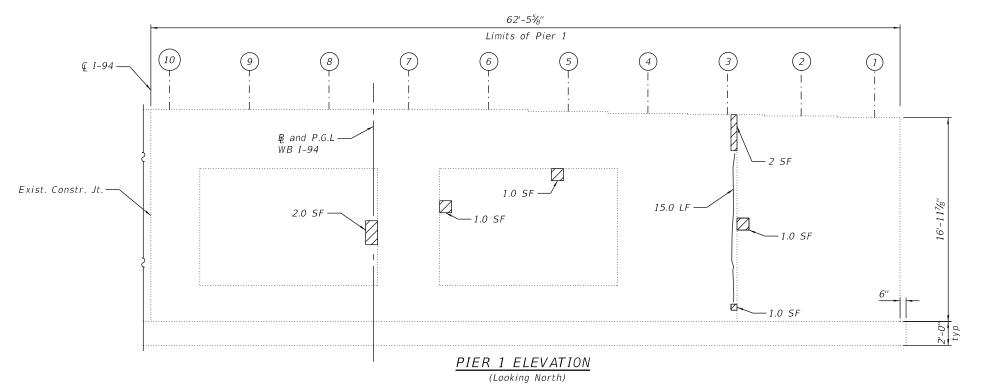


13/9/2024

Accurate GROUP, INC.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT REPAIRS STRUCTURE NO. 016-0161 (WB)



### <u>BILL OF MATERIAL</u>

ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	15
Structural Repair Of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	17

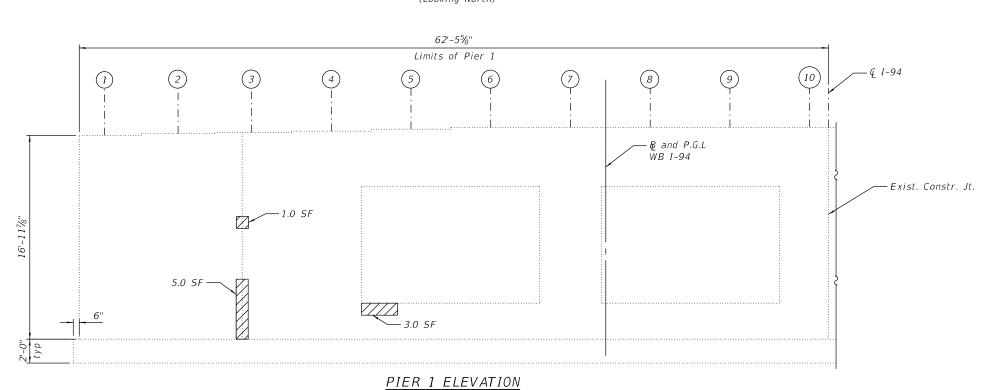
### *LEGEND*

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

——— Epoxy Crack Injection

LF Linear Foot

SF Square Foot

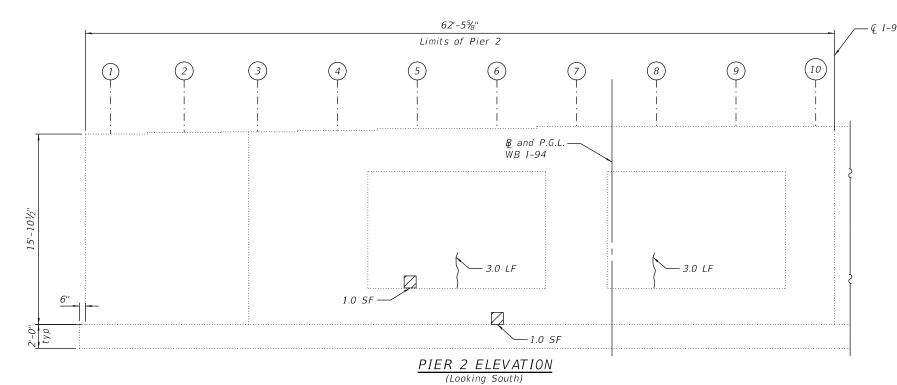


### <u>NOTE:</u>

1. Quantities and limits shown are estimated for bidding purpose only. The actual areas to be repaired and the type(s) of repairs to be used will be determined by the engineer in the field at the time of construction.



USER NAME	=	imranh	DESIGNED	-	SUR	REVISED	-
			DRAWN	-	ME	REVISED	-
PLOT SCALE	-	8:0.0000 ':" / in.	CHECKED	-	JL	REVISED	-
PLOT DATE	-	12/9/2024	DATE	-	12/6/2024	REVISED	-



## BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	22
Structural Repair Of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	31

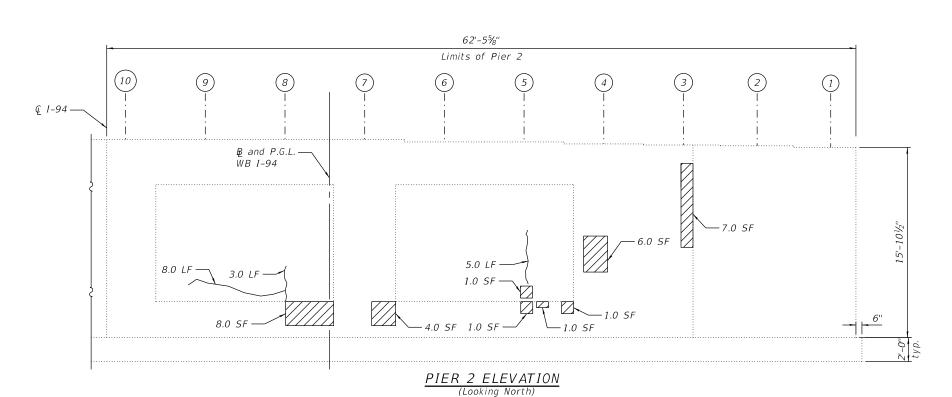
### <u>LEGEND</u>



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Epoxy Crack Injection

LF Linear Foot
SF Square Foot



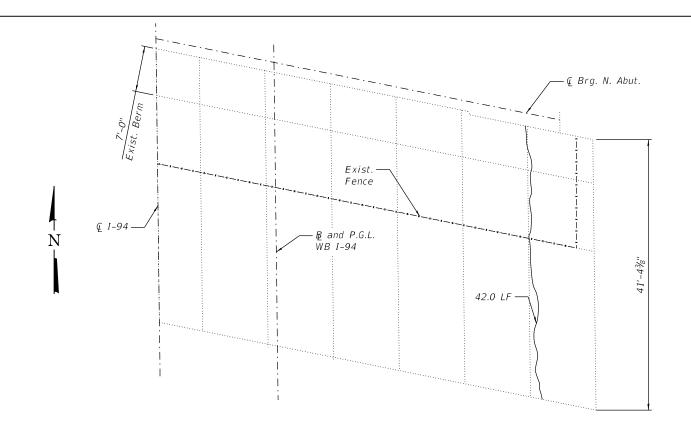
### NOTE:

1. Quantities and limits shown are estimated for bidding purpose only. The actual areas to be repaired and the type(s) of repairs to be used will be determined by the engineer in the field at the time of construction.

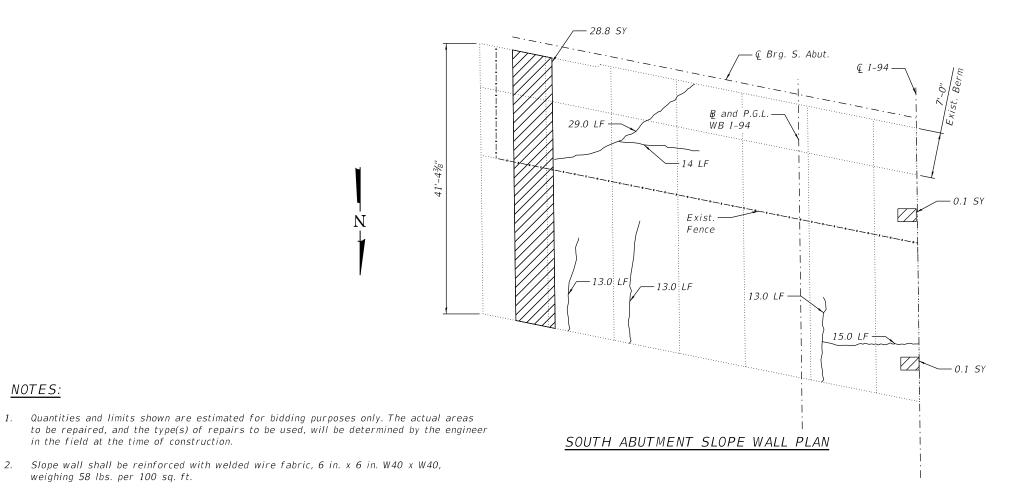
_						
E NAME:	A	С	<b>U</b> ROUP		t	е

USER NAME =	imranh	DESIGNED	-	SUR	REVISED	-	
		DRAWN	-	ME	REVISED	-	
PLOT SCALE =	8:0.0000 '." / in	CHECKED	-	JL	REVISED	-	
PLOT DATE =	12/9/2024	DATE	-	12/6/2024	REVISED	-	
							_

F.A.I.				TOTAL	SHFF
RTE	SECTION		COUNTY	SHEETS	
94	(42-B-11-1) BR, BJR	24	соок	761	603
			CONTRACT	NO. 6	2W87
	ILLINOIS	FED. All	D PROJECT		



### NORTH ABUTMENT SLOPE WALL PLAN



### BILL OF MATERIAL NORTH SLOPE WALL

ITEM	UNIT	QUANTITY
Slope Wall Crack Sealing	LF	42

### BILL OF MATERIAL SOUTH SLOPE WALL

ITEM	UNIT	QUANTITY
Porous Grandular Embankment	Cu Yd	10
Slope Wall Removal	Sq Yd	29
Slope Wall 4 Inch	Sq Yd	29
Slope Wall Crack Sealing	Foot	97

### LEGEND

Slope Wall Removal and Replacement with 4 inch Slope Wall

---- Slope Wall Crack Sealing

LF Linear Foot

SF Square Foot

Accurate

in the field at the time of construction.

weighing 58 lbs. per 100 sq. ft.

USER NAME = imranh DESIGNED - SUR REVISED -DRAWN - ME REVISED -14:8.0000 ':" / in. CHECKED - JL REVISED -PLOT DATE = 12/9/2024 DATE - 12/6/2024 REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

SLOPE WALL REPAIRS STRUCTURE NO. 016-0161 (WB) SHEET S05-20 OF S05-27 SHEETS

SECTION 94 (42-B-11-1) BR, BJR 24 COOK 761 604 CONTRACT NO. 62W87

12/9/2024 2:53:15 PM

NOTES:

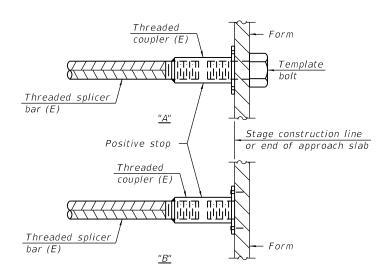
### STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

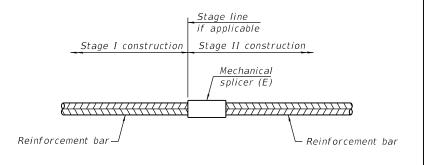
Location	Bar size	No. assemblies required	Minimum Iap length
N. Abut.	#5	10	3'-6"
N. Abut.	#6	6	4'-10"
S. Abut.	#5	10	3'-6"
S. Abut.	#6	6	4'-10"



### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

### NOTES:

- Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
- 2. All reinforcement shall be lapped and tied to the splicer bars.

  Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
- 3. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

05-15-2023

Accurate GROUP, INC.

USER NAME	-	imranh	DESIGNED	-	SUR	REVISED	-
			DRAWN	-	ME	REVISED	-
PLOT SCALE	=	0:2.0000 ':" / in.	CHECKED	-	JL	REVISED	-
PLOT DATE	-	12/9/2024	DATE	-	12/6/2024	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY & MECHANICAL SPLICER DETAILS STRUCTURE NO. 016-0161 (WB)

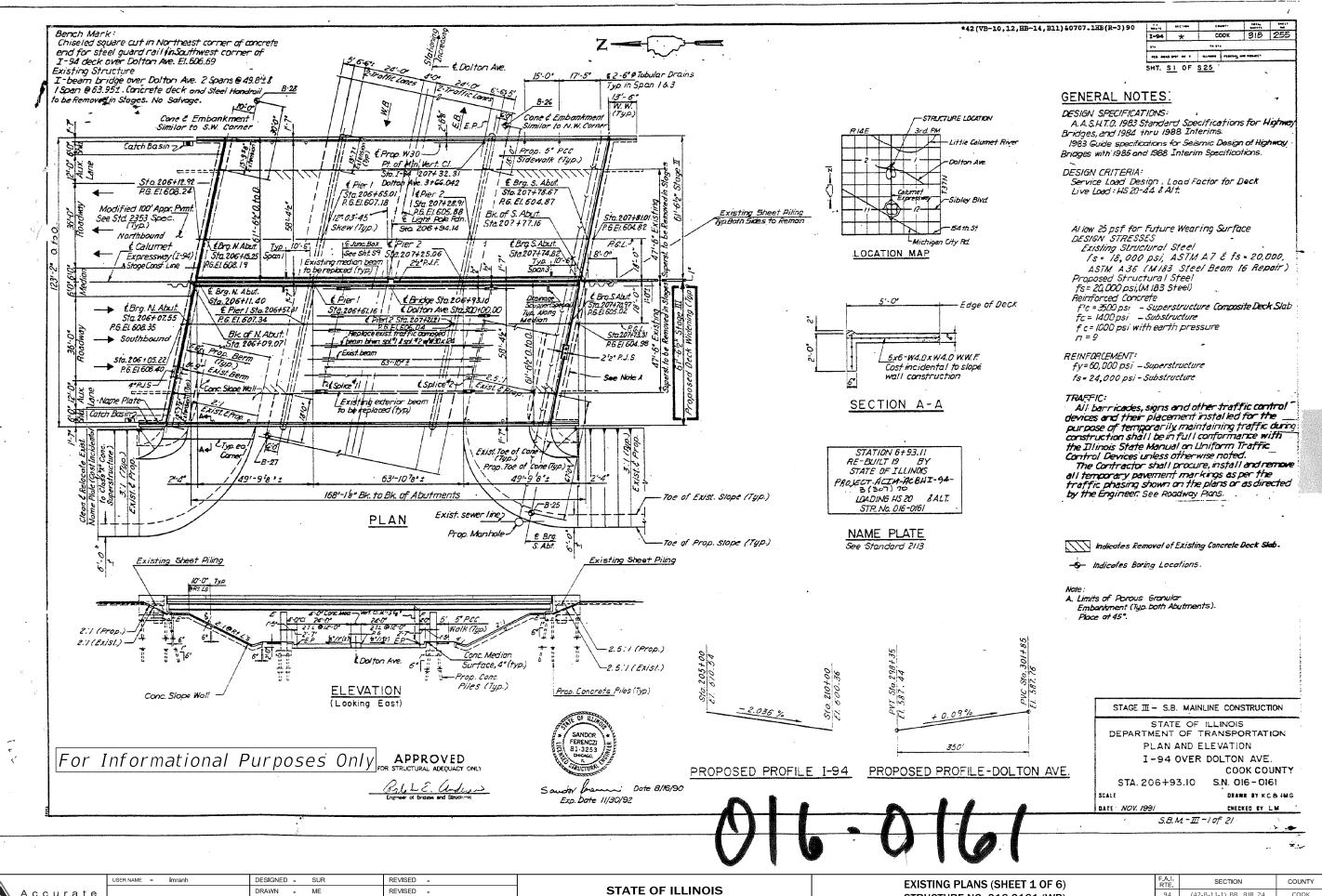
SHEET 505-21 OF 505-27 SHEETS

 
 FA.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 605

 CONTRACT NO.
 62W87

12/9/2024 2:53:17 PM



12/9/2024 2:53:19 PM

0:2.0000 ':" / in.

12/9/2024

PLOT DATE =

CHECKED -

- 12/6/2024

DATE

REVISED -

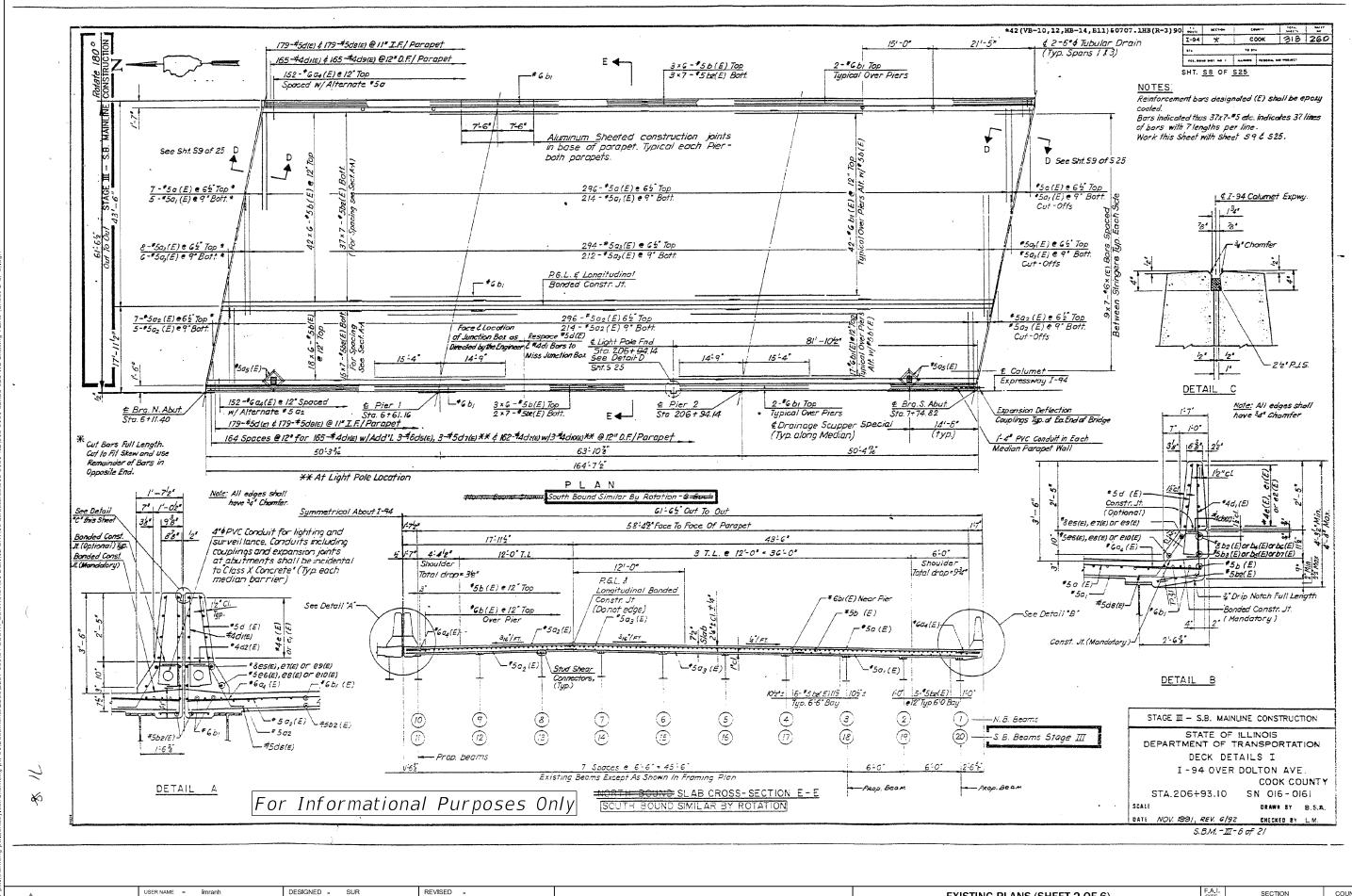
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (SHEET 1 OF 6)
STRUCTURE NO. 016-0161 (WB)
SHEET 505-22 OF 505-27 SHEETS

ALI SECTION COUNTY TOTAL SHEETS NO.

94 (42-B-11-1) BR, BJR 24 COOK 761 606

CONTRACT NO. 62W87



12/9/2024 2:53:28 PM

Accurate

DRAWN - ME

- 12/6/2024

CHECKED -

DATE

0:2.0000 ':" / in.

