additional shear bearing blocks to provide a minimum of three rows Segmental Tread of bolts in double shear. Existing shear bearing block location and quantity of € Track connections shall be field verified.

LIGHTING POLE - DETAIL 1

SEGMENTAL GIRDER REPAIR DETAIL

 $\%_{16}$ " Square Core Holes for ½" Dia. Carriage Bolts

with Hex Nuts, top and

bottom rails, typ.

Drill 3⁹16" dia.

holes, typ.

SEGMENTAL GIRDER REPAIRS

LEGEND

O New Fastener in Existing or Field Drilled Hole

• New Fastener in New Hole

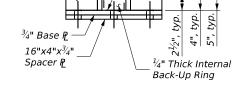
Existing Fastener to Remain

Existing Member to Remain

New Member

€ *U2* Lace Bars To Be Removed 3'-4" Existing Top Chord Existing Bent P

BASCULE LIGHTING POLE



LIGHTING BASE

PLATE - SECTION A-A

4" O.D. Pole Shaft (3³/₈" I.D.)

½" H.S.

Bolt, typ.

*REFERENCE DRAWINGS:

Drawing Sheet No. Track Girder Repair 38306 - 19, 20 38306 - 23 Segmental Girder Details Railing Post Details 38306 - 50

*For Reference Drawings Key, see Sheet S-02.

Existing dimensions shown are based on the original 1931 plans, the 1931 shop drawings, and the 1984 rehabilitation plans. The Contractor shall field verify all dimensions before beginning fabrication and installation to confirm proper fit up with the new components.

Light pole and base plate to be paid for under "Lighting Unit Complete, Special".

Existing rivet holes in the top chord from lacing bar connections removed during installation of the light pole base structures shall be filled with new $\frac{7}{8}$ " Dia. H.S. bolts as required.

For lighting details, see sheet LT-01 thru LT-06. All existing rivet holes outstanding from the removal of existing light fixtures on the bridge shall be filled with matching diameter H.S. bolts.

The Contractor shall field verify the location of existing shear backup blocks. New shear backup blocks shall be installed on either side of the segmental girder web plates to provide a minimum of three rows of shear capacity. Existing rivets shall be replaced with new 11/8" H.S. bolts. Cost of repairs shall be included in "Structural Steel Repair". Connections to segmental girder are primary.

The location shown for new high strength bolts attached to existing steel shall match existing holes. Contractor shall use existing post as a template for new

For replacement post locations, See Sheet S-29. Cost of replacing railing posts shall be included in "Structural Steel Repair".

BILL OF MATERIAL

Item		Unit		Total	
Stee	el Repair	Pound		2932	
A.P.	SECTION	COUNTY		DTAL	SHEET

55'-0" To ℚ Bridge Existing Top Chord **Q** Light Pole Splice & Fill P's See Notes Existing Gusset P

ELEVATION - RAILING POST REPLACEMENT

RAILING POST REPLACEMENT

U Z. -50P46

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BASCULE SPAN - MISCELLANEOUS DETAILS II STRUCTURE NO. 099-0101 SHEET S-38 OF S-48 SHEETS

C.POSTS-50P3 C-POSTS-50P7 6

8. Do -50P76

1-POST -50P6

1. 2 -50PK

Structural Steel SHEET NO. 607 2018-067-BR WILL 128 72 CONTRACT NO. 62M79

LIGHT POLE INSTALLATION - PLAN VIEW

LIGHT POLE INSTALLATION - ELEVATION VIEW

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

Remove and Replace

w/ ¾" Dia. H.S. Bolts

for $\frac{1}{2}$ " dia. Carriage Bolts

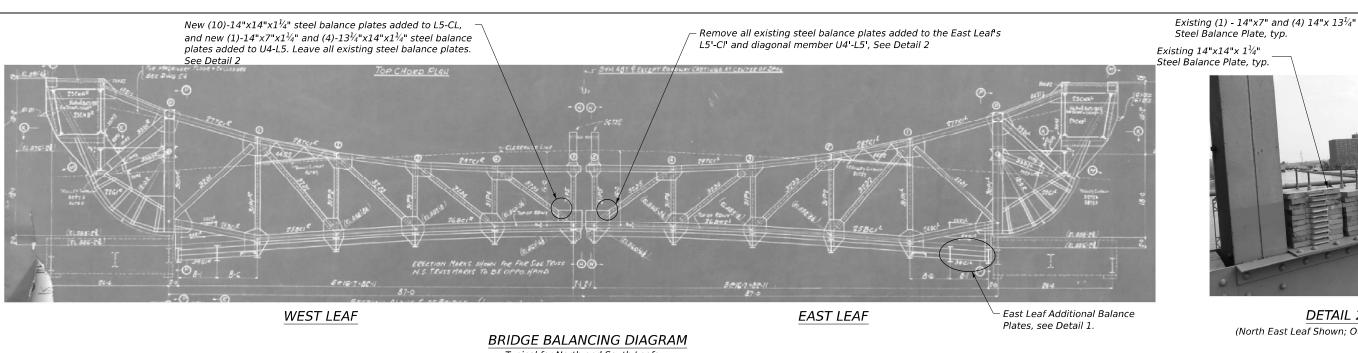
with hex nuts, top and

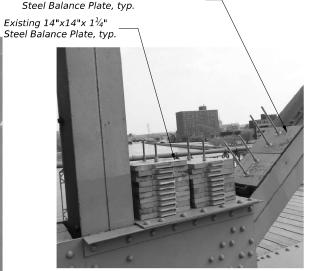
Remove and Replace 💹 🧏

Existing Railing Post

New $\frac{3}{4}$ " Dia. H.S. Bolts in New ^{13/}16" Dia. Drilled

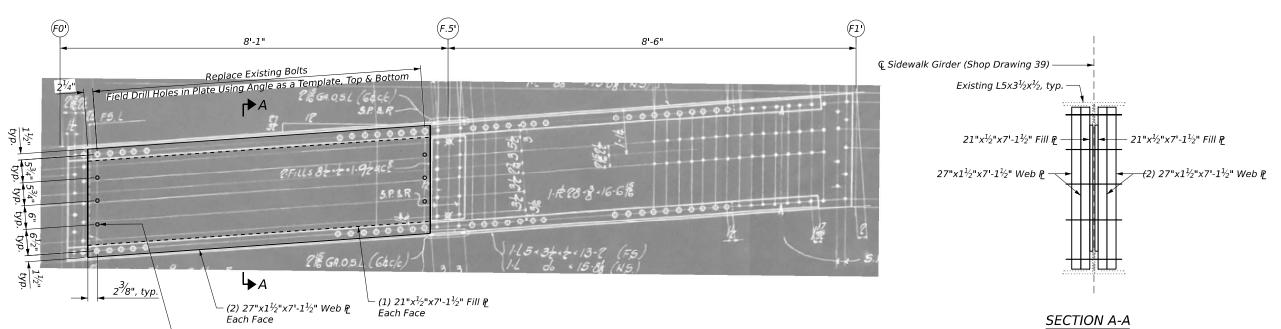
bottom rails, typ.





DETAIL 2 (North East Leaf Shown; Other Leaves Similar)

Typical for North and South Leafs.



DETAIL 1

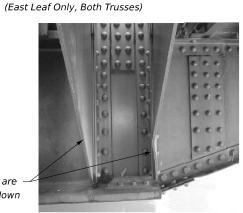
NORTH END OF FB1'

Southwest face of gusset is bent $\pm 3\frac{1}{2}$ " at point 1'-4" down from bottom lower chord.

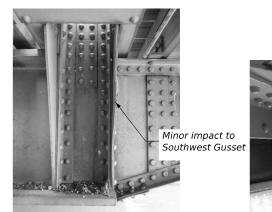
New A325 High Strength Bolt Field Drilled in New Web Plate, New Fill Plate, and

Existing Sidewalk Girder, typ.

West face of both gussets are bent $\pm 1\frac{1}{2}$ " at point 2'-3" down from bottom lower chord. See notes. GUSSET PLATE STRAIGHTENING



GUSSET PLATE STRAIGHTENING SOUTH END OF FB4



GUSSET PLATE STRAIGHTENING SOUTH END OF FB4'

BOTTOM FLANGE STRAIGHTENING LO-L1 OF SOUTH TRUSS WEST LEAF

STRUCTURE NO. 099-0101

SHEET S-39 OF S-48 SHEETS

*REFERENCE DRAWINGS:

Erection Diagram 630-F3 Sidewalk Backets & Girders 630-39 *For Reference Drawings Key, see Sheet S-02.

Existing dimensions shown are based on the original 1931 plans, the 1931 shop drawings, and the 1984 rehabilitation plans. The Contractor shall field verify all dimensions before beginning fabrication and installation to confirm proper fit up with the new components.

The modification to the sidewalk girder on the East leaf including the additional web and fill plates and field drilling shall be included in the cost for "Bridge Balancing".

The removal and addition of existing steel balance plates shall be included in the cost for "Bridge Balancing". Methods of straightening shall meet the requirements of Special Provision "Structural Steel Repair".

BASCULE SPAN - MISCELLANEOUS DETAILS III

N. edge of bottom flange

pent up to $\frac{3}{4}$ " at point

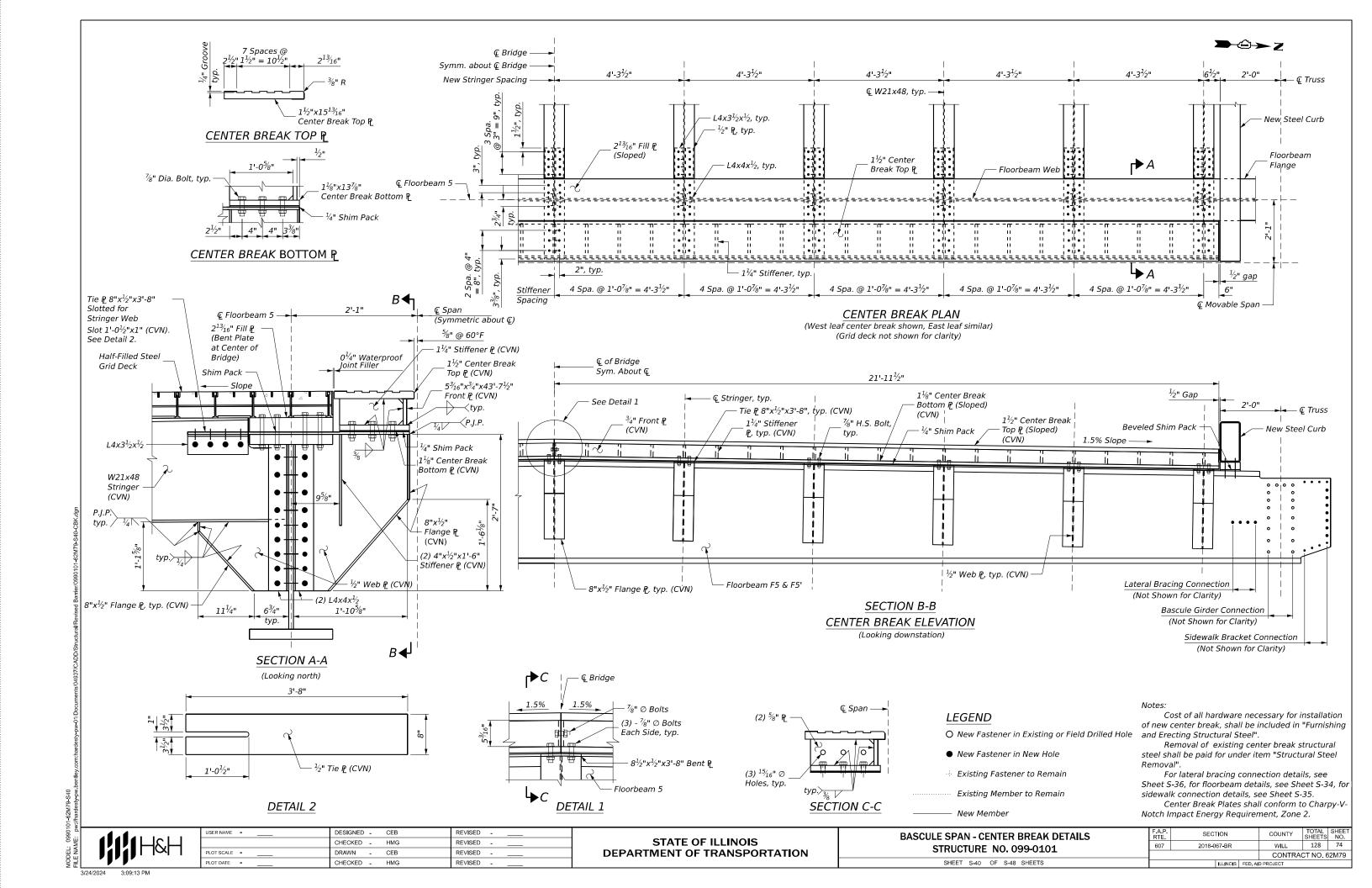
±46" W. of FB1W Q.

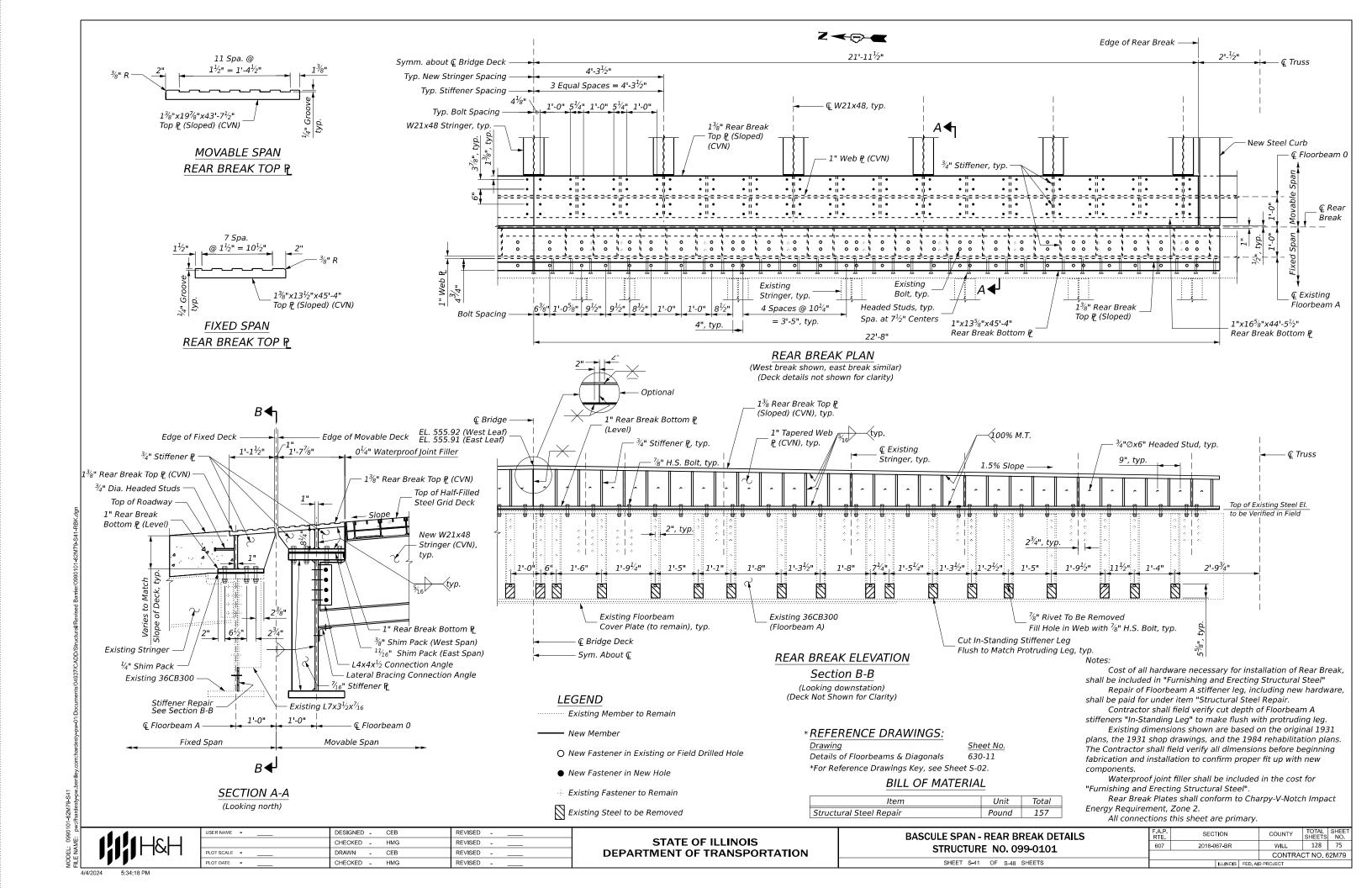
See notes.

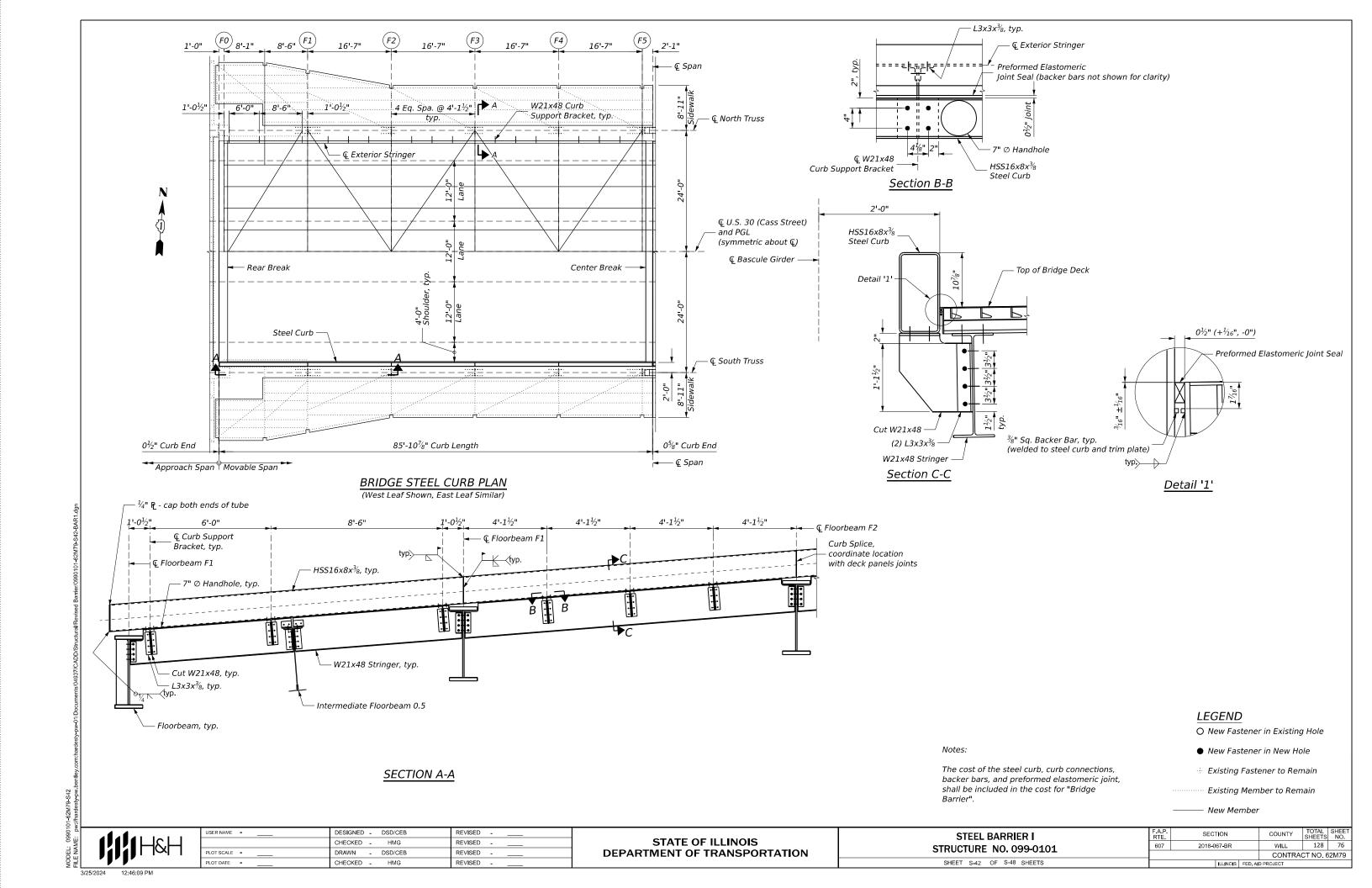
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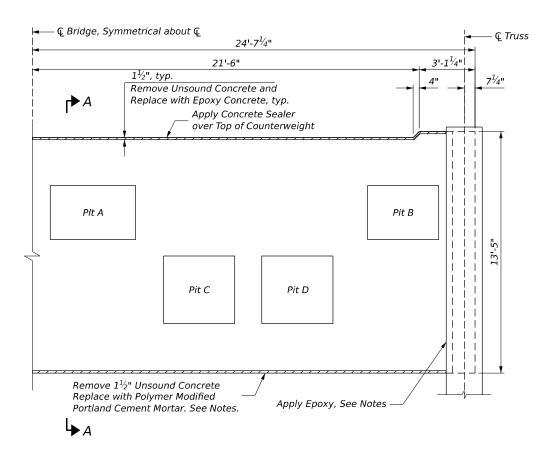
CONTRACT NO. 62M79

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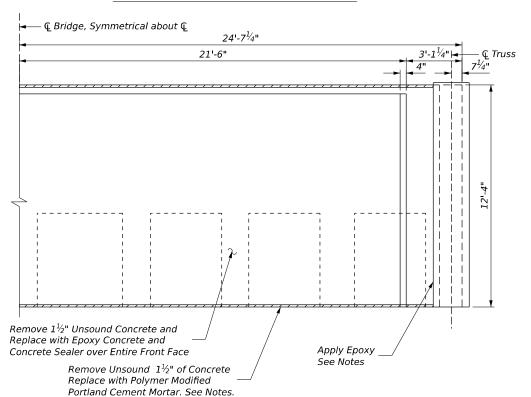




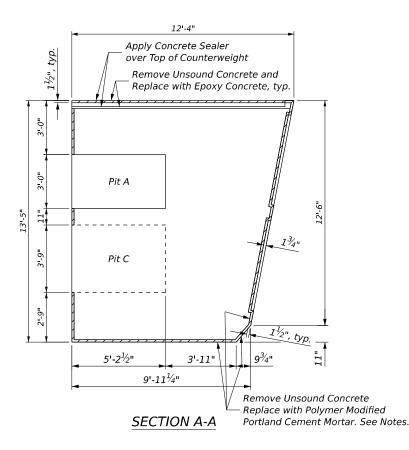


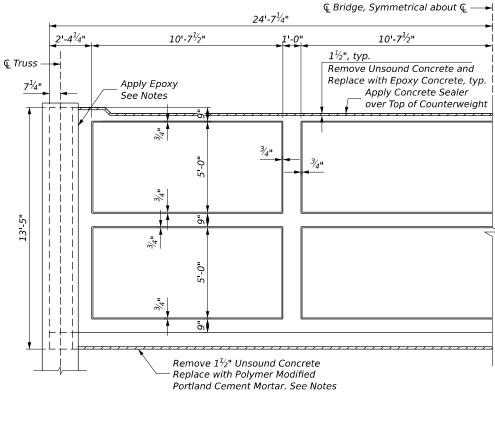


FRONT ELEVATION COUNTERWEIGHT



TOP PLAN COUNTERWEIGHT





REAR ELEVATION COUNTERWEIGHT

Polymer Modified
Portland Cement
Mortar

Expansion Insert

Sign Dia. Exp. Anchor Bolts
12" Max. Grid Spacing for
Sides and Bottom
24" Max. Spacing for Top.
(Estimated 25 SF of Repair on East

Counterweight and 15 SF on West

Counterweight)

Patch Hole in Counterweight Pocket Door East Span, South Top Pocket, with 11x 8½"x¾" Plate, Weld Around, See Notes

DOOR REPAIR DETAILS

LEGEND

Concrete Removal and Repair Limits

Note:

Seal between all steel and concrete along flanges to a minimum depth of 4" with epoxy. See Special Provisions.

The depth of concrete removal and repair is a minimum of $\frac{1}{2}$ " below the finished dimensions. In areas where the depth of repair greater than $1\frac{1}{2}$ ", welded wire fabric and anchor bolts will be required as shown on the plans (See Detail 1). All anchor bolts shall be stainless steel. All welded wire fabric shall be epoxy coated. Engineer shall be notified if depth of repair exceeds 2".

Removal of unsound concrete, new polymer modified portland cement mortar, new welded wire fabric, new anchor bolts, epoxy injection, and concrete sealer are included in the bid item "Counterweight Repair Materials". See Special Provisions.

The polymer modified portland cement mortar is to be applied in lifts as specified by the manufacturer. The Contractor may form up the areas where the depth of repair is greater than $1^{1}\!\!/_{2}$ " and add $^{3}\!\!/_{8}$ " pea gravel to the polymer mortar. The maximum amount of pea gravel that can be added is 40% by weight or as per manufacturers specifications.

Repair of East span's South top pocket shall be included in the cost of "Structural Steel Repair".

