#### DESIGN SPECIFICATIONS

2010 AASHTO LRFD Bridge Design Specifications, 5th Ed. with 2010 Interims

## DESIGN STRESSES

FIELD UNITS f'c = 3,500 psi

fy = 60,000 psi (Reinforcement) fy = 50,000 psi (M270 Grade 50)

#### PRECAST UNITS f'c = 6,000 psi

f'ci = 5,000 psi  $fpu = 270,000 psi (l_2'' \phi low lax stands)$   $fpbt = 201,960 psi (l_2'' \phi low lax strands)$ 

#### LOADING HL-93

Allow 50#/sq. ft. for future wearing surface.

#### SEISMIC DATA

Seismic Performance Zone (SPZ) = 1 Design Spectral Acceleration at 1.0 sec.  $(S_{D1}) = 0.094$  g Design Spectral Acceleration at 0.2 sec. (S<sub>DS</sub>) = 0.171 g Soil Site Class = C



#### ∠ = 47°10′49.46′′ $D = 7^{\circ}30''$ $T = 333.38^{\circ}$ L = 628.66E = 69.618' R = 763.44'P.I. = Sta. 23+04.98 S.E. = 0.015 ft/ft Attain S.E. from Sta. 18+90 to Sta. 19+50, full S.E. from Sta. 19+50 to Sta. 25+85.26.

25.00 . 21+00.00 96 8 V.P.C. Sta. 19+75.0 Elev. 752.21



. *Sta.* 1548+00.00 734.08

V.P.I. Elev.

250' V.C.

. Sta. 1546+00.00 728.64

V.P.I. Elev.

DRAWN

V.P.I. Sta. 1546+50.00 Elev. 730.00

DESIGNED - DEWEY H. COULTAS

CHECKED - D.H.C. / N.R.B.

CHECKED - NICHOLAS R. BARNETT

MICHAEL B. MOSSMAN

V.P.I. Sta. 1547+00.00 Elev. 731.38

V.P.I. Sta. 1547+50.00 Elev. 732.80

PROFILE GRADE (E.B. I-74)

## PROFILE GRADE (C.H. 50)

V.P.I. Sta. 1548+50.00 Elev. 735.33

*Sta.* 1549+00.00 736.68

V.P.I. Elev.

EXAMINED

PASSED



PROFILE GRADE (W.B. I-74)

30'-2<sup>3</sup>8''

Back of

south Abut.

2'-4'4'

10%

Back of south

approach bent

CURVE DATA (F.A.I. 74)

P.I. = Sta. 1551+13.42

S.E. = 0.032 ft/ft

⊿ = 17° 19′03′ D = 1° 08′

T = 769.90'

L = 1528.02

 $R = 5055.65^{\circ}$ 

 $F = 58.29^{\circ}$ 

Sta. 2

9°00′21′

121'-1158''

1'-6'8''

9°00′21′

138′-5′<sub>8</sub>′′

Local tangent to Q

at Sta. 21+29.17

F.A.S. Rte. 1520 (C.H. 50)

.90°

— Ç Pier

F.A.S. Rte. 1520

(C.H. 50) & PG

OFFSET SKETCH

Sta. 21+29.17

2'-2'2"

10°12′24

12'-

Back of

north Abut.

3 -

8 -

9

10 -

11

12

13-15 -

16-20 -

22-24 -

26-28 -

29-31 -

35

36

37

38

39-41 -

42

4.3

44 -

45 -

46 -

57 -

58 -

6″

Pier

59-62 - Soil Boring Logs

Back of

abutment

47-49 -

50-51 -

52-54

55-56

32-34

25

21

40'-534'

3'-8''

20

Back of north

approach bent

General Plan & Elevation

General Data

Footing Layout

4-7 - Top of Slab Elevations

Superstructure

Parapet Railing

Structural Steel

Bearing Details

South Abutment

North Abutment

Superstructure Details

10°12′24′′

INDEX OF SHEETS

Top of Vaulted Abutment Slab Elevations

Top of South Approach Slab Elevations

Top of North Approach Slab Elevations

South Vaulted Abutment Approach Span

North Vaulted Abutment Approach Span

Top of South Vaulted Abutment Slab Elevations

Top of North Vaulted Abutment Slab Elevations

South Vaulted Abutment Approach Span Details

North Vaulted Abutment Approach Span Details

6″



STATION 21+29.17

BUILT 20 BY

STATE OF ILLINOIS

F.A.I. RT. 74 SEC. 10-4BR

LOADING HL-93

STRUCTURE NO. 010-0289

NAME PLATE

See Std. 515001

6″\_

\* 1:6 (V:H)