PLOT DATE = 12/9/2024

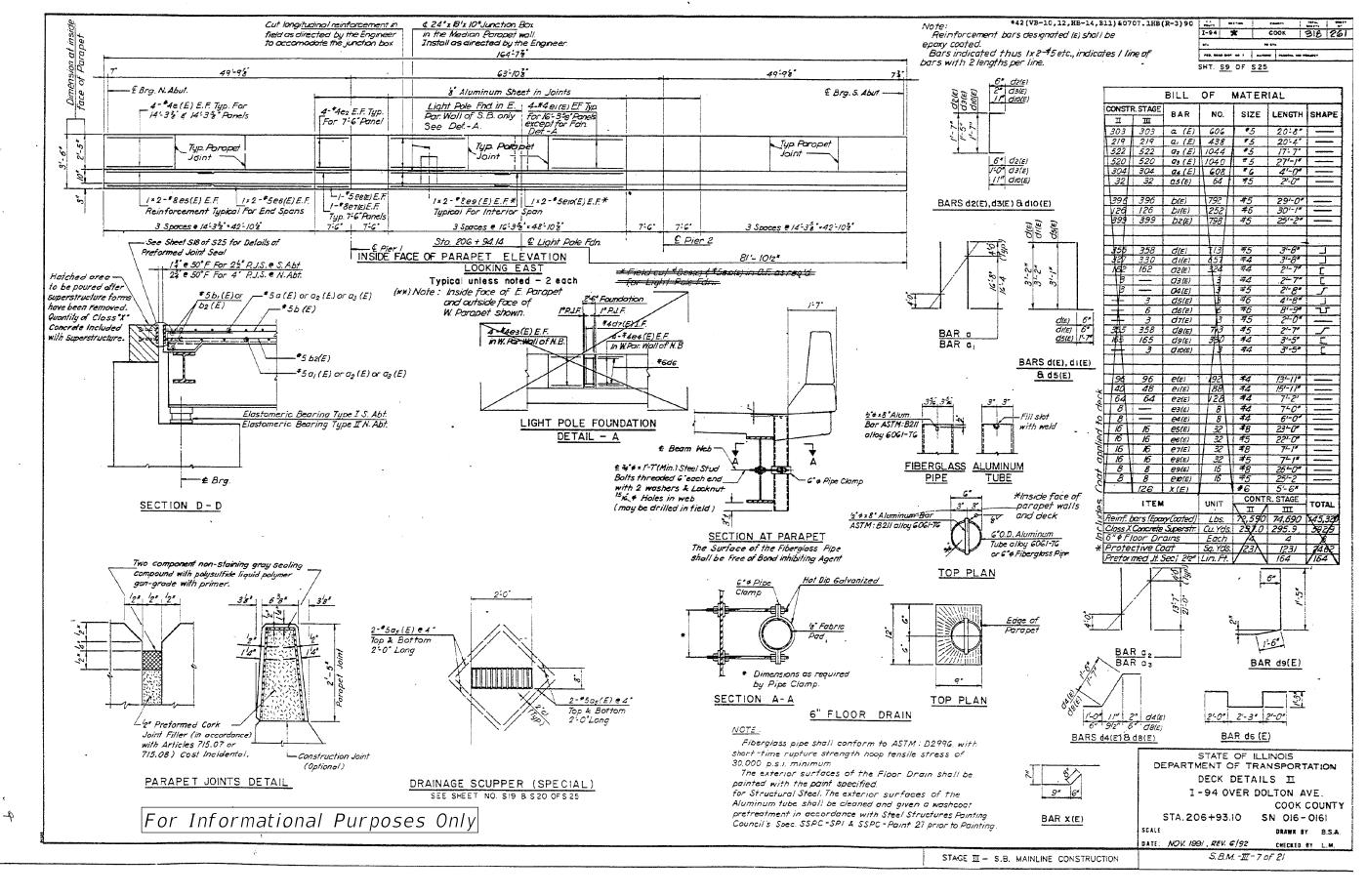
REVISED -

REVISED -

REVISED -

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION**  **EXISTING PLANS (SHEET 2 OF 6)** STRUCTURE NO. 016-0161 (WB) SHEET S05-23 OF S05-27 SHEETS

SECTION COUNTY 94 (42-B-11-1) BR, BJR 24 COOK 761 607 CONTRACT NO. 62W87



A c c u r a t e

 USER NAME
 =
 imranh
 DESIGNED
 SUR
 REVISED

 PLOT SCALE
 =
 0:2,0000 "." / in.
 CHECKED
 JL
 REVISED

 PLOT DATE
 =
 12/9/2024
 DATE
 12/6/2024
 REVISED

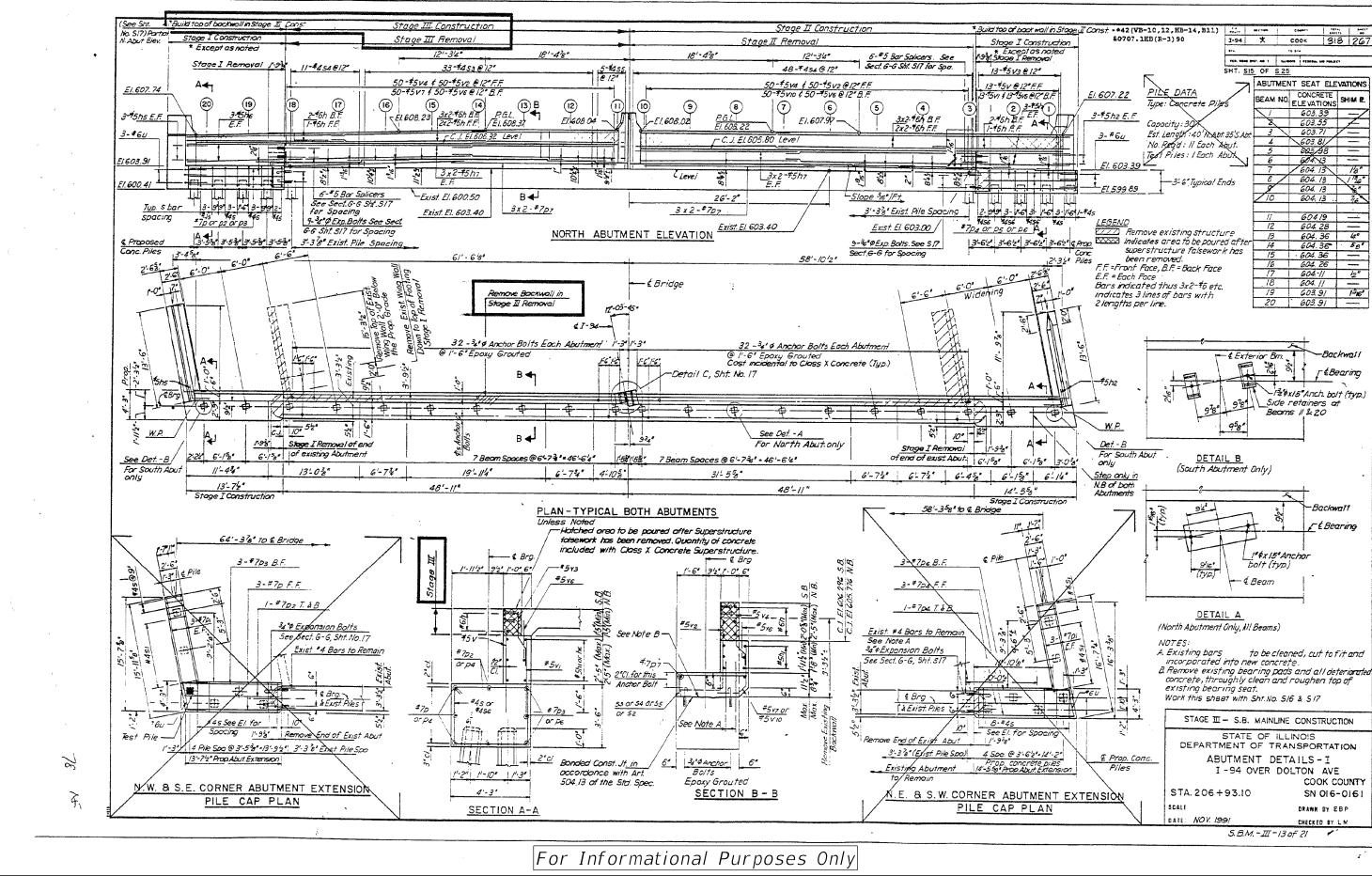
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (SHEET 3 OF 6)
STRUCTURE NO. 016-0161 (WB)

SHEET 505-24 OF 505-27 SHEETS

F.A.I. RTE. SECTION COUNTY TOTAL SHEETS NO.
94 (42-B-11-1) BR, BJR 24 COOK 761 608

| CONTRACT NO. 62W87



FILE NAME: pw

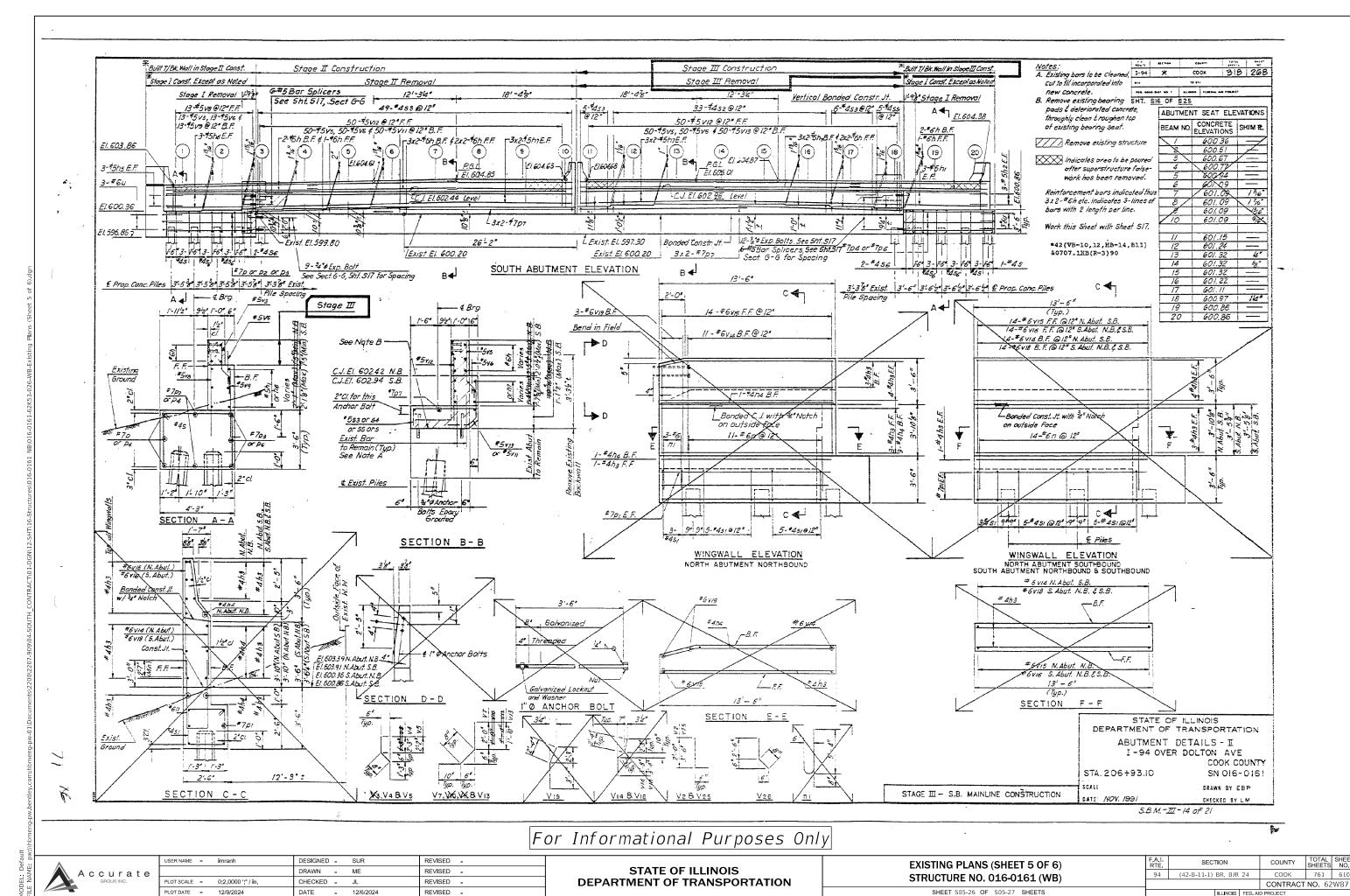
Accurate GROUP, INC.

 USER NAME
 =
 imranh
 DESIGNED
 SUR
 REVISED

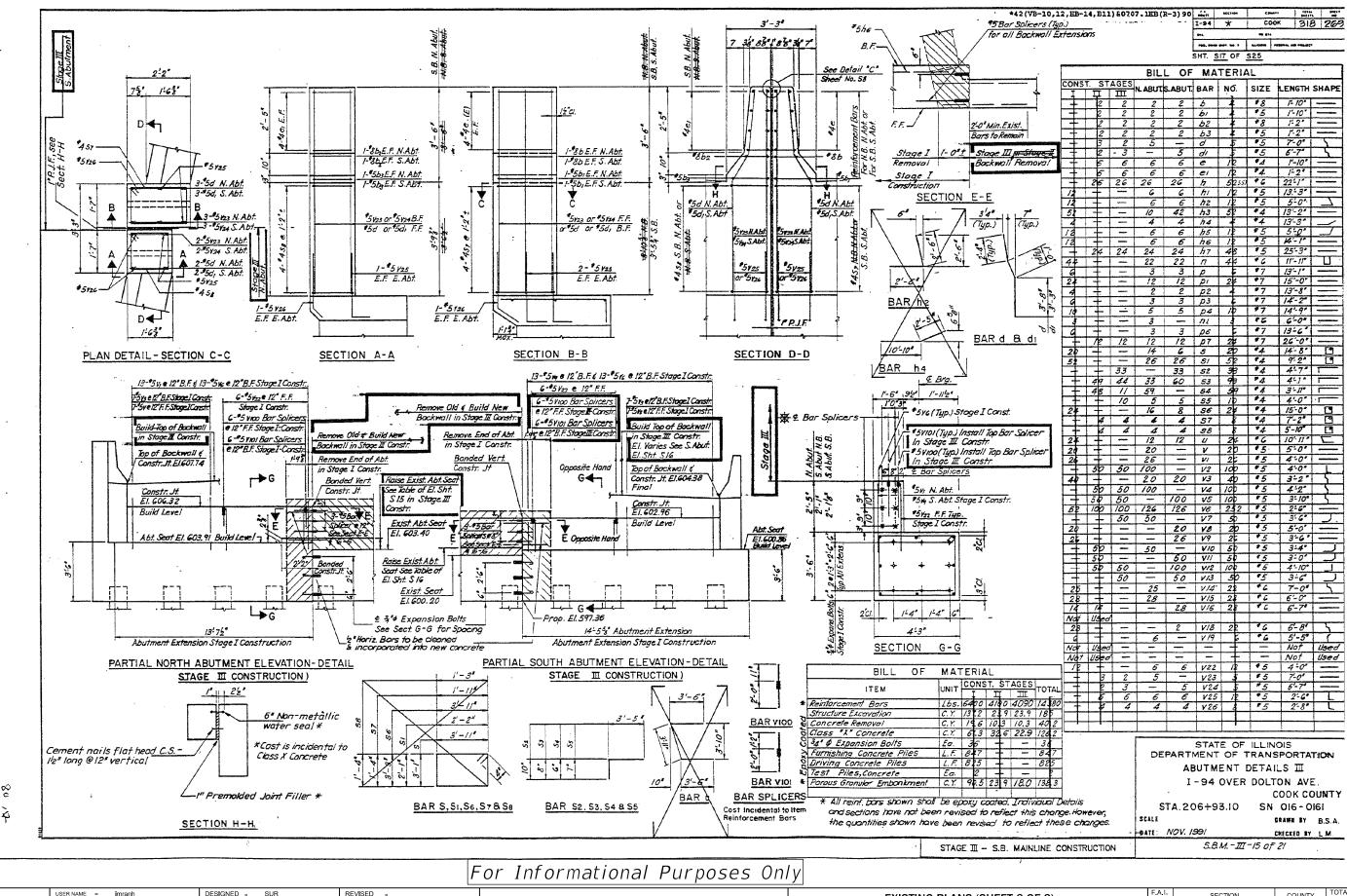
 DRAWN
 ME
 REVISED

 PLOT SCALE
 =
 0:2,0000 ':" / in.
 CHECKED
 JL
 REVISED

 PLOT DATE
 =
 12/9/2024
 DATE
 12/6/2024
 REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION EXISTING PLANS (SHEET 4 OF 6) STRUCTURE NO. 016-0161 (WB) 

12/9/2024 2:53:56 PM



FILE NAME: pw:\\hbme

Accurate GROUP, INC.

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (SHEET 6 OF 6) STRUCTURE NO. 016-0161 (WB)  Existing Structure: LOADING HS 20-44 The bridge was constructed in 1948 under Section 0707.1-HB. It bridge was widened in 1990 under Section 42 (VB 10, 12) and 0707.1 HB (B-Y-I-86). In 1992, the bridge was reconstructed under Section 42 (VB 10, 12, HB 14, B-11) and 0707.1 HB (R-3)90. This structure carries eight 12' lanes (four westbound & four eastbound) of 1-94 over Dolton Avenue. The outer shoulder is 6' wide, No future wearing surface is allowed. and the inner shoulder is 4'-4½". The bridge has three spans (49'-9½", 63'-10½", and 49'-9½") and a Skew of 12°-03'-45". The abutments are pile-supported stub abutments. The bridge has DESIGN SPECIFICATION reinforced concrete column piers on pile foundations with an extension at each end. 2002 AASHTO Standard Specifications for Highway Bridges, 17th Edition Traffic is to be maintained utilizing stage construction. 168'-11/8" Bk. to Bk Abutments EXISTING DESIGN STRESSES No Salvage. N. Approach S. Approach 49'-91/8" 63'-107/8" 49'-91/8" (1992 RECONSTRUCTION) Span 1 Span 2 Span 3 2'-4" 77'-2<sup>7</sup>/8" f'c = 3500 psi (Superstructure)@ Brg. S. Abut. fy = 60,000 psi (Reinforcement)Limits of Protective Shield Reconstruct Exp. Jt. 1'-3" 5'-5" Superstructure) Sidewall Sidewalk - Reconstruct Exp. Jt. fs = 24,000 psi (Structural)— @ Pier 1 @ Pier 2 -PROPOSED DESIGN STRESSES f'c = 4,000 psi (Superstructure) Exist. Fence Exist. W30 Beams fy = 60,000 psi (Reinforcement)to remain & Dolton Ave. Perform Structural Repair Perform Structural Repair 4'-0" Exist. Fence of Concrete and Epoxy of Concrete at North Abut. Perform Slope Wall Conc Med. Crack Injection at Repairs (typ. South Abut. at both Abut.) Exist. Ground Perform Structural Repair line Exist. Concrete Piles, of Concrete and Epoxy Crack 12'-0" 12'-0" 12'-0" Injection at Pier 1 (typ. at Abutements) Perform Structural Repair Lane Lane Lane JUNSHAN LIU Exist. Concrete Piles of Concrete at Pier 2 (typ. at Piers) ELEVATION 081-008224 (Looking East) N. Approach S. Approach 168'-11/8" Bk. to Bk. Abut. ATE OF ILLIN 2'-41/8" 49'-91/8" 63'-10<sup>7</sup>/8' 49'-91/8" Span 1 Span 2 Span 3 Exp: 11/30/2026 Date: 12/05/2024 Drainage Scupper ——— (Special), typ. Exist. fence -Brg. N. Abut to remain Pier 1 ← Structure
 Sta 477+90.79 & Brg. S. Abut Sta. 478+72.50 Sta. 478+40.57 Sta. 479+54.24 478+00 1479+00 Bk. of N. Abut Stations Increase B and P.G.L. -Bk. of S. Abut Sta. 477+88.44 Range 14E, 3rd P.M. Sta. 479+56.58 EB I-94 © Pier 2 Sta. 479+04.48 Harbor 61'-6½" -to-0ut Perform Partial Depth Belt Deck Slab Repairs Perform ¾" Concrete Bridge Deck Scarification and Apply © Dolton Ave ¾" Bridge Deck Thin Polymer xist. fence to remain Stony Island N. Slope Wall S. Slope Wall Reconstruct Exp. Jt. Reconstruct Exp. Jt LOCATION SKETCH GENERAL PLAN AND ELEVATION 17'-07/8" 17'-07/8 EB I-94 OVER DOLTON AVE V.I.F. V.I.FF.A.I. ROUTE 94 30'-0" SECTION 2019 180-RS & T 4'-0" COOK COUNTY 6" Ø Floor Drair 21'-5" 15'-0" 60'-0" Spacing typ. STATION: 478+72.50 S.N. 016-0161 All stations are to the I-94 EB P.G.L. and taken from existing plans. PLANDESIGNED - SUR REVISED -COUNTY STATE OF ILLINOIS DRAWN - ME REVISED -Accurate 94 (42-B-11-1) BR, BJR 24 COOK 761 612 STRUCTURE NO. 016-0161 (EB) 20:0 ':" / in. CHECKED -REVISED -**DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 62W87 SHEET S06-01 OF S06-28 SHEETS PLOT DATE = 12/9/2024 DATE - 12/6/2024 REVISED -