BILL OF MATERIAL

Item	Unit	Total
Counterweight Repair Materials	L. Sum	1
Structural Steel Repair	Pound	10



STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

COUNTERWEIGHT REPAIRS
STRUCTURE NO. 099-0101

SHEET S-43 OF S-48 SHEETS

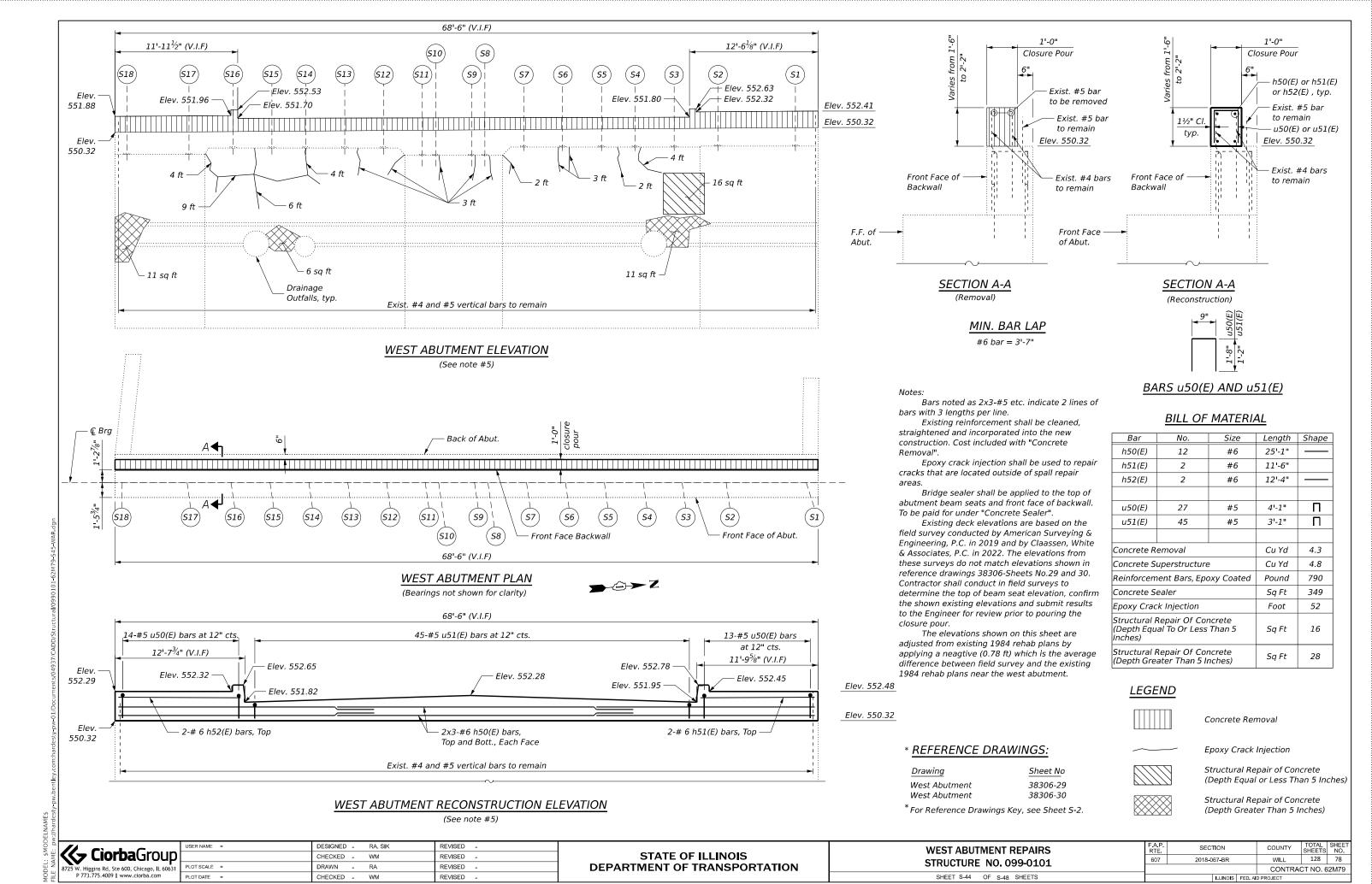
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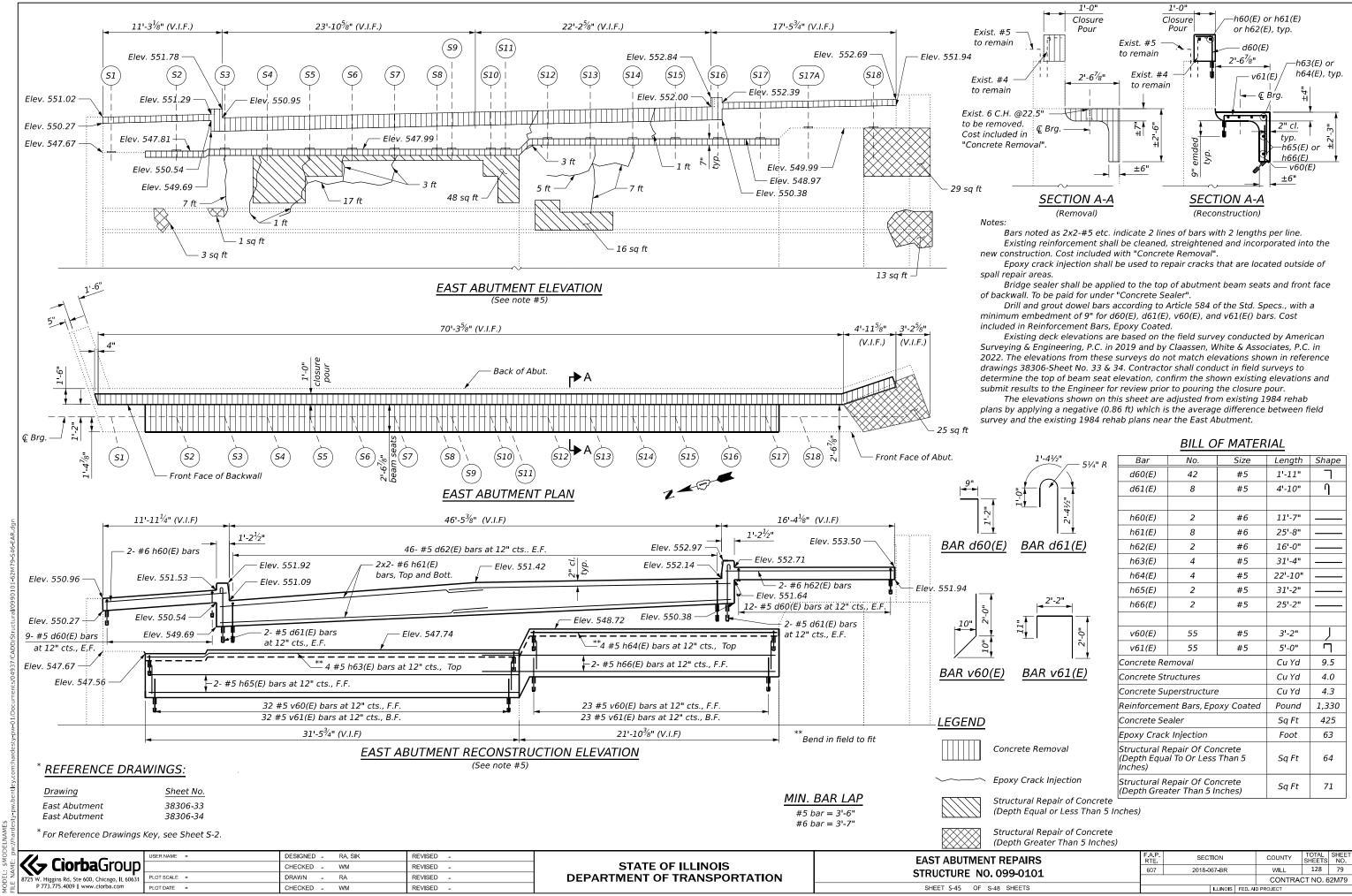
 CONTRACT NO. 62M79

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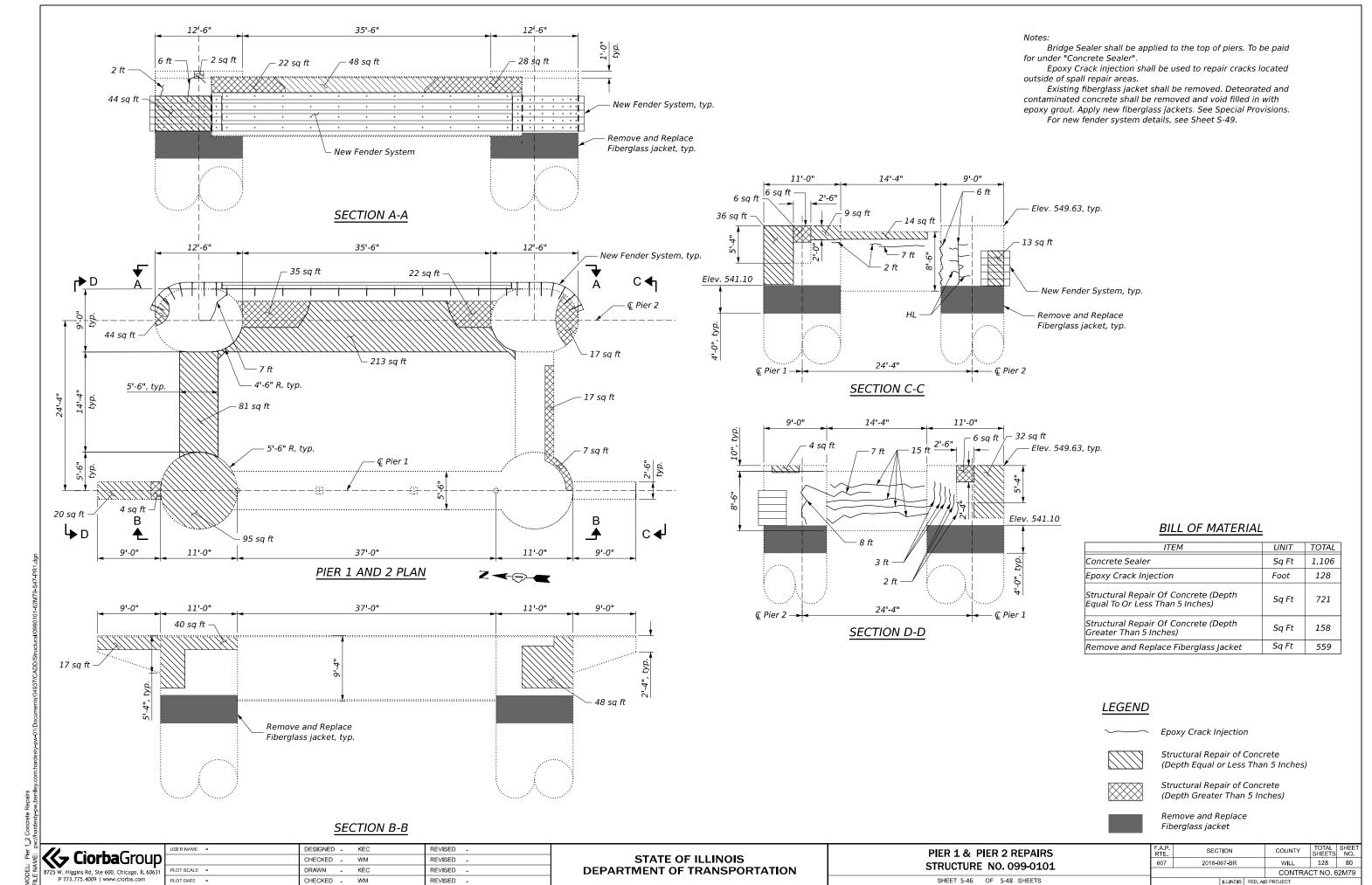
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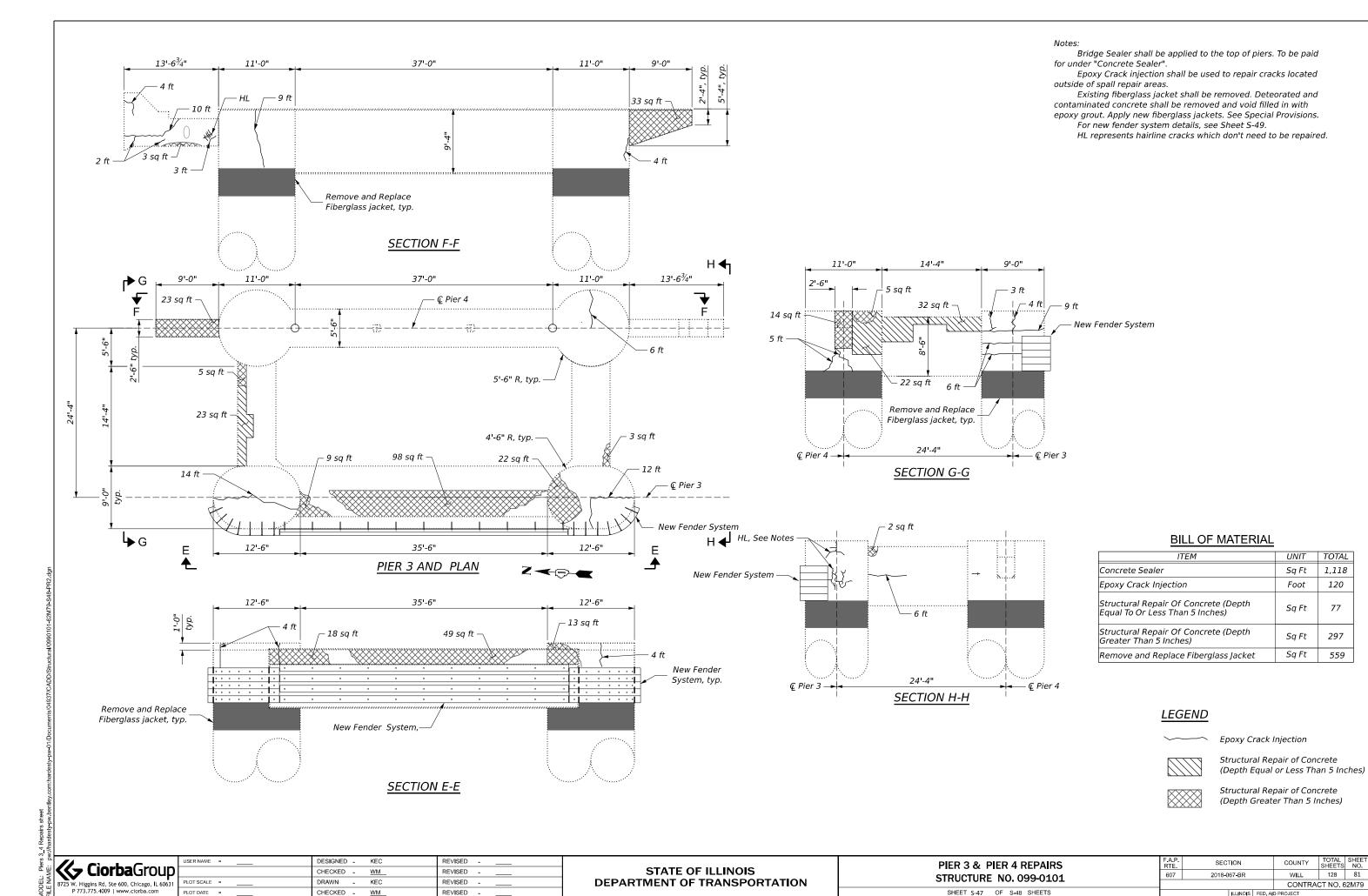


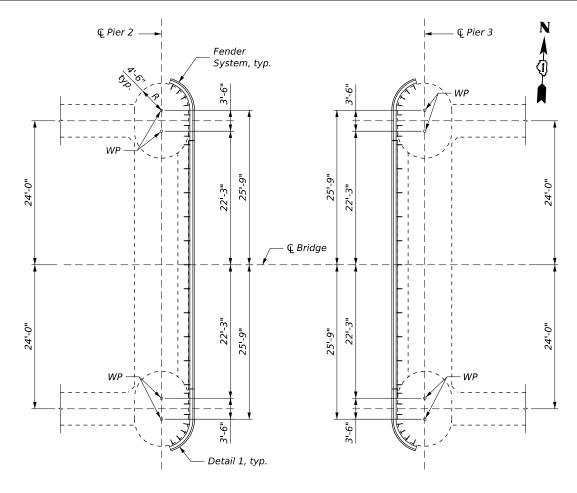
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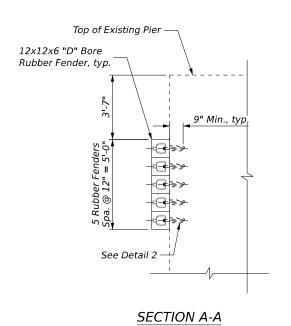


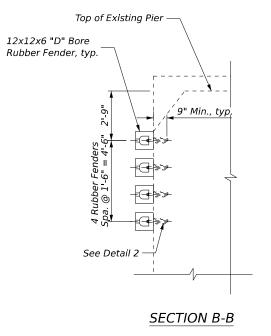
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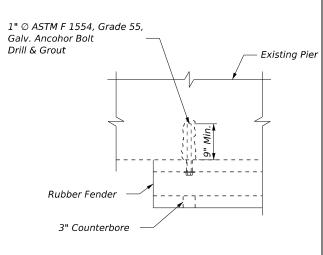






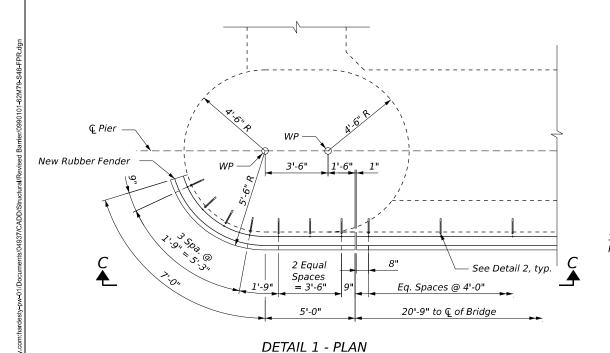






DETAIL 2

PLAN FENDER SYSTEM



(Pier 2, South Side Shown. Pier 2 North Side and Pier 3 Similar) 12x12x6 "D" Bore Rubber Fender, typ.

B

B

B

CEI. 549.63

B

EI. 548.80

D

CEI. 548.80

D

Notos

All hardware: bolts, nuts, washers, and plate washers shall be galvanized in accordance with AASHTO F 2329. Bridge structure not shown for clarity. Existing anchor bolt holes shall be filled in with non-shrink

Existing anchor bolt holes shall be filled in with non-shri grout, see "Removal of Existing Timber Material".

Cost of grout and hardware necessary to install new fender system shall be included in "Fender System".

Existing dimensions shown are from existing drawings. The Contractor shall verify all existing dimensions prior to ordering any material.

BILL OF MATERIAL

Item	Unit	Total
Fender System	L. Sum	1
Removal of Existing Timber Material	L. Sum	0.3

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FENDER PROTECTION
STRUCTURE NO. 099-0101

SHEET S-48 OF S-48 SHEETS

A.P. SECTION COUNTY TOTAL SHEETS NO.

507 2018-067-BR WILL 128 82

CONTRACT NO. 62M79

General Notes

- Refer to the existing shop drawings and as-built drawings, which are furnished upon request from Illinois Department of Transportation.
- Shims must be 1/2" nominal thickness, unless otherwise specified, with adjustment variations as described in the Special Provisions.
- Machinery dimensions shown on plans are minimum dimensions after machining.
- Fill any open holes resulting from removal of existing machinery with new ASTM Grade A325 bolts. This is considered incidental to the mechanical work.
- Machinery dimensions shown on drawings are dimensions after machining. Unless otherwise indicated or required for the proper assembly of parts, dimensional tolerances for machinery in general are as follows:

- 0.005 Straightness: (Per linear foot) 0.005 Flatness: // 0.005 -Parallelism: 1 0.005 -Perpendicularity: (Per linear foot) ∠ 0.020 -Angularity: (Degrees per linear foot) ⊕ 0.005 -Position: (Features within a component) ◎ 0.005 -Concentricity: / 0.005 -Circular Runout:

- Model numbers and details of motors, couplings, and other standard components are based on manufacturer's catalog data current at the time the plans were prepared. Equivalent models from other manufacturer's may be substituted at the option of the Contractor and with the approval of the Engineer. All related structural, mechanical, architectural, and electrical details are to be revised by the Contractor to suit the certified dimensions of the components actually furnished at no additional cost to the department. Mention of a manufacturer's name or model number does not represent a preference, but is used to set a standard.
- Machine all new mating surfaces of machinery parts, supports and external edges. Blend all transitions of surfaces of machinery parts.
- Detail fasteners that require tapped holes with a minimum thread engagement of 1.5 times the nominal thread diameter unless otherwise noted. Detail countersunk fasteners with a minimum of a 1/16" recess.
- Detail and machine the edges and corners of all machinery parts with suitable fillets and chamfers. In general, the minimum edge or corner, radius or chamfer must be 1/8" if the part thickness is less than 1" and 1/4" if equal to or greater, unless otherwise noted. In the case of mating parts, allowance must be made for the proper fit and assembly. Show such details in the shop drawings.
- 10. Provide machinery covers as indicated by dashed lines and as per the general machinery specification. Provide all mounting hardware and fasteners as required. Configure the mounting hardware using plate (straight or bent), angles and channels with a minimum thickness of 1/2" diameter (2 per support connection). The support is to be rigid and resist movement during span operation. Maximum spacing of supports shall be 2 feet. Sumbit all machinery cover details on shop drawings for approval.
- 11. Verify all field sensitive dimensions for proper coordination with supports.
- 12. Weldments to be fabricated per the requirements of the structural steel specifications, with supplemental requirements as per the mechanical specifications and drawings.
- 13. All dimensions for machine finished surfaces shall be held to plus/minus 0.010°, except as otherwise required by the plans or specifications.
- 14. Machine all surfaces of forgings to the dimensions shown on plans.

General Notes (continued)

15. The machinery fits and finishes are as follows, unless otherwise noted:

Surface Description	Fit (ANSI)	Finish (Microinches)
Machinery Base on Steel		125
Machinery Base on Masonry		500
Machinery Supports		125
Machinery Parts in Fixed Contact		125
Shaft Journals	RC6	8
Journal Bushing	RC6	16
Split Bushing in Base	LC1	125
Solid Bushing in Base (To 1/4" Wall)	FN1	63
Solid Bushing in Base (Over 1/4" Wall)	FN2	63
Hubs on Shafts (To 2" Bore)	FN2	32
Hubs on Shafts (Over 2" Bore)	FN2	63
Turned Bolts in Finished Holes	LC6	63
Keys and Keyways	B17.1, CL2	63

Note: The above fits and finished for cylindrical parts must also apply to the major dimensions of non-cylindrical parts.

Unless otherwise indicated or required for the proper assembly of parts, dimensional tolerances for machinery in general must be as follows:

Surface	Tolerance		
Machined (To 1")	+/- 0.015"		
Machined (Over 1")	+/- 0.030"		
Rolled	+/- 0.030"		
Non-Machined Cast (to 1")	+/- 0.030"		
Non-Machine Cast (Over 1")	+/- 0.060"		
Component Locations	+/- 0.030"		
Bolt Hole Locations	+/- 0.030"		
Angular	+/- 0.5 Deg		

- 17. All transitions of surfaces of machinery parts are to be blended in smooth. Machine all mating surfaces of machinery parts and supports.
- 18. Contractor to field verify dimensions before shop drawing submission.

Machinery Bolts

- 19. Furnish and install positive locks for all nuts which are to be torqued to less than 70% proof. Double nuts or jam nuts shall be used except for tapped holes which shall use SS safety wire for hex head bolts and permanent thread locking fluid for countersunk bolts, unless otherwise noted.
- 20. All high strength (H.S.) bolts shall be installed with a hardened plain washer meeting ASTM F436 where shown on the drawings but at a minimum under the turned element.
- 21. See specifications for detailed definition of bolt types. Unless otherwise noted, the four main types of bolts used for machinery bolts in these drawings are H.S. bolts, turned bolts, finished body bolts (FBB), and countersunk (CSK) bolts:

ASTM F3125 Grade 325 Bolt and 1/16" hole clearance for all bolts 1/2" or larger, ASTM A449 -H.S. bolts: or SAE 1429 Grade 5 Cap Screws and 1/32" hole clearance for all bolts under 1/2". Pretension to slip critical requirements

-Turned bolts: ASTM A449 Bolt U.O.N. with fit and finish in accordance with the fits and finishes table on this sheet. Torque to snug tight.

ASTM A449 Bolt or SAE J429 Grade 5 Cap Screws U.O.N., no more than 0.01" clearance U.O.N. -FBB: Pretension to clip critical for permanent connections and to 70% proof for reusable

connections unless otherwise required by the manufacturer or these drawings. Socket Flat Countersunk Head Cap Screws conforming to ASTM D879 (Stainless Steel) for

diameters less than 5/8" and ASTM F835 (Alloy Steel) for diameters greater than or equal to 5/8" U.O.N.

Basis of Design

- The design of the machinery systems conforms to the applicable requirements of the 2007 AASHTO LRFD Movable Highway Bridge Design Specifications, 2nd Edition, with interim revisions through 2018, unless noted otherwise.
- Machinery loading conforms to AASHTO 1988 Condition B loading with two motors and Condition A with one motor.

Loading Condition A: 2.5 psf Wind Loading Condition B: 2.5psf Wind + 2.5 psf Ice

- Max load at rack not to exceed 151,000 lb. at 200% Full Load Motor Torque.
- The mechanical systems are design for the span to be normally left in the lowered (seated) position.



Expires 11/30/2025

BILL OF MATERIAL

Item	Unit	Total
Live Load Bearings	Each	4
Junction Box, Stainless Steel, Attached to Structure, 6"x6"x4"	Each	4
Junction Box, Stainless Steel, Embedded in Structure, 8"x8"x6"	Each	4
Electrical Equipment Removal and Salvage	Each	18
Miscellaneous Electrical Work	L. Sum	1

Note: This Bill of Material covers sheets M01 through M02 and E01 through E21.

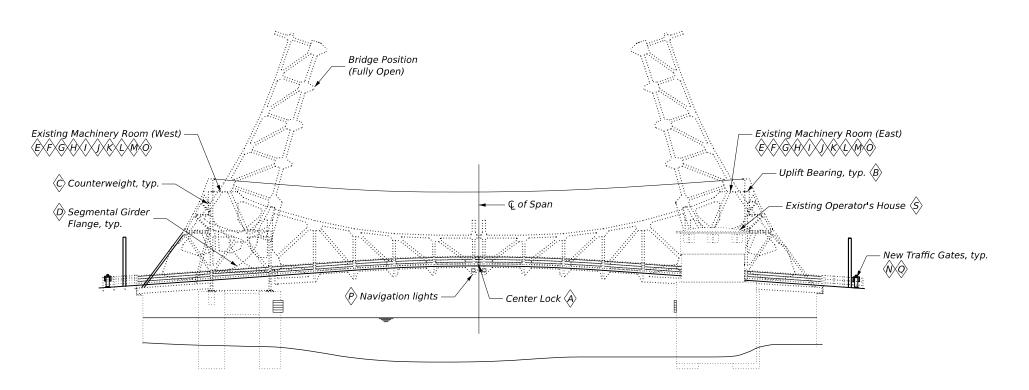
See individual sheets for additional pay items pertaining to work shown on each sheet.

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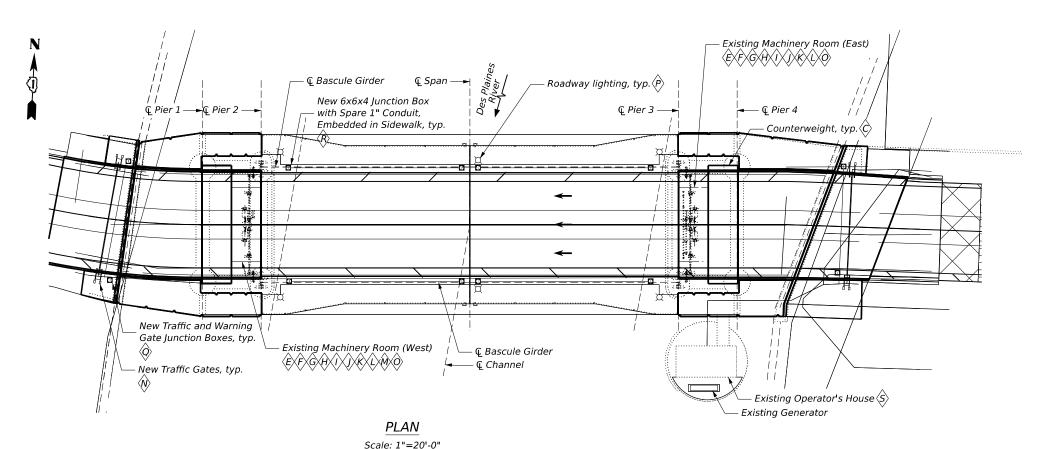
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ELEVATION

Scale: 1"=20'-0"



MECH./ELEC. GENERAL PLAN & ELEVATION STRUCTURE NO. 099-0101 SHEET M-02 OF M-13 SHEETS

SCOPE OF WORK

- $\langle \hat{A} \rangle$ Repair and realign the jaw and diaphragm center lock.
- (B) Reshim live load bearings.
- Rebalance the bridge to a 2 kip toe heavy condition and maintain during construction. Maintain angle of CG between ±25 degrees.
- Install shear backup blocks to the segmental girder webs in bearing with flange angles to provide three rows of shear capacity for the heavier deck where not installed. Refer to sheet S38.

Machinery Room:

Mechanical

- Replace all machinery bearings and bearing supports.
- $\langle F \rangle$ Replace all machinery shafts.
- Replace all brakes and brake wheels. Replace or modify brake supports as needed.
- Replace racks, pinons, and all other open gearing in all four quadrants.
- Remove bearing G on both leaves (4 total) and reconfigure the machinery arrangement to incorporate floating shafts.
- \rangle Remove and replace couplings.
- Refurbish gearboxes on both leaves.
- $\langle \hat{L} \rangle$ Remove and replace rotary cam limit switch.
- Remove all hand-operation machinery including shafting, bearings, chain wheels, and gears.

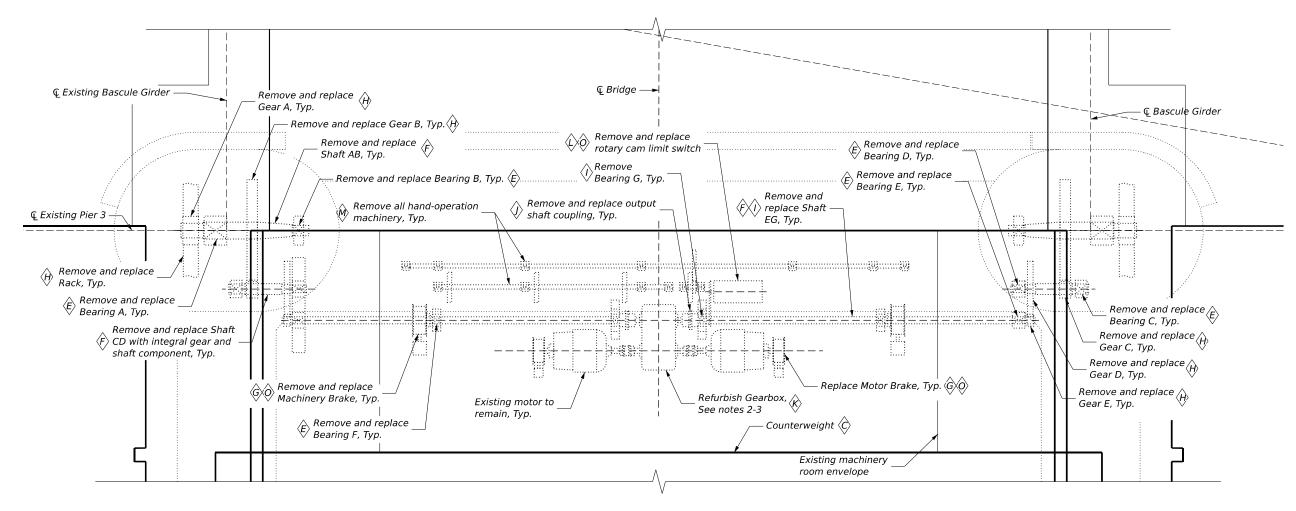
Electrical

- $\langle \hat{N} \rangle$ Remove and replace traffic warning gates.
- Remove and replace machinery brake and limit switch wiring.
- Remove and replace navigation lights and bridge roadway lighting.
- Install new Traffic Signals and Warning Gate junctions boxes adjacent to new signals. Install new conduits and wiring to new equipment.
- Install new spare pull junction boxes and spare conduit.
- (\$\hat{s}\) Install new flux vector drive per Sheet E-01

Notes:

Coordinate electrical work with current contract
 60P55





MACHINERY ROOM PLAN

(East side shown, West side similar) Scale: 3/8"=1'-0"

Notes:

- 1. See Sheet M-02 for Scope of Work descriptions.
- Gearbox to be removed and rehabilitated by the OEM (Horsburgh & Scott) per Quotation No. 606685 issued on 08/10/2023. Min. 40 weeks prior to removing from service, an inspector from H&S is to be called out to visually inspect gearbox to determine if gears will require replacement. If deemed necessary, new replacement gears will be fabricated prior to removing the gearbox from service (estimated fabrication lead time 6 months). The new gears will be installed when gearbox is sent out and disassembled.
- 3. Install rehabilitated gearbox using new shims and existing bolt holes in the structure and such that the input shaft is in alignment with the existing motor output shaft. Turned bolts may be reused if in good condition.

