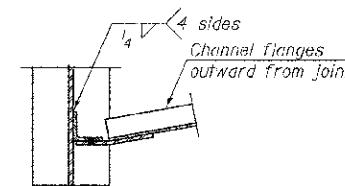


$I_s$ ,  $S_c$ : Non-composite moment of inertia and section modulus of the steel section used for computing  $f_s$  (Total-Strength I, and Service II) due to non-composite dead loads ( $I_n^4$  and  $I_n^3$ ).  
 $I_c(n)$ ,  $S_c(n)$ : Composite moment of inertia and section modulus of the steel and deck based upon the modular ratio, "n", used for computing  $f_s$  (Total-Strength I, and Service II) due to short-term composite live loads ( $I_n^4$  and  $I_n^3$ ).  
 $I_c(3n)$ ,  $S_c(3n)$ : Composite moment of inertia and section modulus of the steel and deck based upon 3 times the modular ratio, "3n", used for computing  $f_s$  (Total-Strength I, and Service II) due to long-term composite (superimposed) dead loads ( $I_n^4$  and  $I_n^3$ ).  
 Un-factored non-composite dead load (kip·ft.).  
 $DC_1$ : Un-factored moment due to non-composite dead load (kip·ft.).  
 $M_{DC_1}$ : Un-factored long term composite (superimposed excluding future wearing surface) dead load (kip·ft.).  
 $DC_2$ : Un-factored moment due to long-term composite (superimposed excluding future wearing surface) dead load (kip·ft.).  
 $M_{DC_2}$ : Un-factored long-term composite (superimposed future wearing surface only) dead load (kip·ft.).  
 $DW$ : Un-factored moment due to long-term composite (superimposed future wearing surface only) dead load (kip·ft.).  
 $M_{DW}$ : Un-factored live load moment plus dynamic load allowance (impact) (kip ft.).  
 $M_L + IM$ : Factored design moment (kip·ft.).  
 $M_u$  (Strength I):  $1.25(M_{DC_1} + M_{DC_2}) + 1.5 M_{DW} + 1.75 M_L + IM$   
 Compact composite positive moment capacity computed according to Article 6.10.7.1 (kip·ft.).  
 $\phi_f M_{Nc}$ : Compact non-composite negative moment capacity computed according to Article A6.1.1 (kip·ft.).  
 $\phi_f M_{Nc}$ : Sum of stresses as computed from the moments below (ksi).  
 $f_s$  (Service II):  $M_{DC_1} + M_{DC_2} + M_{DW} + 1.5 M_L + IM$   
 Sum of stresses as computed from the moments below on non-compact section (ksi).  
 $f_s$  (Total)(Strength I):  $1.25(M_{DC_1} + M_{DC_2}) + 1.5 M_{DW} + 1.75 M_L + IM$   
 $V_f$ : Maximum factored shear range in composite portion of span computed according to Article 6.10.10.

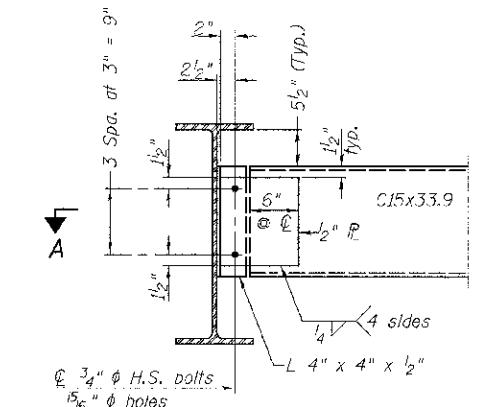
INTERIOR BEAM MOMENT TABLE				
Units	0.4 Span 1 & 0.6 Span 3	Piers 1 & 2	0.5 Span 2	
$I_3$	(in <sup>4</sup> )	5,360	5,360	5,360
$I_c(n)$	(in <sup>4</sup> )	14,916	-	14,916
$I_6(3n)$	(in <sup>4</sup> )	10,920	-	10,920
$Z$	(in <sup>3</sup> )	355	355	355
$S_c(n)$	(in <sup>3</sup> )	534	-	534
$S_c(3n)$	(in <sup>3</sup> )	481	-	481
DC1	(K/ft.)	0.825	0.825	0.825
MDC1	(K)	151	557	223
DC2	(K/ft.)	0.144	0.144	0.144
MDC2	(K)	31	47	52
DW	(K/ft.)	0.325	0.325	0.325
MDW	(K)	7.3	108	122
$M_E + IM$	(K)	612	427	770
$M_U$ (Strength II)	(K)	1,409	1,414	1,874
$\Delta M_{min}$ or $M_{nc}$	(K)	2,748	-	2,674
fs DC1	(k.s.i.)	5.10	12.06	7.55
fs DC2	(k.s.i.)	0.78	1.58	1.30
fs DW	(k.s.i.)	1.82	3.65	3.03
fs 1.3( $E + IM$ )	(k.s.i.)	17.89	18.76	22.49
fs (Service II)	(k.s.i.)	25.59	36.05	34.38
fs (Total)(Strength II)	(k.s.i.)	-	47.78	-
VF	(K)	41.2	-	37.4