#### GENERAL NOTES

- 1. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 2. During repair operations the contractor shall locate and protect any utilities or facilities including but not limited to the fiber optic and/or electrical conduits, conduits under the bridge deck, under lighting, traffic signals or signs attached to the structure. This work is to be performed to the satisfaction of the engineer and will not be paid for separately, but shall be included with the contract. It will be the contractor's responsibility to restore and replace any damage utilities or facilities to the satisfaction of the engineer and the department.
- 3. All exposed concrete edges shall have a  $\frac{3}{4}$ " x 45" chamfer except where shown otherwise.
- 4. Protective Coat shall be applied to the top and inside face of parapets.
- 5. Repairs shown are based upon inspection carried out at the time of plan preparation are for bidding purposes only. Actual area to be repaired and the type(s) of repairs to be used shall be determined by the engineer in the field at the time of construction.
- 6. The contractor shall take the necessary precautions for the protection of passing vehicles, bicycles, and pedestrians from falling objects and/or materials until completion of work.
- 7. Where underpass lighting is present on the structure, the Contractor shall adjust the Protective Shielding to ride above the existing lighting fixtures in order to maintain the existing level of lighting on the roadway underneath. Details shall be approved by the Engineer before installation.
- 8. Any adjustment done to the Protective Shield System must not change the load carrying capacity (or containment specifications) as indicated in the Standard Specifications, Cost of adjusting shielding is included in the cost of Protective Shield.
- 9. Concrete Sealer shall be applied to the designated areas of the abutments.
- 10. Prior to the application of the Concrete Sealer, the Contractor shall clean all existing debris from the abutment seats. The method of debris removal shall not damage the existing concrete and shall be approved by the Engineer. See Special provision for Debris Removal.
- 11. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose detrimental foreign material shall be removed from the surfaces in contact with concrete (SSPCSP3 standards). Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be paid for according to Article 109.04 of the Standard Specifications. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding ¼ inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 12. Cost of removal and re-installation of all members necessary to complete the work as detailed on the plans and as specified in the Special Provisions shall be included with Structural Steel Repairs.
- 13. Existing reinforcement extended into the removal area shall be cleaned, straightened and incorporated into the new construction cost is included with concrete removal. Any reinforcement bars that are damaged during concrete removal operations shall be replaced using an approved bar splicer or anchorage system at the Contractor's expense.
- 14. Cleaning and field painting of structural steel shall be done under a separate painting contract.
- 15. The Contractor is responsible to remove, support and reinstall all existing electrical conduits interfering with the work. See special provision "Protection and Maintenance of Existing Underpass Luminaires".
- 16. Reinforcement bars designated (E) shall be epoxy coated.

- 17. The Contractor is responsible to protect the existing conduit and junction box embedded in the parapet during removal and construction. Any damage to the existing conduit and junction box shall be repaired by the Contractor at no additional cost to the Department.
- 18. No field welding is permitted except as specified in the contract documents.
- 19. The Engineer shall show actual locations and size of deck repairs on As-built Plans.
- 20. Bars indicated thus, 3x2-#5, indicates 3 lines of #5 bars with 2 lengths of bar per line.
- 21. Joint openings shall be adjusted according to Article 520.04 of the Standard Specification when the deck is poured at an ambient temperature other than 50° F.
- 22. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures", and the Standard Specifications. The color of the final finish coat shall be Gray, Munsell No. 5B 7/1. Cost included with Structural Steel Repair.
- 23. All new structural steel shall be hot-dip galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel".
- 24. Fasteners shall be ASTM F 3125 Grade A325 Type 1, mechanically galvanized bolts in painted areas. Bolts  $\frac{3}{4}$  in. diameter, holes  $\frac{15}{16}$  in. diameter, unless otherwise noted.
- 25. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

#### INDEX OF SHEETS

	<u> </u>
506-01	General Plan and Elevation
S06-02	General Notes, Index of Sheets & TBOM
506-03	Stage Construction (Sheet 1 of 2)
506-04	Stage Construction (Sheet 2 of 2)
S06-05	Temporary Concrete Barrier
<i>506-06</i>	Deck Repair Plan
<i>S06-07</i>	S. Abut. Joint Removal & Replacement (Sht. 1 of 3)
<i>S06-08</i>	S. Abut. Joint Removal & Replacement (Sht. 2 of 3)
<i>506-09</i>	S. Abut. Joint Removal & Replacement (Sht. 3 of 3)
S06-10	N. Abut. Joint Removal & Replacement (Sht. 1 of 3)
506-11	N. Abut. Joint Removal & Replacement (Sht. 2 of 3)
S06-12	N. Abut. Joint Removal & Replacement (Sht. 3 of 3)
S06-13	Preformed Joint Strip Seal
S06-14	Framing Plan
S06-15	Beam Straightening Details
S06-16	Structural Steel Repair Details
<i>S06-17</i>	South Abutment Repairs
<i>506-18</i>	North Abutment Repairs
506-19	Pier 1 Repairs
506-20	Pier 2 Repairs
<i>506-21</i>	Slope Wall Repairs
<i>S06-22</i>	Bar Splicer Assembly & Mechanical Splicer Details
506-23	Existing Plans (Sheet 1 of 6)
506-24	Existing Plans (Sheet 2 of 6)
<i>S06-25</i>	Existing Plans (Sheet 3 of 6)
506-26	Existing Plans (Sheet 4 of 6)
<i>S06-27</i>	Existing Plans (Sheet 5 of 6)
<i>S06-28</i>	Existing Plans (Sheet 6 of 6)

#### SCOPE OF WORK

- 1. Provide Protective Shield within limits indicated on the plans.
- 2. Scarify %" from the bridge deck slab.
- 3. Perform Deck Slab Repairs (Partial).
- 4. Remove and Reconstruct Expansion joints at North and South abutments and install new Preformed Joint Strip Seals.
- 5. Apply ¾" Thin Polymer Overlay on Bridge Deck.
- 6. Refer to Roadway plans for Approach Pavement Rehabilitation.
- 7. Apply Protective Coat to the top of reconstructed transverse joint areas, top and inside faces of parapets.

### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	Cu Yd	-	1	1
Concrete Removal	Cu Yd	18.2	-	18.2
Slope Wall Removal	Sq Yd	-	2	2
Protective Shield	Sq Yd	529	-	529
Concrete Superstructure	Cu Yd	18.2	-	18.2
Protective Coat	Sq Yd	203	-	203
Reinforcement Bars, Epoxy Coated	Pound	3,150	-	3,150
Bar Splicers	Each	32	-	32
Slope Wall 4 Inch	Sq Yd	-	2	2
Preformed Joint Strip Seal	Foot	124	-	124
Anchor Bolt, 1"	Each	1	-	1
Concrete Sealer	Sq Ft	-	928	928
Epoxy Crack Injection	Foot	-	30	30
Slope Wall Crack Sealing	Foot	-	154	154
Structural Steel Repair	Pound	190	-	190
Beam Straightening	L Sum	0.33	-	0.33
Bridge Deck Scarification ¾"	Sq Yd	1,042	-	1,042
Bridge Deck Thin Polymer Overlay %"	Sq Yd	1,042	-	1,042
Structural Repair of Concrete (Depth Equal to or less than 5")	Sq Ft	-	93	93
Deck Slab Repair (Partial)	Sq Yd	46	-	46

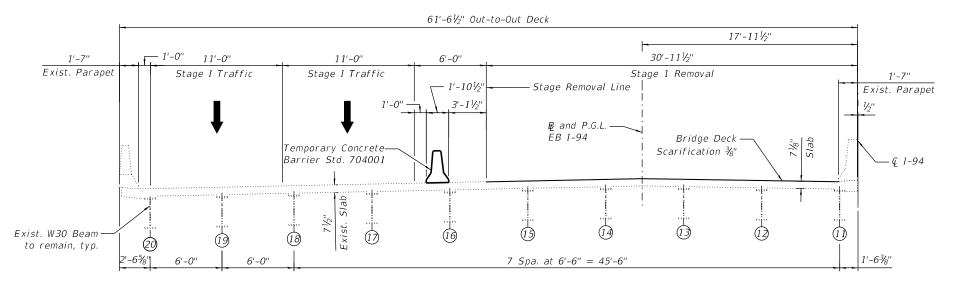
- 8. Perform structural concrete repairs to abutments and piers, as noted on plans.
- 9. Perform structural steel repairs to beams, as noted on plans.
- 10. Perform Slope Wall repairs.

-								
- 1	Δ Δ	C	С	ш	r	а	t	•
Š		•		ROUP			٠	`
1								

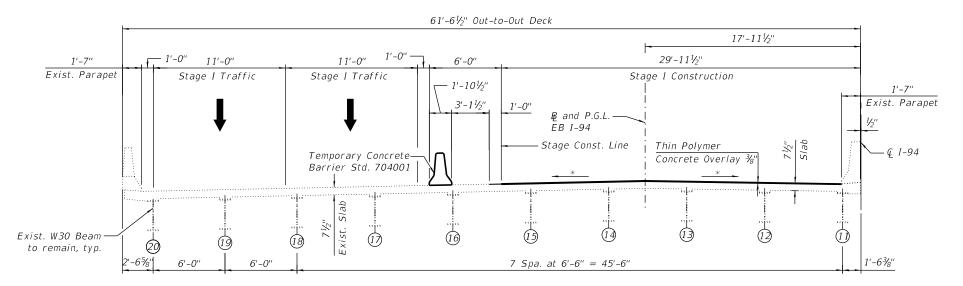
USER NAME	-	imranh	DESIGNED	-	SUR	REVISED	-
			DRAWN	-	ME	REVISED	-
PLOT SCALE	-	0:2.0000 ':" / in	CHECKED	-	JL	REVISED	-
PLOT DATE	-	12/9/2024	DATE	-	12/6/2024	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1) BR, BJR	соок	761	613	
			CONTRACT	NO. 62	2W87
	u i moro	EED AU	D DDO IFOT		



STAGE I REMOVAL (Looking North)



STAGE I CONSTRUCTION (Looking North)

### STAGE I REMOVAL

- Install temporary concrete barrier as shown to locate traffic on the West side of the existing structure.
- 2. Perform 3/8" bridge deck scarification.
- 3. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the North and South Abutments.

### STAGE I CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct transverse expansion joints and install new preformed joint strip steals within the limits of Stage I Construction.
- 3. Perform Structural repair of concrete and epoxy crack injection for the abutments and piers.
- 4. Apply 3%" bridge deck thin polymer overlay.
- 5. Refer to Roadway plans for Approach Pavement Rehabilitation.
- 6. Apply protective coat to top and inside faces of East parapet, and reconstructed transverse expansion joints.
- 7. Perform Slope Wall repairs as shown on the plans.

\*Match existing cross slopes



USER NAME = imranh	DESIGNED -	SUR	REVISED -	1
	DRAWN -	ME	REVISED -	1
PLOT SCALE = 8:0 ':" / in.	CHECKED -	JL	REVISED -	1
PLOT DATE = 12/9/2024	DATE -	12/6/2024	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

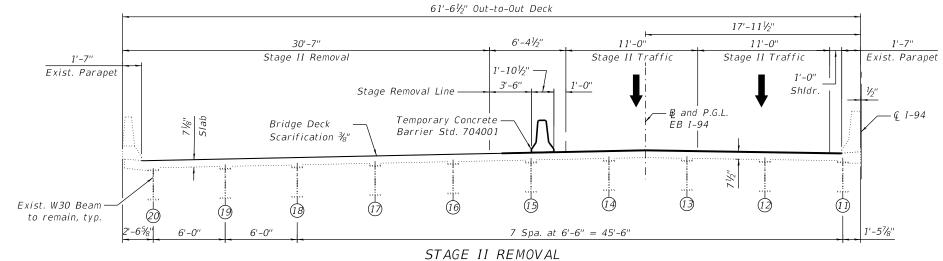
STAGE CONSTRUCTION (SHEET 1 OF 2)
STRUCTURE NO. 016-0161 (EB)

SHEET S06-03 OF S06-28 SHEETS

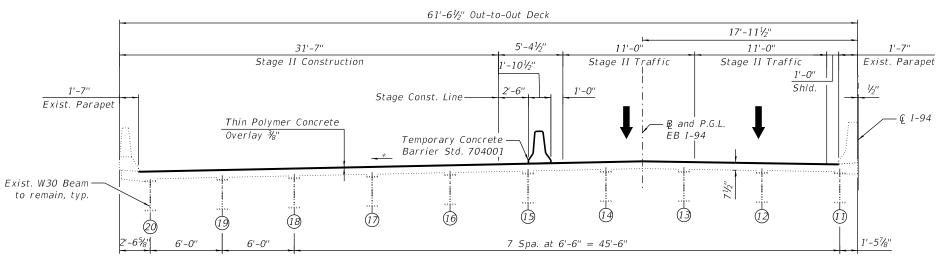
 
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEET NO.

 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 614

 CONTRACT NO. 62W87

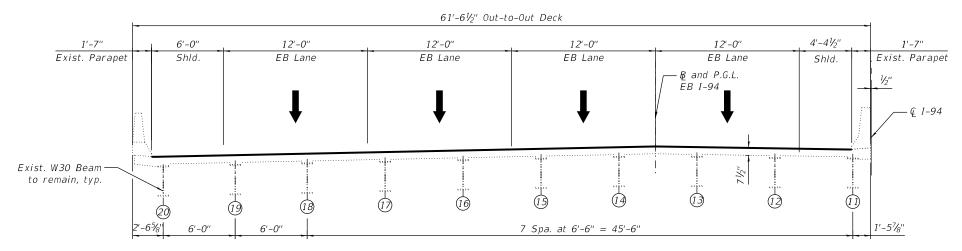


# (Looking North)



### STAGE II CONSTRUCTION

(Looking North)



### FINAL CROSS SECTION

(Looking North)

### STAGE II REMOVAL

- Install temporary concrete barrier as shown to locate traffic on the East side of the existing structure.
- 2. Perform 3/8" bridge deck scarification.
- 3. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the North and South Abutments.

### STAGE II CONSTRUCTION

- Perform bridge deck slab repairs.
- Reconstruct expansion joints and install new preformed joint strip seal within the limits of Stage II Construction.
- Perform structural repair of concrete and epoxy crack injection for the abutments and piers.
- 4. Apply ¾" bridge deck thin polymer overlay.
- Apply protective coat to top and inside faces of West parapets, and reconstructed abutment expansion joint areas.

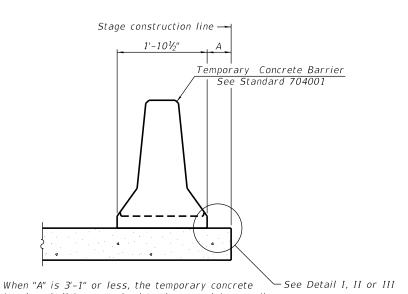
\*Match existing cross slopes

Accurate

JSER NAME = imranh DESIGNED - SUR REVISED -DRAWN - ME REVISED -LOT SCALE = 8:0 ':" / in. CHECKED - JL REVISED -PLOT DATE = 12/9/2024 DATE - 12/6/2024 REVISED -

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION**  STAGE CONSTRUCTION (SHEET 2 OF 2) STRUCTURE NO. 016-0161 (EB) SHEET S06-04 OF S06-28 SHEETS

94 (42-B-11-1) BR, BJR 24 соок 761 615 CONTRACT NO. 62W87



barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

Stage removal line ← Stage removal line 1'-101/5" 1'-101/5" Temporary Concrete Barrier See Standard 704001 min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint \* When hot-mix asphalt wearing surface is present, embedment is required when "A" is greater than 3'-1".

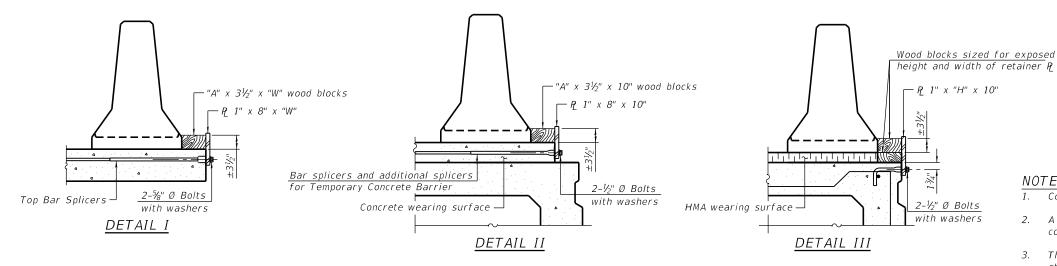
shall be 3" plus the wearing surface depth.

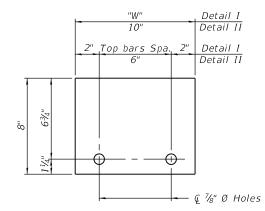
EXISTING DECK BEAM

### NEW SLAB OR NEW DECK BEAM

### SECTIONS THRU SLAB OR DECK BEAM

EXISTING SLAB





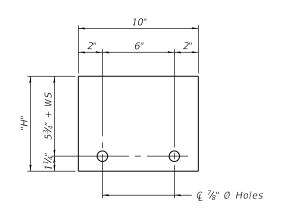
STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)

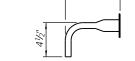
#### RAILING CRITERIA

NCHRP 350 Test Level Railing Weight (plf)

R - 2705-15-2023



### STEEL RETAINER P 1" x "H" x 10" (Detail III)



### BAR SPLICER FOR #4 BAR - DETAIL III

RESTRAINING PIN

Cost of retainer assembly is included with Temporary Concrete Barrier.