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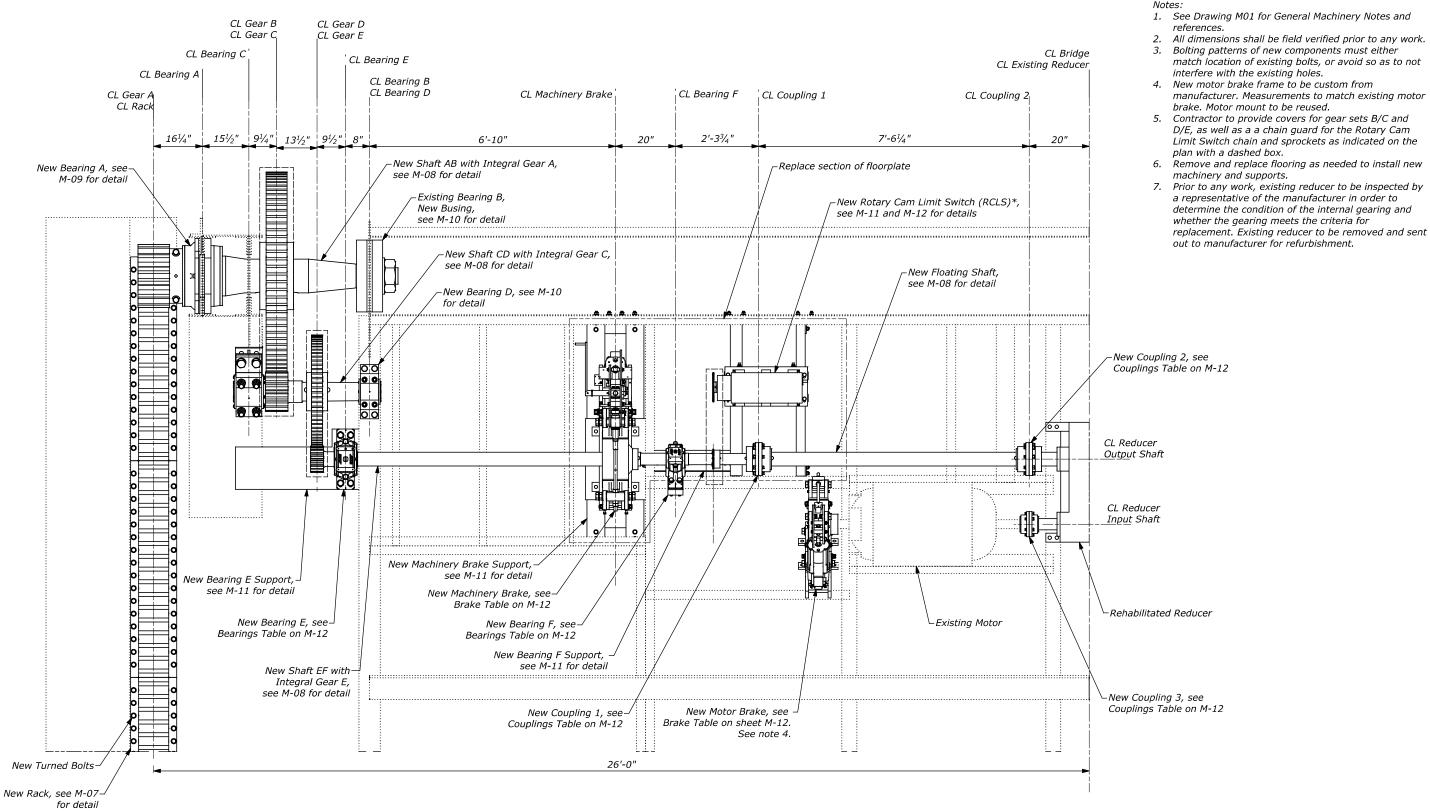
Item	Unit	Total
Mechanical Demolition	L. Sum	1



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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION MACHINERY ROOM DEMOLITION PLAN **STRUCTURE NO. 099-0101** SHEET M-03 OF M-13 SHEETS

SECTION COUNTY 607 2018-067-BR WILL 128 85 CONTRACT NO. 62M79



Machinery Room Layout Plan

Scale: 3/4" = 1'-0"

Machinery arrangement symmetrical about Bridge CL except where noted* East side shown, West side similar *Only one RCLS required per leaf

BILL OF MATERIAL

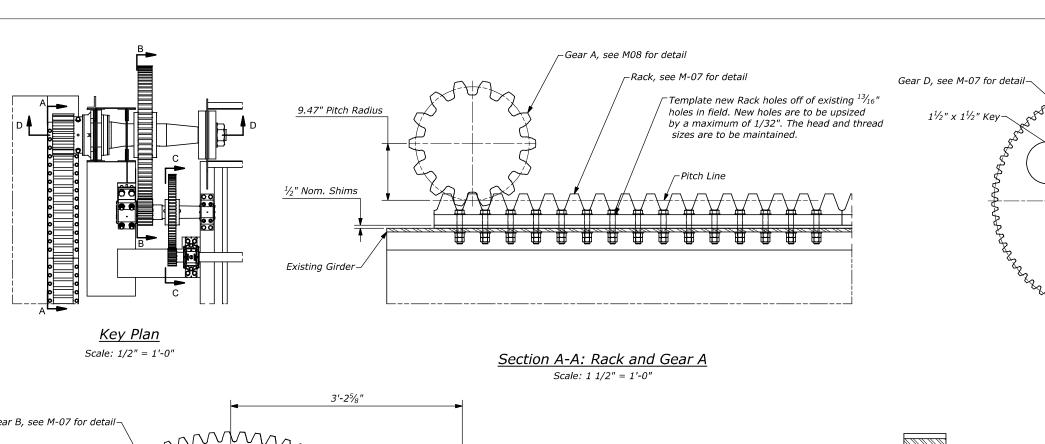
Item	Unit	Total
Refurbishing of Operating Machinery	L. Sum	1

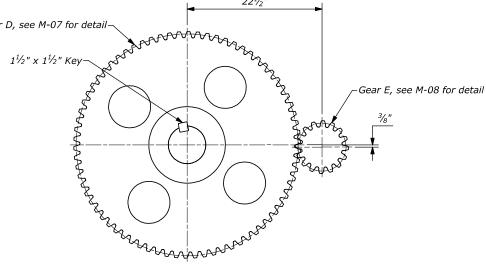


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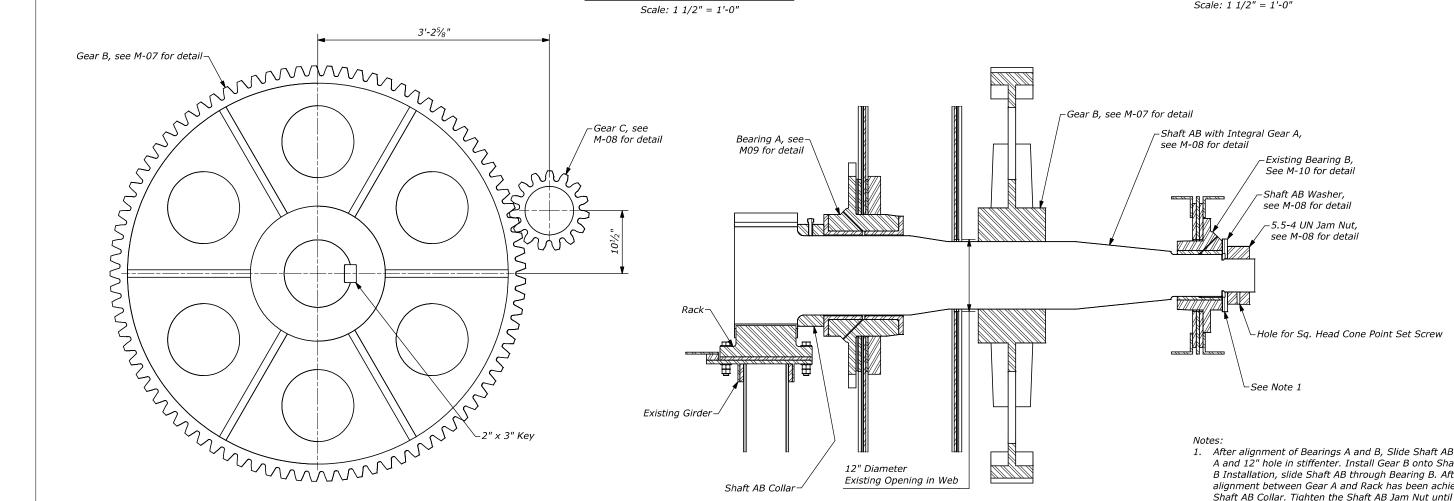




221/2"

Section C-C: Gears D & E

Scale: 1 1/2" = 1'-0'



Section B-B: Gears B & C

Scale: 1 1/2" = 1'-0"

Section D-D: Shaft AB Assembly

Scale: 1 1/2" = 1'-0"

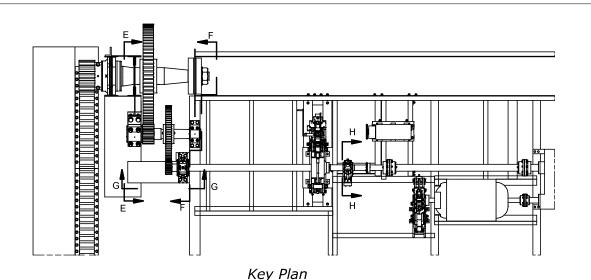
1. After alignment of Bearings A and B, Slide Shaft AB through Bearing A and 12" hole in stiffenter. Install Gear B onto Shaft AB. After Gear B Installation, slide Shaft AB through Bearing B. After axial alignment between Gear A and Rack has been achieved, install Shaft AB Collar. Tighten the Shaft AB Jam Nut until light contact is made between Bearing B, the Shaft AB Washer, and the Shaft AB Jam Nut. Back the jam nut off to provide 0.030" clearance. Once the 0.030" clearance has been achieved, install and fully tighten the Sq. Head Cone Point Set Screw into the Jam Nut.

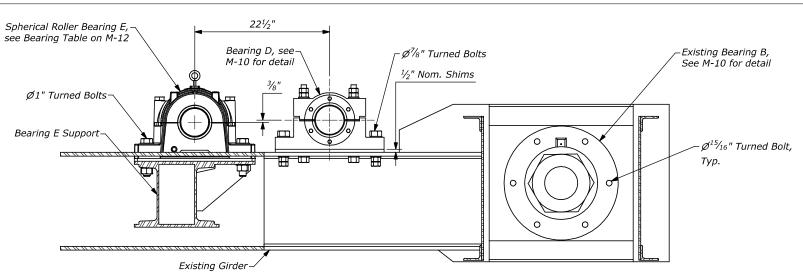


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OPEN GEARING SECTIONS STRUCTURE NO. 099-0101					
SHEET M-05	OF	M-13	SHEETS		

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607	2018-067-BR		WILL	128	87
		CONTRAC	T NO. 62	M79	





-Shaft EF with

Integral Gear E

−Ø⁵/8" H.S. Bolts

Section F-F: Bearings B, D & F

Scale: 1 1/2" = 1'-0"

Scale: 3/8" = 1'-0"

see M-11 for detail Existing Web- $- {\it \emptyset} 1^{\it 3}\!/\!\! s$ " Finished Body Bolt, Typ. Bearing C, see -Bearing E M-10 for detail Ø1" Turned Bolts, See Note 2 - $\emptyset 1^{5/16}$ " Turned Bolt, Bearing E Support, -see M-11 for detail Typ., See Note 1 Bearing E Support 3/8" Nom. Shim 5%" H.S. Bolts, See Note 2 $L4x3x^{3/8}$ Existing Girder $-\frac{7}{8}$ " H.S. Bolts, See Note 3 Existing Girder *∟Existing Girder*

L5x3x³/₈-5/8" H.S. Bolt, Typ.-

Section H-H: Bearing F Assembly

-Ball Bearing F, see

*L4x3x*³/₈

Bearing Table on M-12

 $- \phi^{3}/_{4}$ " Turned Bolts

 $\frac{1}{2}$ " Nom. Shim

1/4" Nom. Shim

Bearing F Support, see M-11 for detail

Scale: 1 1/2" = 1'-0"

Section E-E: Bearing C Assembly

Scale: 1 1/2" = 1'-0"

3'-25/8"

Section G-G: Bearing E Assembly

Scale: 1 1/2" = 1'-0"

Notes:

- 1. Reuse existing bolt holes to fasten Bearing C to structure. Contractor to verify size and location of existing bolt holes and ream holes to a maximum 1/16" diameter larger. Furnish turned bolts with a shank size that produces an ANSI LC6 fit with the reamed holes. Thread and head sizes are to match existing bolts.
- 2. Tapered washers to be utilized at the underside of channels where bolt cannot be flush mounted.
- Remove existing rivets where Bearing E support connects to existing girder. Fasten new bolts in existing rivet holes. Contractor to verify size and location of existing rivets.

Bearing A and B Alignment Proceduere:

- 1. Contractor is to establish the bridge center of roll by fabricating a jig that can aid in the establishment of bridge center of roll based off the existing curved segments
- Prior to installation of Bearing B, provide center of roll measurements along with Bearing B hole centers in the structural supports to the EOR for review.
- After EOR review, install Bearing B.
- Bearing A installation can follow. Bearing A and B are to be aligned to produce colinearity of Shafts AB within 1/64".



CL Shaft AB-

Bearing C Support,

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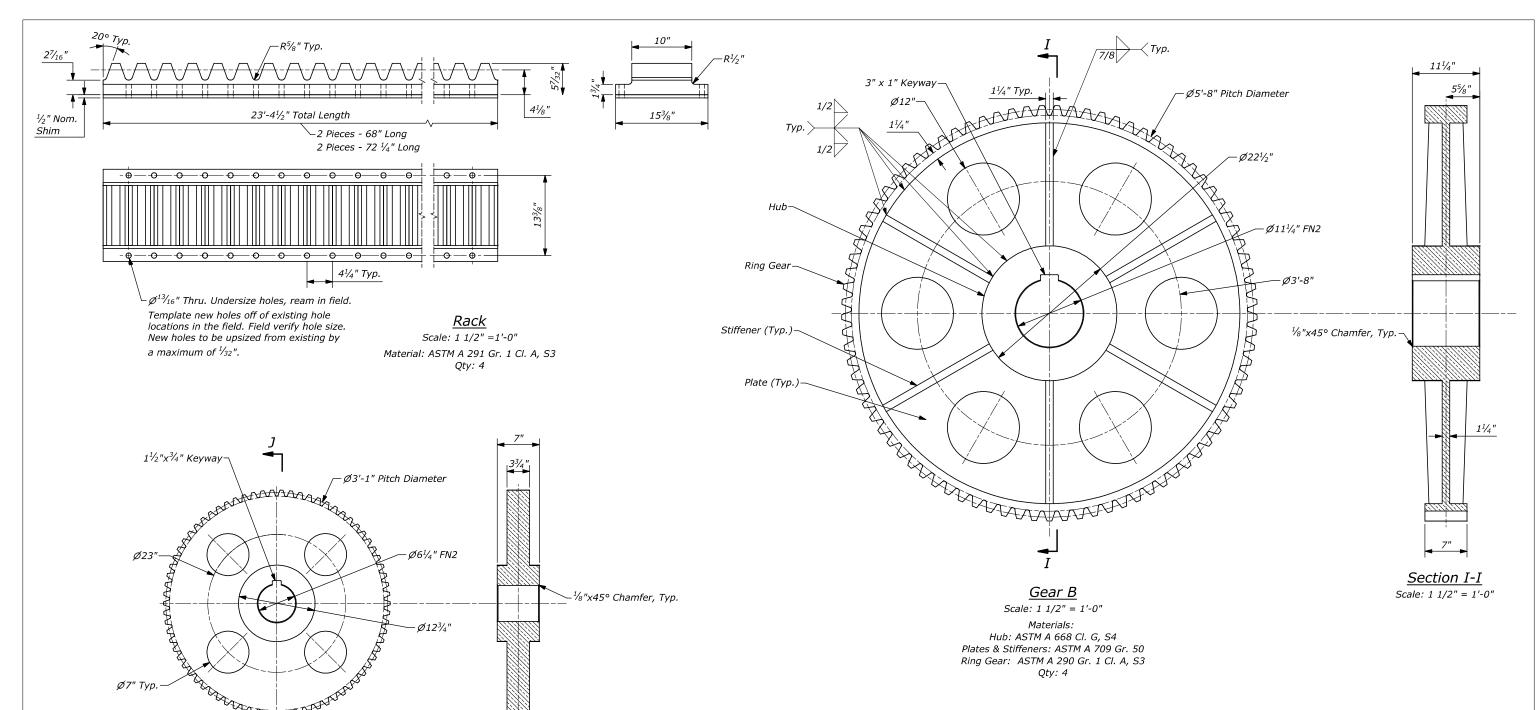


	Table of Gears								
Mark	Qty.	No. of Teeth	Face Width	Pitch	Pitch Dia.	Addendum Dia.	Dedendum Dia.	Teeth	Key Size
Rack	4	66	10"	4 1/4" C.P.	1	-	-	20 deg. involute stub tooth profile	N/A
Α	4	14	10 1/2"	4 1/4" C.P.	18.94"	21.10"	16.23"	20 deg. involute stub tooth profile	N/A
В	4	85	7"	1 1/4" D.P.	68"	69.28"	66.40"	20 deg. involute stub tooth profile	3" x 2"
С	4	15	7 1/2"	1 1/4" D.P.	12"	13.28"	10.40"	20 deg. involute stub tooth profile	N/A
D	4	74	3 3/4"	2" D.P.	37"	37.80"	36.00"	20 deg. involute stub tooth profile	1 1/2" x 1 1/2"
E	4	16	4 1/4"	2" D.P.	8"	8.80"	7.00"	20 deg. involute stub tooth profile	N/A

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<u>Gear D</u>

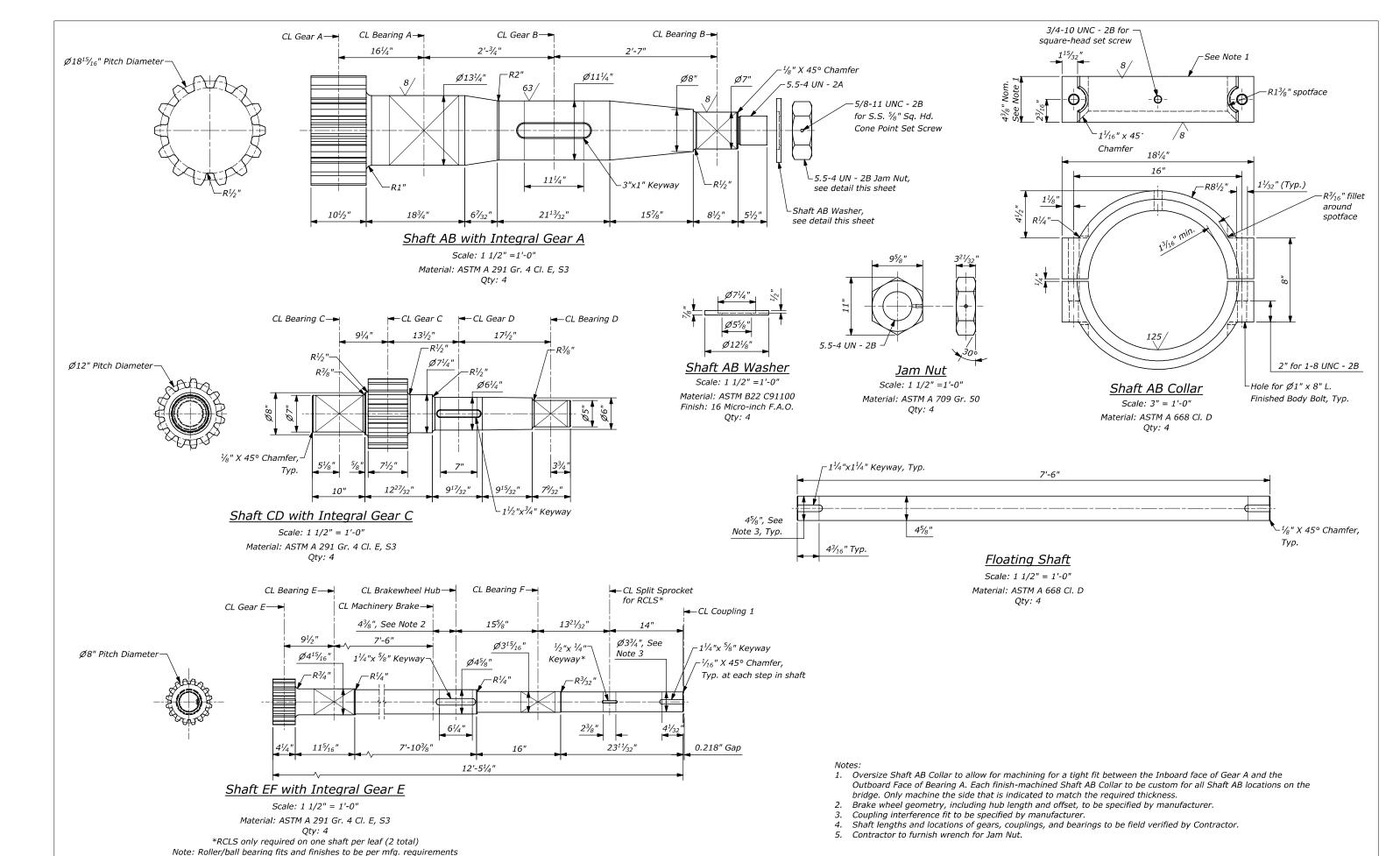
Scale: 3" = 1'-0" Material: ASTM A 291 Gr. 1 Cl. A, S3 Qty: 4

USER NAME =	DESIGNED - CMS	REVISED
	CHECKED - KMC	REVISED
PLOT SCALE = AS SHOWN	DRAWN - CMS	REVISED
PLOT DATE = JAN 2024	CHECKED - KMC	REVISED

Scale: 3" = 1'-0"

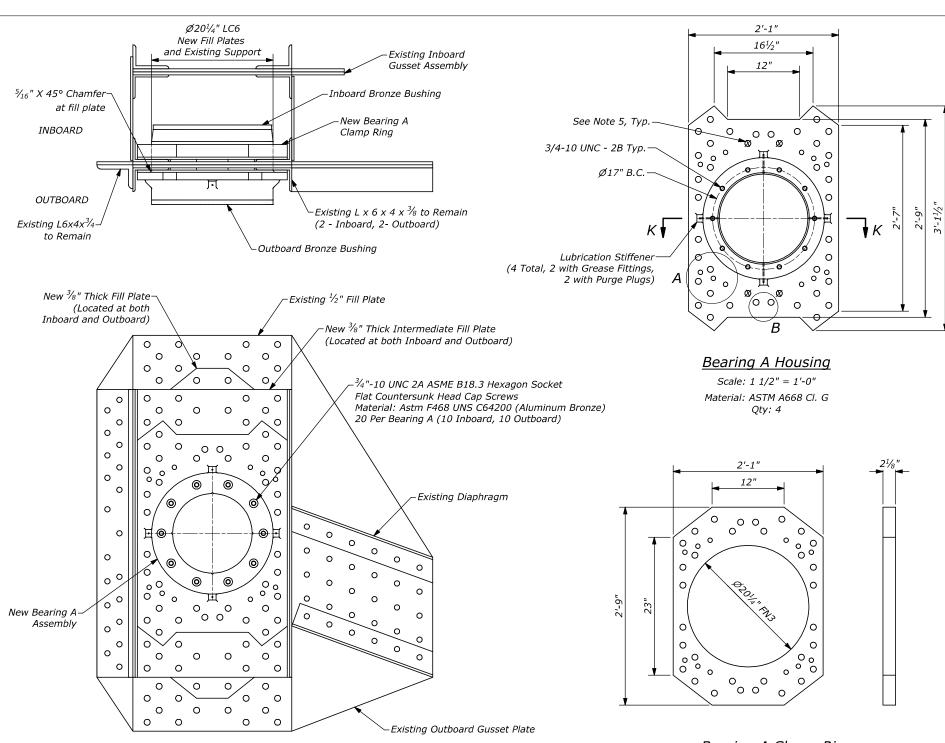
OPEN GEARING DETAILS	F.A.P. RTE	
STRUCTURE NO. 099-0101	607	20
SHEET M-07 OF M-13 SHEETS		

	SECTION			COUNTY	TOTAL SHEETS	SHEET NO.
	2018-067-BR			WILL	128	89
,				CONTRAC	CT NO. 6	2 M 79
ILLINOIS FED. AI				D PROJECT		



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SHAFTIING DETAILS STRUCTURE NO. 099-0101		SEC.	TION		COUNTY	TOTAL SHEETS	SHEET NO.
		607 2018-067-BR		WILL	128	90	
					CONTRA	CT NO. 6	32 M 79
				FFD 41	D DDO IFOT		



Bearing A Clamp Ring

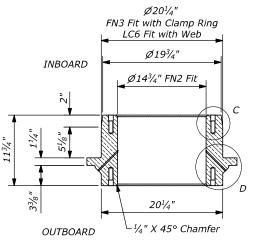
Scale: 1 1/2" = 1'-0" Material: ASTM A709 Gr. 50

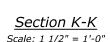
Qty: 4

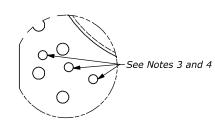
Outboard Gusset Plate - Bearing A Assembly Scale: 3"=1'-0"

Notes:

- 1. Bearing A connection to structural support is slip-critical.
- 2. It is the Contractor's responsibility to field verify all existing bolt and rivet patterns as well as their respective sizes. All data shall be provided to the EOR prior to construction.
- 3. Holes indicated in Detail A and B are from previous repairs not detailed in the 1931 Plans or the 1984 Rehabilitation Plans. It is the Contractor's responsibility to field verify the size and location of the bolts that were not indicated in the 1931 or 1984 plans and to incorporate them into the new Bearing A bolted connection. Contractor is to provide field measurments and observations to the EOR prior to
- 4. Existing bolt and rivet patterns and sizes indicated in the 1931 Plans and 1984 Plans used in the Bearing A connection are to also be field verified. All data and observations gathered are to be submitted to the EOR prior to construction.
- Contractor is to submit a written installation procedure for Bearing A prior to fabrication and

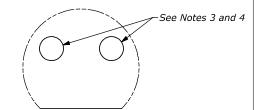






Detail A

Scale: 3" = 1'-0" (4 Locations per Housing)

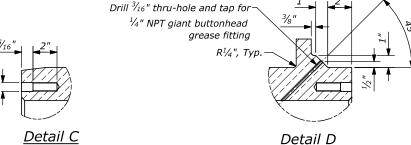


<u>De</u>tail B

Scale: 3" = 1'-0'

(2 Locations per Housing)

Scale: 3" = 1'-0"



Detail C Scale: 3" = 1'-0"

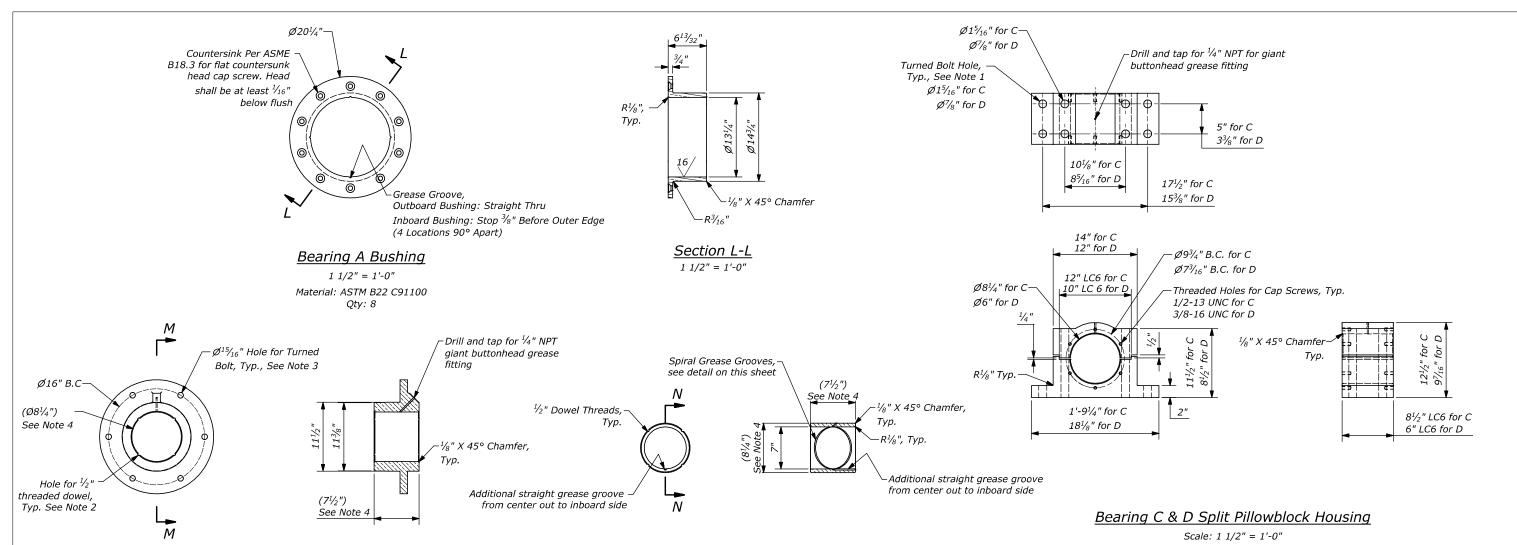
Recommended Bearing A Installation Procedure:

- 1. Bearing A is to be installled after Bearing B. Refer to Bearing A and B Alignment Procedure on M-06. The
- center of roll is to be established with a jig prior to any boring of the structural support or new fill plates. 2. Install the new fill plates with temporary undersized bolts with locations templated off the existing bolts. Contractor to verify new fill plates match the existing fill plate thicknesses.
- 3. Bearing A requires the structural support to be bored to $20\frac{1}{4}$ " with an ANSI LC6 fit with Bearing A. No flame cutting is permitted. The bore is to be machined in the field.
- 4. Bearing A is to be installed with temporary bolts held at the outermost corners of the Bearing A housing. Bearing A Clamp Ring can then be installed on to the Bearing A housing from the Inboard side of the structural support. Locations where turned bolts are to be placed can be sub-drilled to permit temporary bolts to be placed immediately after the Bearing A Clamp Ring is mounted. No liquid Nitrogen is permitted during this process.
- 5. Holes denoted with an "X" are to be filled with turned bolts of a size 1/16" larger in diameter than what exists in the field. Turned bolts are to be furnished with double nuts. Hole locations are to be templated off
- 6. All remaining bolts are to be $\frac{7}{8}$ " finished body bolts and the locations are to be templated off the locations of the existing holes in the field. Ensure all Bearing A housing and fill plate temporary bolts have been removed and replaced with new finished body bolts or turned bolts as indicated in the plans. Should larger bolts be required after field verifying the existing holes, the Contractor is to notify the EOR.
- 7. Refer to the suggested installation sequence of the operating machinery in the Specifications.
- Bearing A replacement must only proceed with the bridge in the fully open position and the deck removed to reduce the loads in U0-L0.