- \* Compact sections
- \*\* Non-Compact and slender section.

INTERIOR BEAM REACTION TABLE		
Units	S. & N. Abuts.	Piers 1 & 2
R <sub>D1</sub> (K)	15.9	59.9
R <sub>D2</sub> (K)	3.0	10.0
R <sub>DW</sub> (K)	6.9	23.2
R <sub>E + IM</sub> (K)	75.2	127.6
R <sub>Total</sub> (K)	101.0	220.7

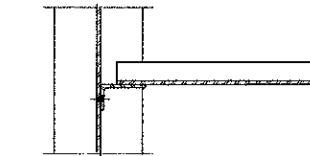


SECTION A -

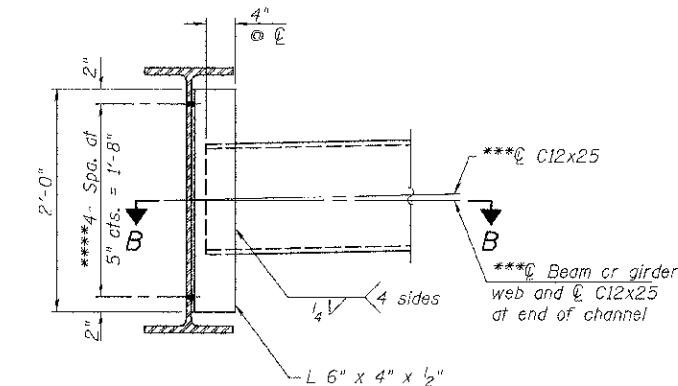


DIAPHRAGM D.

Note:  
Two hardened washers required for each set of oversized holes.



SECTION B-B



### *DIAPHRAGM D*

*Note:* Two hardened washers required for each

set of oversized holes.  
\*\*\*Alternate channels C12x30, are permitted to facilitate material acquisition. Calculated weight of structural steel is based on the lighter section.

The alternate, if utilized, shall be provided at no additional cost to the Department.  
\*\*\*\*\*3." A HS bolts 15- "A holes

10

All diaphragms shall be installed as steel is erected and secured with erection pins and bolts except as otherwise noted. Individual diaphragms at supports may be temporarily disconnected to install bearing anchor rods.

FILE NAME *	USER NAME = smurtsi	DESIGNED BY	REVISED -
V:\2655\CAD SHEETS\STRUCTURES	Rec'd 02-27-2013\2555b18.dgn	DRAWN TAC	REVISED -
	PLOT SCALE = NONE	CHECKED CTM	REVISED -
	PLOT DATE: 2/26/2013	DATE -	REVISED -

**DEKALB COUNTY  
C.H. 26 (FIVE POINTS RD.)  
OVER SOUTH BRANCH OF KISHWAUKEE RIVER**

STRUCTURAL STEEL DETAIL

STRUCTURAL STEEL DETAILS				RTE. NO.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
SCALE: N/A	SHEET NO. 18 OF 28 SHEETS	STA. N/A	TO STA. N/A	CH 26	05-00044-01-BR	DEKALB	49	33
						CONTRACT NO. 87477		