1x8 UNC

US Std. 11/16" I.D. x 21/2" O.D. x approx. 8 gauge thick washer

- 2. A retainer assembly shall be located at the approximate © of each temporary concrete barrier.
- 3. The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.
- 4. When the 'A' dimension is less than  $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.
- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

Accurate

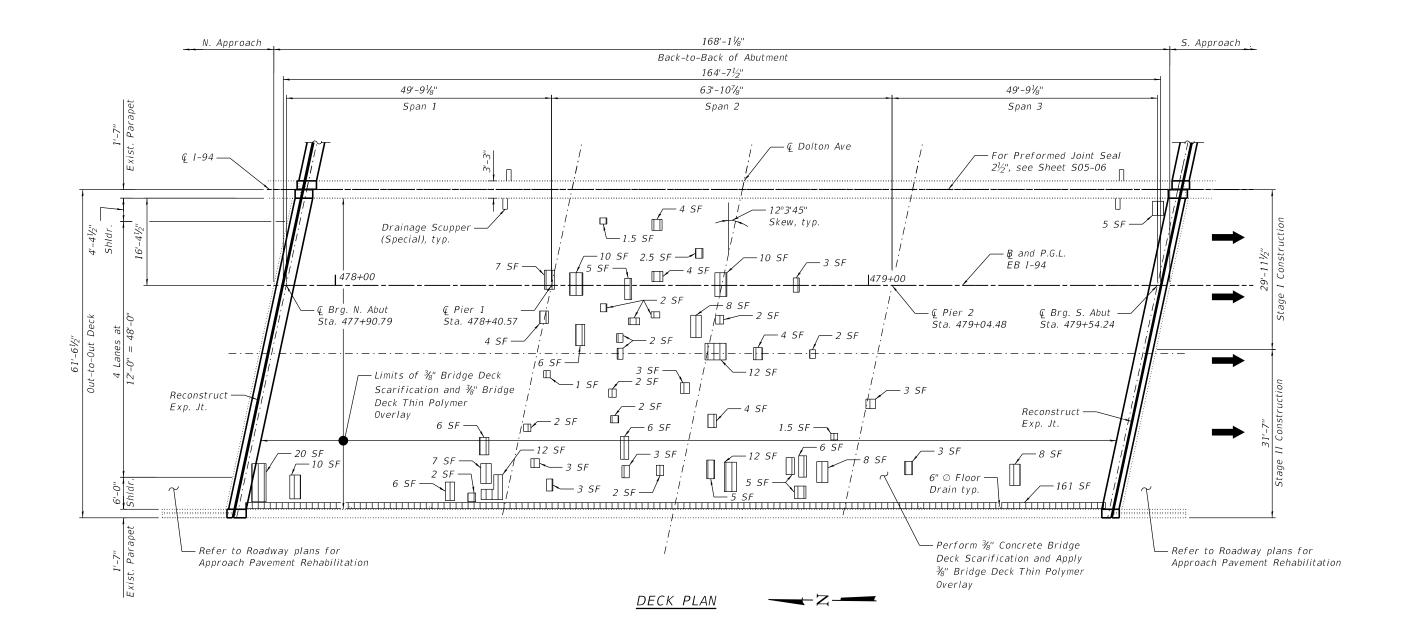
USER NAME	-	imranh	DESIGNED	-	SUR	REVISED	-
			DRAWN	-	ME	REVISED	-
PLOT SCALE	-	0:2.0000 ':" / in.	CHECKED	-	JL	REVISED	-
PLOT DATE	-	12/9/2024	DATE	-	12/6/2024	REVISED	-

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION** 

TEMPORARY CONCRETE BARRIER						
STRUCTURE NO. 016-0161 (EB)						
QUEET SAS AS OF	SAG 20 CHEETS					

A I. TE.	SEC.	TION		COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1) BR, BJR 24			COOK	761	616
				CONTRACT	<b>NO</b> . 6	2W87
		ILLINOIS	FED. All	D PROJECT		

12/9/2024 3:14:16 PM



### NOTES:

- 1. Areas of deck repair shown are estimated. The Engineer shall show actual locations of deck repairs at the time of construction.
- 2. For bridge deck final cross section, See Sheet S06-04.
- 3. For North and South Joint Removal and Replacement, See Sheets S06-07 through S06-12.
- 4. Protective Coat shall be applied to top and inside face of parapets, and the joint areas.
- 5. Any reinforcement bars that are damaged during concrete removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.
- 6. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to the existing steel beams and/or diaphragms to remain caused by the Contractor in the performance of his/her work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.

Deck

<u>LEGEND</u>

Deck Slab Repair (Partial Depth)

ITEM	UNIT	QUANTITY
Protective Coat	Sq Yd	149
Bridge Deck Thin Polymer Overlay ¾"	Sq Yd	1042
Bridge Deck Scarification ¾"	Sq Yd	1042
Deck Slab Repair (Partial)	Sa Yd	46

BILL OF MATERIAL

Accurate GROUP, INC.

	USER NAME	=	imranh	DESIGNED	-	ME	REVISED -
Э				DRAWN	-	SUR	REVISED -
	PLOT SCALE	=	18:0.0000 ':" / in.	CHECKED	-	JL	REVISED -
	PLOT DATE	-	12/9/2024	DATE	-	12/6/2024	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

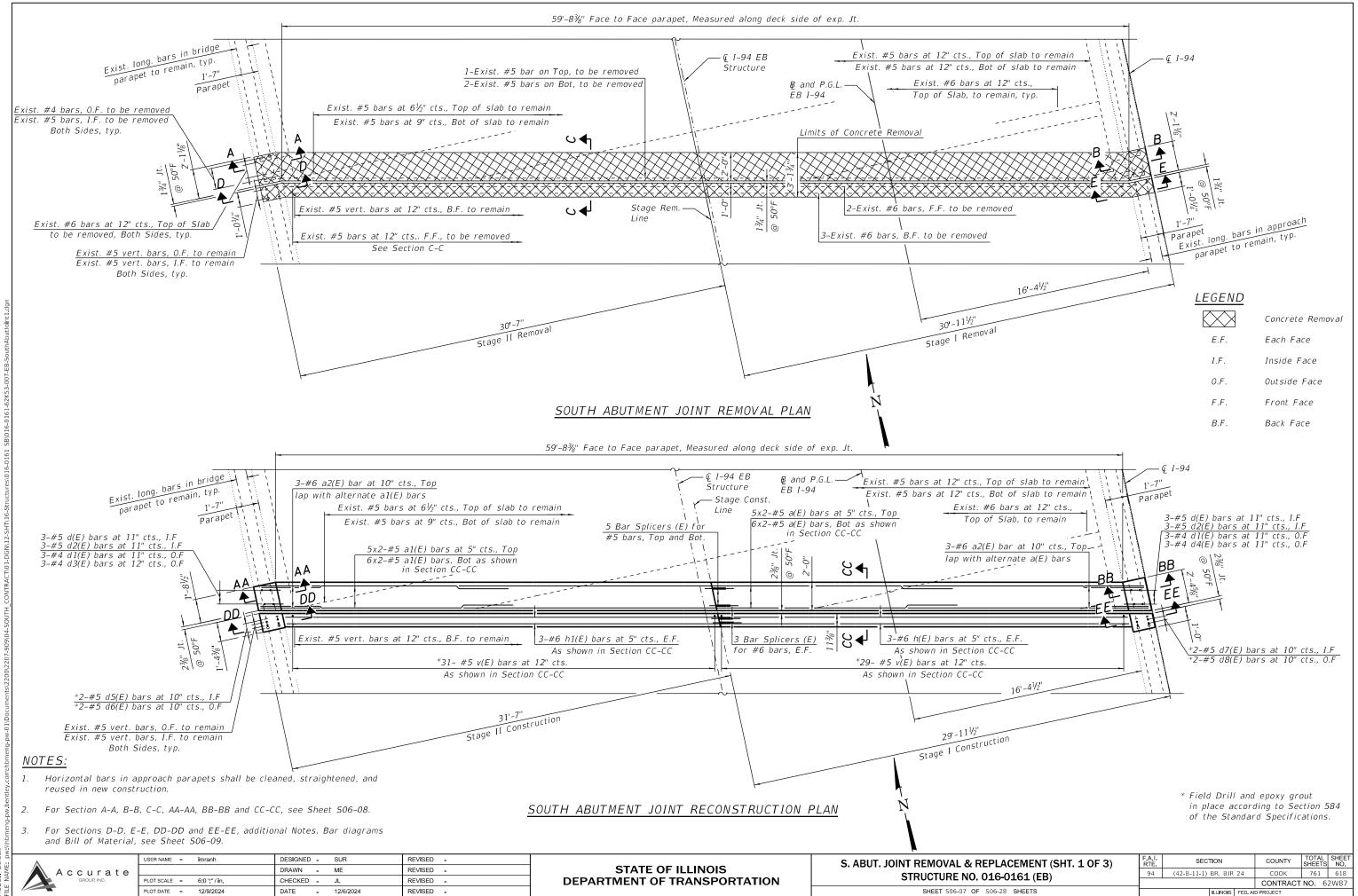
 DECK REPAIR PLAN
 F.A.I. RTE.
 SEC

 STRUCTURE NO. 016-0161 (EB)
 94 (42-B-11-1)

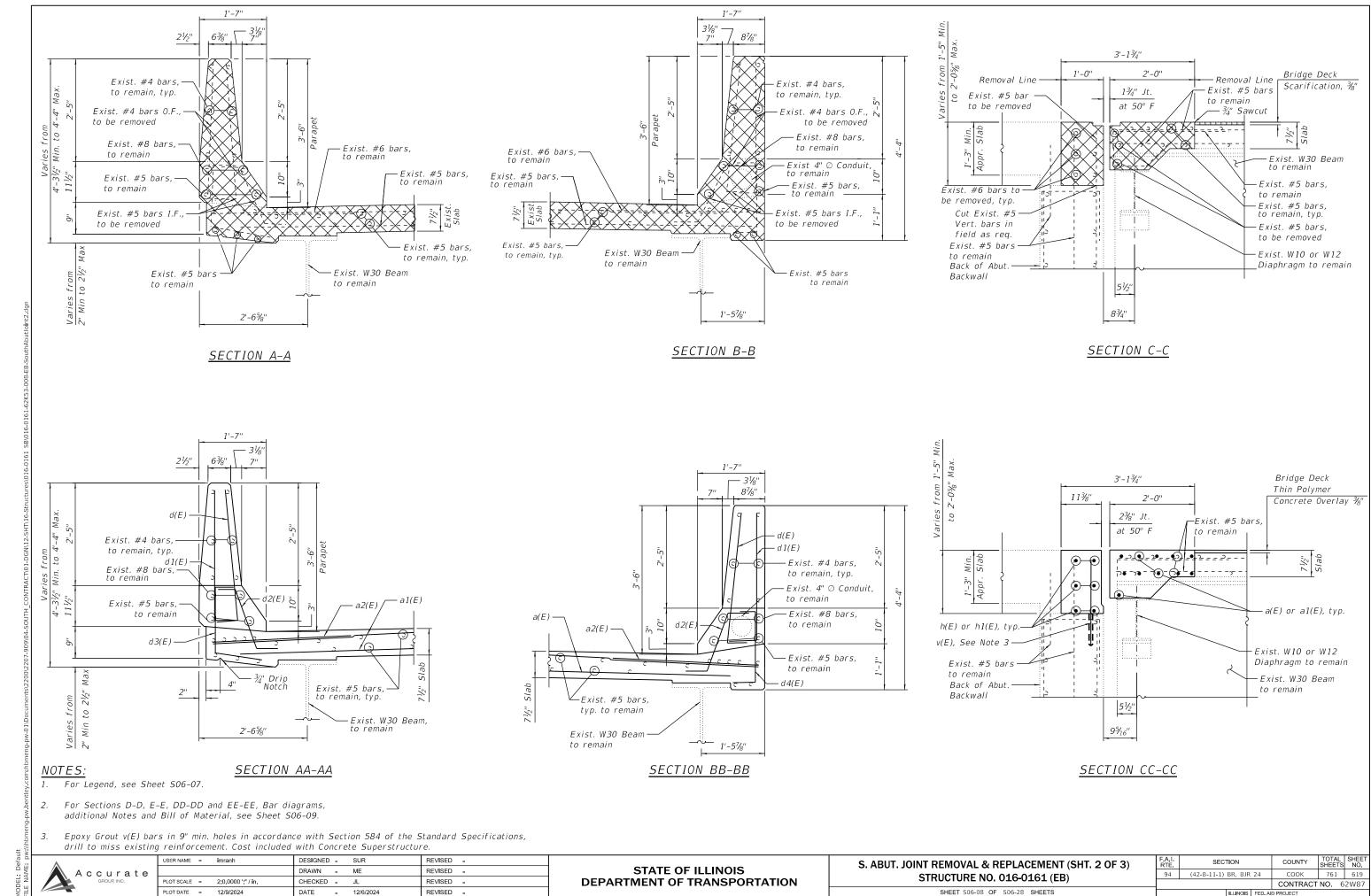
F.A.I. SECTION COUNTY TOTAL SHEETS NO.

94 (42-B-11-1) BR, BJR 24 COOK 761 617

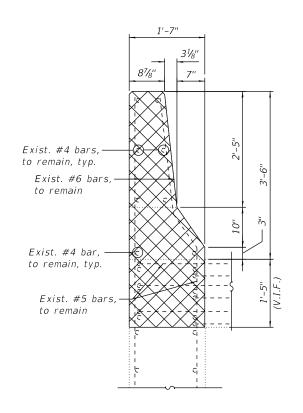
CONTRACT NO. 62W87



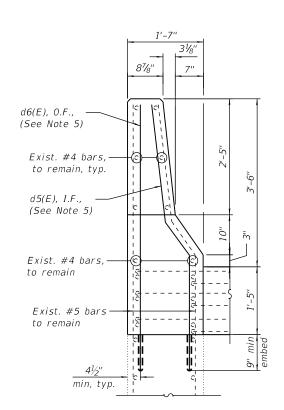
12/9/2024 3:14:22 PM



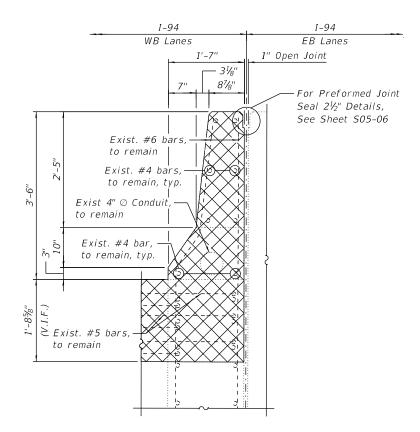
12/9/2024 3:14:25 PM



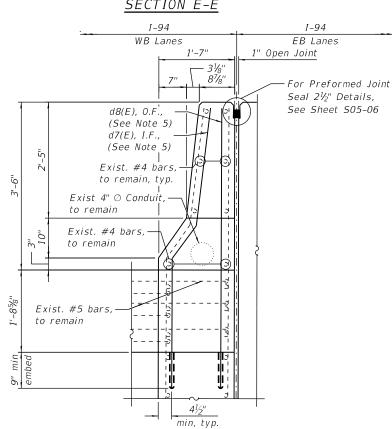
## SECTION D-D



SECTION DD-DD



### SECTION E-E

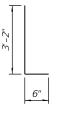


SECTION EE-EE

### BILL OF MATERIAL

### MIN BAR LAPS 3'-6" 4'-10"

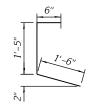
Bar	No.	Size	Length	Shape
a(E)	22	#5	17'-1"	
a1(E)	22	#5	17'-10"	
a2(E)	6	#6	6'-6"	
d(E)	6 #5		3'-8"	L
d1(E)	6 #4		3'-8"	L
d2(E)	6	#5	2'-7"	~
d3(E)	3	#4	3'-5"	C
d4(E)	3	#4	2'-7"	С
d5(E)	(E) 3 (E) 3 (E) 2 (E) 2 (E) 2 (E) 2		5'-8"	/
d6(E)	2	#5	5'-6"	
d7(E)	2	#5	6'-1"	
d8(E)	2	#5	5'-10"	
h(E)	6	#6	28'-10"	
h1(E)	6	#6	30'-6"	
v(E)	60	#5	2'-0"	
Concrete	Removal	Cu Yd	8.9	
Concrete	Superst	Cu Yd	8.9	
Protectiv		Sq Yd	27	
Reinforce Epoxy Co		Pound	1,640	



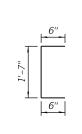




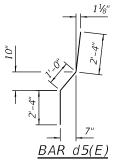
BAR d2(E)



**BAR** d3(E)



BAR d4(E)



**BAR** d7(E)

### NOTES:

- 1. For Legend, see Sheet S06-07.
- 2. For Preformed Joint Strip Seal Details, see Sheet S06-13.
- For Bar Splicer Assembly Details, see Sheet S06-22.
- Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 5. Epoxy grout d5(E), d6(E), d7(E) and d8(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.

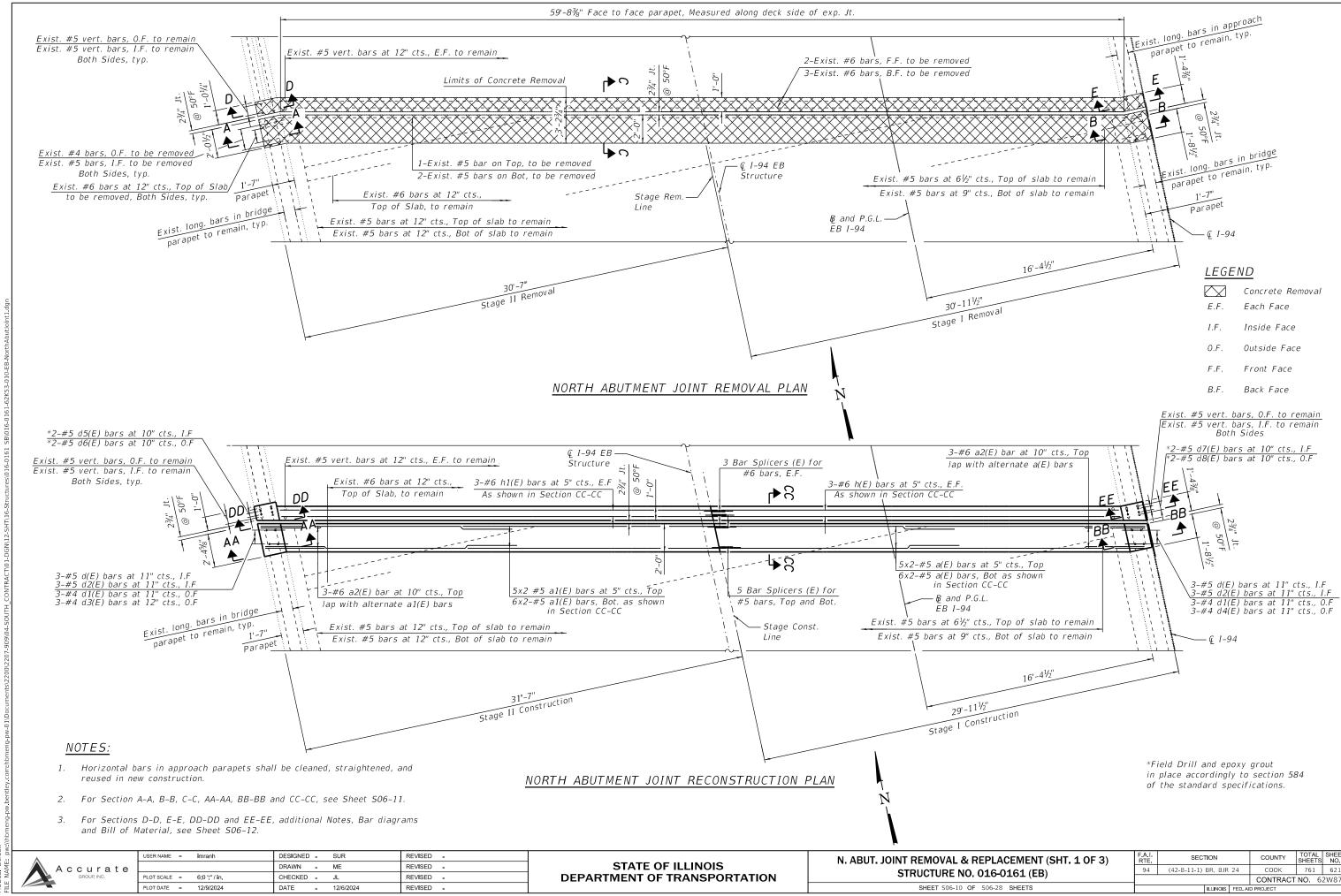


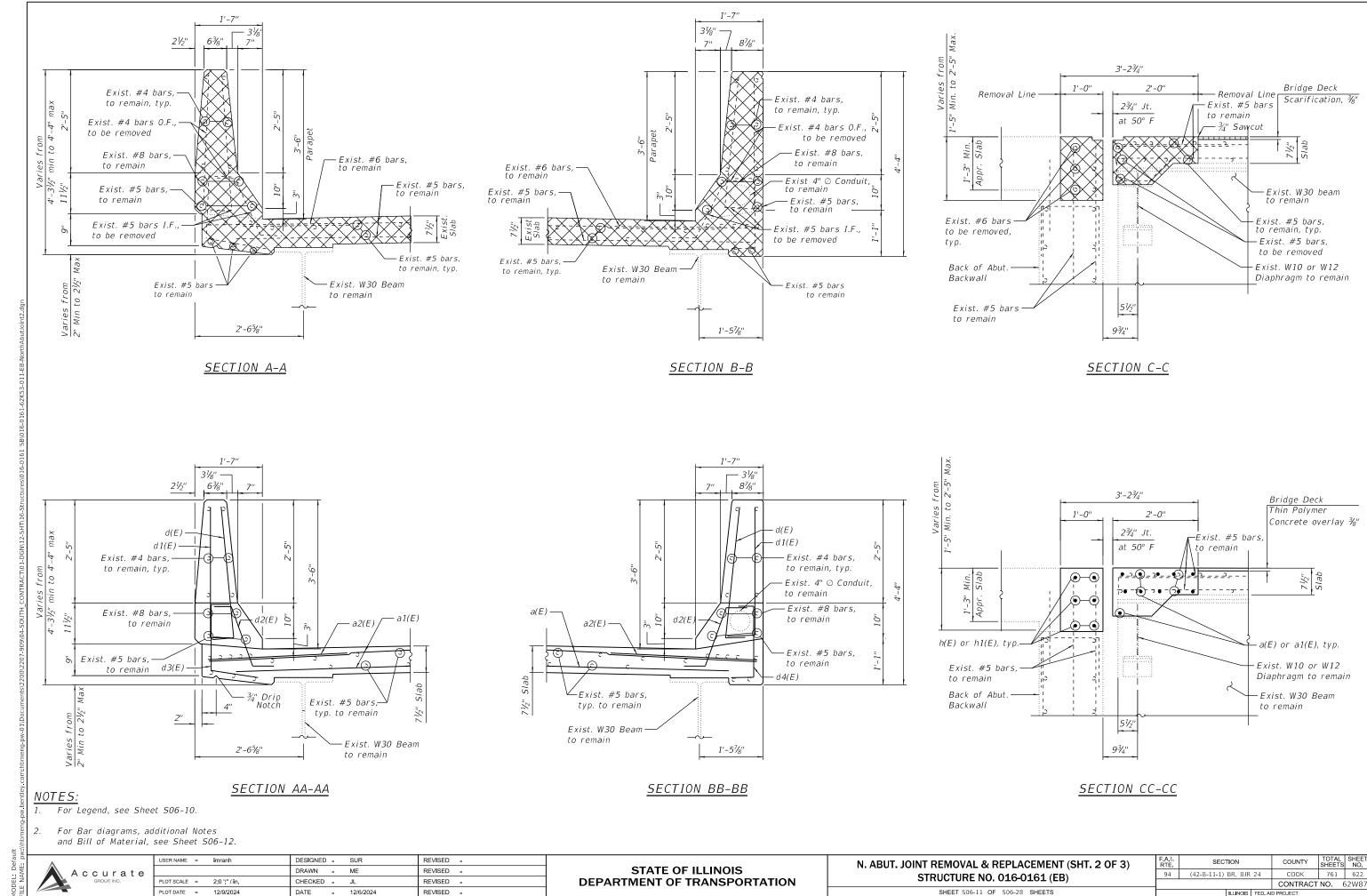
USER NAME	=	imranh	DESIGNED	-	SUR	REVISED	-
			DRAWN	-	ME	REVISED	-
PLOT SCALE	=	2:0.0000 ':" / in.	CHECKED	-	JL	REVISED	-
PLOT DATE	-	12/9/2024	DATE	-	12/6/2024	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  S. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 3 OF 3) STRUCTURE NO. 016-0161 (EB) SHEET S06-09 OF S06-28 SHEETS