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	CHECKED -	KMC	REVISED -
PLOT SCALE =	DRAWN -	MAD	REVISED -
PLOT DATE = JAN 2024	CHECKED -	KMC	REVISED -

F.A.P. SECTION				COUNTY	TOTAL SHEETS	SHEET NO.
607	2018-067-BR			WILL	128	91
			CONTRACT NO. 62M79			
		ILLINOIS	D PROJECT			



Existing Bearing B Housing

Scale: 1 1/2" = 1'-0" Qty: 4 <u>Section M-M</u>
1 1/2" = 1'-0"

<u>Bushing B</u> Scale: 1 1/2" = 1'-0"

Scale: 1 1/2 = 1-0

Material: ASTM B22 C91100

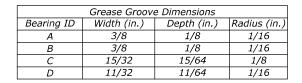
Qty: 4

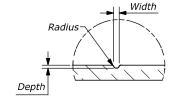
<u>Section N-N</u>
1 1/2" = 1'-0"

Scale: 1 1/2" = 1'-0"

Material: ASTM A 668 Cl. G

Qty: 4 each of Bearings C and D

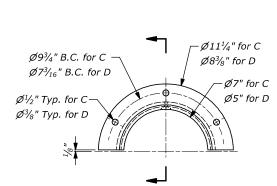


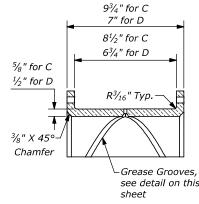


Grease Groove Detail

Scale: 6" = 1'-0"

$8\frac{1}{2}$ " LC6 for C Grease Groove Depth $1\frac{1}{8}$ " for C $1\frac{1}{8}$ " for D $1\frac{1}{8}$ " for D





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- Contractor to verify existing size and location of turned bolts for Bearing C and D. New turned bolts to be ¹/₁₆" larger than existing, and reuse existing holes in structure.
- 2. Existing Bearing B housing to remain. Replace existing threaded dowels and bushings with new. Replace Existing Bearing B ½" threaded dowels with new ASTM A449 dowels machined at same dimensions as existing.
- 3. Reuse existing bolt holes to re-fasten Bearing B to structure. Contractor to verify size and location of existing bolt holes and ream holes to a maximum 1/32" diameter larger. Furnish turned bolts with a shank size that produces an ANSI LC6 fit with the reamed holes. Thread and head sizes are to match existing bolts.
- 4. Contractor to verify Existing Bearing B dimensions prior to machining bushing.

Bearing C & D Liner

Scale: 1 1/2" = 1'-0"

Material: See Specs Qty: 2 per Bearing C (8 total) 2 per Bearing D (8 total)

Bearing C & D Bushing

1 1/2" = 1'-0"

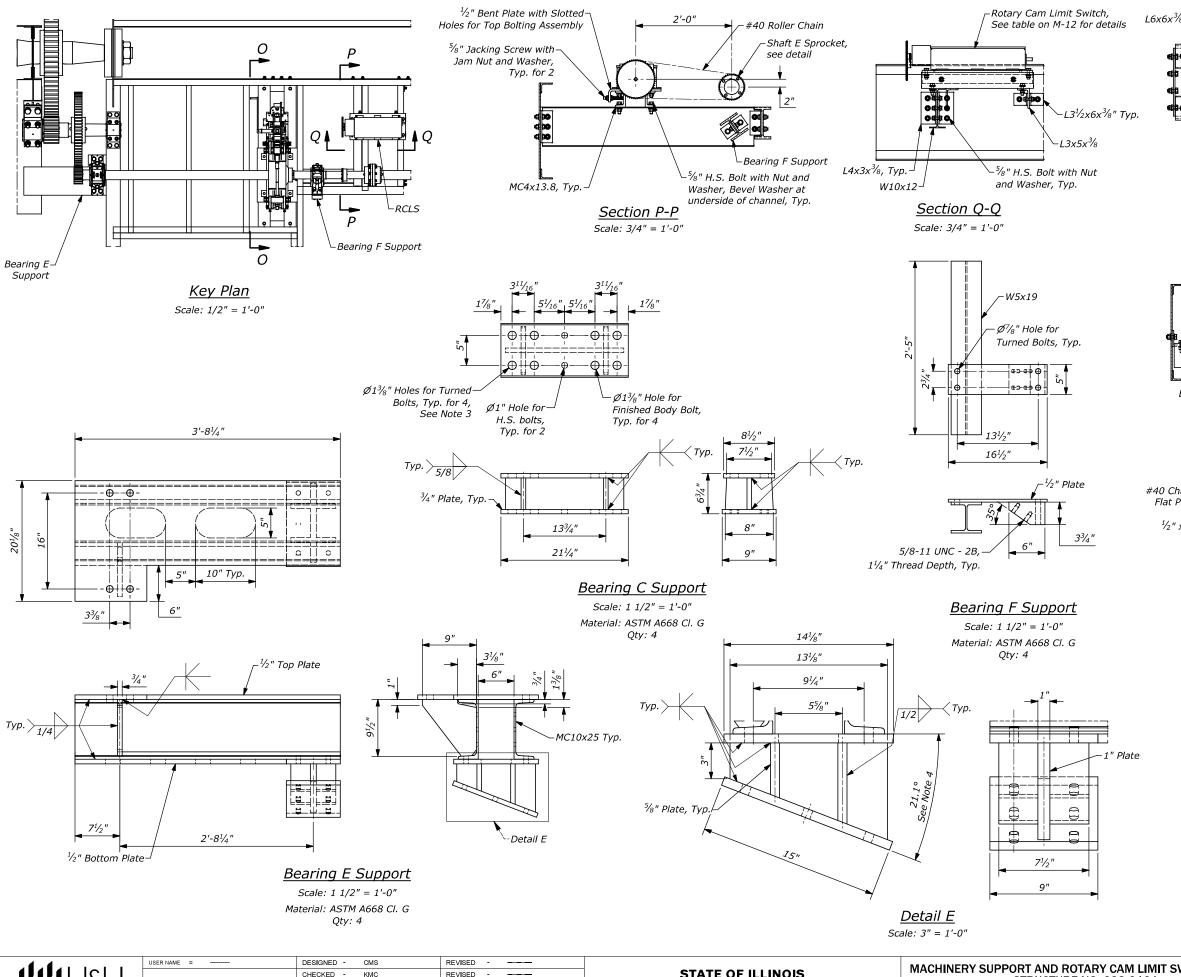
Material: ASTM B 22 C93700 Qty: 2 per Bearing C (8 total) 2 per Bearing D (8 total)

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PLOT DATE = JAN 2024	CHECKED	-	KMC	REVISED	-

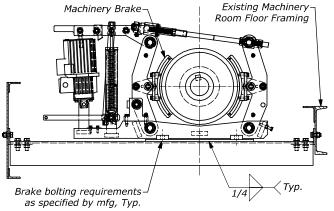
BEARING DETAILS STRUCTURE NO. 099-0101					
SHEET M-10	OF	M-13	SHEETS		

F.A.P. RTE.				COUNTY	TOTAL SHEETS	SHEE NO.
607	2018-067-BR			WILL	128	92
			CONTRAC	CT NO. 6	2 M 79	
				D DDG IFOT		

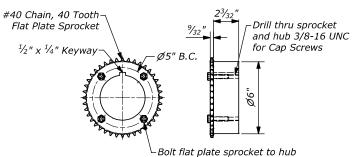


Weld $\frac{1}{2}$ " plate- $L6x3^{1/2}x^{3/8}$, Typ.- $L6x6x^{3/8}$, Typ. \neg to angle supports, machine flat 00 00 -¾" H.S. Bolts, Nuts, CL Brakewheel and Washers, Typ. Note: Machinery Brake and Framing Plan Top

Flanges not shown in plan view for clarity



Section 0-0 Scale: 1" = 1'-0"



Shaft E Sprocket Assembly

using 3/8" UNC Cap Screws

Scale: 3" = 1'-0"

Material: Hub - ASTM A 709 Gr. 50

- 1. RCLS Reducer Input Shaft Sprocket is a standard sprocket for ANSI 40 roller chain ($\frac{1}{2}$ " pitch) with hub and keyway. Coordinate sprocket bore size with selected RCLS input shaft size.
- One RCLS per leaf required.
- Reuse existing bolt holes to fasten Bearing C to structure. Contractor to verify size and location of existing bolt holes and ream $\frac{1}{16}$ " larger diameter for new turned bolts.
- Bearing E Support wedge angle to match existing girder
- angle. Contractor to field verify angle.

 Bearing E support wedge bolt holes to match location of existing rivets. Contractor to verify location and size of existing rivets.
- 6. Machinery mounting surfaces must be machined after stress relieving. Plate thicknesses shown are after final



USER NAME =	DESIGNED) -	CMS	REVISED	-	
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PLOT SCALE = AS SH	IOWN DRAWN	-	CMS	REVISED	-	
PLOT DATE = JAN 20	024 CHECKED	-	KMC	REVISED	-	

CHINERY SUPPORT AND ROTARY CAM LIMIT SWITCH DETAILS STRUCTURE NO. 099-0101						
SHEET M-11 OF M-13 SHEETS						

F.A.P. RTE	SEC	COUNTY	TOTAL SHEETS	SHEET NO.		
607	607 2018-067-BR			WILL	128	93
			CONTRAC	CT NO. 6	2M79	
		ILLINOIS	D PROJECT			

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Rotary Cam Limit Switch									
Mark	Qty.	Description	Shaft E Sprocket	Input Shaft Sprocket	Manufacturer				
RCLS	2	Nema 4X Rotary Cam Limit Switch with Straight Drive Gear Reducer Ratio 100:1, 16 Circuits	#40 Chain, 40 Tooth See M-11 for detail	#40 Chain, 60 Tooth	GEMCO or approved equal				

	<i>Bearings</i>								
Mark	Qty.	Туре	Shaft Size (in.)	Manufacturer					
А	4	Plain Bearing with New Bushings in New Custom Flanged Bearing Housing, See Drawings M09 for details	13¾	Custom					
В	4	Plain Bearing with New Bushings in Existing Flanged Bearing Housing, See Drawing M10 for details	7	Custom					
С	4	Plain Bearing with New Bushings in New Custom Split Pillowblock Housing, See Drawing M10 for details	7	Custom					
D	4	Plain Bearing with New Bushings in Custom Split Pillowblock Housing, See Drawing M10 for details	5	Custom					
Ε	4	SAFS 22528 Split Pillowblock with Spherical Roller Bearings on Adapter Sleeve. Fixed Bearing	4 ¹⁵ ⁄ ₁₆	SKF or approved equal					
F	4	SAFS 1522 Split Pillowblock with Self-Aligning Ball Bearings on Adapter Sleeve. Float Bearing.	3 ¹⁵ ⁄ ₁₆	SKF or approved equal					

	Brakes (See Notes)							
Mark	Mark Qty. Type							
Motor Brake	13" Brake Wheel Diameter, Brake Torque Setting 660 lb-tt		Magnetek or approved equal					
Machinery Brake	4	Hi-Tork 300M Series Type MBTE, 3-Phase AC Hy-Thrust 19" Brake Wheel Diameter, Brake Torque Setting 1800 lb-ft.	Magnetek or approved equal					

	Brake Wheels									
Mark	Qty.	Wheel Dia. "D" (in.)	Hub Dia. "S" (in.)	Bore Dia. (in.)	Hub Length "E" (in.)	Keyway (in. x in.)	Wheel Width "C" (in.)	Offset "O" (in.)	Max Speed (RPM)	
Motor Brake Wheel	4	13	6.75	3*	4	¾ x ¾ *	5.75	3.5	3600	
Machinery Brake Wheel	4	19	7	4.62	6.25	1¼ x ⅓	8.75	7.5	2470	

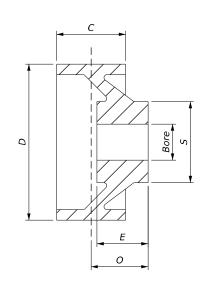
^{*} Field Verify Dimension

Notes

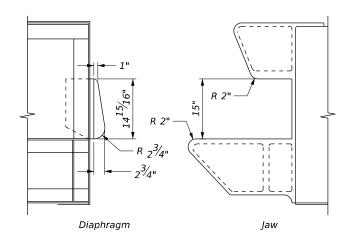
- 1. All quantities listed are for both leafs.
- Machinery brakes and motor brakes will be supplied by the Department.

Turned Bolts								
Component	Component Qty. Per Component		Bolt Size (in.)					
Rack	124	496	13 / 16 *					
Bearing A	4	16	7⁄8"∗					
Bearing B	6	24	¹⁵ / ₁₆ *					
Bearing C	4	16	1¾ ₁₆ *					
Bearing D	4	16	7⁄8*					
Bearing E	4	16	1					
Bearing F	4	16	¾					

*Bolt sizes for these components are based on existing bolt sizes. Field verify existing bolt sizes prior to selecting new bolt sizes.



Brake Wheel Detail Scale: NTS



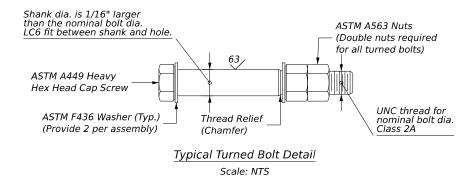
Center Lock Detail Scale: 1" = 1'-0"

Center Lock Rehabilitation

- 1. Contractor to throughly clean and inspect center lock jaw and diaphragm prior to commencing work.
- 2. The geometry of the parts that are determined to be out of tolerance are to be built back up by means of aluminum bronze brazing. If required, a combination of brazing and grinding may be used to restore the jaw and diaphragm to their original profiles as shown on the detail on this drawing.
- 3. Contractor to use Ampco-Trode 10 ECuAl-A2 conforming to AWS A5.6 or approved equal.
- 4. Submit a brazing procedure for approval prior to starting work.

Live Load Bearing Adjustment

1. Once work on the center lock jaw and diaphragm has been completed, the live load bearings, located above the counterweights, are to be shimmed to attain a tight fit when the bridge is fully closed.



BILL OF MATERIAL

Item	Unit	Total
Replacement of Center Locks	Each	4



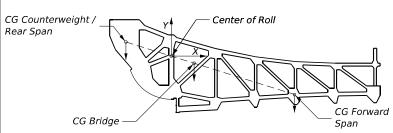
USER NAME =	DESIGNED - CMS	REVISED
	CHECKED - KMC	REVISED
PLOT SCALE = NOT TO SCALE	DRAWN - CMS	REVISED
PLOT DATE = JAN 2024	CHECKED - KMC	REVISED

MACHINERY TABLES, AND CENTER LOCK DETAIL STRUCTURE NO. 099-0101							
SHEET M-12	OF M-13 SHEETS						

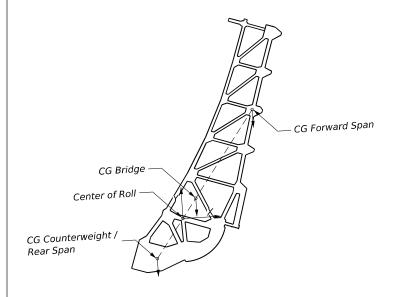
A.P. TE.	SEC	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEET NO.
07	2018-067-BR			WILL	128	94
				CONTRAC	CT NO. 6	2M79
		HI MICIO	EED A	D DDG IFOT		

Balance Tables

Note: Values given in table are for the span in the fully closed position.



Elevation - Bascule Closed Scale: NTS



		V	Vest Leaf			
Item		Weight	Loc	ation	Moment	
		Total Weight (kips)	X (ft)	Y (ft)	Mx (kip*ft)	My (kip*ft,
Existing CG		1,818.00	-0.06	0.00	-100.64	-8.33
Removed Items						
Existing Steel Curb		23.31	43.99	-15.73	1,025.46	-366.63
Existing Open Grid De	eck	90.21	43.70	-18.60	3,942.43	-1,677.97
Existing Deck Joist Re	emoval	14.79	43.58	-19.08	644.64	-288.22
Existing Stringer Rem	noval	75.05	43.95	-18.41	3,298.46	-1,382.03
Existing Floorbeam R	emoval	102.28	42.01	-19.64	4,296.77	-2,008.40
Existing Rear Break		7.51	1.65	-20.12	12.40	-151.16
Existing Center Break	(12.65	86.32	-16.49	1,091.88	-208.62
Existing Steel Plates : See Note 6	1,	1.39	80.00	-19.45	111.12	-27.02
Total Removed		327.19	44.08	-18.66	14,423.15	-6,104.06
Added Items						
Bridge Barrier		19.05	43.99	-15.48	837.71	-294.74
Half Filled Grid Deck		153.41	44.02	-17.52	6,752.72	-2,687.30
Stringer		48.41	41.39	-18.63	2003.69	-901.65
Floorbeam		95.17	43.46	-19.74	4,135.85	-1,878.37
Rear Break		10.63	1.96	-21.98	20.82	-233.58
Center Break		12.21	84.63	-18.66	1,033.72	-227.98
Total Added		338.88	43.63	-18.37	14,784.51	-6,223.62
Balance Blocks	Qty.					
Upper Pocket	100	13.06	-18.00	10.5	-235.15	137.17
Lower Pocket	0	0	-	-	0	0
Proposed CG		1,842.75	0.01	0.13	25.57	248.40
i i oposeu co		1,042.73	0.01	0.15	23.37	240.40

		E	ast Leaf			
Item		Weight	Loc	ation	Mon	nent
		Total Weight (kips)	X (ft)	Y (ft)	Mx (kip*ft)	My (kip*ft)
						4.50
Existing CG		1,818.00	-0.04	0.00	-67.33	1.50
Removed Items						
Existing Steel Curb		23.31	43.99	-15.73	1,025.46	-366.63
Existing Open Grid Deck		90.21	43.70	-17.88	3,942.43	-1613.44
Existing Deck Joist Remo	val	14.79	43.58	-18.36	644.64	-271.63
Existing Stringer Remova	al .	75.05	43.95	-18.41	3,298.46	-1,382.03
Existing Floorbeam Remo	oval	102.65	41.93	-19.64	4,303.79	-2,016.29
Existing Rear Break		7.51	1.65	-20.12	12.40	-151.16
Existing Center Break		12.65	86.32	-16.49	1,091.88	-208.62
Existing Steel Plates 1, See Note 6		2.64	83.25	-19.45	219.78	-51.34
Existing Steel Plates 2, See Note 6		1.19	80.00	-19.22	95.28	-22.89
Total Removed		330.02	44.34	-18.44	14,634.11	-6,084.05
A / / / / /		I				
Added Items		10.05	12.00		007.71	205.20
Bridge Barrier		19.05	43.99	-15.51	837.71	-295.38
Half Filled Grid Deck		153.42	44.02	-17.55	6,677.76	-2,662.61
Stringer		48.41	41.39	-18.66	2,003.86	-903.40
Floorbeam		95.17	43.46	-19.74	4,135.85	-1,879.00
Rear Break		10.63	1.96	-21.28	20.82	-226.12
Center Break		12.21	84.63	-17.97	1,033.72	-219.55
Steel Plate Along Member 39G1, See Note 7		7.62	5.92	-21.53	45.06	-164.00
Total Added		346.50	42.80	-18.41	14,829.98	-6,380.05
Balance Blocks	Qty.					
Upper Pocket	0	0	_	_	0	0
Lower Pocket	264	12.80	-18.29	5.75	-234.19	73.62
		·			•	•
Proposed CG		1843.46	-0.10	-0.15	-184.41	-278.15

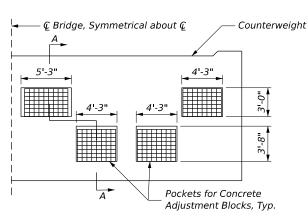
Notes:

- 1. This balance information is for bidding purposes only. The exact values will be determined by the contractor's counterweight balance calculations in accordance with the Special Provision "Bridge Balancing".
- 2. Contractor to document the exact quantity, type, and location of existing balance blocks in all counterweight pockets prior to starting work.
- Contractor to maintain an inventory of the weights and center of gravity locations of all materials removed and added to the lift span and counterweight.
- 4. The span to be maintained in a balanced condition throughout the construction period.
- Contractor to provide additional balance blocks for future adjustment in the amount of 0.5% of the weight of the new deck, with a minimum requirement of 150 blocks.
- Existing Steel Plates 1 and 2 refer to the additional steel plates that have been added to the toe of the bridge. Steel Plates 1 refers to the plates located near the center break, and Steel Plates 2 refers to the steel plates located on the diagonal member. Note that on the East Leaf, all of the existing plates are to be removed. On the West Leaf, only the weight shown is to be removed (approx. 20 plates located near the center break to be removed.)
- 7. See Structural Drawings for details of Steel Plate Along Member 39G1. The additional weight is provided at this location to lower the CG and should be accounted for in any changes the contractor makes once construction begins.
- 8. The desired final condition is: Seated Toe Load = 2,000 lbs. (Toe-Heavy) $CG Angle = 0 \pm 25^{\circ}$ Imbalance Moment = 174 kip*ft

BILL OF MATERIAL

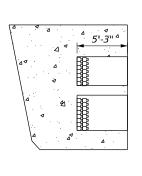
Item	Unit	Total
Bridge Balancing	L. Sum	1

Elevation - Bascule Open Scale: NTS

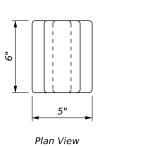


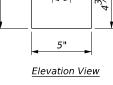
Counterweight Elevation - Front

Scale: NTS



Section A-A Scale: NTS





Balance Block Detail

Scale: 3"=1'-0" Material: Cast Steel ASTM A27 Weight: 48.5 lbs ea.

USER NAME	=		DESIGNED	-	CMS	REVISED	-	
			CHECKED	-	KMC	REVISED	-	
PLOT SCALE	-	AS SHOWN	DRAWN	-	CMS	REVISED	-	
PLOT DATE	=	JAN 2024	CHECKED	-	KMC	REVISED	-	

COUNTERWEIGHT BALANCE TABLES AND BALANCE BLOCK DETAIL	F.A.P. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 099-0101		2018-067-BR	WILL	128	95
STREETERE NO. 033 OLOI			CONTRA	CT NO. 6	2M79
SHEET M-13 OF M-13 SHEETS		ILLINOIS EED AL	D DDO IECT		

General Notes

- 1. All electrical work shall be performed in accordance with the requirements of the National Electric Code (nec), American Association of State Highway and Transportation officials (AASHTO), U.S. Coast Guard and local ordinance and regulations. Coordinate all electrical work with the Illinios Department of Transportation (IDOT) and other contractors on the
- 2. All electrical work shall be coordinated with the work of other trades and shall be scheduled consistent with the overall construction staging sequence.
- 3. The plans are diagrammatic and are not to be scaled. The locations of equipment and routing of conduits shown on the contract drawings are approximate. Exact locations shall be determined based upon approved shop drawings submitted by the Contractor.
- 4. The location and number of raceways and junction boxes shown on the plans are of schematic type and do not purport to be exact. The Contractor shall furnish and install all required raceways, junction boxes, conduit fittings, elbows, and hardware for a complete installation in accordance with the nec whether or not they are explicitly shown or indicated on the contract drawings.
- 5. All conduit shall be 3/4" minimum pvc coated rigid galvanized steel unless otherwise noted or required by nec and shall meet all the additional requirements for materials, construction, and installation contained in the specification.
- 6. All wiring and conduit furnished and installed under this contract shall be new. Electrical conductors shall be minimum size no. 12 awg stranded type xhhw-2 except for internal wiring in control cabinets and control desk which shall be minimum size no. 14 awg type sis. All wires and cables shall meet all the additional requirements for material, construction and installation contained in the relevant specifications.
- 7. The Contractor shall furnish and install expansion fittings of the approved type wherever conduits pass through structural expansion joints. Deflection fittings shall also be furnished and installed as necessary.
- 8. Provide equipment grounding per nec requirements running separate grounding wire in each conduit. Ground conductors shall be provided in all flexible cables. Minimum size ground conductor shall be #12 awg. All cabinets, terminal and junction boxes shall be grounded in accordance with the nec.
- 9. Run conduit at right angles or parallel to house/bridge lines. Rack neatly and fasten securely all conduits. Use insulated bushings and double nuts on movable span and where indicated in the specifications. Provide pull/junction boxes as required to facilitate wiring.
- 10. Install all conduit supports per NEC and AASHTO standards. Limit total angular conduit bends between pull boxes or access points to 270 degrees. Radius of conduit bend shall be per nec table 346-10. All required pull boxes are not necessarily shown on the
- 11. The Contractor shall coordinate the installation of all electrical components, conduits, hangers, supports, etc. With the other disciplines or as required by the Engineer.
- 12. Structural steel shall not be cut, drilled, or welded to, except as may be explicitly authorized by the Engineer in writing.
- 13. Upon completion of electrical installation, the Contractor shall test the comeplete electrical system for short circuits, grounds and proper operation in the presence of the
- 14. Electrical loads shall be equally distributed on all three phases.

Scope of Work

The work shall consist of furnishing, installing, and placing in satisfactory operating condition motor drives, traffic gates, rotary cam limit switches, junction boxes, conduit and cabling. The work shall also include installing and placing in satisfactory operating condition new motor brakes and machinery brakes that will be supplied by the Department. Where not specifically covered on the plans, Specifications, or Special Provisions, all equipment shall be installed according to the manufacturer's published recommendations.

Removal, relocation, storage, and/or disposal of all existing material that is to be replaced is Contractor's responsibility. If, at the discretion of the Department existing components are to be salvaged, the Contractor shall coordinate delivery to an agreed upon location.

Any existing conduit encased in concrete, which are to be abandoned, shall be cut back to the concrete surface, threaded and plugged with a steel plug.

A total of eight brakes (four motor brakes and four machinery brakes) shall be installed for purposes of holding the leaf when stopped and for stopping the leaf in an emergency situation. Each brake will be provided with a brake set, brake released, and brake hand-released limit switch. Limit switches will be integrated into the existing control system.

Traffic gates:

Four traffic gates with flashing arm lights and steady burn tip light shall be furnished and installed, two on each approach to the bascule span as indicated on the plans. The gates shall be interfaced with the existing control system and shall be controlled by selector switches located on the control desk, Each gate shall be equipped with front and rear door interlock switches and a hand crank limit switch that will prevent electrical operation of the gate when an access door is open or when the handcrank is inserted. New conduit and wire shall be furnished and installed to traffic gates.

One rotary cam limit switch and resolver assembly shall be provided on each span to provide continuous span position information to the existing control system. A rotary cam limit switch shall give back up position information to the existing control system at nearly closed, nearly open, fully open, mating and over-travel positions.

Conduit, Wire, Boxes:

Conduit and wire shall be installed to new brakes, traffic gates, and instrumentation in the machinery rooms, bascule leaf and approaches. Except where otherwise noted, all wiring shall be enclosed in PVC coated rigid metal conduit. Conductors shall be single type, xhhw-2 except when noted on the plans. All junction boxes shall be stainless steel type 316l with continuous hinge access door. Any backplates for mounting of equipment shall be 316l stainless

Motor Drives:

Each existing span motor shall be provided with new flux vector drives providing closed loop speed regulation. Each drive shall be heavy duty compatible with the motor and shall be sized to provide 200% of full torque for 60 seconds and a continuous output current rating of the selected motors FLA. The drives shall be configured as per the manufacturer specifications and recommendations. Both motors and drives are required to operate their respective spans. The control system shall be modified to allow for single motor operation in the event of a motor or drive failure.

