CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions, ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs, concentrated live load.

DESIGN STRESSES: Field Units f'c = 3,500 p.s.i. fy = 60,000 p.s.i. (reinforcement)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Specificiations.

MATERIALS: Aluminum Alloys as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W\*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer. The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent malerial acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to AASHTO M314 Gr. 105 with a minimum Charpy V-Notch (CVN) energy of 15 lb,-ft. at 10° F.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final around line at each foundation shall be cleaned and coated with Bridge Seat Sealer in accordance with the Standard Specifications,

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

\* If M270 Gr. 50W (M222) steel is proposed. chemistry for plate to be used shall first be approved by the Engineer as suitable for galvanizing and welding.

TOTAL	BILL	OF	MATERIAL	
	T. [ ] ] (			

ER	REVISION	DATE	ITEM	UNIT	TOTAL
			OVERHEAD SIGN STRUCTURE CANTILEVER TYPE I-C-A	Foot	24'-1"
			OVERHEAD SIGN STRUCTURE CANTILEVER TYPE II-C-A	Foot	27'-6"
			OVERHEAD SIGN STRUCTURE CANTILEVER TYPE III-C-A	Foot	62'-0"
			OVERHEAD SIGN STRUCTURE WALKWAY, TYPE A	Foot	70′-6″
			DRILLED SHAFT CONCRETE FOUNDATIONS	Cu. Yds.	22.4

0S	r-	۸	_	1
US	C -	А	_	1

12-1-08						
USER NAME =	DESIGNED - PMK	REVISED -		CANTILEVED SIGN STRUCTURES	F.A.I. SECTION	COUNTY TOTAL SHEET
	CHECKED - MPW	REVISED -	STATE OF ILLINOIS		* 82-1SG	ST. CLAIR 145 103
PLOT SCALE =	DRAWN - PMK	REVISED -	DEPARTMENT OF TRANSPORTATION	GENERAL PLAN & ELEVATION	*998/70/64	CONTRACT NO. 76C45
PLOT DATE =	CHECKED - MPW	REVISED -		SHEET NO. 1 OF 12 SHEETS	ILLINOIS	FED. AID PROJECT
	PLOT SCALE =	CHECKED - MPW PLOT SCALE = DRAWN - PMK	CHECKED         -         REVISED         -           PLOT SCALE =         DRAWN         -         PMK         REVISED         -           PLOT DATE =         CHECKED         -         MRW         PEVISED         -	CHECKED     MPW     REVISED     STATE OF ILLINOIS       PLOT SCALE =     DRAWN     PMK     REVISED     DEPARTMENT OF TRANSPORTATION	CHECKED     MPW     REVISED     STATE OF ILLINOIS     CANTILEVER SIGN STRUCTORES       PLOT SCALE =     DRAWN     PMK     REVISED     DEPARTMENT OF TRANSPORTATION     GENERAL PLAN & ELEVATION	CHECKED     MPW     REVISED     STATE OF ILLINOIS     CANTILEVER SIGN STRUCTORES     RTE.     SECONDA       PLOT SCALE =     DRAWN     PMK     REVISED     DEPARTMENT OF TRANSPORTATION     GENERAL PLAN & ELEVATION     •     82-15G       PLOT SCALE =     DRAWN     PMK     REVISED     -     DEPARTMENT OF TRANSPORTATION     •     998/70/64

Diag	rnate Direction of Horizontal nonal Bracing for Each Bay in Des of Upper and Lower Chords Bracing, typ.
	Lower Chord Bracing, typ.
	TYPICAL PLAN (Walkway not shown)
Sign Panel —	Alternate Vertical Diagonal Bracing for Each Bay in Planes of Front and Back Chords
Depth of Sign (D.	
Lowest part of structure above Elevation A.	Walkway, railing and lights (if required) omitted for clarity Cantilever Length (L) and Basis of Payment <u>© Steel</u> Post Support
Elev. A	Cantilever Length (L) and Basis of Payment <u>© Steel</u> Post Support (along © of truss) Edge of
(Location varies)	Pavement
earance to sign, walkwo	TYPICAL ELEVATION

TYPICAL ELEVATION

Looking in Direction of Traffic

Sign support structures may be subject to damaging vibrations and oscillations when sign panels are not in place during erection or maintenance of the structure. To avoid these vibrations and oscillations, consideration should be aiven to attaching temporary blank sign panels to the structure.

Note:

Structure

Number

8C082I070R002.6

8C082I064L003.9

8C082I064R004.3

Trusses shall be shipped individually with adequate provision to prevent detrimental motion during transport. This may require ropes between horizontals and diagonals or energy dissipating (elastic) ties to the vehicle. The contractor is responsible for maintaining the configuration and protection of the trusses.

(1) After adjustments to level truss and insure adequate vertical

clearance, all top and leveling nuts shall be tightened against

steel mesh shall then be placed around the perimeter of the

base plate. Secure to base plate with stainless steel banding.

the base plate with a minimum torque of 200 lb.-ft. Stainless

Desian Cantilever

III-C-A 31'-O"

Maximum Sign Area

340 Sq. Ft.

30 p.s.f. on

Maximum Sign Area

(See Table)

Maximum Length (See Table)

DESIGN WIND LOADING DIAGRAM

Installations not within dimensional limits shown require special analysis for all components.

Parameters shown are basis for I.D.O.T. Standards

400 Sa.

Length

(1)

Truss

Type

Station

- C - A

## — @ Upper Chord

Total

Sign Area

Dim. D

 $D_s$ 

Elev. A

 75+44.55
 III-C-A
 31'-O"
 415.44
 14'-O"
 15'-O"
 255 S0 F1

 68+00.00
 II-C-A
 27'-6"
 415.47
 15'-6"
 15'-0"
 180 S0 FT

 68+98.98
 I-C-A
 24'-1"
 449.60
 13'-0"
 10'-6"
 137 S0 FT

MM 4.3 III-C-A 30'-O" 432.58 I5'-6" 8'-0" 100 SQ FT

Maximum Lenath

30 Ft.

40 Ft.

10 p.s.f.

TIRE TRANSFER

Bottom of

ANT SALENT

1

Base Plate

## GENERAL NOTES

DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

FOUNDATIONS: The contract unit price for Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.