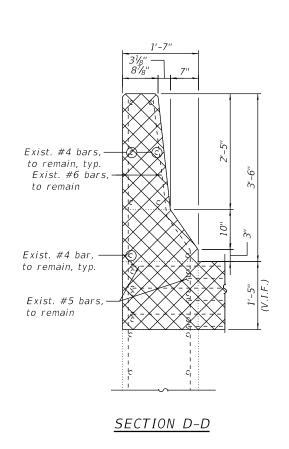
A.I.	SECT	TION		COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1)	BR, BJR	24	COOK	761	620
CONTRACT NO. 62W87						2W87

12/9/2024 3:14:28 PM





12/9/2024 3:14:34 PM



1'-7"

8<sup>7</sup>/8''

d5(E), I.F., (See Note 5)

Exist. #4 bars,

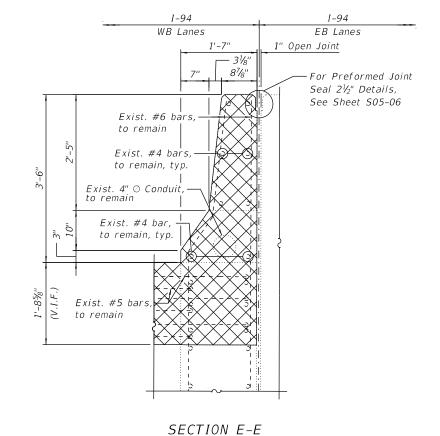
to remain, typ.

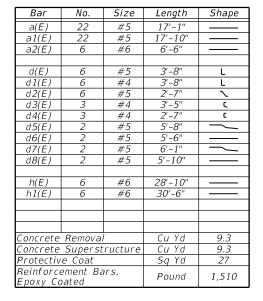
d6(E), 0.F., -

(See Note 5)

Exist. #4 bars, to remain

Exist. #6 bars, to remain





BILL OF MATERIAL

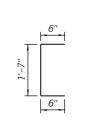
MIN BAR LAPS

3'-6" 4'-10"

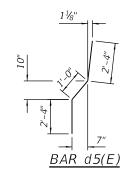
BARS d(E) & d1(E)

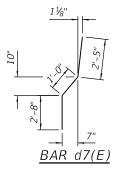
BAR d2(E)

 $BAR \ d3(E)$ 

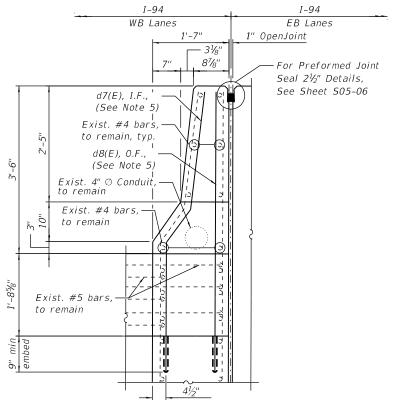


BAR d4(E)





I-94 WB Lanes



SECTION EE-EE

#### NOTES:

- For Legend, see Sheet S06-10.
- For Preformed Joint Strip Seal Details, see Sheet S06-13.
- For Bar Splicer Assembly Details, see Sheet S06-21.
- Removal and disposal of the existing expansion joints is included with Concrete Removal.
- Epoxy grout d5(E), d6(E), d7(E), and d8(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.

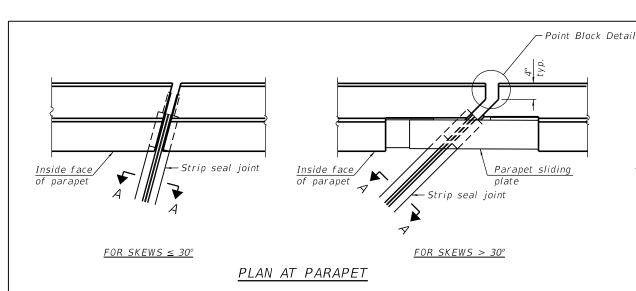
## SECTION DD-DD

USER NAME = imranh	DESIGNED - SUR	REVISED -
	DRAWN - ME	REVISED -
PLOT SCALE = 2:0.0000 '." / in.	CHECKED - JL	REVISED -
PLOT DATE = 12/9/2024	DATE - 12/6/2024	REVISED -

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION**  N. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 3 OF 3) STRUCTURE NO. 016-0161 (EB) SHEET S06-12 OF S06-28 SHEETS

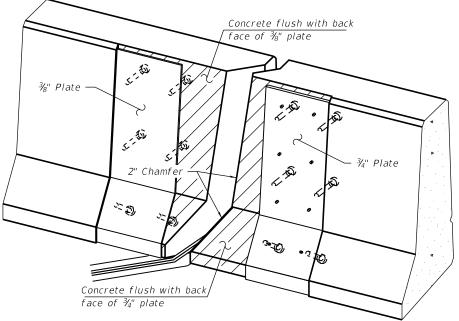
94 (42-B-11-1) BR, BJR 24 COOK 761 623 CONTRACT NO. 62W87

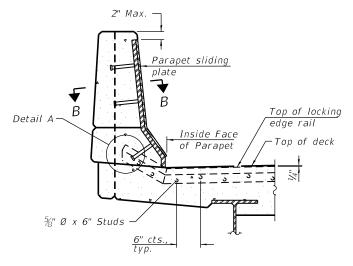
Accurate



#### \* ¾" Ø x 6" Studs 1'-0" (8 per side 39" parapet) (10 per side 44" parapet) $^{\circ}$ $\Gamma$ $^{\circ}$ $^{\circ}$ $^{\circ}$ Embedded plate I full depth 3/8" Embedded plate Min. lap full depth $\frac{1}{2}$ " Parapet sliding plate ¾" Ø Countersunk bolts 1'-0" (10 per side 39" parapet) (12 per side 44" parapet) <u>Direction</u> of traffic

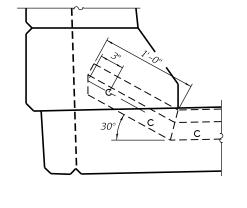
### SECTION B-B





### ELEVATION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)



DETAIL A

### Notes:

The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

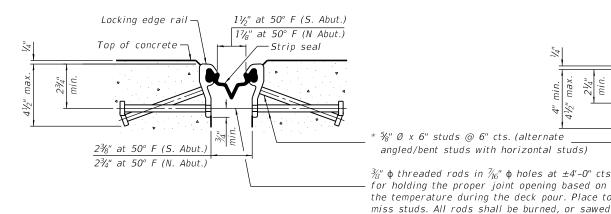
Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

39" constant slope barrier shown, 44" constant slope barrier

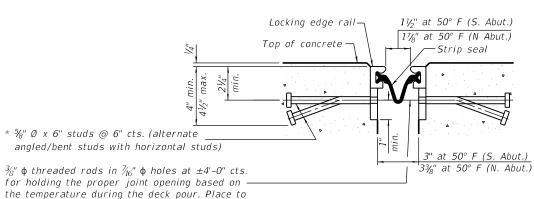
The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

## TRIMETRIC VIEW

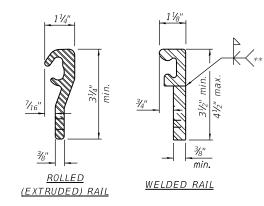
(Showing embedded plates only)



SHOWING ROLLED RAIL JOINT

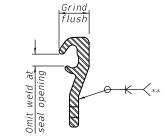


SHOWING WELDED RAIL JOINT off flush with the plates after concrete is set.



### LOCKING EDGE RAILS

\*\* Back gouge not required if complete joint penetration is verified by mock-up.



#### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

#### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	124

#### SECTION A-A

\* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

Accurate

USER NAME	-	imranh	DESIGNED	-	SUR	REVISED	-
			DRAWN	-	ME	REVISED	-
PLOT SCALE	-	0:2.0000 ':" / in	CHECKED	-	JL	REVISED	-
PLOT DATE	-	12/9/2024	DATE	-	12/6/2024	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  PREFORMED JOINT STRIP SEAL STRUCTURE NO. 016-0161 (EB) SHEET S06-13 OF S06-28 SHEETS

щŽ	SECTIO	ON		COUNTY	TOTAL SHEETS	SHEET NO.
4	(42-B-11-1) B	R, BJR	24	COOK	761	624
				CONTRACT	NO. 6	2W87
		LINOIC	EED AU	D DDO IECT		

12/9/2024 3:14:39 PM

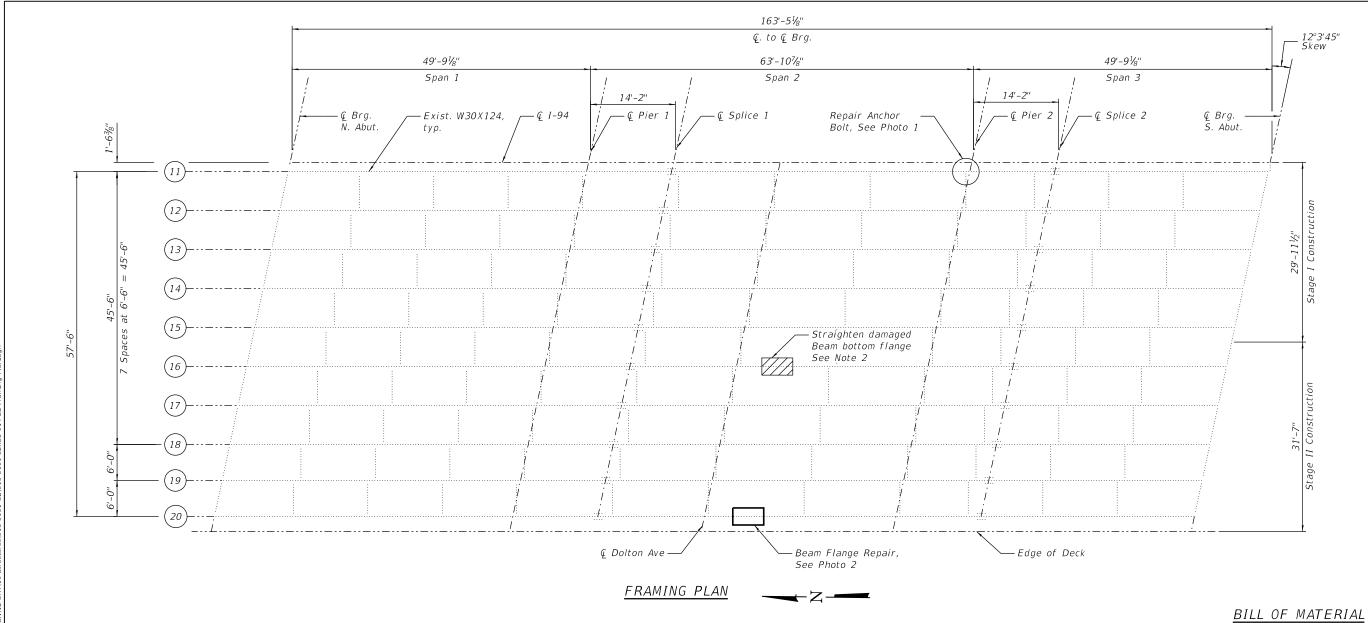




PHOTO 1: REPAIR ANCHOR BOLT



PHOTO 2: BEAM FLANGE REPAIR

ITEM	UNIT	QUANTITY
Beam Straightening	L Sum	0.33
Anchor Bolts, 1"	Each	1

### <u>LEGEND</u>

Straighten Damaged Beam Flange

Perform Beam Mid-Span Plating, Paid for as Structural Steel Repair

### NOTES:

- 1. All work is to be performed utilizing stage construction, See Sheets S06-03 and S06-04 for details.
- 2. Gouges caused by the impact should be ground to eliminate sharp or sudden irregularities in the beam surface. Grinding should be done in such a way as to provide a smooth transition with a maximum slope of 3:1 between the damaged and undamaged surfaces. Cost included in Structural Steel Repair.
- 3. See Sheet S06-16 for Structural Steel Repair Details.



imranh	DESIGNED -		IH	REVISED	-	
	DRAWN -		SUR	REVISED	-	
16:0 ':" / in	CHECKED -		JL	REVISED	-	
12/9/2024	DATE -		12/6/2024	REVISED	-	
	16:0 ':" / in.	DRAWN - 16:0 ':" / in. CHECKED -	DRAWN - 16:0 ':" / in. CHECKED -	DRAWN - SUR  16:0 '." / in. CHECKED - JL	DRAWN - SUR REVISED  16:0 '." / in. CHECKED - JL REVISED	DRAWN - SUR REVISED -  16:0 '." / in. CHECKED - JL REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

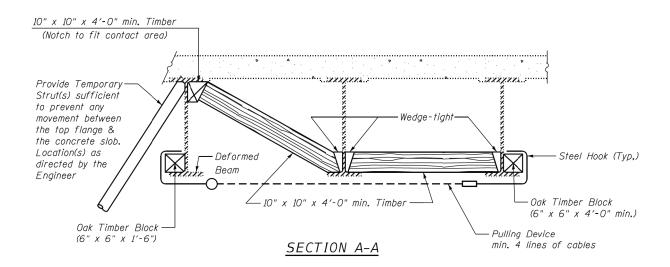
FRAMING PLAN STRUCTURE NO. 016-0161 (EB) SHEET S06-14 OF S06-28 SHEETS

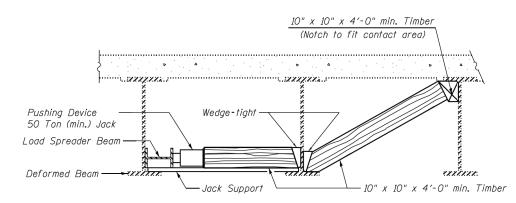
94 (42-B-11-1) BR, BJR 24 COOK 761 625 CONTRACT NO. 62W87

12/9/2024 3:14:42 PM

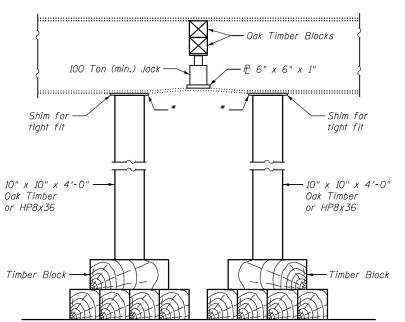
### PARTIAL PLAN SUGGESTED BEAM STRAIGHTENING METHODS

NOTE: Straightening force shall be maintained on all load transfer blocking during beam straightening.





### SECTION B-B



### SUGGESTED VERTICAL STRAIGHTENING DETAIL

(To correct localized vertical flange deformations.)

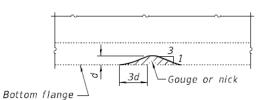
\* Edge of plate shall line up with edge of deformation.

#### NOTE:

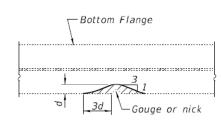
Braces and jack assembly shall be placed on same side of web. Bent bottom flange shall be straightened before starting any horizontal straightening operations.

## ·Oak Timber Blocks 50 Ton (min.) Jack Place on HP Bm. or a 6" I-Bm. min. ±1'-6" in length\_on Bottom Flange. Bm. shall hove $\frac{3}{8}$ " Stiffeners (3 ea. Deformed Beam side of Bm.) welded in place. Bent Bott. Flange shall be straightened before starting any Horiz. Straightening operation. -Shim Æ's as req'd. Timber Wedge 10" x 10" x 4'-0" Oak Timber or HP8x36 50 Ton (min.) Jack — Timber Block

### VERTICAL STRAIGHTENING DETAIL



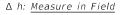
#### ELEVATION



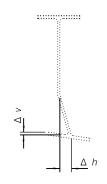
PLAN

### GRINDING DETAIL

Grind existing nicks, gouges and shallow cracks in the damaged beams as detailed. Grinding shall be done parallel to the longitudinal axis of the member. Ground surfaces shall be inspected for cracks using dye penetrant or magnetic particle testing prior to initiating any beam straightening operations. Any cracks that cannot be removed by grinding approximately  $lam{1}{4}$  deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. Ground surfaces shall be spot cleaned and painted with an aluminum epoxy mastic primer followed by a finish coat to match the color of the existing beam. Cost of grinding, testing and spot painting is included with Beam Straightening.



#### Δ v: <u>Measure in Field</u>



### EXISTING DEFORMATION TO BE STRAIGHTENED

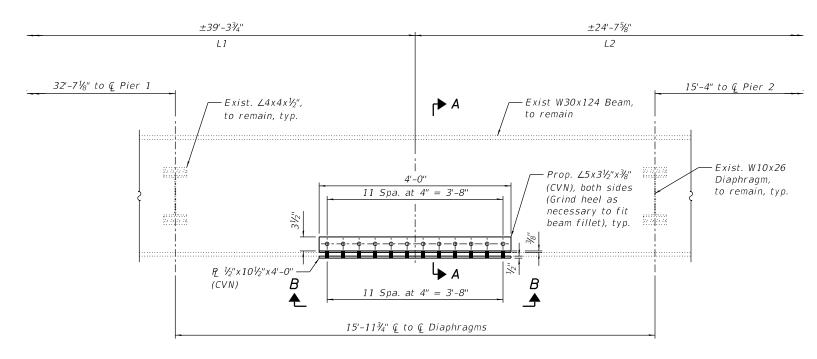
(Looking North) (Approximate max. deflections) Deflected length of beam to be straightened is approximately 4'-0".