3/25/2024

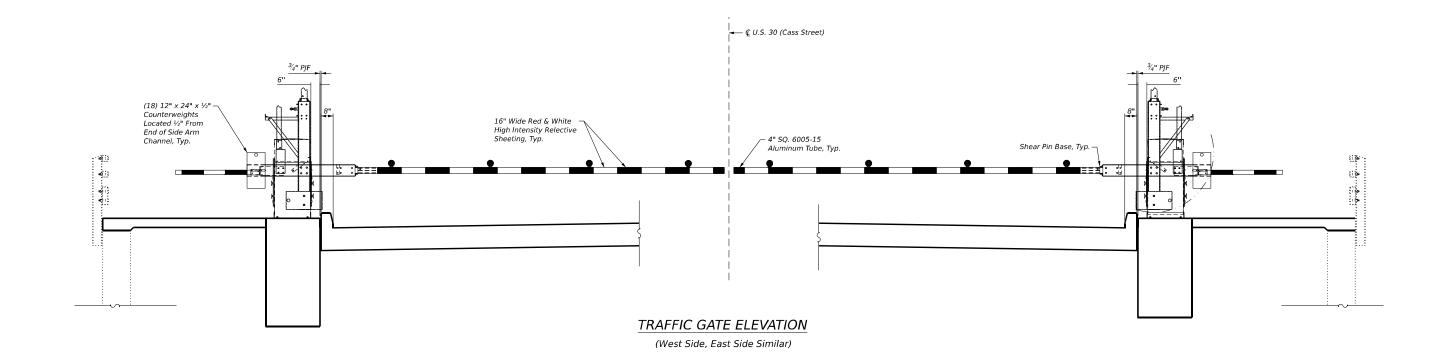
Paul Skelton, P.E. Licensed Professional Engineer State of Illinois 062-051949 Expires 11/30/2025

REVISED DESIGNED RL CHECKED AC REVISED -REVISED CHECKED RL

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **ELECTRICAL NOTES AND SCOPE OF WORK STRUCTURE NO. 099-0101**

SHEET E-01 OF E-21 SHEETS

SECTION 128 96 2018-067-BR CONTRACT NO. 62M79



BILL OF MATERIAL

Item	Unit	Total
Traffic Gates	Each	4

NOTES:

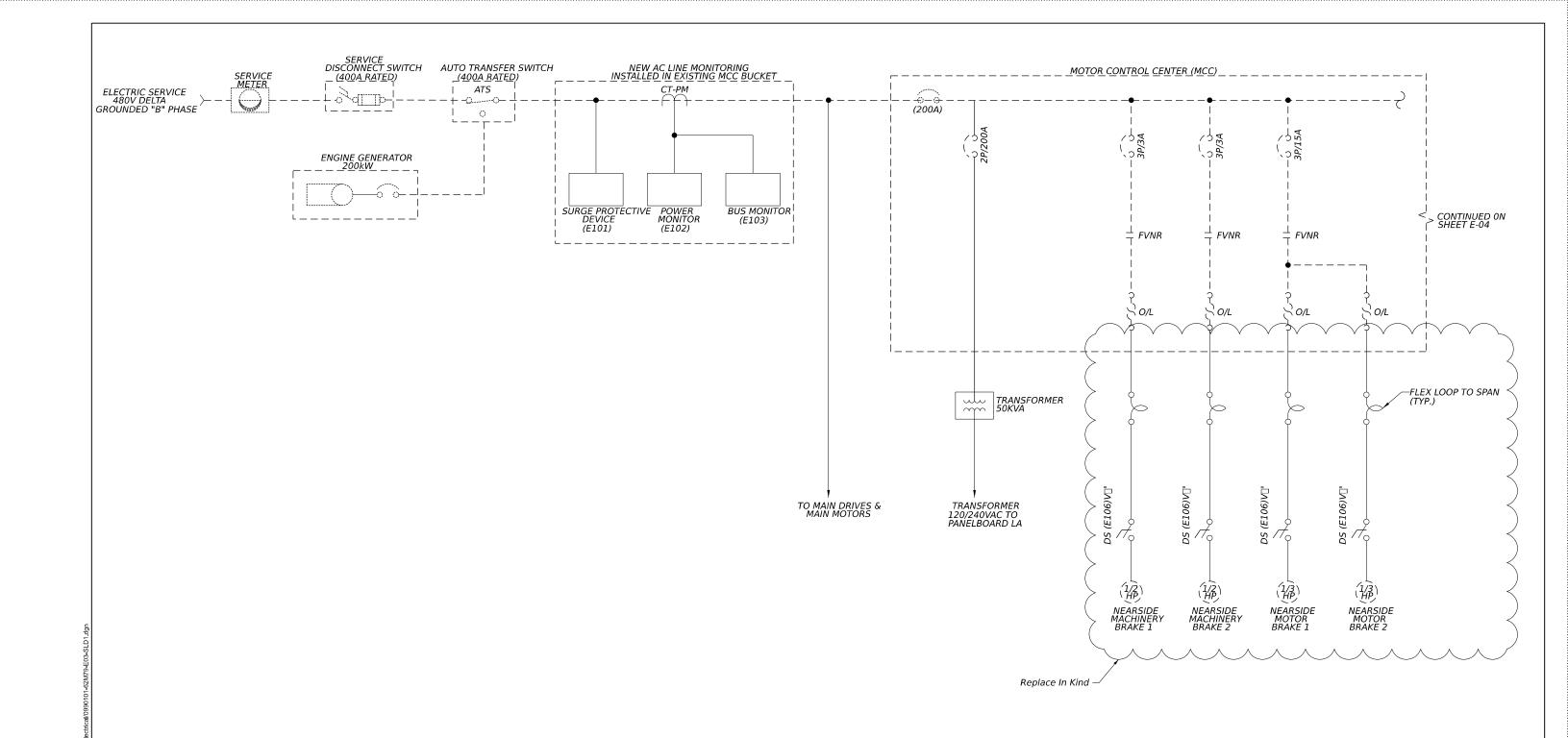
- Contractor shall verify all dimensions, paying close attention to boxed dimensions.
- 2. Four (4) gates required as shown two (2) oncoming and two (2) offgoing.
- 3. Traffic gate housing, side arm channels, crossmember and counterweights shall be hot dip galvanized steel per Special Provisions "BRIDGE ELECTRICAL WORK".
- 4. Traffic gates shall have removable lockable doors keyed alike with padlock straps. Padlocks shall be by others.

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TRAFFIC GATE - DETAILS		SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
STRUCTURE NO. 099-0101	607	2018-067-BR		WILL	128	97
311001011E 110: 033-0101				CONTRA	CT NO. 6	32M79
SHEET E-02 OF F-21 SHEETS		ILLINOIS	FED. AII	PROJECT		



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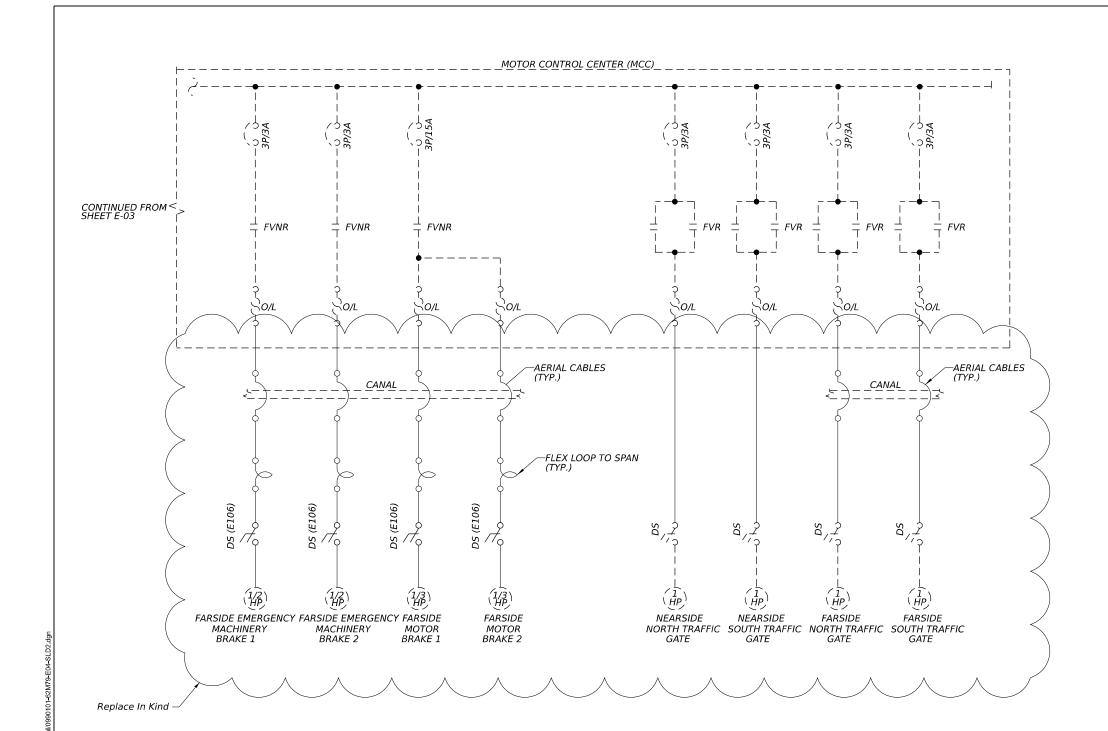
 All items located outside of the bubbled areas are existing to remain and are shown for reference only.

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SINGLE LINE DIAGRAM I
STRUCTURE NO. 099-0101

SHEET E-03 OF E-21 SHEETS



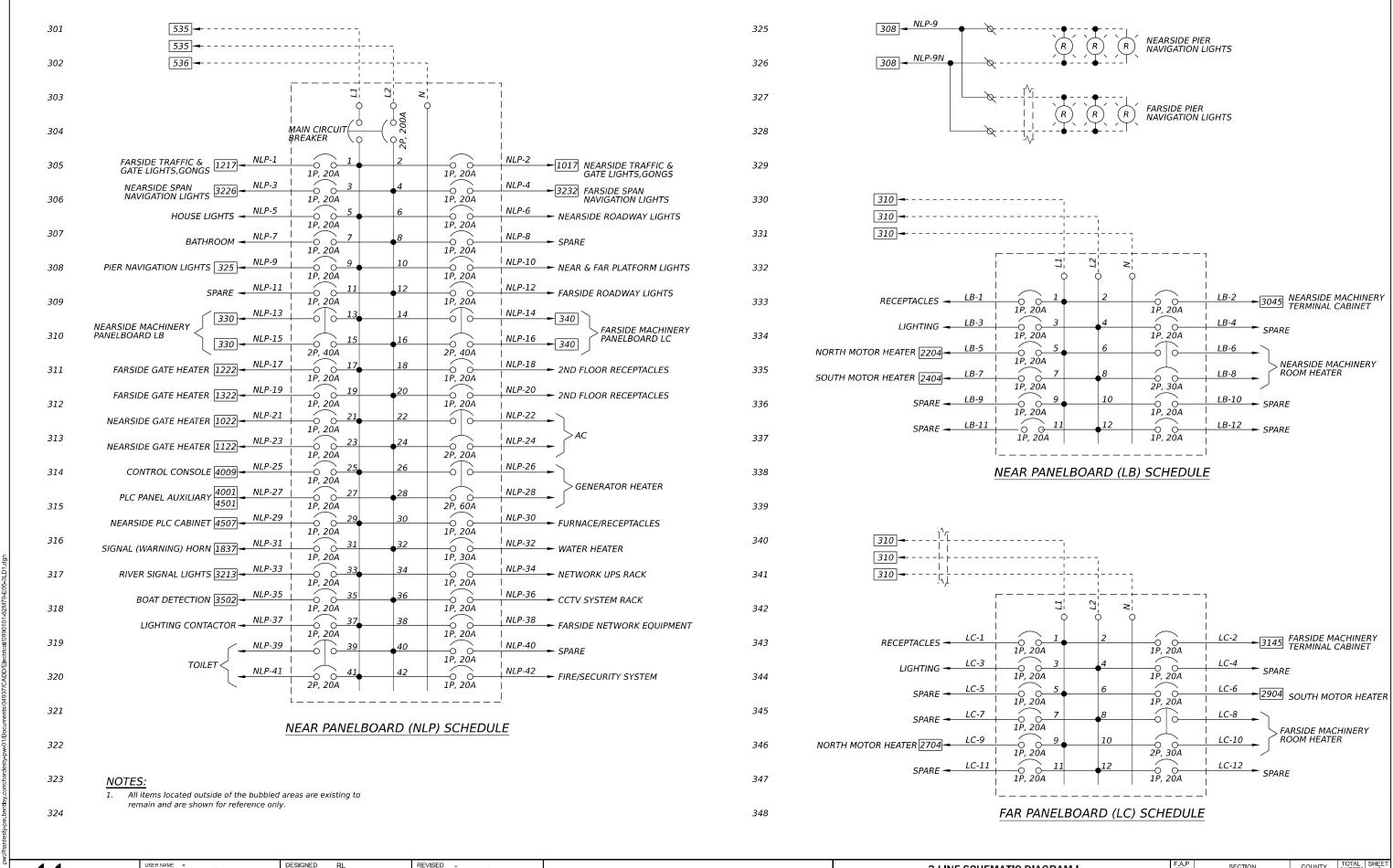
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SINGLE LINE DIAGRAM II	F.A.P RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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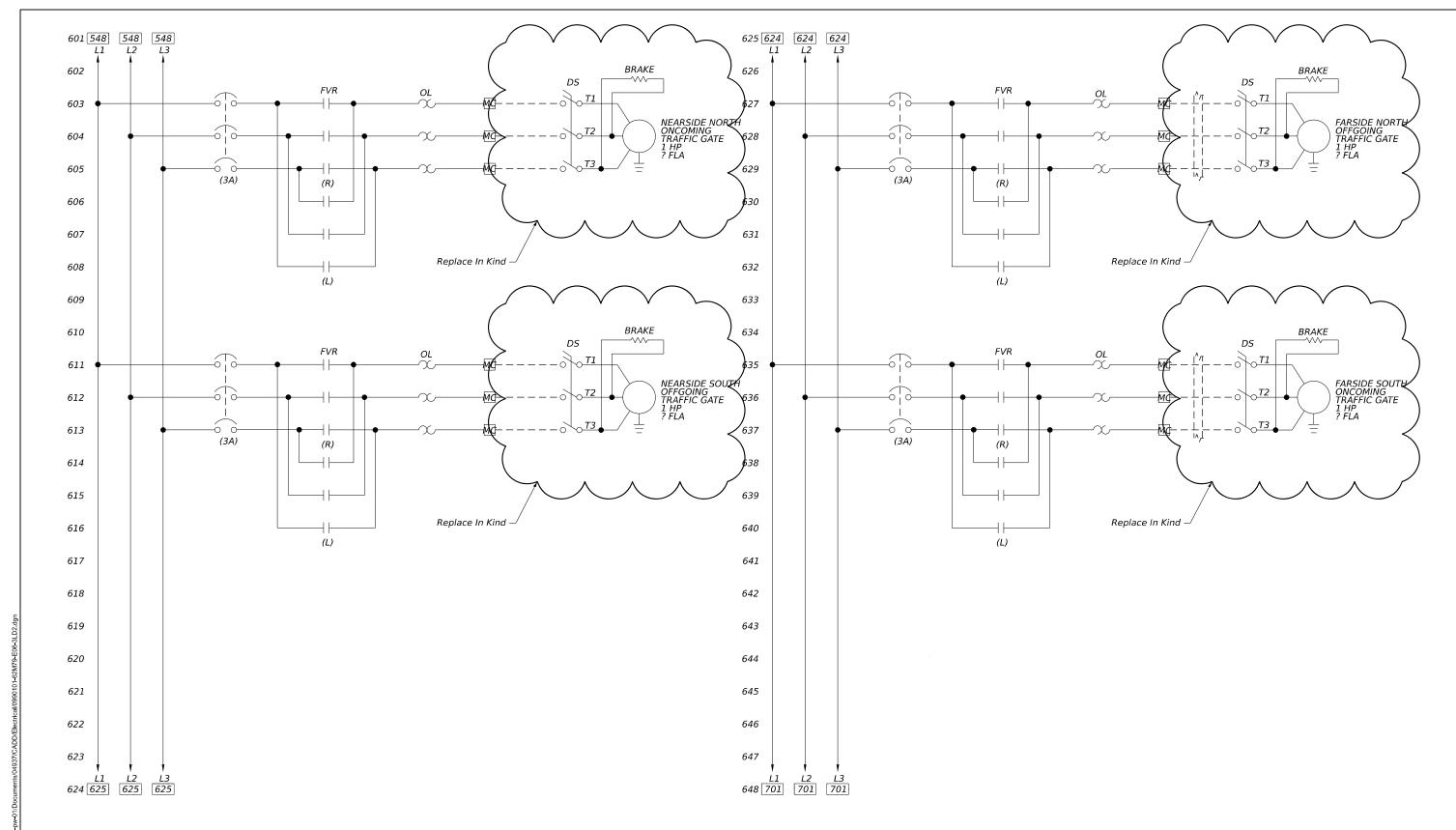


STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

3 LINE SCHEMATIC DIAGRAM I STRUCTURE NO. 099-0101
 FAP RTE.
 SECTION
 COUNTY SHEETS
 TOTAL NO.
 SHEET NO.

 607
 2018-067-BR
 WILL
 128
 100

 CONTRACT NO. 62M79



TERMINAL LEGEND:

- CC CONTROL CONSOLE
- PC PLC CABINET
- MC MOTOR CONTROL CENTER

NOTES:

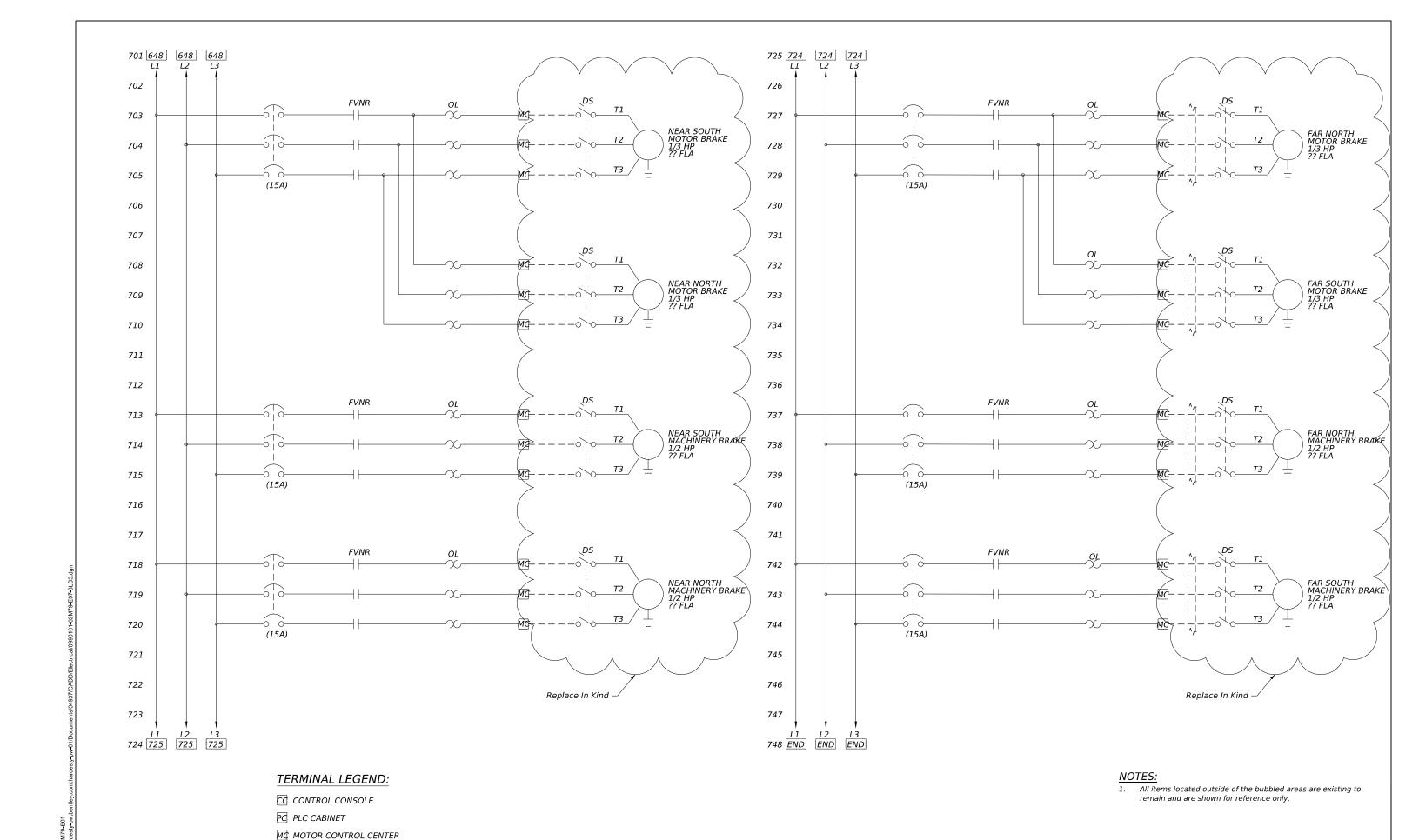
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

3 LINE SCHEMATIC DIAGRAM II STRUCTURE NO. 099-0101

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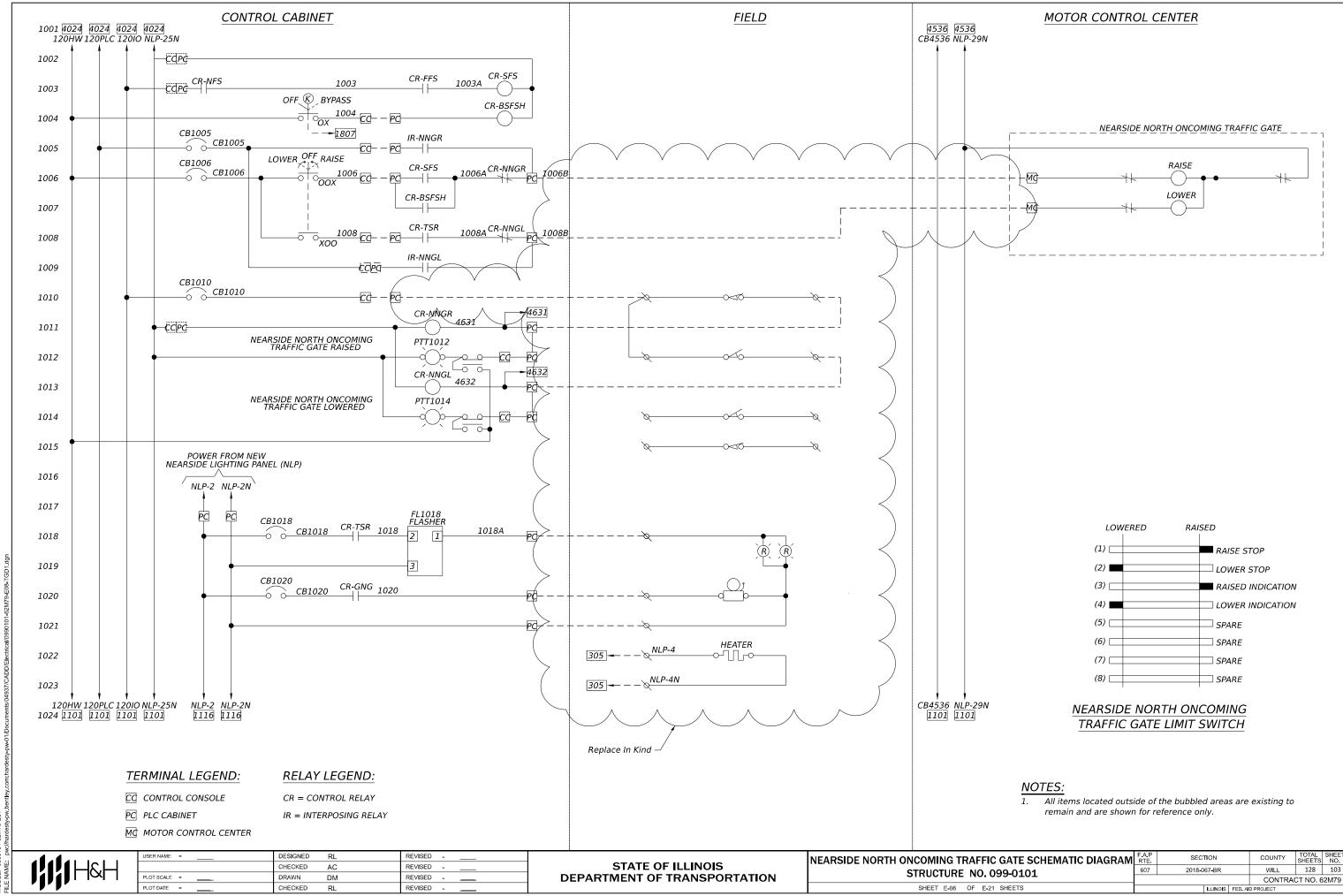


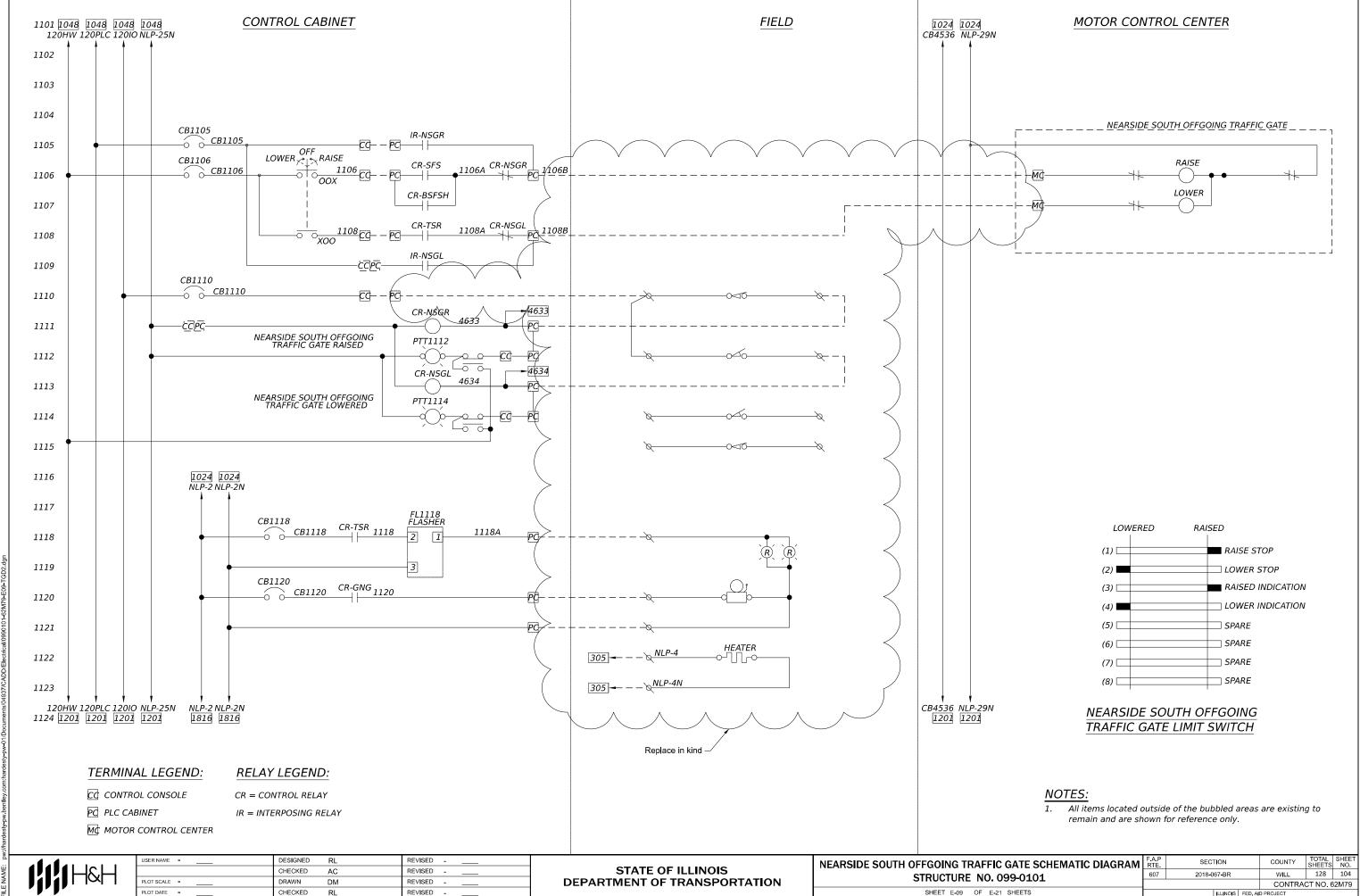
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