## SECTION THRU CONCRETE SLOPEWALL



SECTION A-A

# Munsell No. 7.5G 4/8. Inventory: HS 6.0 Operating: HS 10.0 equipment. provisions.

DATE FEBRUARY 25, 2013 **GENERAL D** STATE OF ILLINOIS STRUCTURE NO.0 REVISED **DEPARTMENT OF TRANSPORTATION** SHEET NO. 2 OF 6 REVISED

# GENERAL NOTES

Fasteners shall be ASTM A325 Type 1, mechanically galvanized bolts. Bolts  $7_{B}$  in  $\phi$ , holes  $^{15}_{16}$  in.  $\phi$ , unless otherwise noted.

Calculated weight of Structural Steel = 474,930 Lbs. (AASHTO M270, Grade 50) No field welding is permitted except as specified in the contract documents. Reinforcement bars designated (E) shall be epoxy coated.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of  $l_{B}$  inch (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.

Concrete Sealer shall be applied to the designated areas of the abutments.

The Organic Zinc Rich Primer / Epoxy / Urethane Paint System shall be used for painting of new structural steel except where otherwise noted. The entire system shall be shop applied, with the exception of the exterior surfaces and bottom of the bottom flange of fascia beams, masked off connection surfaces, field installed fasteners, and damaged areas,

all of which shall be touched up and finish coated in the field. The color of the final finish coat for all interior steel surfaces shall be Gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Interstate Green,

The embankment configuration shown shall be the minimum that must be

placed and compacted prior to construction of the abutments.

Slopewall shall be reinforced with welded wire fabric, 6" x 6" - W4.0 x W4.0, weighing 58 lbs. per 100 sq. ft.

Slipforming of the parapets is not allowed.

The Contractor shall retain the services of an engineering firm, prequalified in the IDOT consultant selection category of Highway Bridges Advanced Typical, for preparation of the Structural Assessment Report. Contractor's pre-approval shall not be applicable for this project. See Special Provision.

Current Ratings on File for Existing Structure 010-0157:

Live Load Restrictions: Yes (15 Tons)

Inventory and Operating Ratings and Live Load Restrictions are provided for information only. Inventory and Operating Ratings are based on HS loading and configuration. Live Load Restrictions are based on Illinois legal loads and configurations. The Ratings and Live Load Restrictions are not necessarily representative of capacities to support the Contractor's

The Contractor is advised that the existing structure contains members that are in a deteriorated condition with reduced load carrying capacity. It is the Contractor's responsibility to account for the condition of the existing structure when developing construction procedures for the complete or partial removal, or replacement of the structure. An Existing Structure Information Package is available upon request as noted in the special

#### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL				
Removal of Existing Structures	Each			1				
Structure Excavation	Cu. Yd.		435	435				
Concrete Structures	Cu. Yd.		388.7	388.7				
Concrete Superstructure	Cu. Yd.	693.4		693.4				
Bridge Deck Grooving	Sq. Yd.	1,283		1,283				
Protective Coat	Sq. Yd.	2,475		2,475				
Furnishing and Erecting Precast Prestressed Concrete I-Beams, 36 in.	Foot	337		337				
Furnishing and Erecting Structural Steel	L. Sum			1				
Stud Shear Connectors	Each	3,510		3,510				
Reinforcement Bars, Epoxy Coated	Pound	154,970	48,870	203,840				
Bridge Fence Railing	Foot	335		335				
Parapet Railing	Foot	332		332				
Slope Wall 4 Inch	Sq. Yd.		207	207				
Furnishing Metal Shell Piles 12″ x 0.25″	Foot		1,268	1,268				
Furnishing Metal Shell Piles 14″ x 0.25″	Foot		667	667				
Driving Piles	Foot		1,935	1,935				
Test Pile Metal Shells	Each		5	5				
Pile Shoes	Each		67	67				
Name Plates	Each	1		1				
Preformed Joint Strip Seal	Foot	96		96				
Elastomeric Bearing Assembly, Type II	Each	16		16				
Anchor Bolts, 1"	Each		32	32				
Anchor Bolts, 1'4''	Each		16	16				
Concrete Sealer	Sq. Ft.		2,922	2,922				

ΑΤΑ	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.			
010 — 0289	74	10-4BR	CHAMPAIGN	290	87			
	CONTRACT NO. 70700							
2 SHEETS	ILLINOIS FED. AID PROJECT							