Accurate

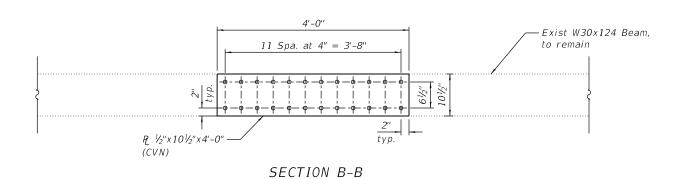
DESIGNED - SUR REVISED -DRAWN - ME REVISED -0:2.0000 ':" / in. CHECKED -REVISED -REVISED -PLOT DATE = 12/9/2024 DATE - 12/6/2024

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION**  **BEAM STRAIGHTENING DETAILS** STRUCTURE NO. 016-0161 (EB) SHEET S06-15 OF S06-28 SHEETS

SECTION 94 (42-B-11-1) BR, BJR 24 COOK 761 626 CONTRACT NO. 62W87



### BEAM ELEVATION



### NOTES:

- 1. For locations of Beam bottom flange Repairs, See Framing Plan on Sheet S06-14.
- 2. All Beam bottom flange repair plate and bottom flange repair angles shall conform to the requirements of AASHTO M270 Grade 36.
- 3. Beam bottom flange plate & bottom flange repair angles connection holes shall be  $^{15}\!\!f_{16}$ " Ø for  $^3\!\!4$  Ø bolts.
- All proposed beam bottom flange repair plate, bottom flange repair angles, field drilling and associated bolts and fasteners shall be paid for as Structural Steel Repair.
- Load carrying components designated "CVN" shall conform to the impact Testing Requirement, Zone 2.
- 6. Contractor to field verify hole locations before ordering material. Contractor can elect to field drill holes in repair plates.

Exist. ∠4 to remair	
Exist. W. Diaphrag to remai	m, Exist W30x124 Beam
Prop. $\angle 5x3\frac{1}{2}x\frac{3}{6}$ " (CVN), both sides (Grind heel as necessary to fit beam fillet), typ.	2" R 1/2" x 10 1/2" x 4'-0" (CVN)

### SECTION A-A

Mid-Span Repair Detail Table							
Direction		Span	Pier	Distance from @			
of Travel	Beam #	Number	Reference	Plate to & Pier			
I-94							
EB	20	2	1	39'-3½" (L1)			
EB	20	2	2	24'-75/8" (L2)			
EB		2	2				

### BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Structural Steel Repair	Pound	190

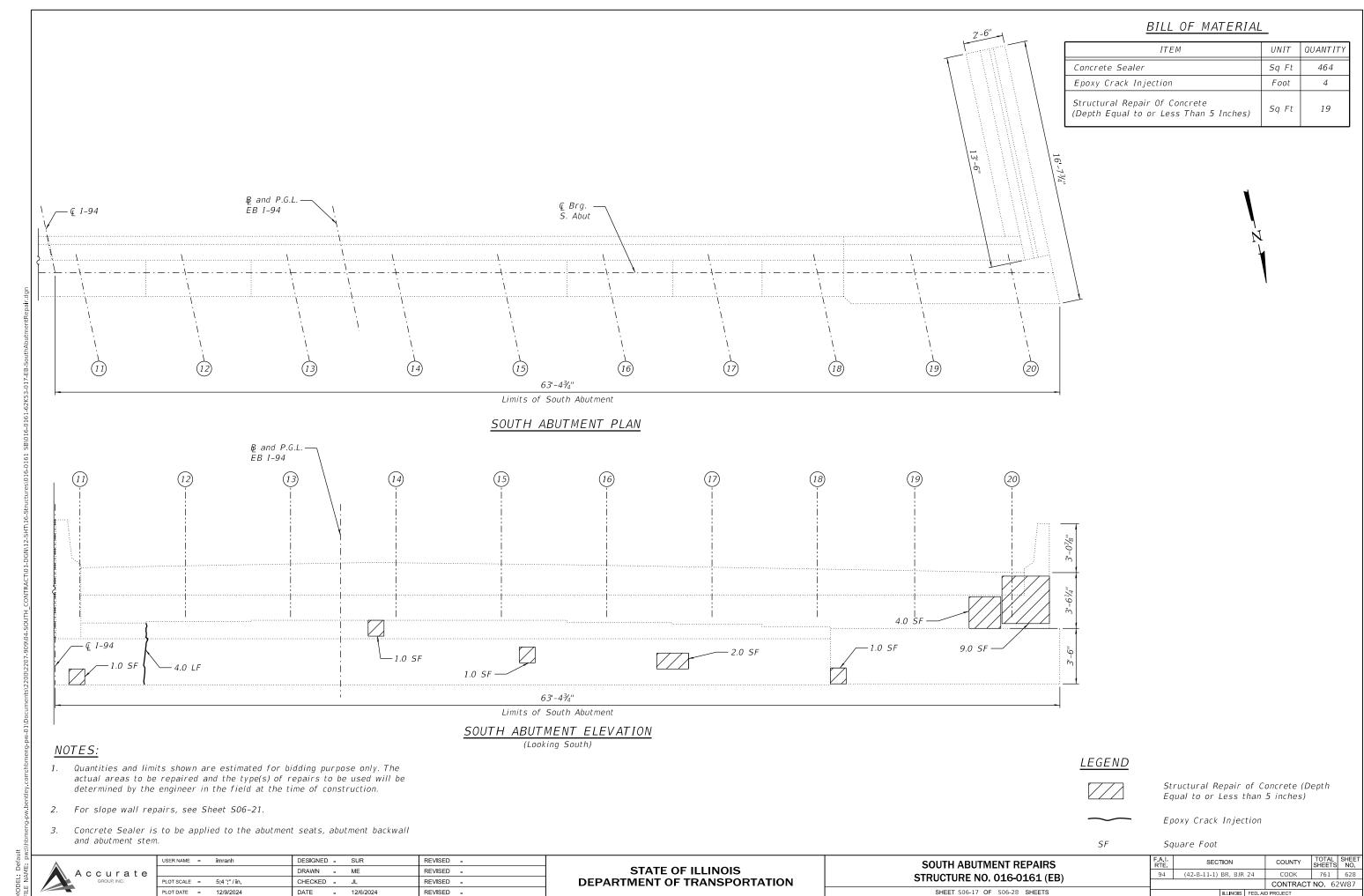
A	Α	С	U		t	е

USER NAME = imranh	DESIGNED - IH	REVISED -
	DRAWN - SUR	REVISED -
PLOT SCALE = 2:0 '." / in.	CHECKED - JL	REVISED -
PLOT DATE = 12/9/2024	DATE - 12/6/2024	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURAL S		 	
	-16 OF		

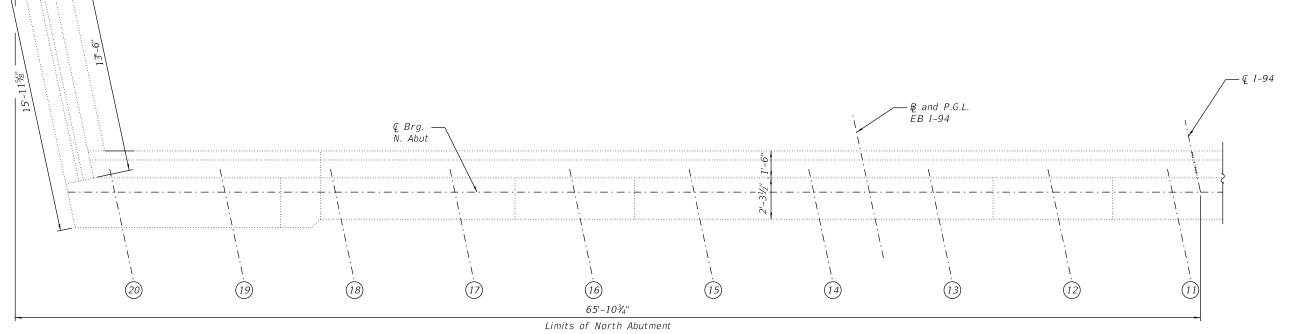
F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE.
94	(42-B-11-1) BR, BJR	24	соок	761	627
			CONTRACT	NO. 6	2W87
	HIMOR	CCD AL	D DDO IECT		



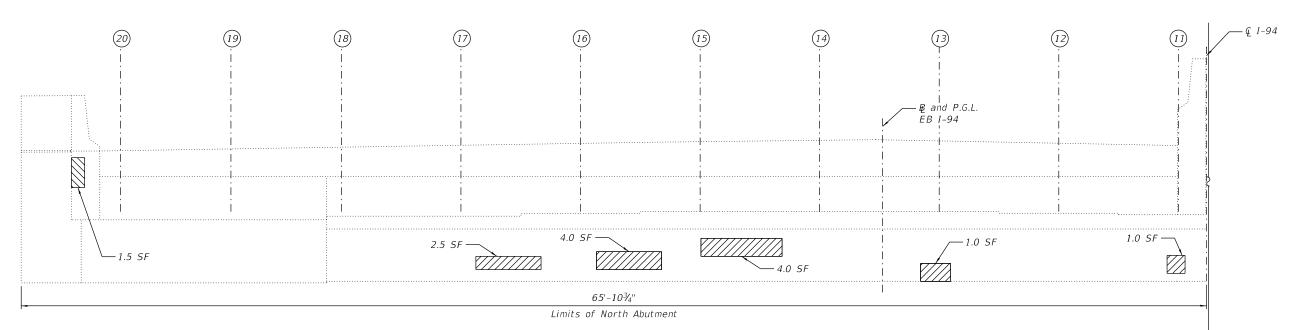
12/9/2024 3:14:57 PM

# BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	464
Structural Repair Of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	14



### NORTH ABUTMENT PLAN



### NOTES:

- 1. Quantities and limits shown are estimated for bidding purpose only. The actual areas to be repaired and the type(s) of repairs to be used will be determined by the engineer in the field at the time of construction.
- 2. For slope wall repairs, see sheet S06-21.
- 3. Concrete Sealer is to be applied to the Abutment Seats, Abutment Backwall and Abutment Stem.

NORTH ABUTMENT ELEVATION

(Looking North)

<u>LEGEND</u>

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Accurate GROUP, INC.

USER NAME =	imranh	DESIGNED	-	SUR	REVISED	-
		DRAWN	-	ME	REVISED	-
PLOT SCALE =	5:4 ':" / in.	CHECKED	-	JL	REVISED	-
PLOT DATE =	12/9/2024	DATE	-	12/6/2024	REVISED	-

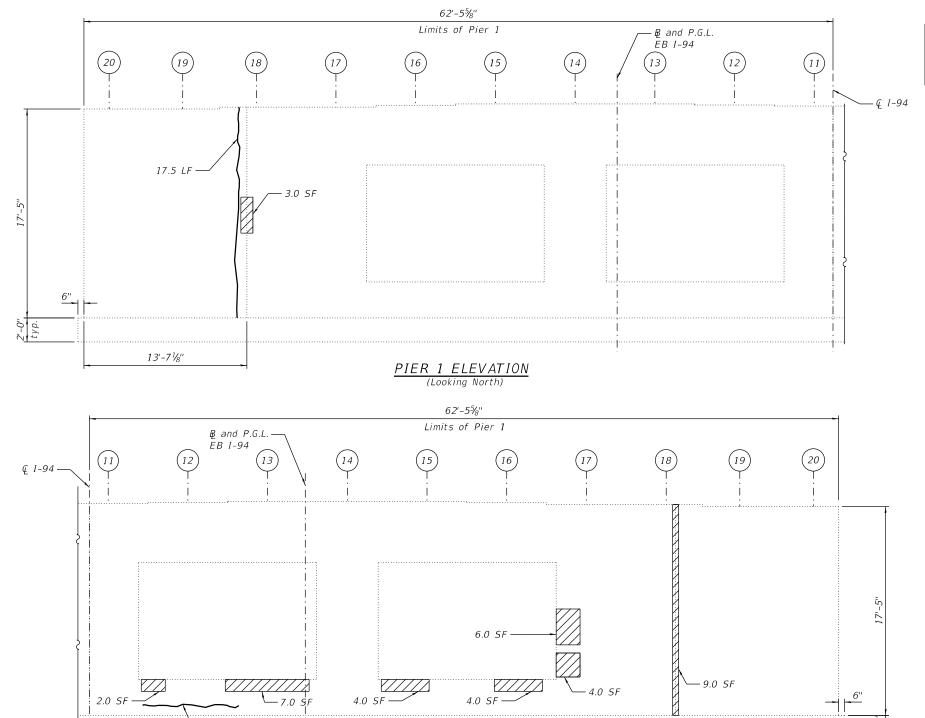
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT REPAIRS STRUCTURE NO. 016-0161 (EB)  
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 629

 CONTRACT NO. 62W87

12/9/2024 3:15:00 PM



PIER 1 ELEVATION (Looking South)

### BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	26
Structural Repair Of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	39

### <u>LEGEND</u>

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

—— Epoxy Crack Injection

Linear Foot

Square Foot

### NOTE:

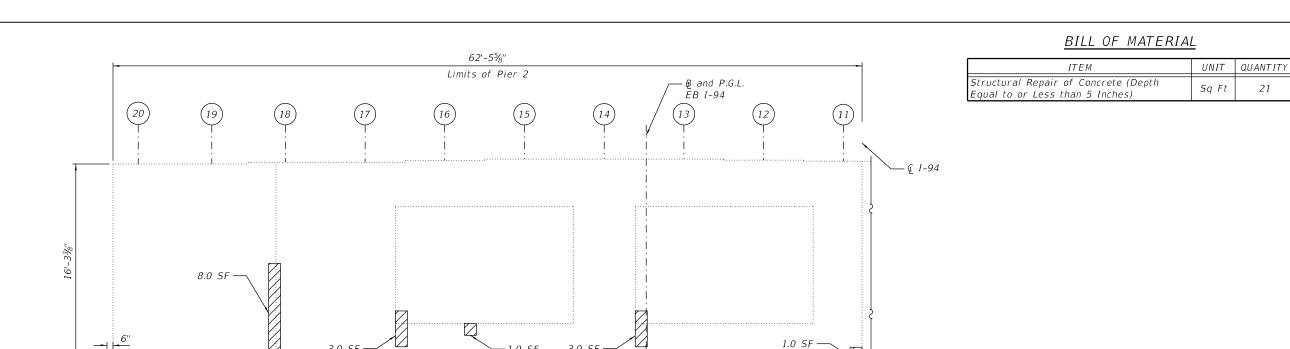
1. Quantities and limits shown are estimated for bidding purpose only. The actual areas to be repaired and the type(s) of repairs to be used will be determined by the engineer in the field at the time of construction.

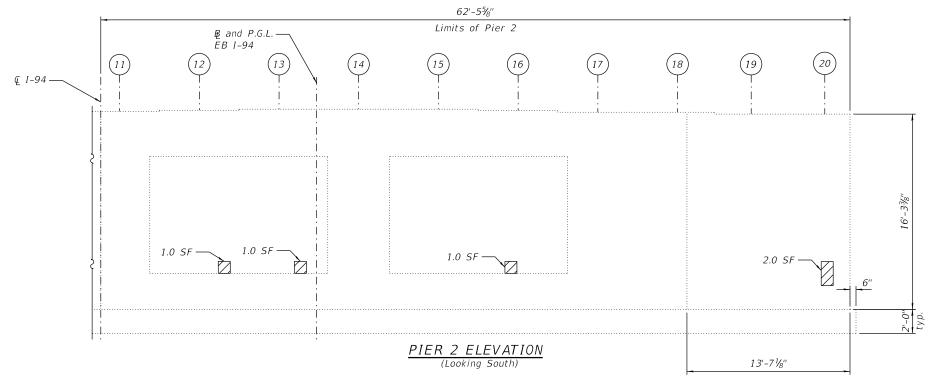


USER NAME =	imranh	DESIGNED -	SUR	REVISED	-
		DRAWN -	ME	REVISED	•
PLOT SCALE =	8:0.0000 ':" / in.	CHECKED -	JL	REVISED	•
PLOT DATE =	12/9/2024	DATE -	12/6/2024	REVISED	-

13'-7½''

F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
94	(42-B-11-1) BR, BJR	24	COOK	761	630
			CONTRACT	NO. 6	2W87
	ILLINOIS	FED. All	D PROJECT		





-1.0 SF

PIER 2 ELEVATION (Looking North)

3.0 SF —

CRACKS ON PIER 2 (NF) (Looking South)

CRACKS ON PIER 2 (SF) (Looking North)

 Quantities and limits shown are estimated for bidding purpose only. The actual areas to be repaired and the type(s) of repairs to be used will be determined by the engineer in the field at the time of construction.

3.0 SF

LEGEND

Structural Repair of Concrete (Depth Equal to or less than 5")

SF Square Foot

Accurate

USER NAME =	imranh	DESIGNED -	SUR	REVISED -
		DRAWN -	ME	REVISED -
PLOT SCALE =	8:0.0000 ':" / in	CHECKED -	JL	REVISED -
PLOT DATE =	12/9/2024	DATE -	12/6/2024	REVISED -

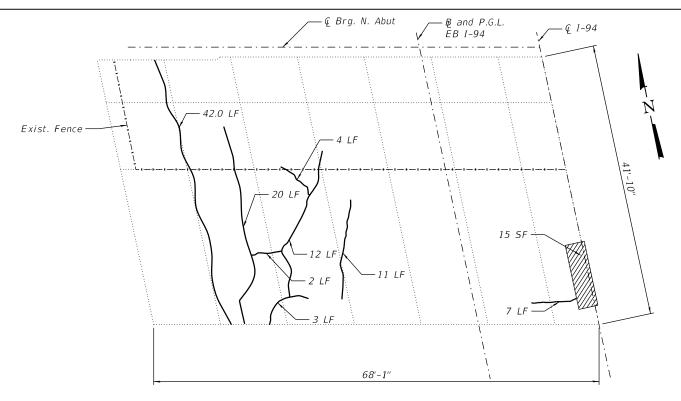
NOTE:

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

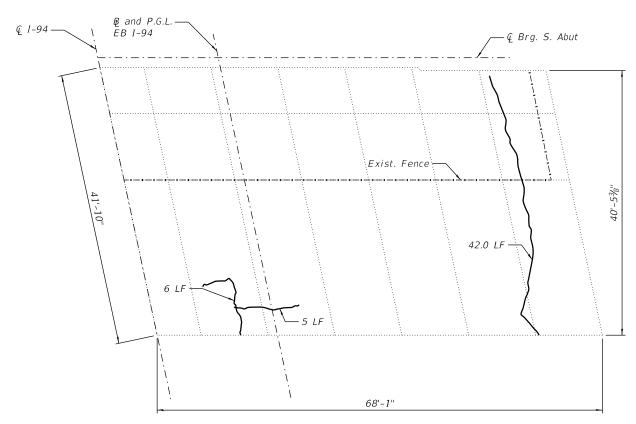
PIER 2 REPA STRUCTURE NO. 016	
SHEET S06-20 OF S06-	-28 SHEETS

I.	SECT	ION		COUNTY	TOTAL SHEETS	
4	(42-B-11-1) E	BR, BJR	24	соок	761	631
				CONTRACT	NO. 6	2W87
		ILLINOIS	FED All	D PROJECT		

21



### NORTH ABUTMENT SLOPE WALL PLAN



### SOUTH ABUTMENT SLOPE WALL PLAN

### NOTES:

- Quantities and limits shown are estimated for bidding purposes only.
   The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the engineer in the field at the time of construction.
- 2. Slope wall shall be reinforced with welded wire fabric, 6 in. x 6 in. W40 x W40 weighing 58 lbs. per 100 sq. ft.

# BILL OF MATERIAL NORTH SLOPE WALL

ITEM	UNIT	QUANTITY
Porous Grandular Embankment	Cu Yd	1
Slope Wall Removal	Sq Yd	2
Slope Wall 4 Inch	Sq Yd	2
Slope Wall Crack Sealing	Foot	101

## BILL OF MATERIAL SOUTH SLOPE WALL

ITEM	UNIT	QUANTITY
Slope Wall Crack Sealing	Foot	53.0

### <u>LEGEND</u>

Slope Wall Removal and Replacement with 4 inch Slope Wall

---- Slope Wall Crack Sealing

LF Linear Foot

SF Square Foot

Accurate GROUP, INC.