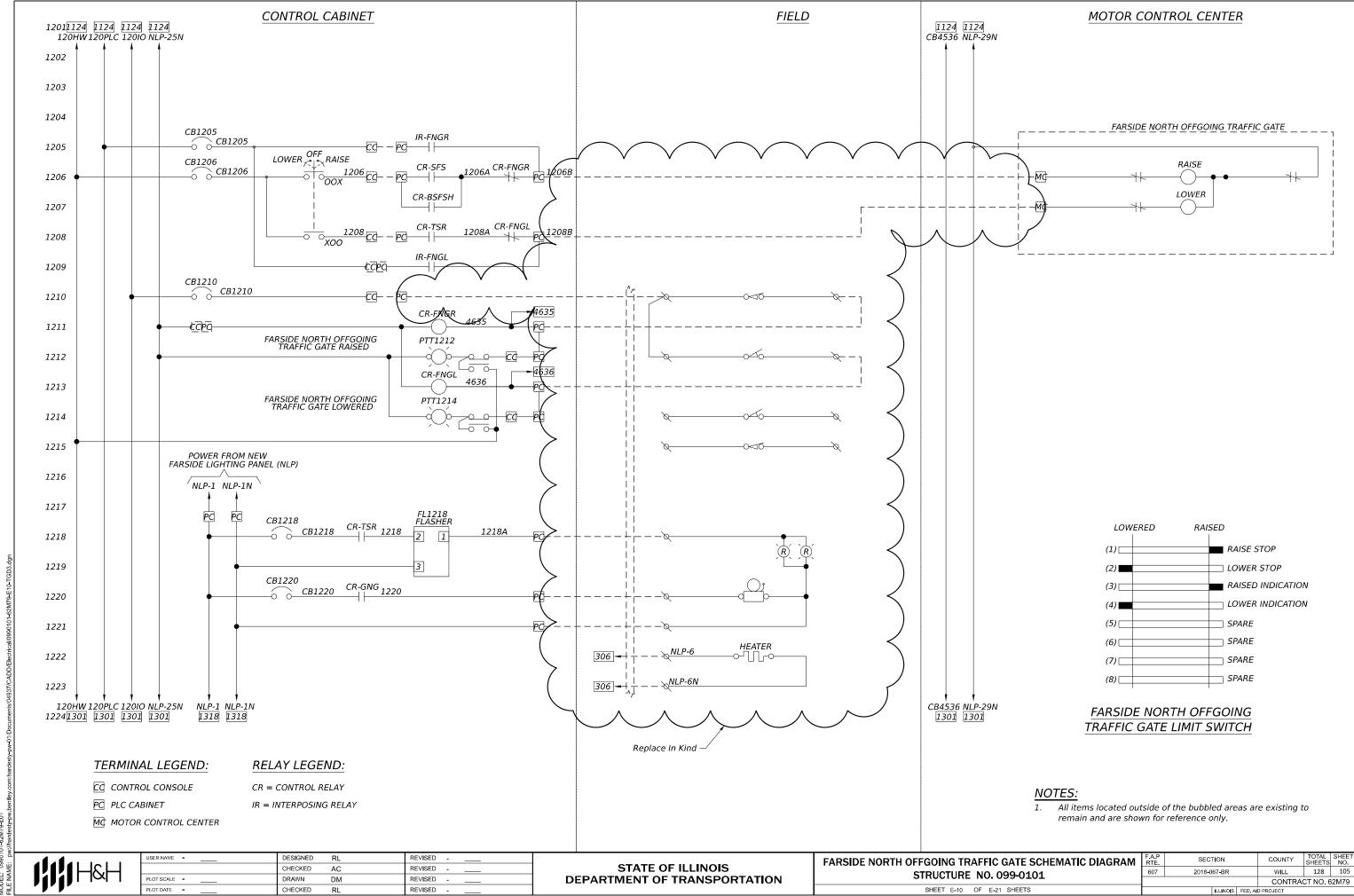
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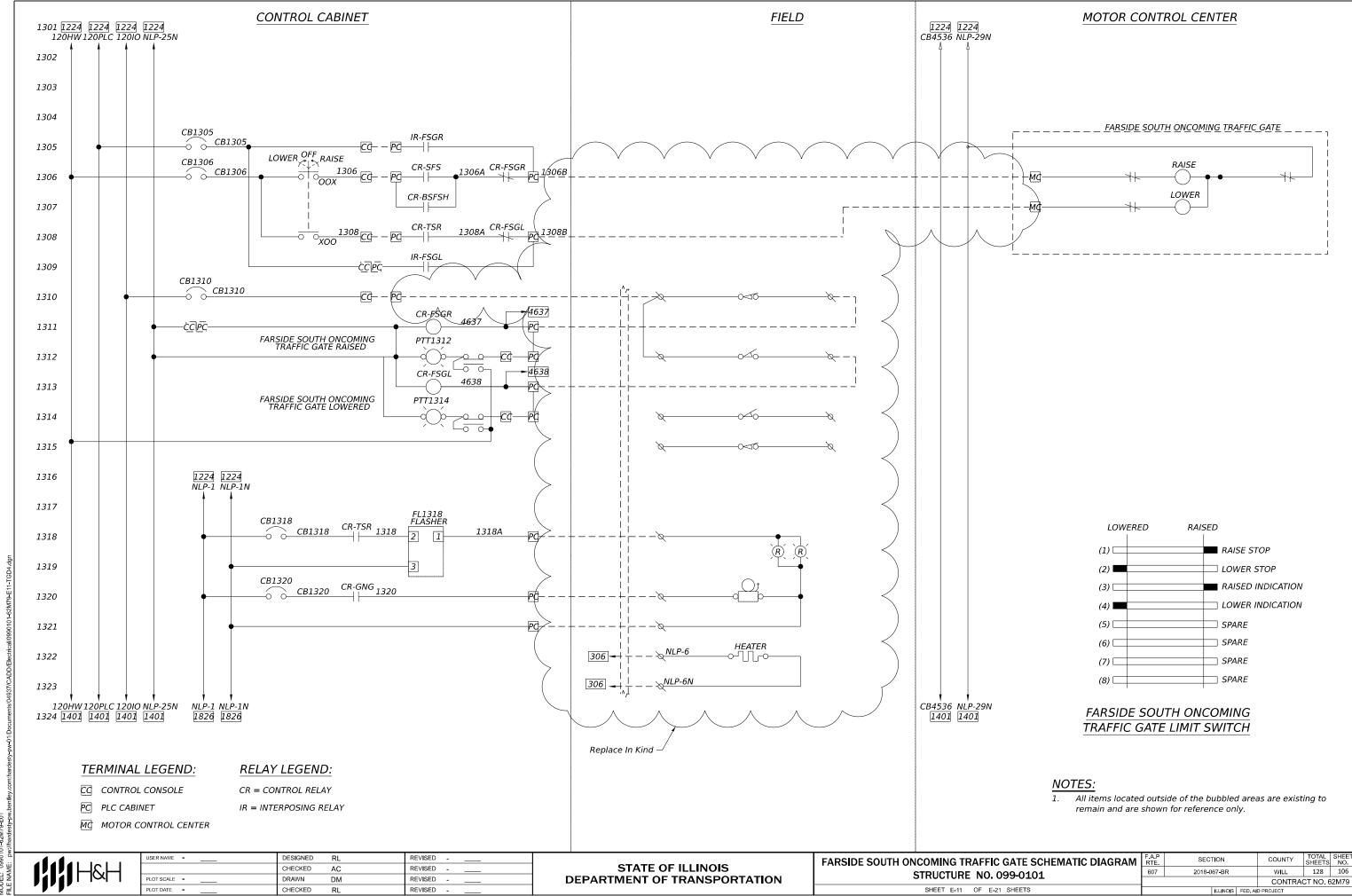




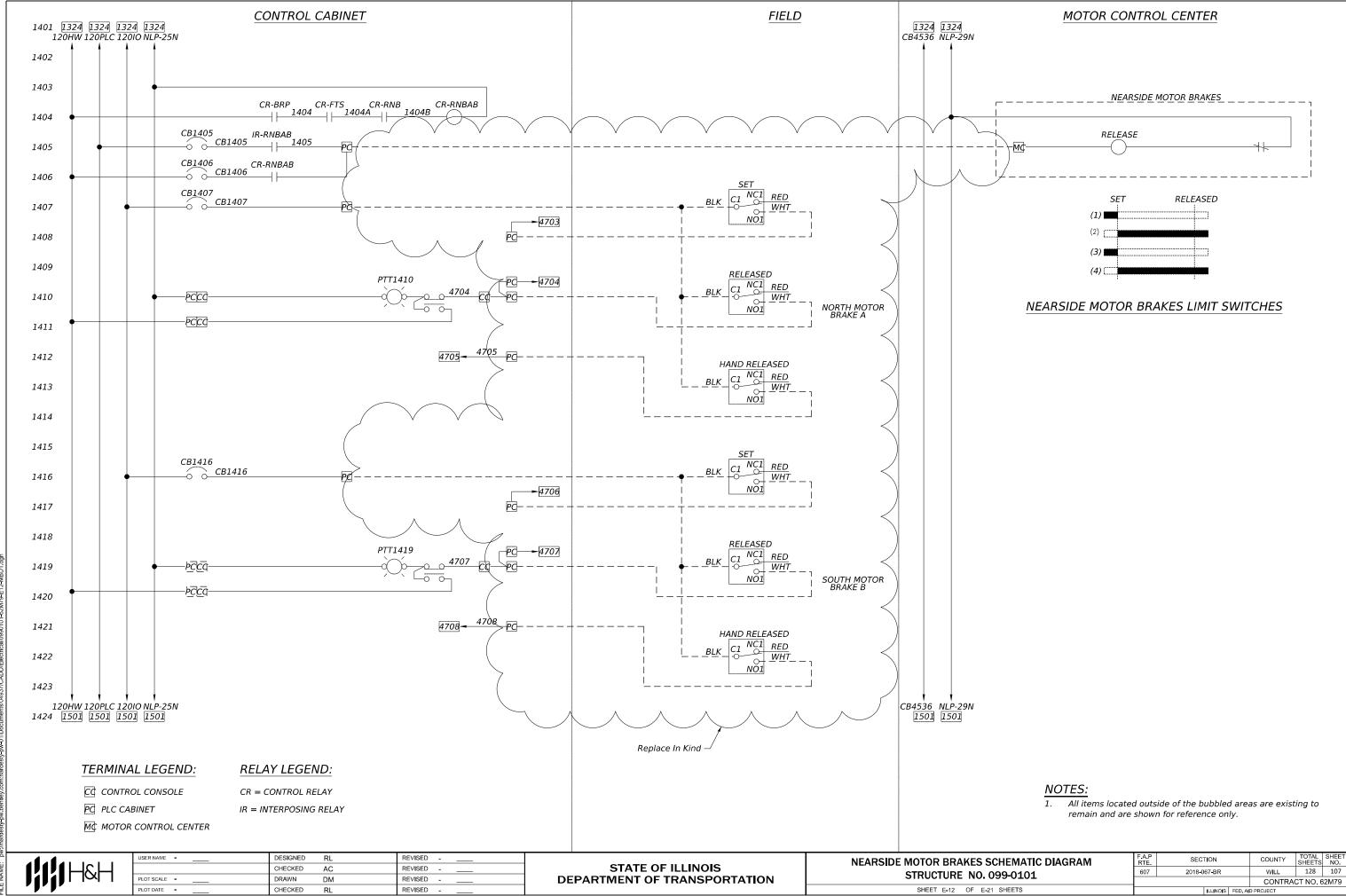
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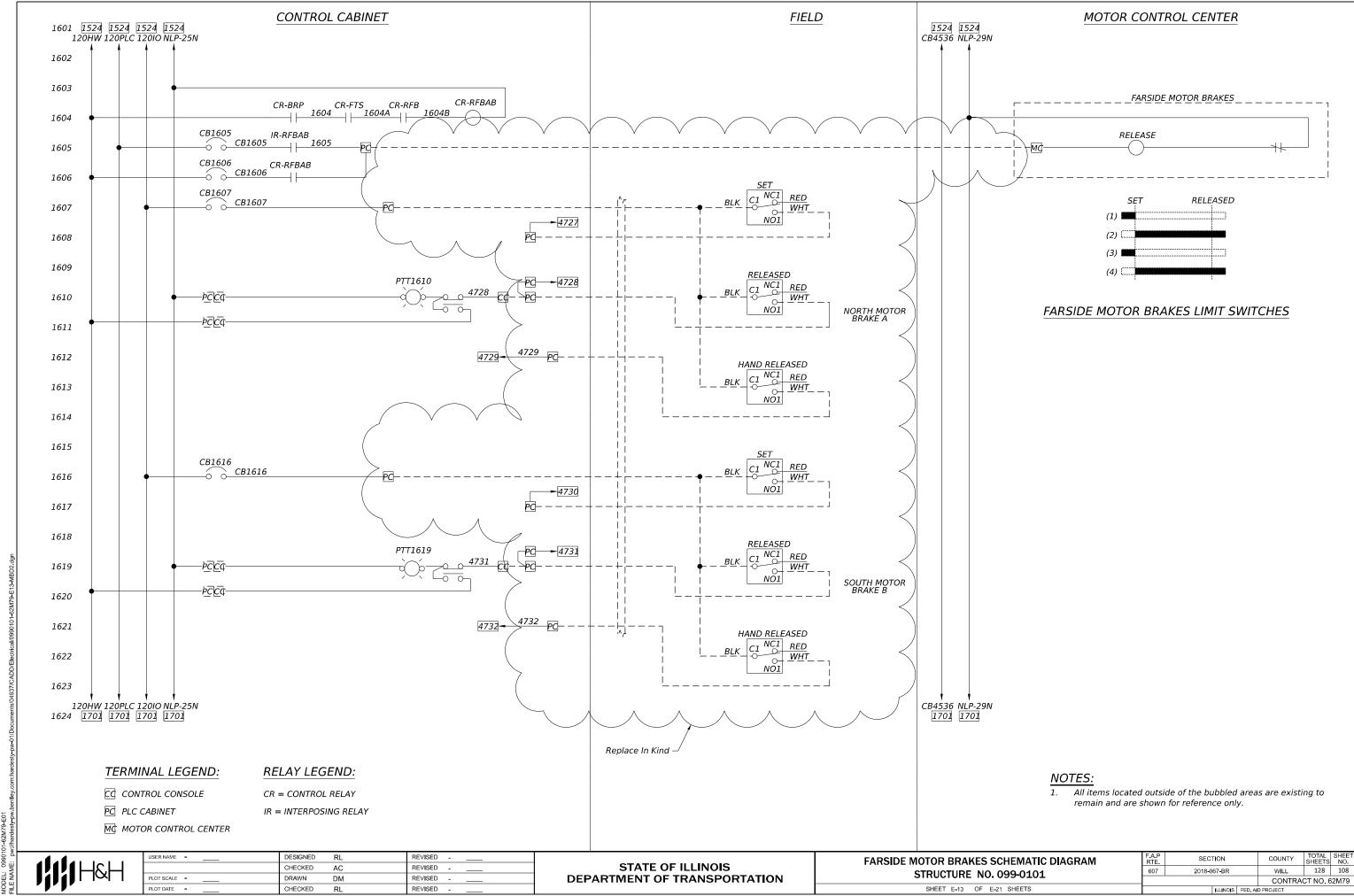
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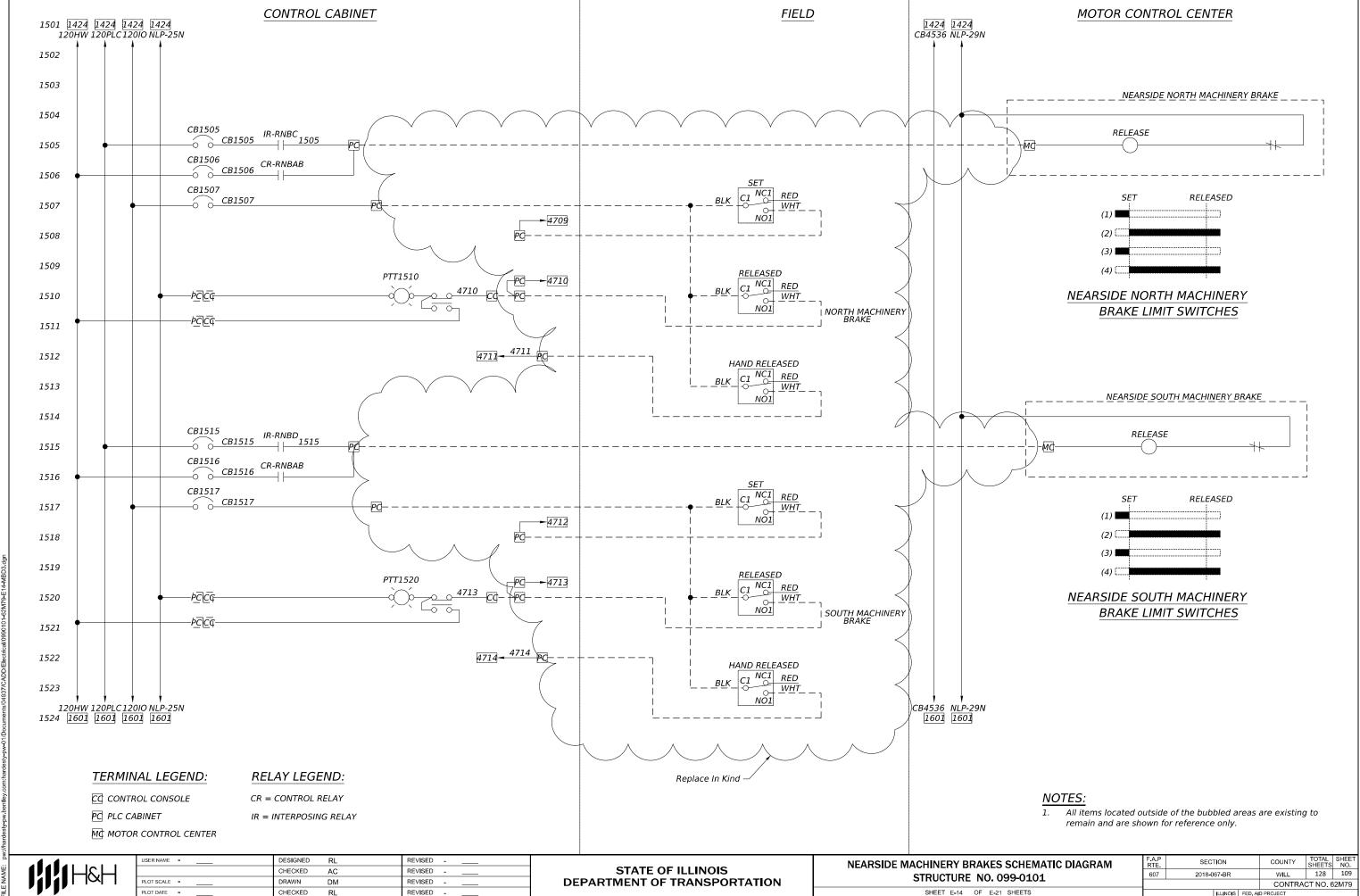
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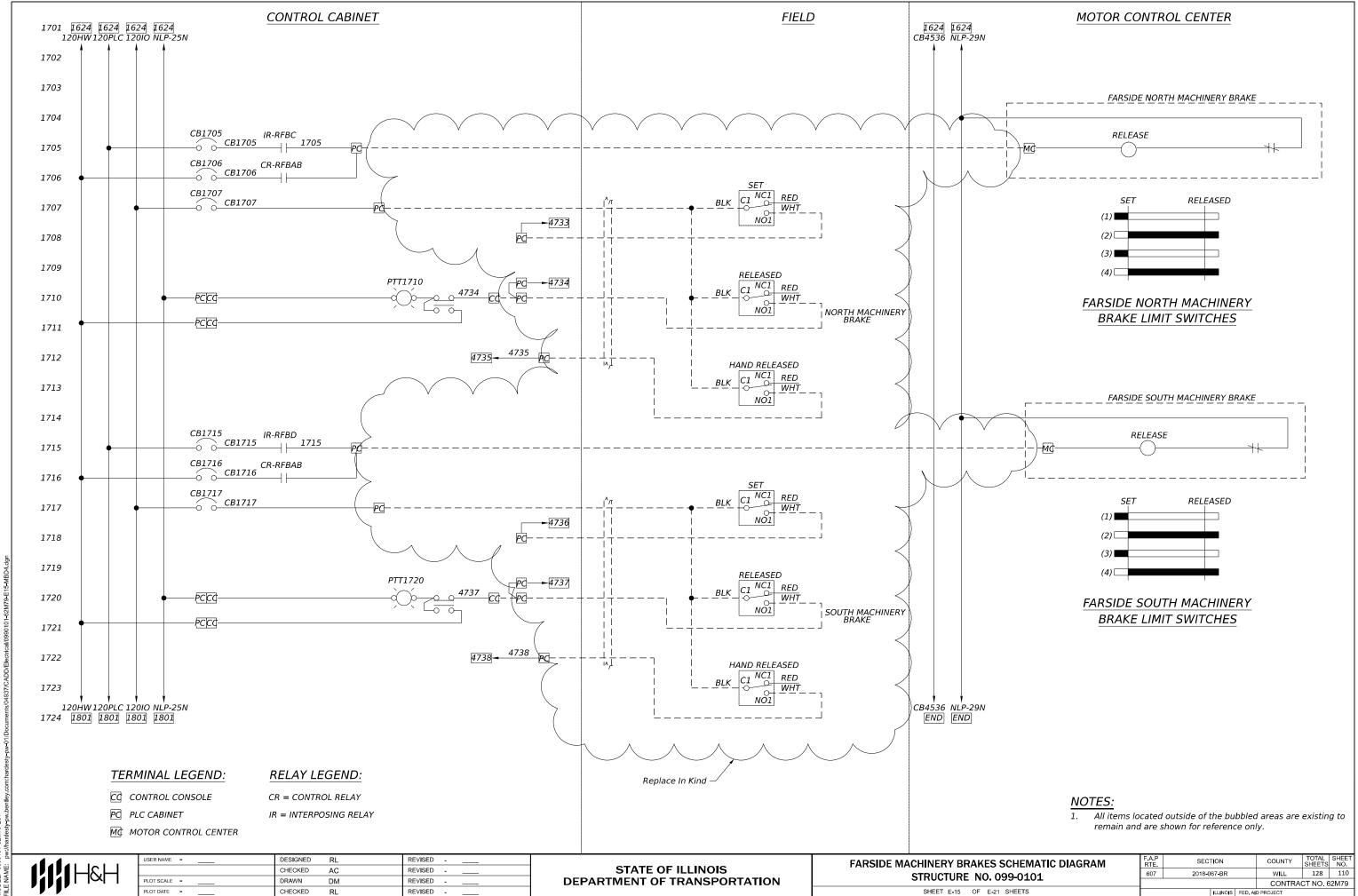
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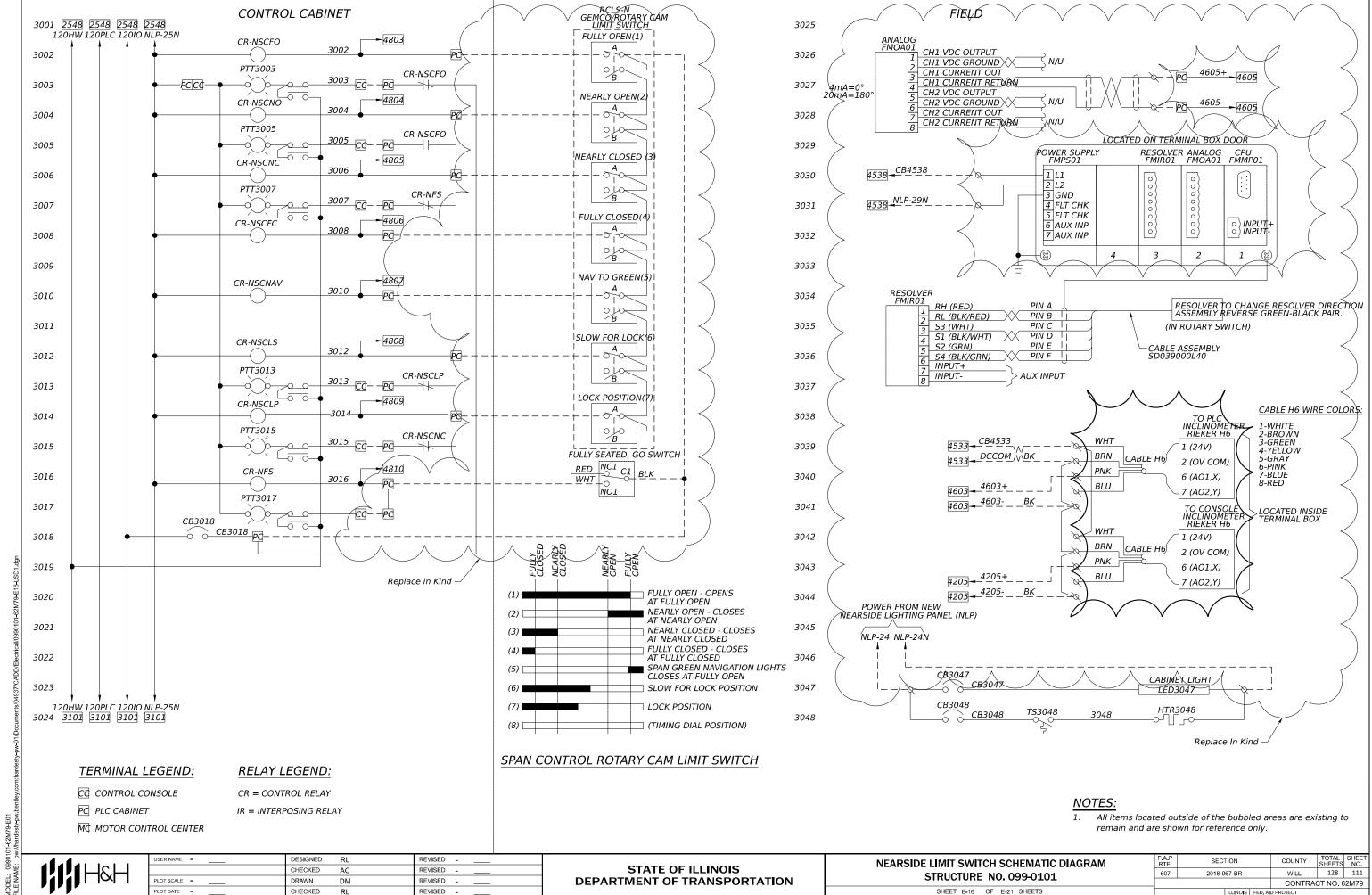
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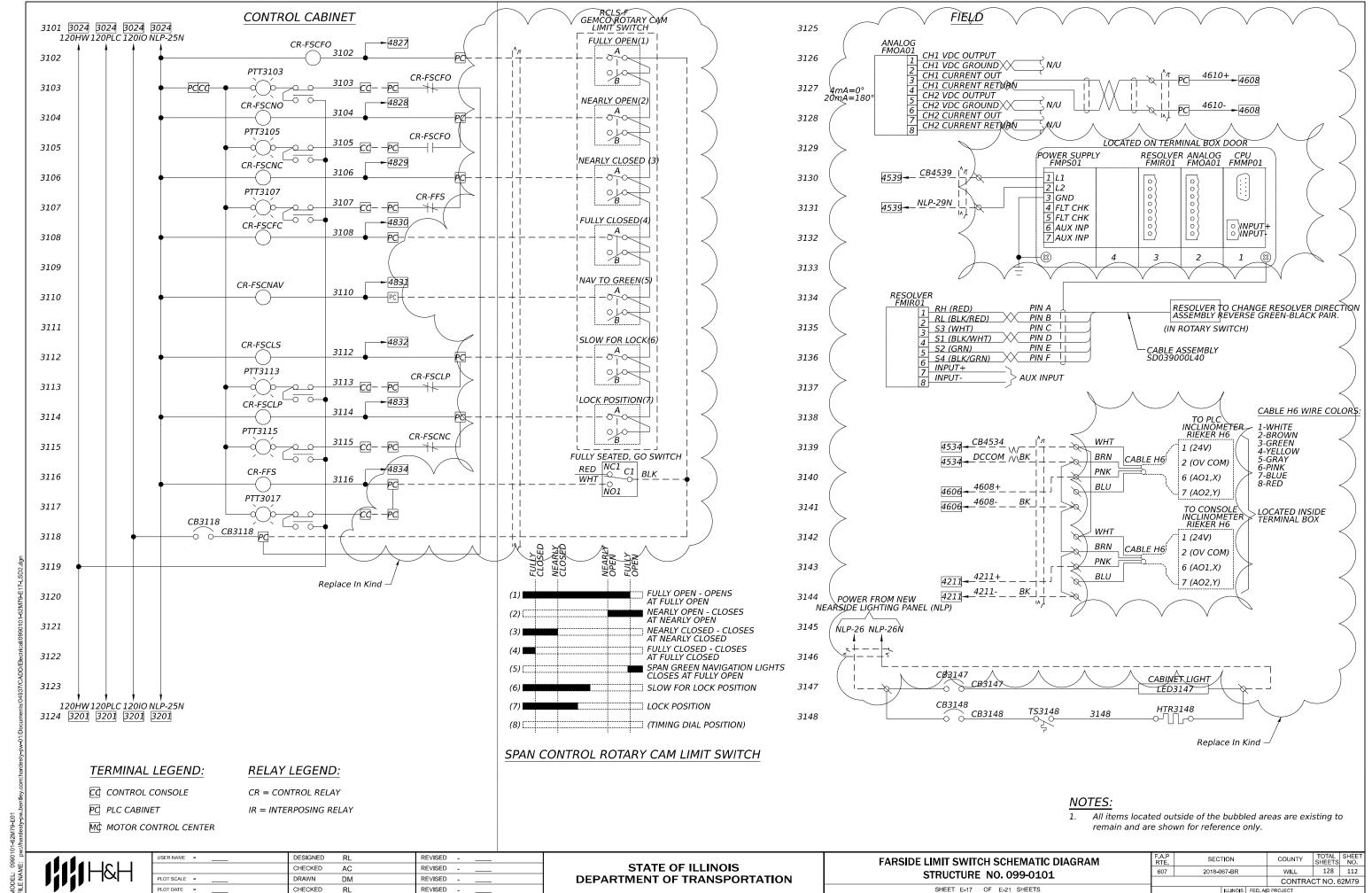
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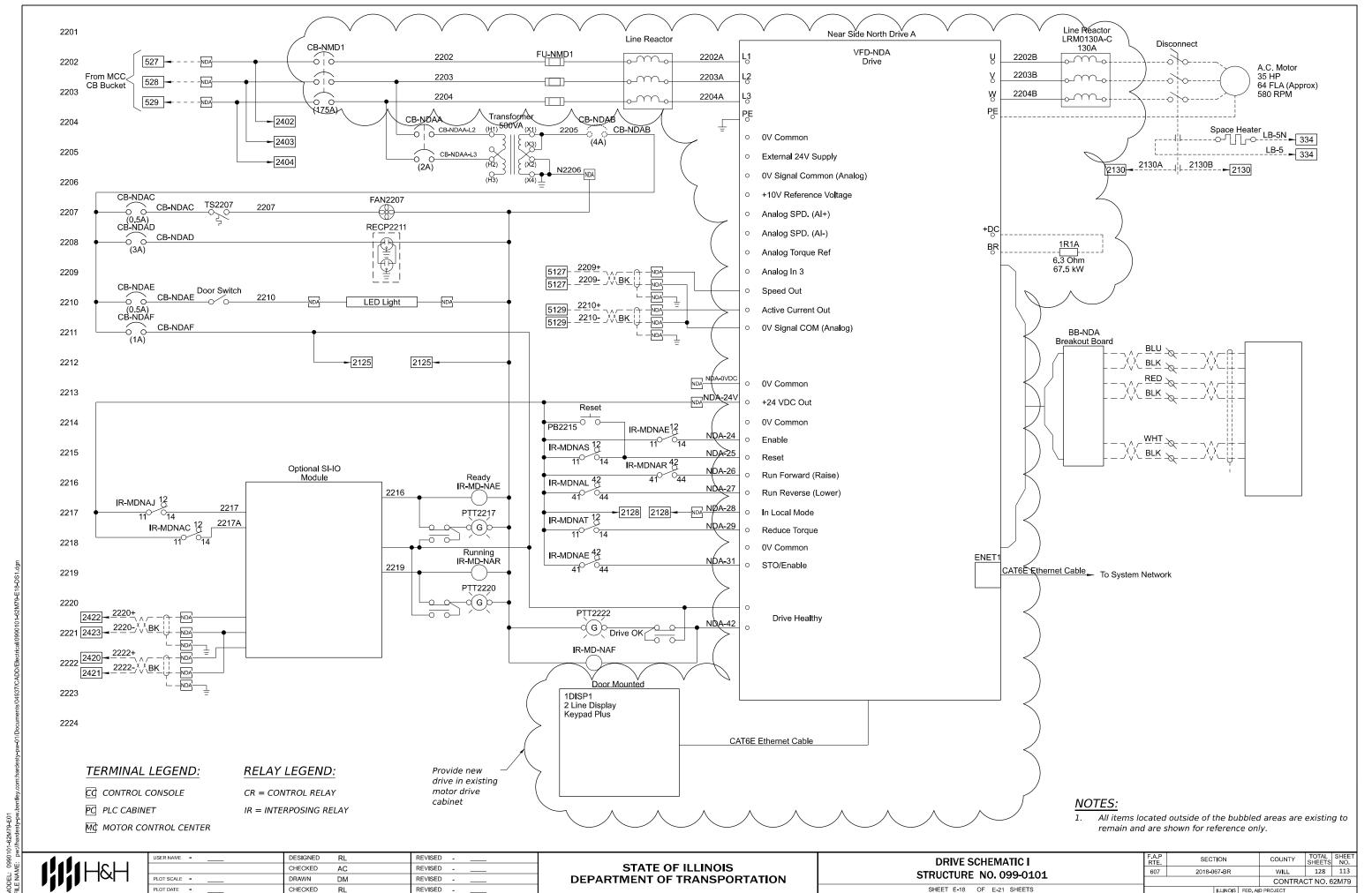
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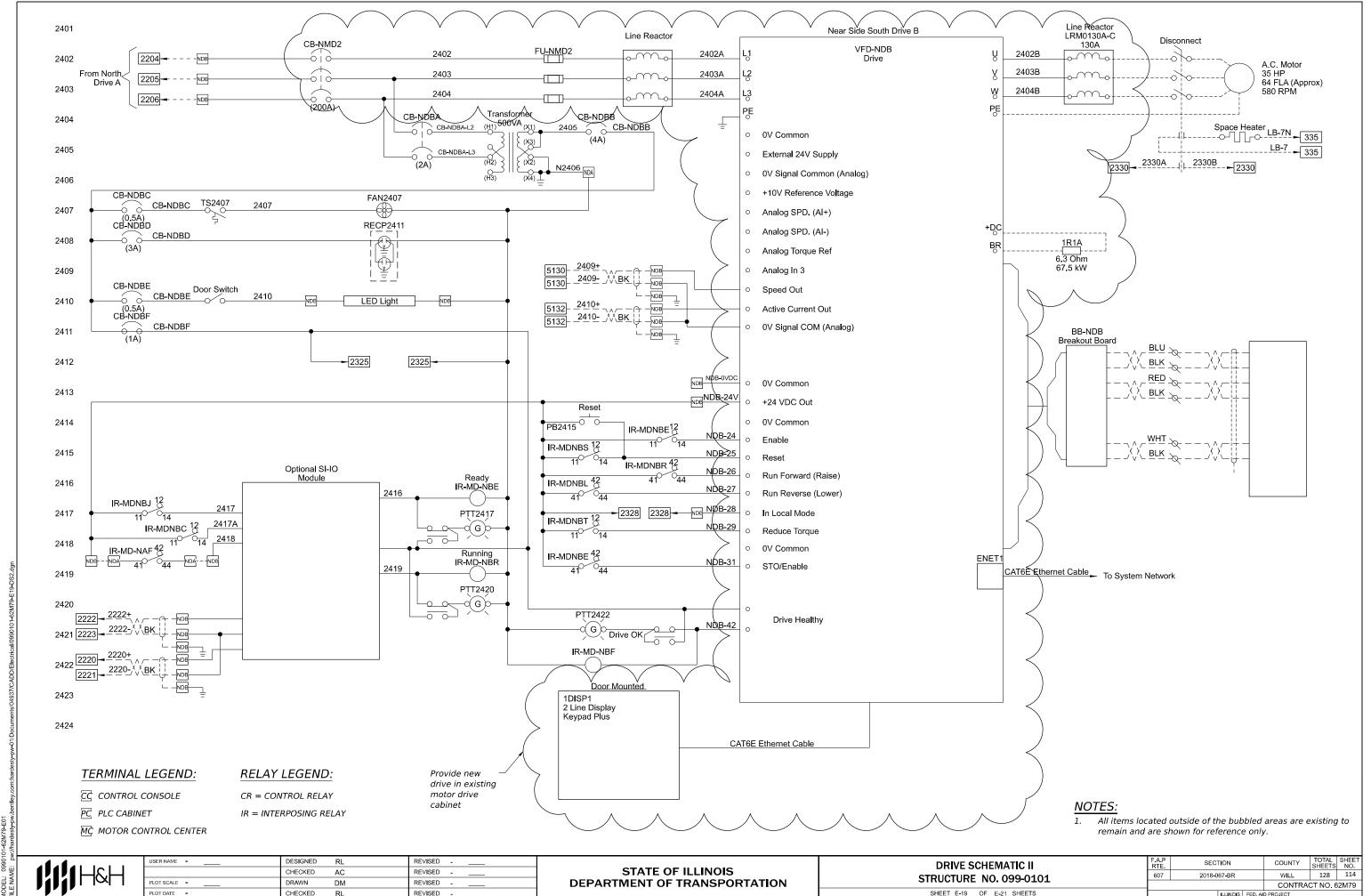
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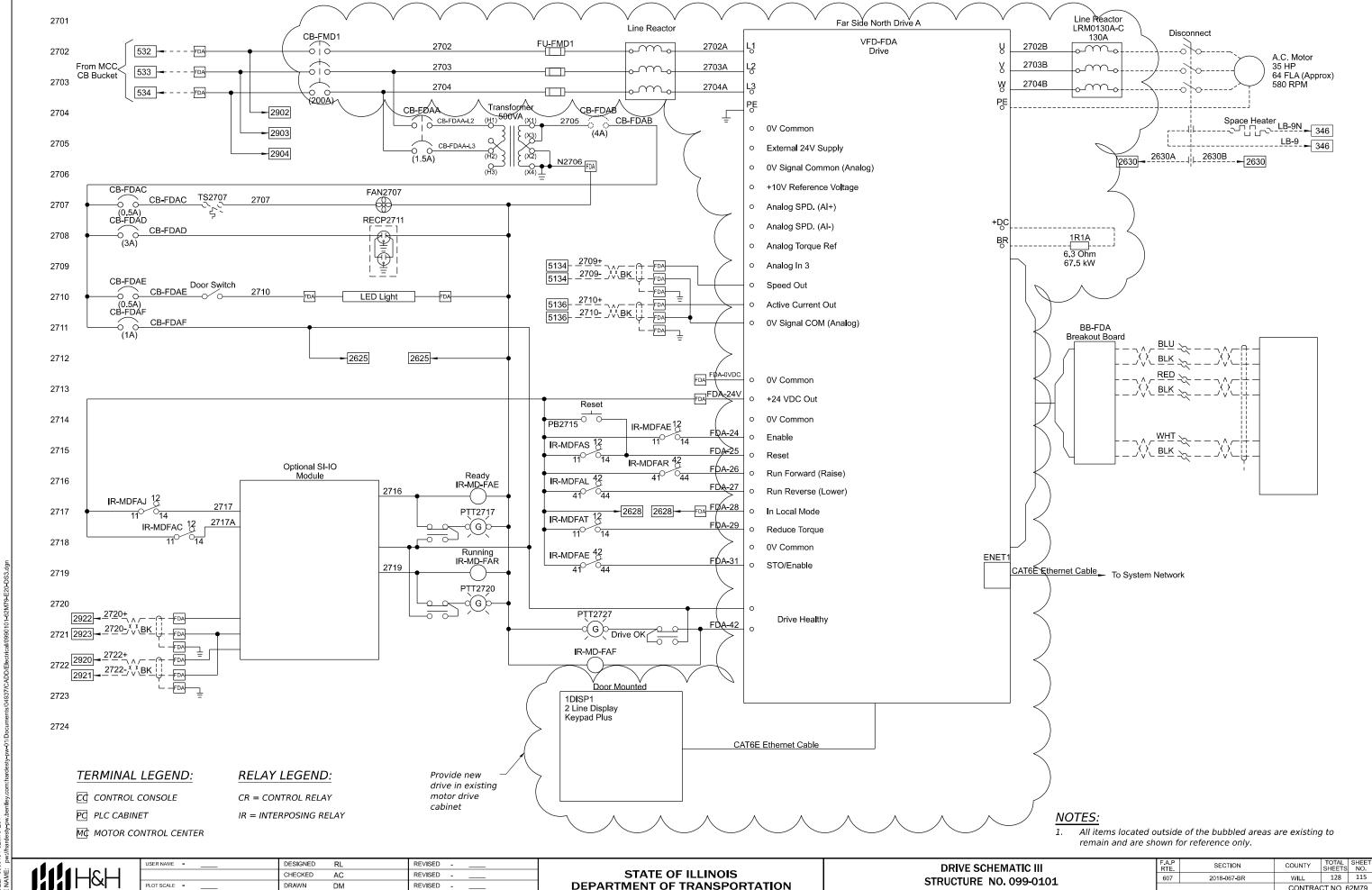
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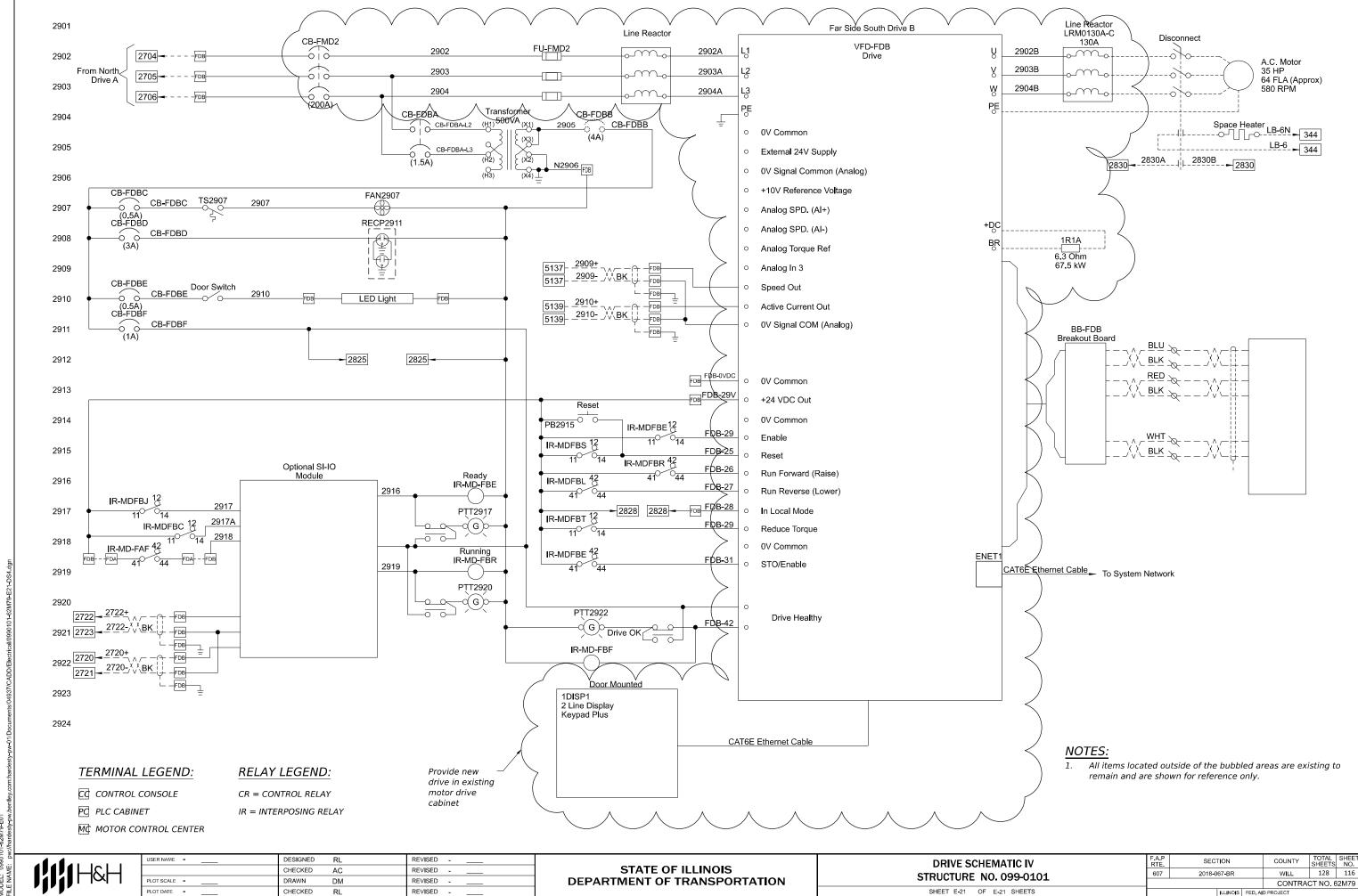
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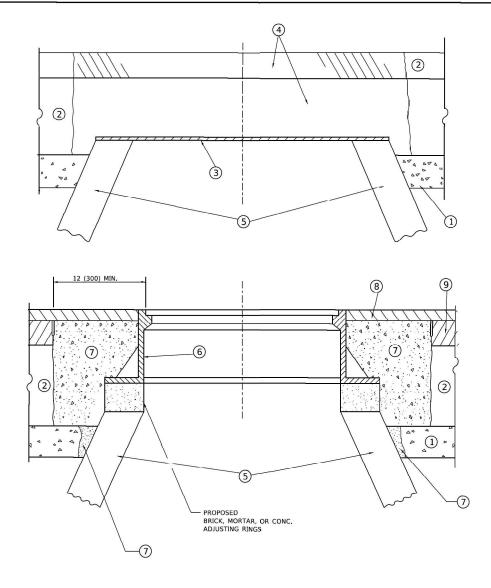
DEPARTMENT OF TRANSPORTATION