USER NAME = imranh	DESIGNED - SUR	REVISED -
	DRAWN - ME	REVISED -
PLOT SCALE = 14:8.0000 ':" / in.	CHECKED - JL	REVISED -
PLOT DATE = 12/9/2024	DATE - 12/6/2024	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SLOPE WALL REPAIRS STRUCTURE NO. 016-0161 (EB) 

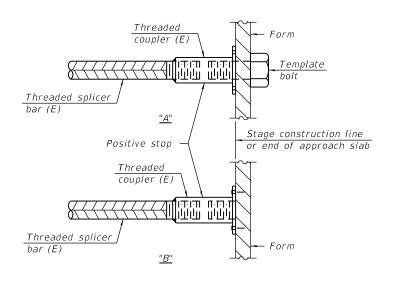
### STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

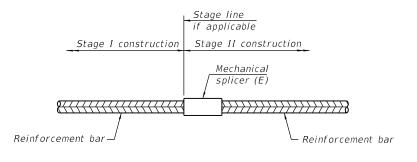
Location	Bar size	No. assemblies required	Minimum lap length	
N. Abut.	#5	10	3'-6"	
N. Abut.	#6	6	4'-10''	
S. Abut.	#5	10	3'-6"	
S. Abut.	#6	6	4'-10''	



### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E) : Indicates epoxy coating.



### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

### NOTES:

- Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.
- 2. All reinforcement shall be lapped and tied to the splicer bars.

  Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.
- 3. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

05-15-2023

Accurate GROUP, INC.

USER NAME	-	imranh	DESIGNED	-	SUR	REVISED	-
			DRAWN	-	ME	REVISED	-
PLOT SCALE	=	0:2.0000 ':" / in.	CHECKED	-	JL	REVISED	-
PLOT DATE	-	12/9/2024	DATE	-	12/6/2024	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY & MECHANICAL SPLICER DETAILS STRUCTURE NO. 016-0161 (EB)

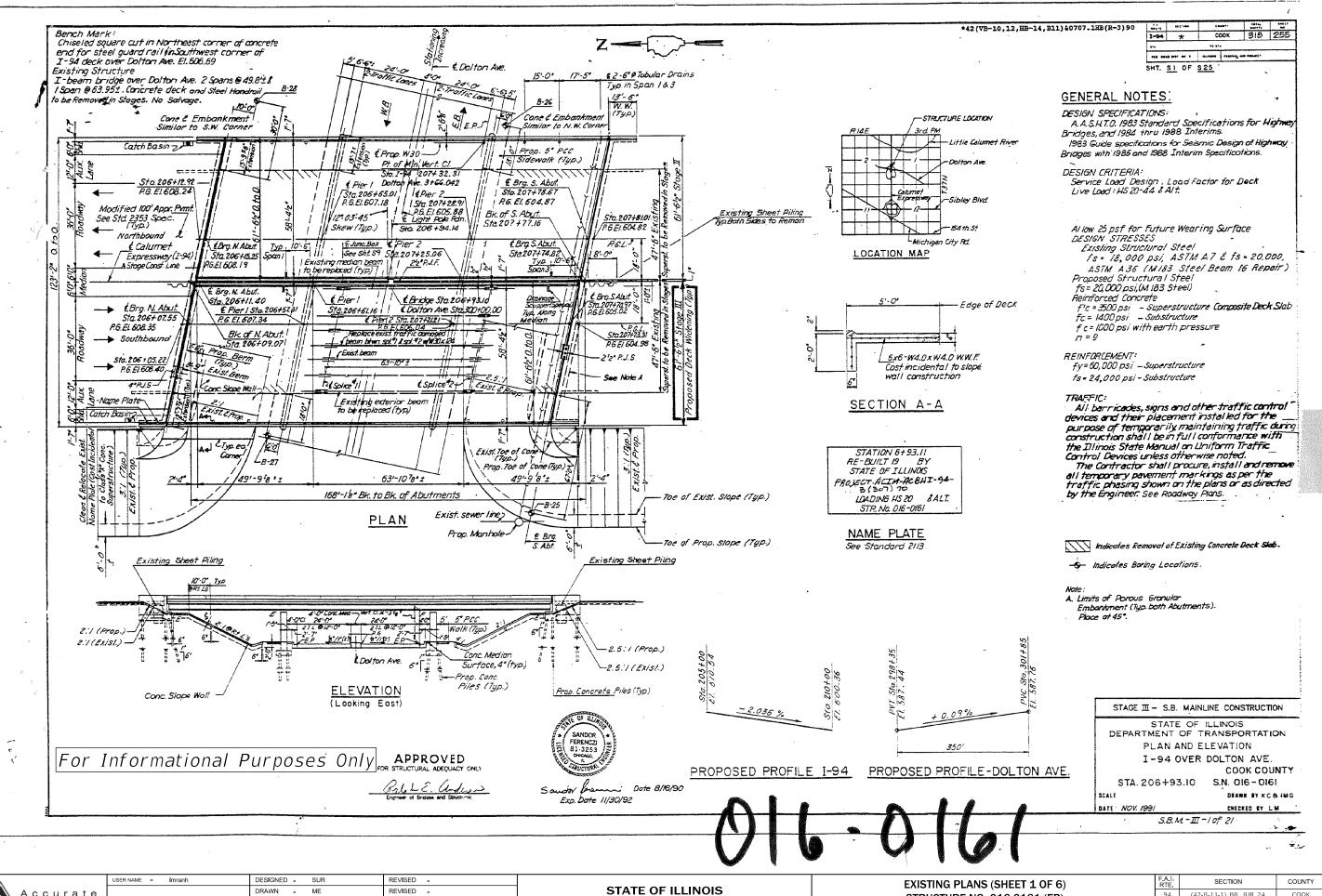
SHEET 506-22 OF 506-28 SHEETS

 
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 633

 CONTRACT NO.
 62W87

12/9/2024 3:15:12 PM



12/9/2024 3:15:15 PM

0:2.0000 ':" / in.

12/9/2024

PLOT DATE =

CHECKED -

- 12/6/2024

DATE

REVISED -

REVISED -

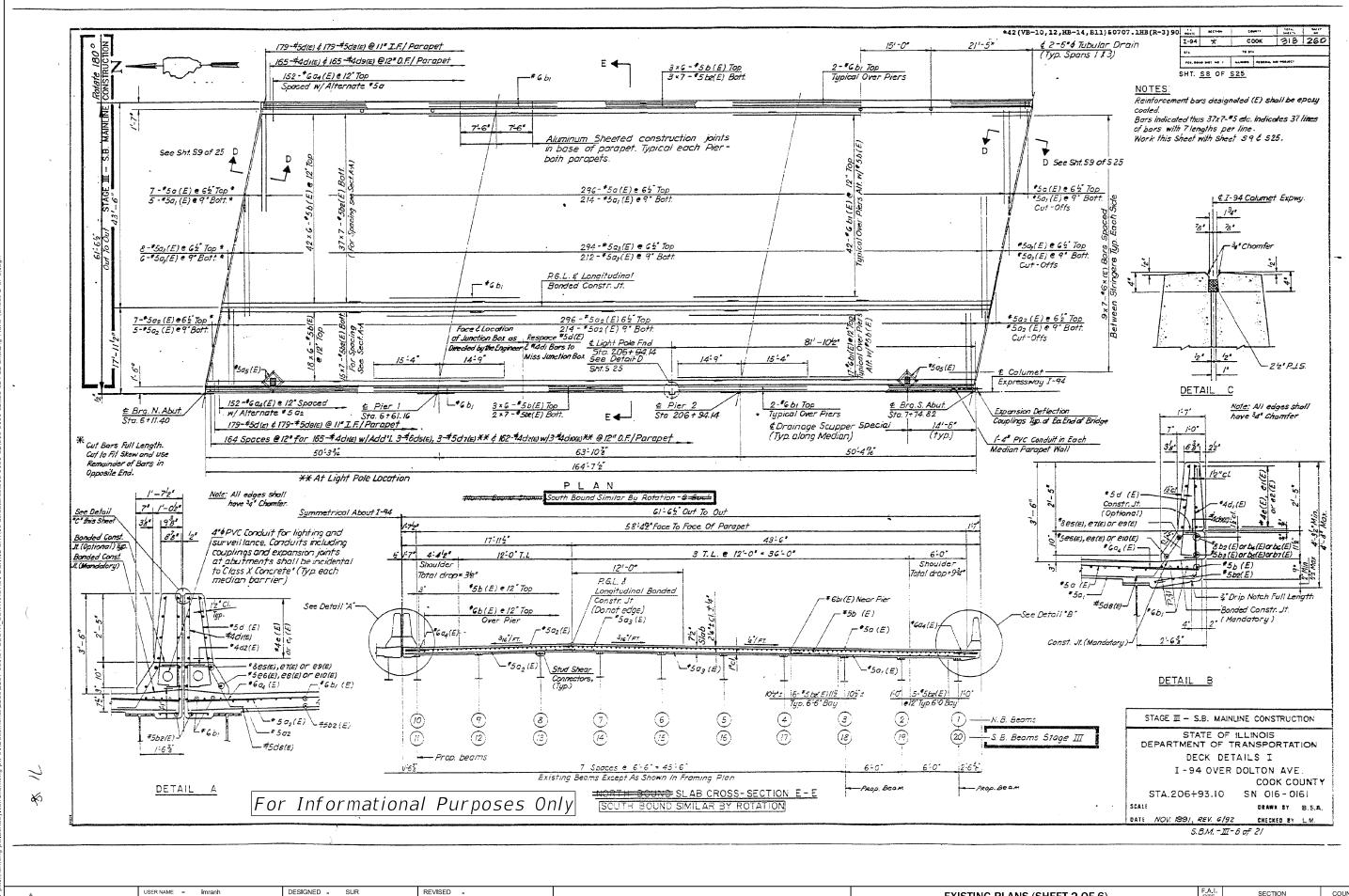
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (SHEET 1 OF 6)
STRUCTURE NO. 016-0161 (EB)
SHEET S06-23 OF S06-28 SHEETS

AL. SECTION COUNTY TOTAL SHEETS NO.

104 (42-B-11-1) BR, BJR 24 COOK 761 634

CONTRACT NO. 62W87



12/9/2024 3:15:24 PM

Accurate

DRAWN - ME

- 12/6/2024

CHECKED -

DATE

0:2.0000 ':" / in.

PLOT DATE = 12/9/2024

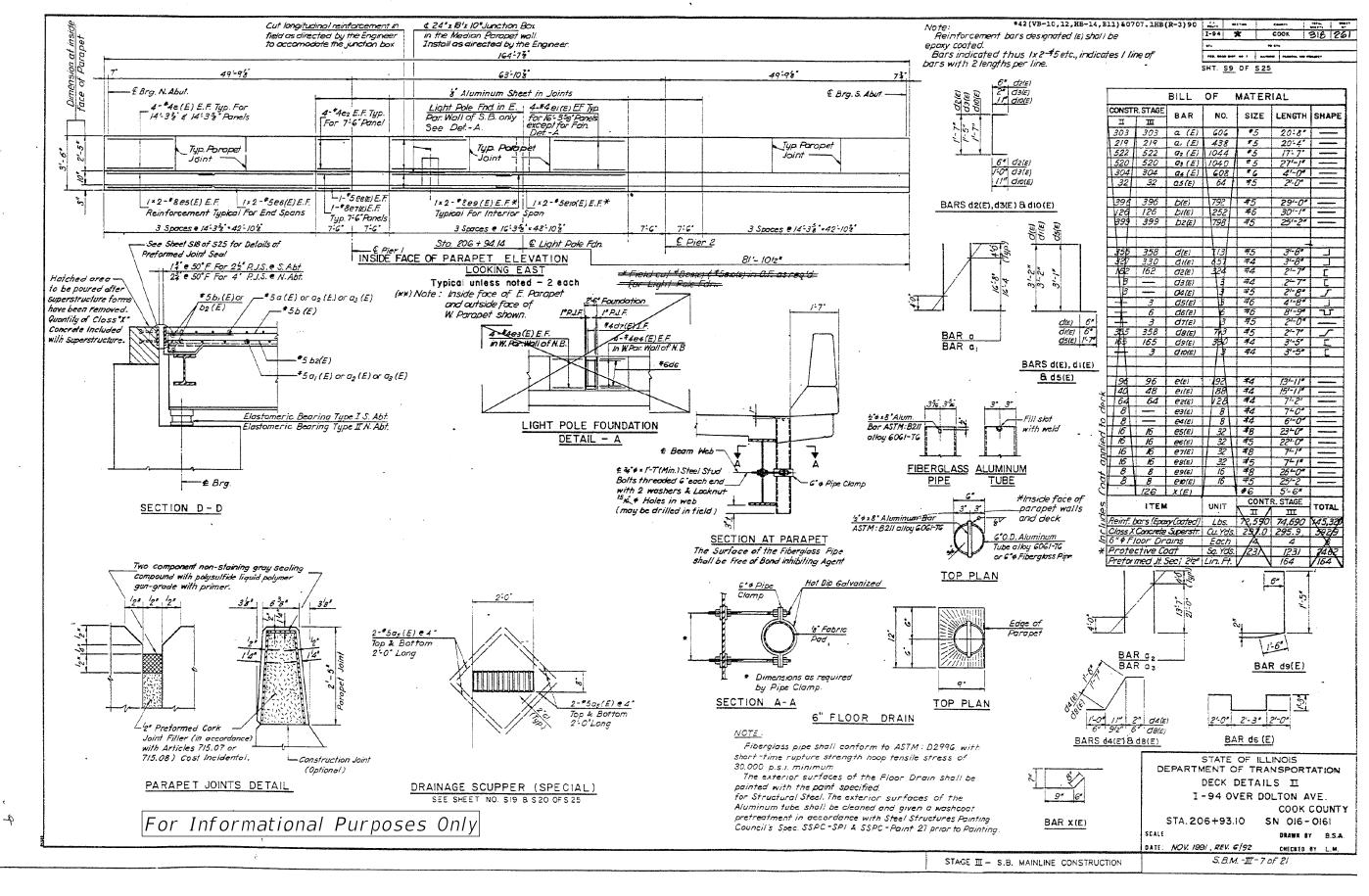
REVISED -

REVISED -

REVISED -

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION**  **EXISTING PLANS (SHEET 2 OF 6)** STRUCTURE NO. 016-0161 (EB) SHEET S06-24 OF S06-28 SHEETS

SECTION COUNTY 94 (42-B-11-1) BR, BJR 24 COOK 761 635 CONTRACT NO. 62W87



Accurate GROUP, INC.

 USER NAME
 =
 imranh
 DESIGNED
 SUR
 REVISED

 PLOT SCALE
 =
 0;2,0000 ':" / in.
 CHECKED
 JL
 REVISED

 PLOT DATE
 =
 12/9/2024
 DATE
 12/6/2024
 REVISED

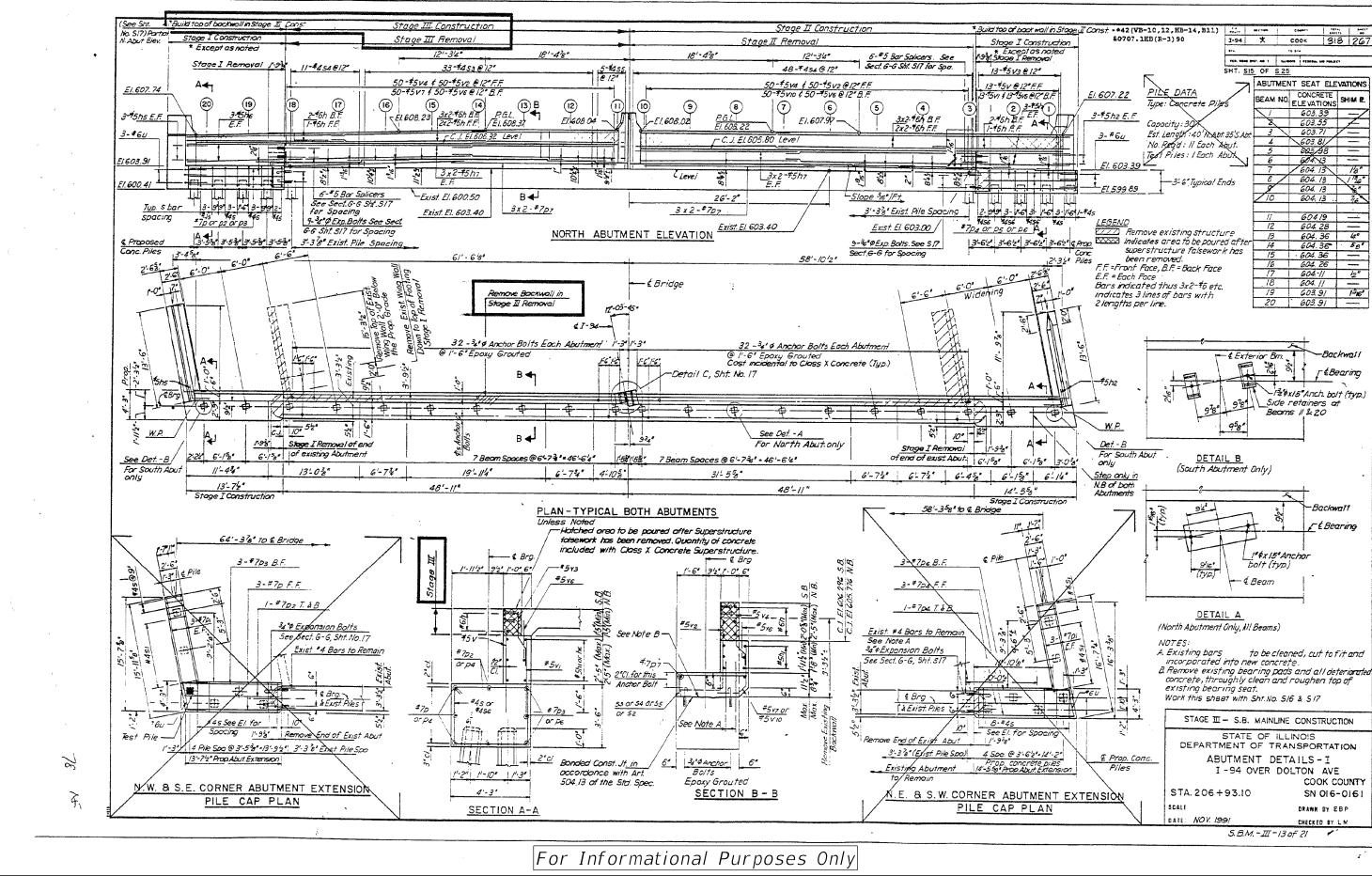
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (SHEET 3 OF 6)
STRUCTURE NO. 016-0161 (EB)

SHEET S06-25 OF S06-28 SHEETS

F.A.I. SECTION COUNTY TOTAL SHEET NO.
94 (42-B-11-1) BR, BJR 24 COOK 761 636

CONTRACT NO. 62W87



Accurate anoup, inc.