SHEET E-20 OF E-21 SHEETS

CONTRACT NO. 62M79



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DETAILS FOR FRAMES AND LIDS ADJUSTMENT WITH MILLING

NOTES

- 1. EXISTING BROKEN FRAMES AND LIDS SHALL BE REMOVED AND DISPOSED OF BY THE CONTRACTOR AND SHALL BE REPLACED AS DIRECTED BY THE ENGINEER. REPLACEMENT FRAMES AND LIDS WILL BE PAID FOR IN ACCORDANCE WITH ARTICLE 109.04 OF THE STANDARD SPECIFICATIONS UNLESS A SEPARATE PAY ITEM HAS BEEN PROVIDED.
- 2. IF THE EXISTING LIDS ARE OPEN, THE FRAME WILL BE ADJUSTED TO THE ELEVATION OF THE MILLED PAVEMENT SURFACE PRIOR TO THE MILLING OPERATION. THE FRAME WILL NOT BE REMOVED AND COVERED BY THE METAL PLATE.
- 3. CITY OF CHICAGO CASTINGS ARE THE PROPERTY OF THE CITY AND THE CONTRACTOR SHALL NOTIFY THE CITY FOR REMOVAL AND DISPOSITION OF THE CASTINGS.
- 4. THE METAL PLATE USED TO COVER THE STRUCTURE SHALL REMAIN THE PROPERTY OF THE CONTRACTOR.
- 5. THE CONTRACTOR SHALL REMOVE ALL TRAFFIC CONTROL DEVICES BY THE END OF EACH WORK SHIFT.

CONSTRUCTION PROCEDURES

STAGE 1 (BEFORE PAVEMENT MILLING)

- A) REMOVE A MINIMUM OF 12 (300) OF THE PAVEMENT FROM AROUND THE STRUCTURE.
- B) REMOVE THE EXISTING FRAME AND LID FROM THE STRUCTURE.
- C) COVER THE STRUCTURE OPENING WITH A 36 (900) DIAMETER METAL PLATE.
- D) BACKFILL WITH CRUSHED STONE AND HMA SURFACE MIX APPROVED BY THE ENGINEER. (MIN. 3 (80) HMA TO REMAIN AFTER MILLING).

STAGE 2 (AFTER PAVEMENT MILLING)

- A) REMOVE THE HMA SURFACE MIX AND CRUSHED STONE.
- B) INSTALL THE FRAME AND LID; ADJUST THE FRAME TO ITS FINAL SURFACE ELEVATION.
- C) THE SURROUNDING SPACE SHALL BE FILLED WITH CLASS PP-2* CONCRETE TO THE ELEVATION OF THE SURFACE OF THE EXISTING BASE COURSE OR THE BINDER COURSE.
- *UNLESS OTHERWISE SPECIFIED IN THE PLANS.

THE PROCEDURE EXPLAINED ABOVE SHALL CONFORM TO THE APPLICABLE PORTIONS OF SECTIONS 353, 406, 602, AND 603 OF THE STANDARD SPECIFICATIONS. EXCEPT THAT "THE CONTRACTOR SHALL ADJUST THE STRUCTURES TO THE FINISHED PAVEMENT ELEVATION NO MORE THAN 5 CALENDAR DAYS PRIOR TO PLACEMENT OF THE FINAL LIFT OF SURFACE UNLESS APPROVED BY THE ENGINEER."

- ① SUB-BASE GRANULAR MATERIAL
- 6 FRAME AND LID (SEE NOTES)
- (2) EXISTING PAVEMENT

5 EXISTING STRUCTURE

- (7) CLASS PP-2* CONCRETE
- 3 36 (900) DIAMETER METAL PLATE
- 8 PROPOSED HMA SURFACE COURSE
- 4 PROPOSED CRUSHED STONE AND HMA SURFACE MIX
- 9 PROPOSED HMA BINDER COURSE

LOCATION OF STRUCTURES

THE CONTRACTOR WILL BE REQUIRED TO KEEP A RECORD OF THE LOCATIONS OF THE BURIED STRUCTURES ACCORDING TO THE STATION AND DISTANCE LEFT OR RIGHT OF THE CENTERLINE OF PAVEMENT. UPON COMPLETION OF THE WORK, THE CONTRACTOR WILL DELIVER THE RECORD TO THE ENGINEER.

BASIS OF PAYMENT

- 1. REMOVING FRAMES AND LIDS ON DRAINAGE AND UTILITY STRUCTURES IN THE PAVEMENT PRIOR TO MILLING, AND ADJUSTING TO FINAL GRADE PRIOR TO PLACING THE SURFACE COURSE, WILL BE PAID FOR AT THE CONTRACT UNIT PRICE EACH FOR "FRAMES AND LIDS TO BE ADJUSTED
- 2. THIS WORK WILL NOT BE PAID FOR WHEN DRAINAGE AND UTILITY STRUCTURES ARE SPECIFIED FOR PAYMENT AS STRUCTURE RECONSTRUCTION.
- 3. NEW FRAMES AND LIDS, WHEN SPECIFIED, WILL BE PAID FOR SEPARATELY.
- 4. WHEN STRUCTURES ARE TO BE ADJUSTED OR RECONSTRUCTED, THE LOWERING AND RAISING OF THE FRAMES AND LIDS WILL NOT BE PAID FOR SEPARATELY BUT WILL BE INCLUDED IN THE COST OF THE CORRESPONDING PAY ITEM.

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN

JSER NAME = Lawrence, DeManch DESIGNED -R. SHAH REVISED - R. BORO 03-09-11 TOTAL SHEET SHEETS NO. COUNTY **DETAILS FOR** DRAWN REVISED - R. BORO 12-06-11 STATE OF ILLINOIS FRAMES AND LIDS ADJUSTMENT WITH MILLING PLOT SCALE = 100.0000 ' / in. CHECKED **DEPARTMENT OF TRANSPORTATION** REVISED - K. SMITH 11-18-22 CONTRACT NO. PLOT DATE = 9/15/202: 10-25-94 REVISED - K. SMITH 09-15-23 SCALE: NONE SHEET 1 OF 1 SHEETS STA.

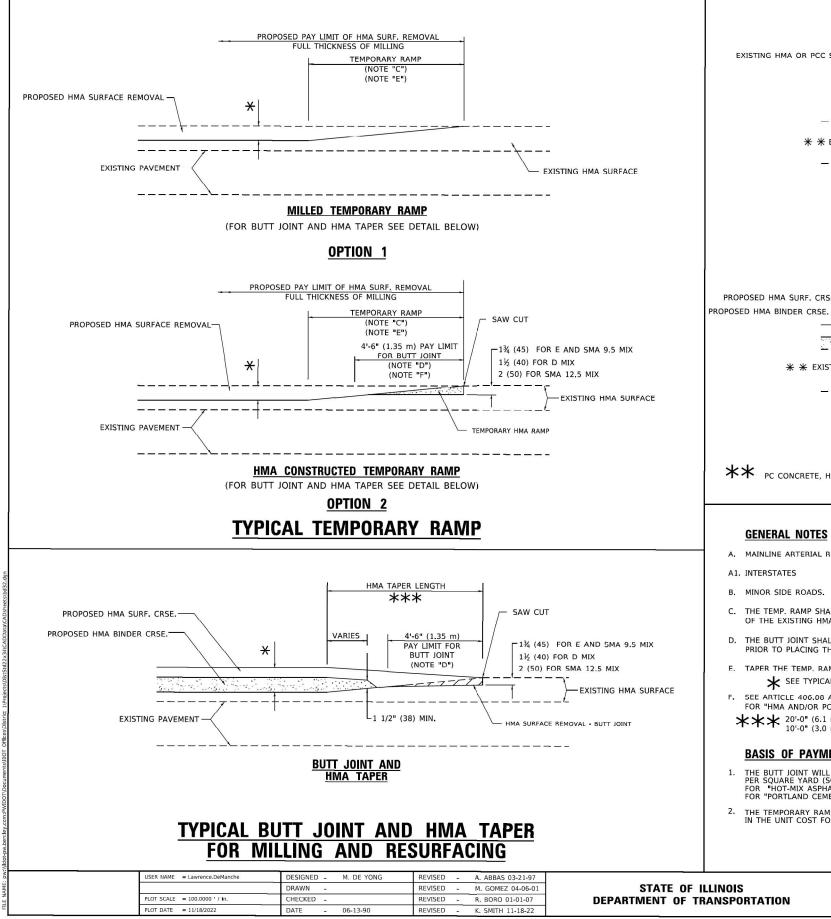
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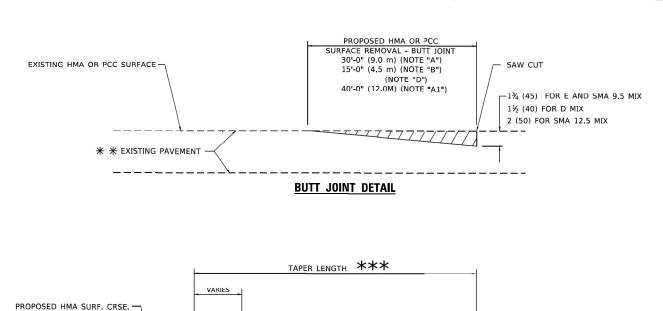
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

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TOTAL SHEE SHEETS NO. SECTION COUNTY 607 2018-067-BP WILL 128 117 CONTRACT NO. 62M79





TYPICAL BUTT JOINT AND HMA TAPER FOR RESURFACING ONLY

HMA TAPER DETAIL

** PC CONCRETE, HMA OR HMA RESURFACED PAVEMENT

* * EXISTING PAVEMENT

GENERAL NOTES

- A. MAINLINE ARTERIAL ROADWAYS AND MAJOR SIDE ROADS.
- A1. INTERSTATES
- B. MINOR SIDE ROADS.
- C. THE TEMP. RAMP SHALL BE CONSTRUCTED IMMEDIATELY UPON REMOVAL OF THE EXISTING HMA SURFACE.
- THE BUTT JOINT SHALL BE CONSTRUCTED IMMEDIATELY PRIOR TO PLACING THE PROPOSED HMA COURSES.
- TAPER THE TEMP. RAMP AT A RATE OF 3' 4" (1.02m) PER 1 INCH (25 mm) OF MILLING THICKNESS. * SEE TYPICAL SECTIONS FOR MILLING THICKNESS.
- SEE ARTICLE 406.08 AND 406.14 OF THE STANDARD SPECIFICATIONS
- FOR "HMA AND/OR PCC SURFACE REMOVAL, BUTT JOINT". *** 20'-0" (6.1 m) PER 1 (25) RESURFACING (NOTE "A") 10'-0" (3.0 m) PER 1 (25) RESURFACING (NOTE "B")

BASIS OF PAYMENT

- THE BUTT JOINT WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER SQUARE YARD (SQUARE METER) FOR "HOT-MIX ASPHALT SURFACE REMOVAL BUTT JOINT" OR FOR "PORTLAND CEMENT CONCRETE SURFACE REMOVAL- BUTT JOINT".
- THE TEMPORARY RAMP AND SAW CUT SHALL BE INCLUDED IN THE UNIT COST FOR HMA OR PCC SURFACE REMOVAL-BUTT JOINT.

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS

-1¾ (45) FOR E AND SMA 9.5 MIX

1½ (40) FOR D MIX 2 (50) FOR SMA 12.5 MIX

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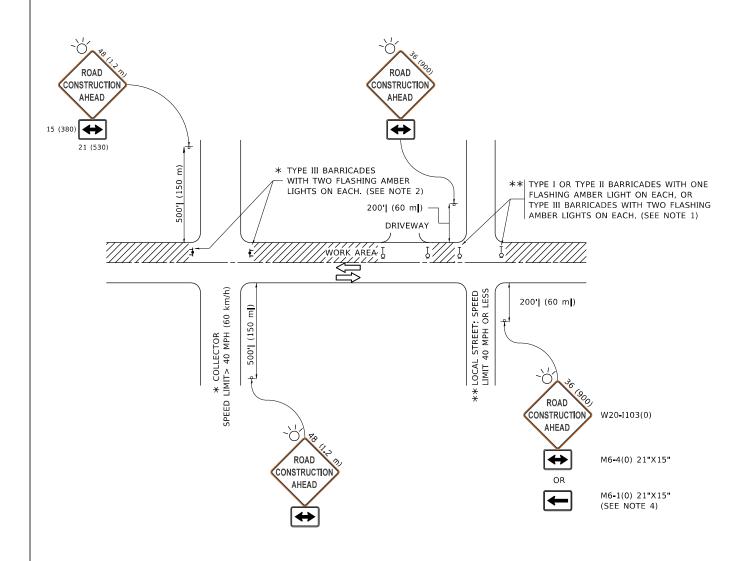
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NOTES:

- 1. SIDE ROAD WITH A SPEED LIMIT OF 40 MPH (60 km/h) OR LESS AS SHOWN ON THE DRAWING AND AS DIRECTED BY THE ENGINEER:
- a) ONE "ROAD CONSTRUCTION AHEAD" SIGN 36 x 36 (900x900) WITH A FLASHER MOUNTED ON IT APPROXIMATELY 200* (60 m) IN ADVANCE OF THE MAIN ROUTE.
- b) THE CLOSED PORTION OF THE MAIN ROUTE SHALL BE PROTECTED BY BLOCKING WITH TYPE I, TYPE II OR TYPE III BARRICADES, 1/3 OF THE CROSS SECTION OF THE CLOSED PORTION.
- SIDE ROAD WITH A SPEED LIMIT GREATER THAN 40 MPH (60 km/h) AS SHOWN ON THE DRAWING AND AS DIRECTED BY THE ENGINEER:
 - a) ONE "ROAD CONSTRUCTION AHEAD" SIGN 48 x 48 (1.2 m x 1.2 m) WITH A FLASHER MOUNTED ON IT APPROXIMATELY 500" (150 m) IN ADVANCE OF THE MAIN ROUTE.
- THE CLOSED PORTION OF THE MAIN ROUTE SHALL BE PROTECTED BY
 b) BLOCKING WITH TYPE III BARRICADES, 1/2 OF THE CROSS SECTION
 OF THE CLOSED PORTION.
- 3. CONES MAY BE SUBSTITUTED FOR BARRICADES OR DRUMS AT HALF THE SPACING DURING DAY OPERATIONS. CONES SHALL BE A MINIMUM OF 28 (710) IN HEIGHT
- WHEN THE SIDE ROAD LIES BETWEEN THE BEGINNING OF THE MAINLINE
 4. SIGNING AND THE WORK ZONE, A SINGLE HEADED ARROW (M6-1) SHALL
 BE USED IN LIEU OF THE DOUBLE HEADED ARROW (M6-4).

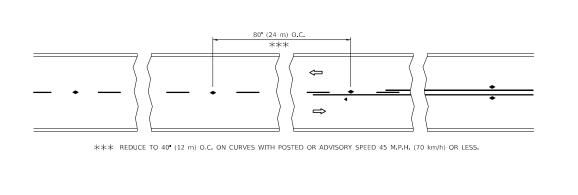
- WHEN WORK IS BEING PERFORMED ON A SIDE ROAD OR DRIVEWAY, FOLLOW THE APPLICABLE STANDARD(S). THE DIRECTIONAL ARROW (M6-1 OR M6-4) SHALL BE COVERED OR REMOVED WHEN NO LONGER CONSISTENT WITH THE TRAFFIC CONTROL SET-UP.
- 6. ADVANCE WARNING SIGNS ARE TO BE OMITTED ON DRIVEWAYS UNLESS OTHERWISE SPECIFIED IN THE PLANS OR BY THE ENGINEER
- THE TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS SHALL BE INCLUDED IN THE COST OF SPECIFIED TRAFFIC CONTROL STANDARDS OR ITEMS.

All dimensions are in inches (millimeters) unless otherwise shown.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

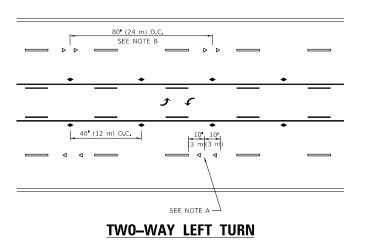
TRAFFIC CONTROL AND PROTECTION FOR SIDE ROADS, INTERSECTIONS, AND DRIVEWAYS

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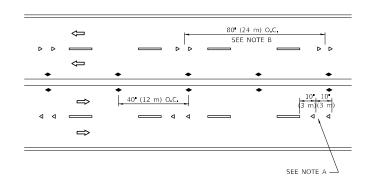


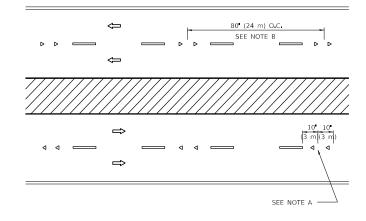
LANE REDUCTION TRANSITION

SEE FIGURE 3B-14 MUTCD



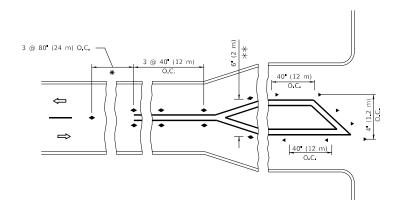
TWO-LANE/TWO-WAY

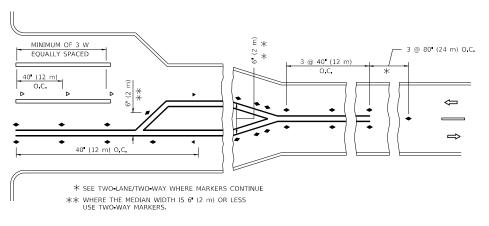




MULTI-LANE/UNDIVIDED







TURN LANES

GENERAL NOTES

- 1. MARKERS USED WITH DASHED LINES SHALL BE CENTERED IN THE GAP BETWEEN SEGMENTS.
- 2. MARKERS USED ADJACENT TO SOLID LINES SHALL BE OFFSET 2 TO 3 (50 TO 75) TOWARD TRAFFIC AS SHOWN.
- 3. MARKERS THROUGH TANGENTS LESS THAN 500 (150 m) IN LENGTH BETWEEN CURVES SHALL BE INSTALLED AT THE LESSER OF THE TWO CURVE SPACINGS,
- 4. MARKERS ARE TO BE USED ADJACENT TO BOTH SOLID WHITE LINES IN DUAL LEFT TURN LANES

LANE MARKER NOTES

- A. USE DOUBLE LANE LINE MARKERS SPACED AS SHOWN.
- B. REDUCE TO 40 (12 m) O.C. ON CURVES WHERE ADVISORY SPEEDS ARE 10 M.P.H (20 km/h) LOWER THAN POSTED SPEEDS.

DESIGN NOTES

- 1. DOUBLE LANE LINE MARKERS SHALL BE USED UNLESS SPECIFIED OTHERWISE.
- EXCEPT AS SHOWN ON THE LANE REDUCTION TRANSITION AND FREEWAY EXIT RAMP DETAIL, MARKERS ARE NOT TO BE SPECIFIED ON RIGHT EDGE LINES.
- 3. THE EXACT MARKER LIMITS, SPACING, AND COLOR SHALL BE INCLUDED IN THE PLANS WHEN STANDARD SPECIFICATIONS ARE NOT BEING USED.
- MARKERS SHOULD NOT BE USED ALONGSIDE CURBS EXCEPT FOR EXTREMELY SHORT SECTIONS OF CURBS WHERE NOT MORE THAN TWO MARKERS WOULD BE INVOLVED.

All dimensions are in inches (millimeters) unless otherwise shown.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TYPICAL APPLICATIONS

RAISED REFLECTIVE PAVEMENT MARKERS (SNOW-PLOW RESISTANT)

SCALE: NONE SHEET 1 OF 1 SHEETS STA. TO STA.

P. SECTION COUNTY TOTAL SHEETS NO.
7 2018-067-BR WILL 128 120

TC-11 CONTRACT NO.

SYMBOLS

ONE-WAY AMBER MARKER

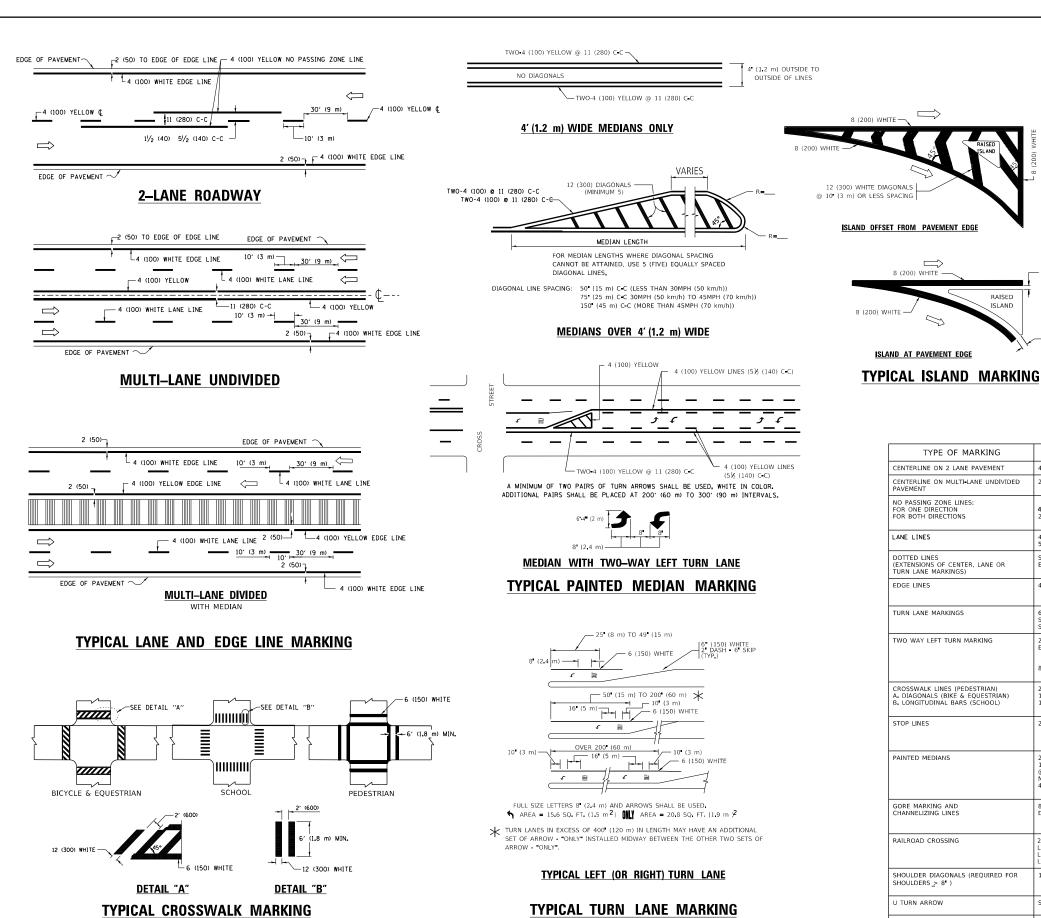
TWO-WAY AMBER MARKER

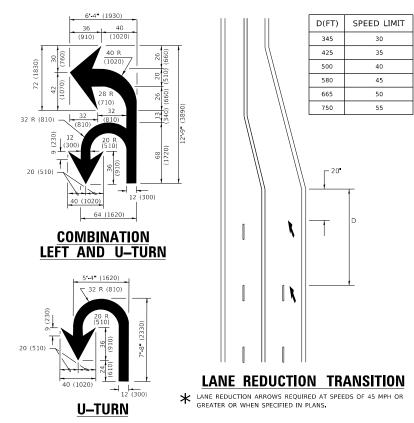
ONE-WAY CRYSTAL MARKER (W/O)

YELLOW STRIPE

WHITE STRIPE

MODEL: Default





TYPE OF MARKING	WIDTH OF LINE	PATTERN	COLOR	SPACING / REMARKS
CENTERLINE ON 2 LANE PAVEMENT	4 (100)	SKIP•DASH	YELLOW	10 (3 m) LINE WITH 30 (9 m) SPACE
CENTERLINE ON MULTI-LANE UNDIVIDED PAVEMENT	2 @ 4 (100)	SOLID	YELLOW	11 (280) C-C
NO PASSING ZONE LINES: FOR ONE DIRECTION FOR BOTH DIRECTIONS	4 (100) 2 @ 4 (100)	SOLID SOLID	YELLOW YELLOW	5½ (140) C-C FROM SKIP-DASH CENTERLINE 11 (280) C-C OMIT SKIP-DASH CENTERLINE BETWEEN
LANE LINES	4 (100) 5 (125) ON FREEWAYS	SKIP-DASH SKIP-DASH	WHITE WHITE	10" (3 m) LINE WITH 30" (9 m) SPACE
DOTTED LINES (EXTENSIONS OF CENTER, LANE OR TURN LANE MARKINGS)	SAME AS LINE BEING EXTENDED	SKIP-DASH	SAME AS LINE BEING EXTENDED	2* (600) LINE WITH 6* (1.8 m) SPACE
EDGE LINES	4 (100)	SOLID	YELLOW-LEFT WHITE-RIGHT	OUTLINE MEDIANS IN YELLOW
TURN LANE MARKINGS	6 (150) LINE; FULL SIZE LETTERS & SYMBOLS (8" (2.4m))	SOLID	WHITE	SEE TYPICAL TURN LANE MARKING DETAIL
TWO WAY LEFT TURN MARKING	2 @ 4 (100) EACH DIRECTION 8* (2.4m) LEFT ARROW	SKIP-DASH AND SOLID IN PAIRS	YELLOW	10 (3 m) LINE WITH 30 (9 m) SPACE FOR SKIP-DASH; 5½ (140) C-C BETWEEN SOLID LINE AND SKIP-DASH LINE SEE TYPICAL TWO-WAY LEFT TURN MARKING DETAIL
CROSSWALK LINES (PEDESTRIAN) A. DIAGONALS (BIKE & EQUESTRIAN) B. LONGITUDINAL BARS (SCHOOL)	2 @ 6 (150) 12 (300) @ 45° 12 (300) @ 90°	SOLID SOLID SOLID	WHITE WHITE WHITE	NOT LESS THAN 6' (1,8 m) APART 2' (600) APART 2' (600) APART 2' (600) APART SEE TYPICAL CROSSWALK MARKING DETAILS.
STOP LINES	24 (600)	SOLID	WHITE	PLACE 4 (1,2 m) IN ADVANCE OF AND PARALLEL TO CROSSWALK, IF PRESENT OTHERWISE, PLACE AT DESIRED STOPPING POINT. PARALLEL TO CROSSROAD CENTERLINE, WHERE POSSIBLE
PAINTED MEDIANS	2 @ 4 (100) WITH 12 (300) DIAGONALS @ 45° NO DIAGONALS USED FOR 4" (1,2 m) WIDE MEDIANS	SOLID	YELLOW: TWO WAY TRAFFIC WHITE: ONE WAY TRAFFIC	11 (280) C-C FOR THE DOUBLE LINE SEE TYPICAL PAINTED MEDIAN MARKING.
GORE MARKING AND CHANNELIZING LINES	8 (200) WITH 12 (300) DIAGONALS @ 45°	SOLID	WHITE	DIAGONALS: 15' (4.5 m) C-C (LESS THAN 30MPH (50 km/h)) 20' (6 m) C-C 30MPH (50 km/h) TO 45MPH (70 km/h)) 30' (9 m) C-C (OVER 45MPH (70 km/h))
RAILROAD CROSSING	24 (600) TRANSVERSE LINES: "RR" IS 6 (1.8 m) LETTERS: 16 (400) LINE FOR "X"	SOLID	WHITE	SEE STATE STANDARD 780001 AREA OF: "R"=3,6 SQ, FT, (0,33 m PEACH "X"=54,0 SQ, FT, (5,0 m PEACH
SHOULDER DIAGONALS (REQUIRED FOR SHOULDERS > 8°)	12 (300) @ 45°	SOLID	WHITE - RIGHT YELLOW - LEFT	50° (15 m) C-C (LESS THAN 30MPH (50 km/h)) 75° (25 m) C-C (30 MPH (50 km/h) TO 45MPH (70 km/h)) 150° (45 m) C-C (OVER 45MPH (70 km/h))
U TURN ARROW	SEE DETAIL	SOLID	WHITE	16.3 SF
2 ARROW COMBINATION LEFT AND U TURN	SEE DETAIL	SOLID	WHITE	30.4 SF

FOR FURTHER DETAILS ON PAVEMENT MARKING REFER TO STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION AND STATE STANDARD 780001,

8 (200) WHITE -

RAISED

All dimensions are in inches (millimeters) unless otherwise shown

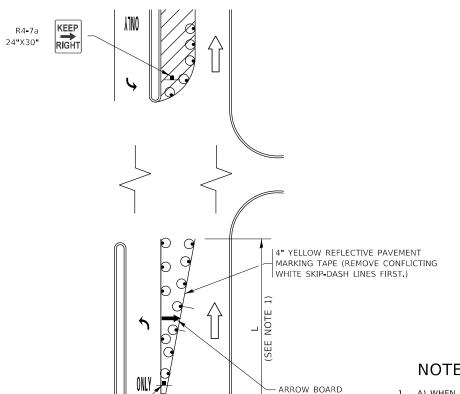
USER NAME ■ footemj	DESIGNED	-	EVERS	REVISED	-	C. JUCIUS 09-09-09
	DRAWN	-		REVISED	-	C. JUCIUS 07-01-13
PLOT SCALE = 50,0000 / in	CHECKED	-		REVISED	-	C. JUCIUS 12-21-15
PLOT DATE ■ 3/4/2019	DATE	-	03-19-90	REVISED	-	C. JUCIUS 04-12-16

* MARKINGS SHALL BE INSTALLED PARALLEL TO THE CENTERLINE OF THE ROAD WHICH IT CROSSES

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

	DISTRICT ONE TYPICAL PAVEMENT MARKINGS						SECTI	ION	COUNTY	TOTAL SHEETS	SHEET NO.
							2018-06	67-BR	WILL	128	121
TITICAL LAVEIVIEW WAIRINGS							TC-13 CONTRACT NO.				-
	SHEET 1 OF	2	SHEETS	STA.	TO STA.			ILLINOIS FED A	ID PROJECT		

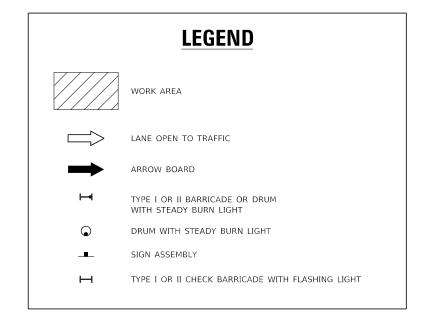
TURN BAY ENTRANCE AT START OF LANE CLOSURE TAPER

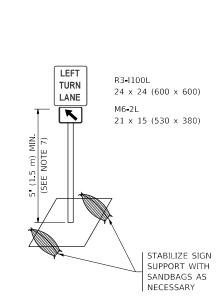


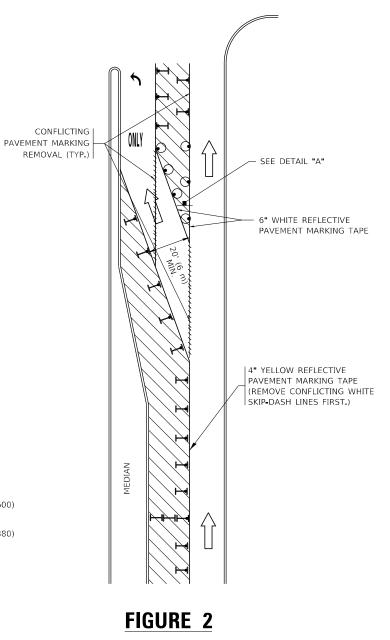
NOTES:

- 1. A) WHEN "L" IS ≤ THE STORAGE LENGTH OF THE TURN LANE (AS SHOWN IN FIG. 1), USE FIGURE 1.
- B) WHEN "L" IS > THE STORAGE LENGTH OF THE TURN LANE OR THE TURN LANE IS WITHIN THE LANE CLOSURE, USE FIGURE 2.
- 2. CONES MAY BE SUBSTITUTED FOR BARRICADES OR DRUMS AT HALF THE SPACING DURING DAY OPERATIONS. CONES SHALL BE A MINIMUM OF 28 (710) IN HEIGHT.
- 3. LIGHTS WILL NOT BE REQUIRED ON BARRICADES OR DRUMS FOR DAY OPERATIONS. ALL LIGHTS SHALL BE MONODIRECTIONAL.
- 4. REFLECTIVE TEMPORARY PAVEMENT MARKINGS SHALL BE PLACED THROUGHOUT THE BARRICADED AREAS OF EACH TURN BAY AS SHOWN WHERE THE CLOSURE TIME IS GREATER THAN FOURTEEN (14) DAYS.
- 5. THIS APPLICATION ALSO APPLIES WHEN WORK IS BEING PERFORMED IN THE RIGHT LANE(S) AND THE RIGHT TURN BAY IS TO REMAIN OPEN. UNDER THIS CONDITION, "RIGHT TURN LANE" R3-I100R 24 x 24 (600 x 600) AND M6-2R 21 x 15 (530 x 380) SHALL BE USED.
- 6. THESE CONTROLS SHALL SUPPLEMENT MAINLINE TRAFFIC CONTROL FOR LANE CLOSURES.
- 7. THE SIGNS SHALL BE MOUNTED ABOVE THE BARRICADES/DRUMS ON SEPARATE SIGN SUPPORTS THAT MEET NCHRP 350 OR MASH PREOUIREMENTS.
- 8. TRAFFIC CONTROL AND PROTECTION AT TURN BAYS (TO REMAIN OPEN TO TRAFFIC) SHALL BE INCLUDED IN THE COST OF SPECIFIED TRAFFIC CONTROL STANDARDS OR ITEMS.

TURN BAY ENTRANCE WITHIN A LANE CLOSURE







DETAIL A

SCALE: NONE

All dimensions are in inches (millimeters) unless otherwise shown

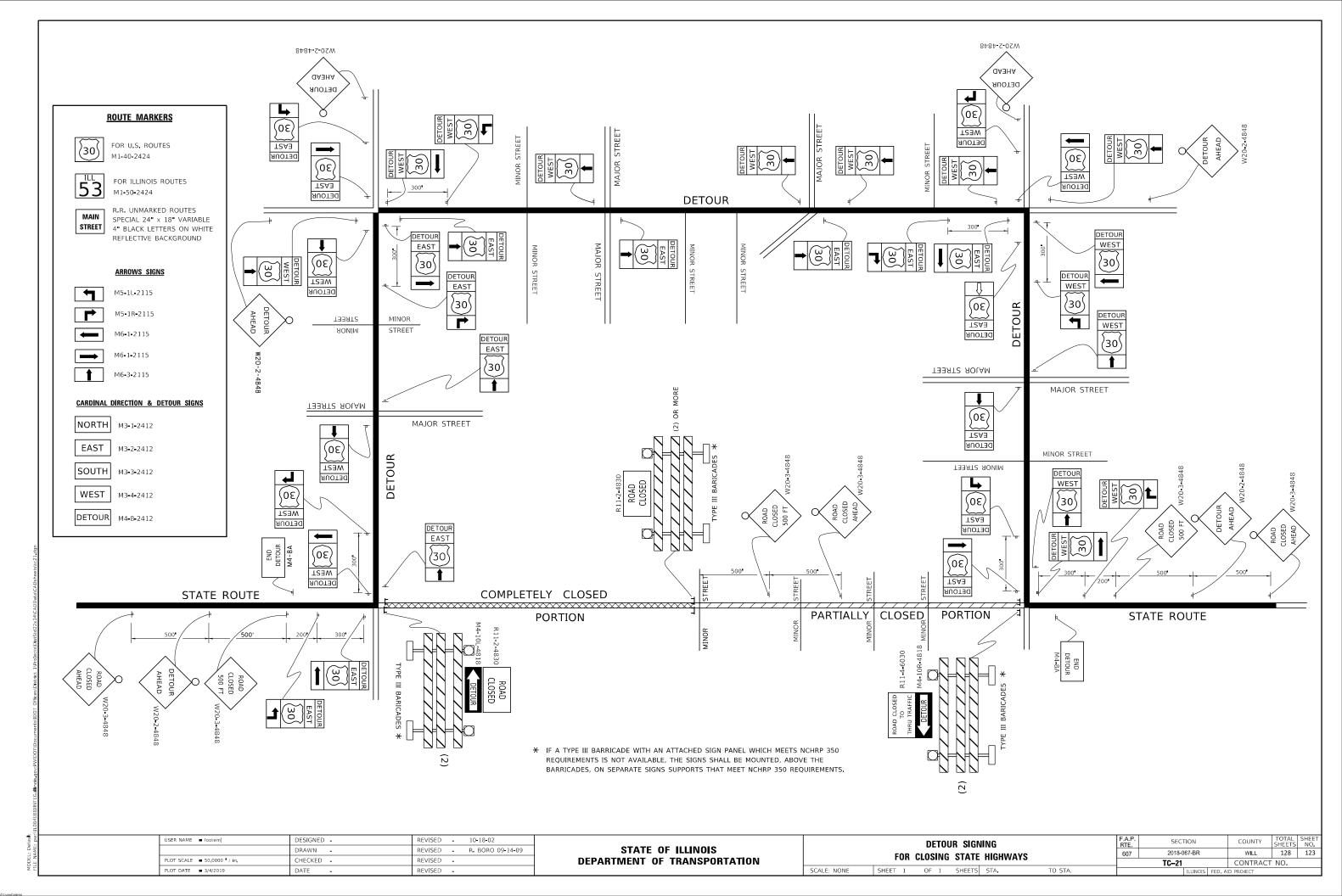
USER NAME ■ footem) DESIGNED -T. RAMMACHER 09-08-94 REVISED - R. BORO 09-14-09 DRAWN - A. HOUSEH 11-07-95 REVISED - A. SCHUETZE 07-01-13 CHECKED - A. HOUSEH 10-12-96 REVISED - A. SCHUETZE 09-15-16 PLOT DATE = 3/4/2019 DATE -T. RAMMACHER 01-06-00 REVISED

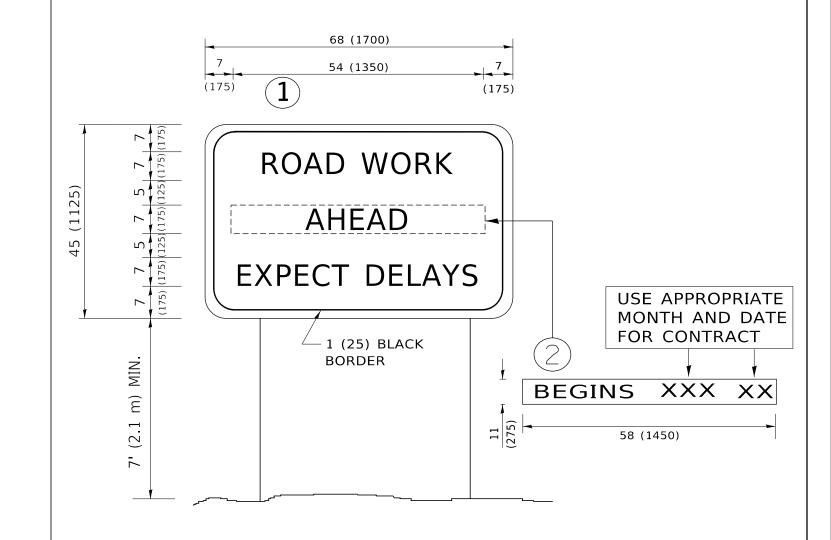
FIGURE 1

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

TRAFE	TRAFFIC CONTROL AND PROTECTION AT TURN BAYS							BAYS	F.A.P. RTE.						
(TO REMAIN OPEN TO TRAFFIC)						607	2018-067-I	3R	WILL	128	122				
(TO REIMAIN OFEN TO TRAFFIC)							TC-14 CONTRACT NO.								
NE	SHEET :	ι	OF	1	SHEETS	STA.		TO STA.		1111	NOIS FED. AL	ID PROJECT			

SEE DETAIL "A"





NOTES:

- 1. USE BLACK LETTERING ON ORANGE BACKGROUND.
- 2. ERECT SIGNS IN ADVANCE OF THE LOCATION FOR THE "ROAD CONSTRUCTION AHEAD" SIGN AT LOCATIONS AS DIRECTED BY THE ENGINEER.
- 3. ERECT SIGN 1 WITH INSTALLED PANEL 2 ONE WEEK PRIOR TO THE START OF CONSTRUCTION.
- 4. REMOVE PANEL(2)SOON AFTER THE START OF CONSTRUCTION.
- 5. SEE SPECIAL PROVISION FOR "TEMPORARY INFORMATION SIGNING" FOR ADDITIONAL INFORMATION.
- 6. ONE SIGN ASSEMBLY EQUALS 25.70 SQ. FT. (2.3 SQ. M.)

SCALE: NONE

7. SHALL BE PAID FOR AS TEMPORARY INFORMATION SIGNING.

ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.

USER NAME ■ footemj	DESIGNED -	REVISED	- R. MIRS 09-15-97
	DRAWN -	REVISED	- R. MIRS 12-11-97
PLOT SCALE = 50,0000 / in	CHECKED -	REVISED	-T. RAMMACHER 02-02-99
PLOT DATE ■ 3/4/2019	DATE -	REVISED	- C ILICIUS 01•31•07

STATE O	F ILLINOIS
DEPARTMENT OF	TRANSPORTATION

	ARTE	RIAL RO	F.A.P. RTE.	SECTION				
	INFORM	IATION	607 2018-067-BR					
	IIII OIIIV	IATION	JIGIN			TC-22		
1	OF 1	SHEETS	STA.	TO STA.		ILLINOIS FED.		