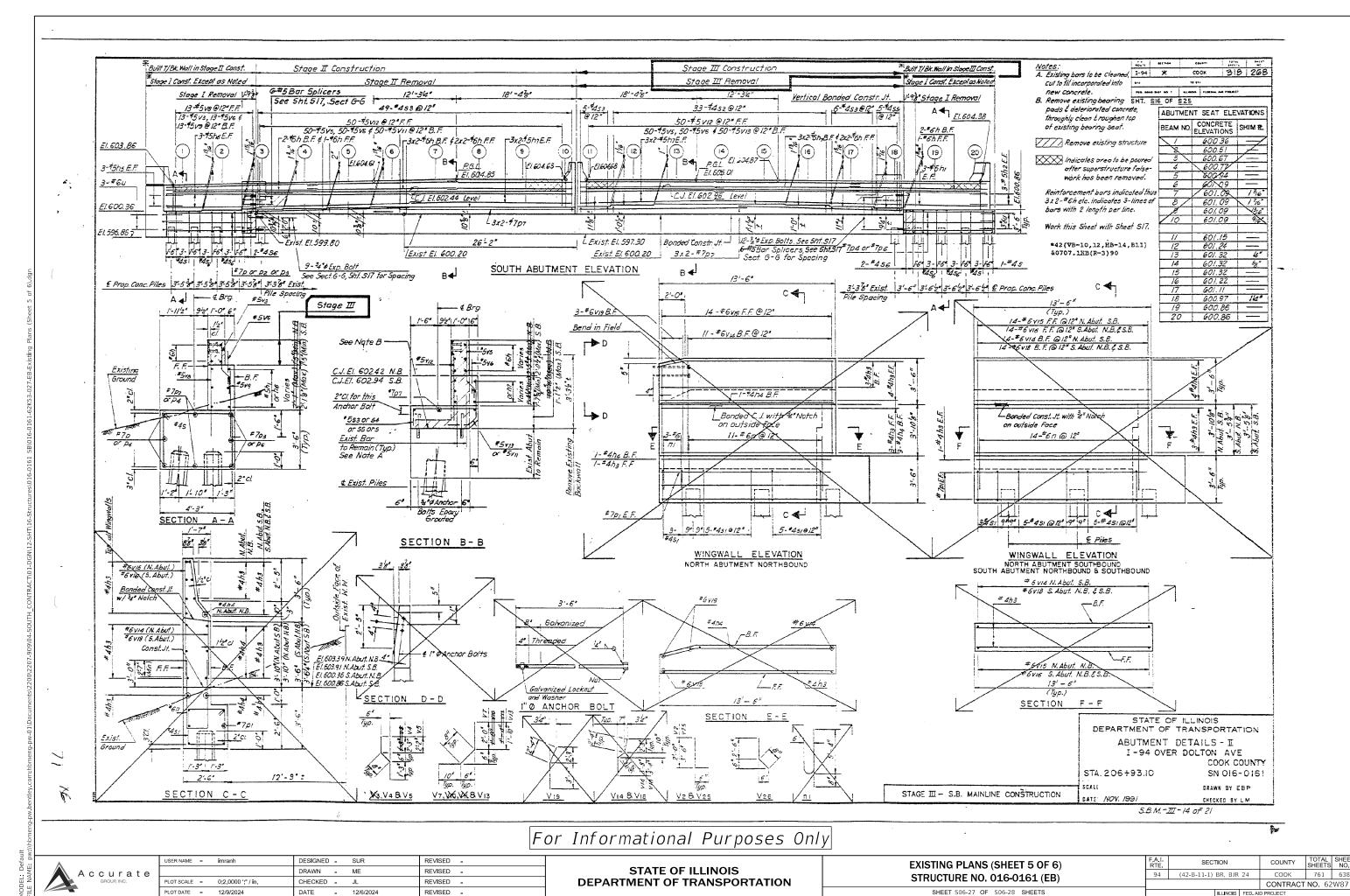
 USER NAME
 =
 imranh
 DESIGNED
 SUR
 REVISED

 DRAWN
 ME
 REVISED

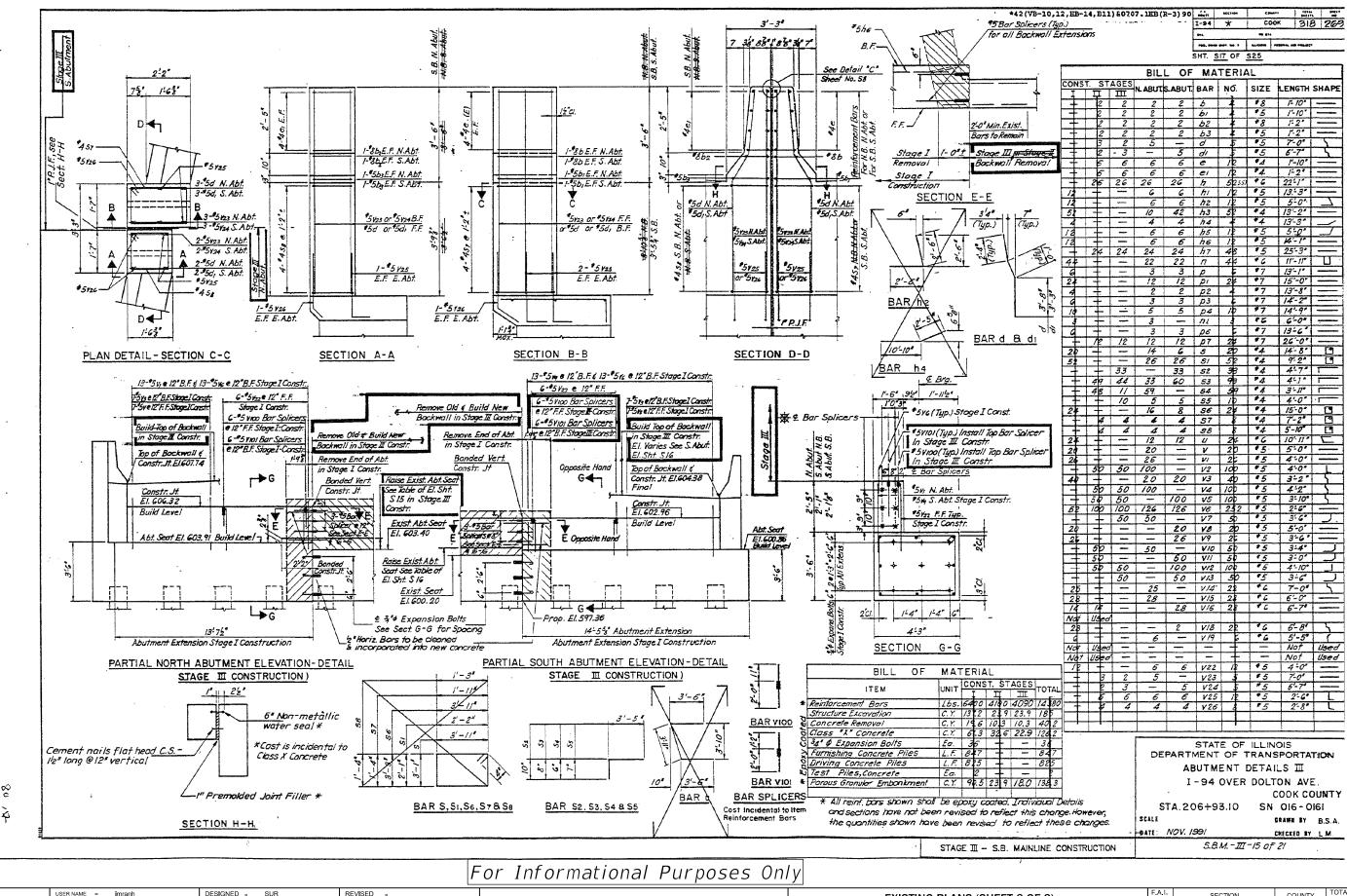
 PLOT SCALE
 =
 0:2.0000 '.\* / in.
 CHECKED
 JL
 REVISED

 PLOT DATE
 =
 12/9/2024
 DATE
 12/6/2024
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (SHEET 4 0F 6) STRUCTURE NO. 016-0161 (EB) 

12/9/2024 3:15:51 PM



MODEL: Default FILE NAME: pw:\\hhme

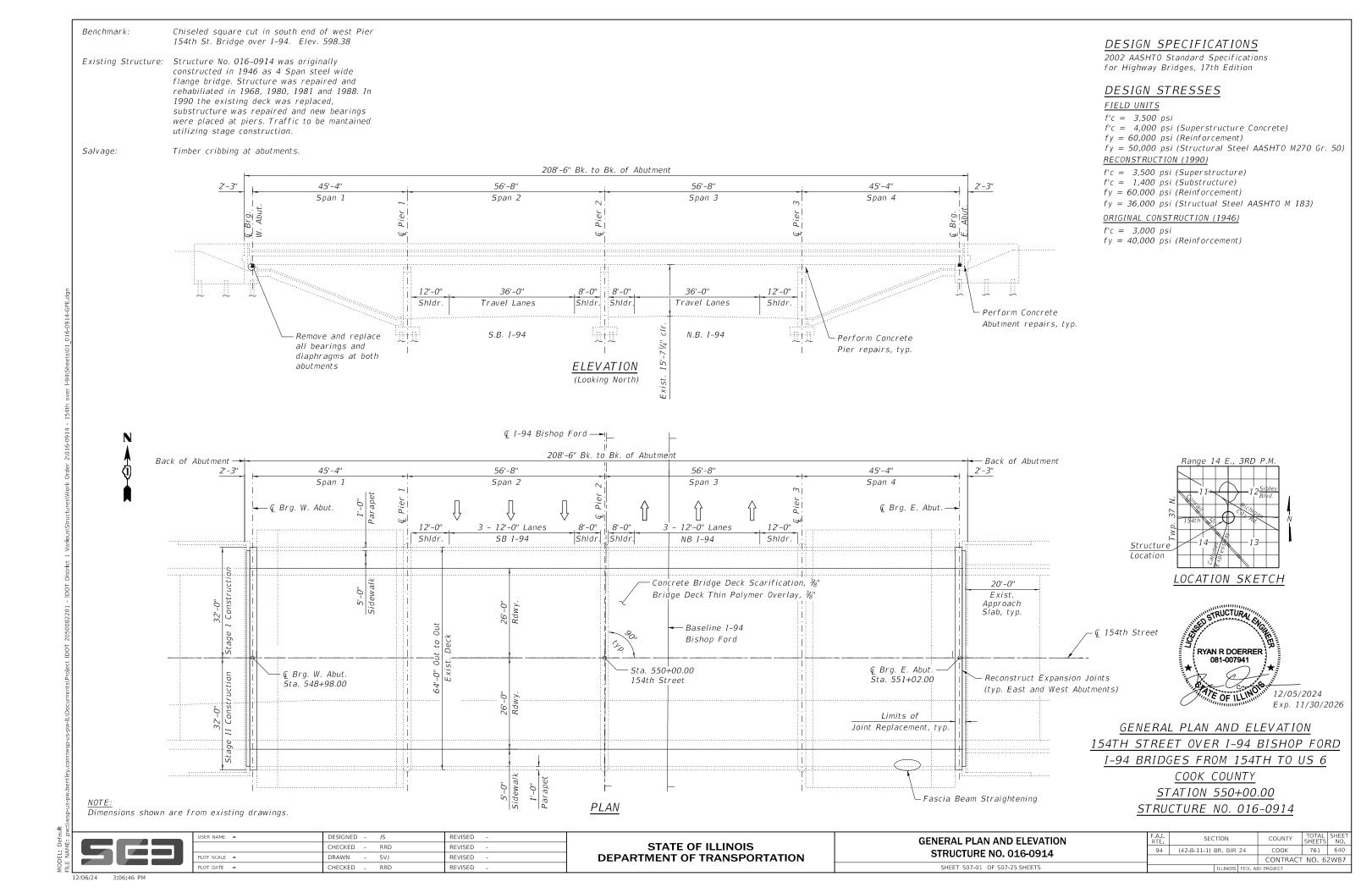
Accurate GROUP, INC.

 USER NAME
 =
 Immanh
 DESIGNED
 SUR
 REVISED

 PLOT SCALE
 =
 0:2.0000 ':" / in.
 CHECKED
 JL
 REVISED

 PLOT DATE
 =
 12/9/2024
 DATE
 12/6/2024
 REVISED

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

EXISTING PLANS (SHEET 6 OF 6) STRUCTURE NO. 016-0161 (EB) 

Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose detrimental foreign material shall be removed from the surfaces in contact with concrete (SSPC-SP3 standards). Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be paid for according to Article 109.04 of the Standard Specifications.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding  $\frac{1}{4}$  inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

Proposed elevations are based on the existing drawing profile and adjusted to account for the proposed overlay.

Fasteners shall be ASTM F 3125 Grade A325 Type 1. Fasteners shall be hot dip galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel." Bolts  $\frac{3}{4}$  in.  $\Phi$  holes  $\frac{13}{16}$  in.  $\Phi$ , unless otherwise noted.

All new structural steel shall be galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel."

Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures."

The existing structural steel coating contains lead. The contractor shall take appropriate precautions to deal with the presence of lead on this

It is the Contractor's responsibility to temporarily support the existing signal interconnect and all other utilities interfering with proposed work, as required. Embedment in the south parapet shall be reestablished. Cost included in Concrete Superstructure.

The contractor shall exercise caution during removal and construction operations to avoid damaging the existing signal interconnect and all other utilities. Any damage to the signal interconnect or other utilities caused by the Contractor in the performance of their work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.

All exposed concrete edges shall have a  $\frac{3}{4}$ " x 45 degree chamfer except where shown otherwise.

The contractor shall salvage the wood cribbing supporting some of the abutment beam ends and diaphragms. The wood cribbing shall be transported, unloaded, and stacked by the Contractor to the District Bridge Yard in Elk Grove at 1101 Biesterfield Road during the week days of Monday-Friday, and between the hours of 8am and 2pm. The Contractor shall notify the District Bridge Office 48 hours in advance of the delivery at (847) 956-1443. Cost included in Structural Steel Removal.

Joint openings shall be adjusted according with Article 520.04 of the Standard Specs. when the joint concrete is poured at an ambient temperature other than 50° F.

Expansion joint shall be fabricated to conform to the existing cross-slopes

Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of  $\frac{1}{8}$  in. (0.01 ft.) Adjustment shall be made either by grinding the surface or by shimming the bearings.

Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost included to "Concrete Removal".

Cleaning and field painting of the structural steel shall be done under a separate painting contract.

Cost of removal and re-installation of all members necessary to complete the work as detailed on the plans and as specified in the Special Provisions shall be included with Furnishing and Erecting Structural Steel or Structural Steel Repairs.

The Engineer shall show actual locations and size of deck repairs on As-built Plans.

### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	CU YD	0	2	2
Concrete Removal	CU YD	17.0	0	17.0
Slope Wall Removal	SQ YD	0	3	3
Protective Shield	SQ YD	758	0	758
Concrete Structures	CU YD	0.0	1	1.0
Concrete Superstructure	CU YD	17.2	0	17.2
Protective Coat	SQ YD	1,443	0	1,443
Furnishing And Erecting Structural Steel	POUND	5,830	0	5,830
Reinforcement Bars, Epoxy Coated	POUND	2,630	0	2,630
Bar Splicers	EACH	26	0	26
Slope Wall 4 Inch	SQ YD	0	3	3
Preformed Joint Strip Seal	FOOT	124	0	124
Elastomeric Bearing Assembly, Type II	EACH	20	0	20
Anchor Bolts, 1"	EACH	80	0	80
Epoxy Crack Injection	FOOT	0	259	259
Jack and Remove Existing Bearings	EACH	0	20	20
Structural Steel Removal	POUND	5,430	0	5,430
Structural Steel Repair	POUND	10,650	0	10,650
Cleaning Drainage System	L SUM	0.34	0	0.34
Concrete Bridge Deck Scarification 3/8"	SQ YD	1,430	0	1,430
Bridge Deck Thin Polymer Overlay, 3/8"	SQ YD	1,430	0	1,430
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	SQ FT	0	679	679
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	SQ FT	0	46	46
Deck Slab Repair (Full Depth, Type I)	SQ YD	1	0	1
Deck Slab Repair (Partial)	SQ YD	4	0	4
Beam Straightening	L SUM	0.34	0	0.34
Temporary Shoring and Cribbing	EACH	0	23	23

### SCOPE OF WORK

1.	Remove	and	replace	the	abutment	bearings	with
	elastomeric be			5.			

2. Remove and replace diaphragms at both abutments. Perform Structural Steel Beam End and Midspan

Repairs at the locations shown in the drawings. 4. Straighten impact damage on Beam 1 in Span 4.

5. Perform Structural Repair of Concrete and Epoxy Crack Injection to the abutments and Piers as shown in the drawings.

6. Perform %" Bridge Deck Scarification.

7. Perform Deck slab repairs as required.

Remove and replace deck expansion joints at the North and South Abutments. Install new Preformed Joint Strip Seals

9. Apply a 3/4" thin Polymer Deck Overlay on Bridge Deck.

10. Apply Protective Coat.

### INDEX OF SHEETS

S07-01 General Plan and Elevation 507-02 General Data S07-03 Removal and Construction Staging (1 of 2) *S07-04* Removal and Construction Staging (2 of 2)

507-05 Temporary Concrete Barrier 507-06 Bridge Deck Repair Plan and Details

S07-07 East and West Abutment Expansion Joint Removal and Replacement Plan

S07-08 East and West Abutment Expansion Joint Removal and Replacement Details

507-09 Preformed Joint Strip Seal (1 of 3) 507-10 Preformed Joint Strip Seal (2 of 3)

S07-11 Preformed Joint Strip Seal (3 of 3)

S07-12 Framing Plan S07-13

Diaphragm Replacement Details S07-14 Beam End Plating Details

507-15 Beam Mid-Span Repair Details 507-16

Bearing Details 507-17 East Abutment Repairs

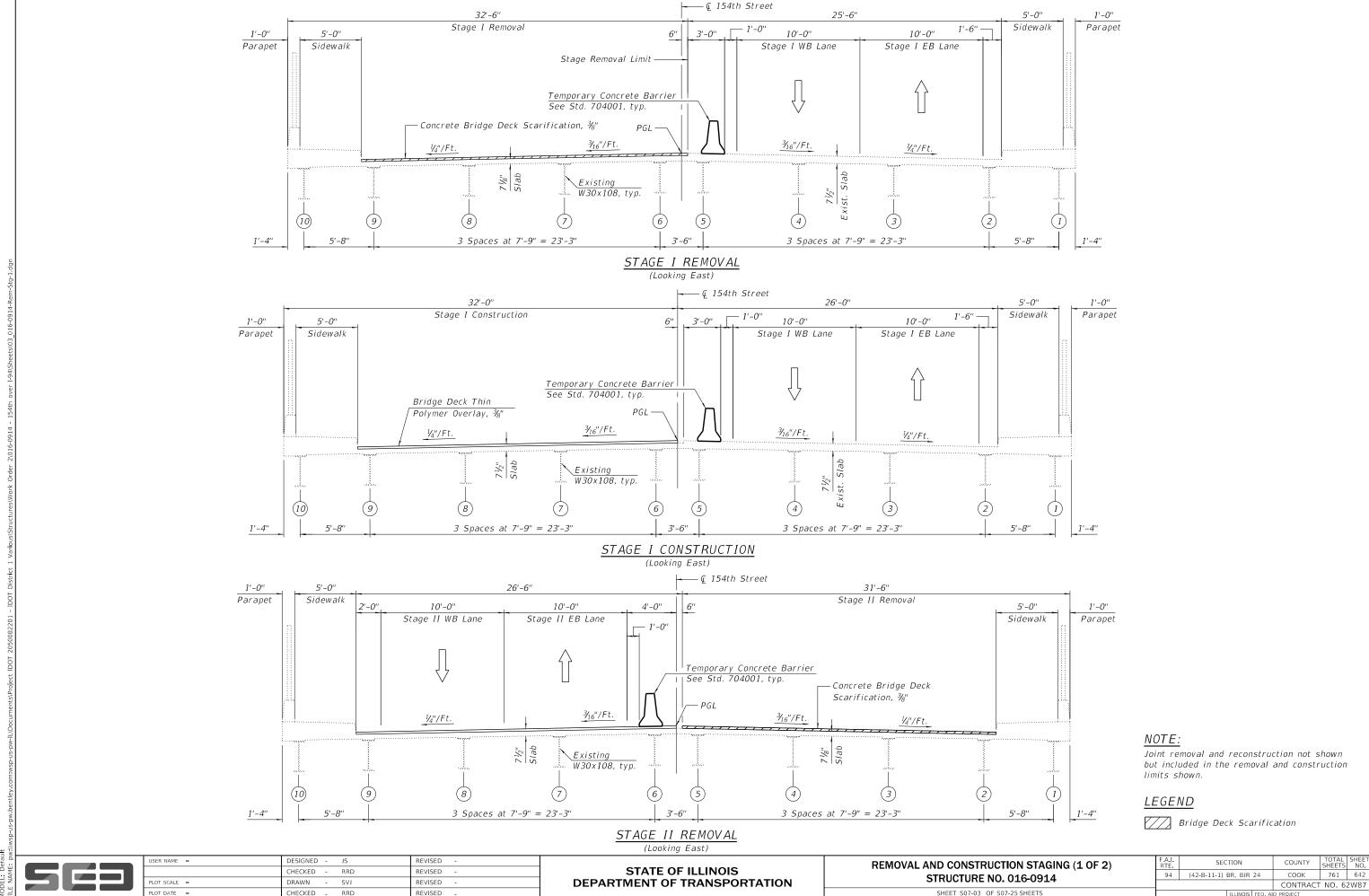
507-18 West Abutment Repairs S07-19 Pier 1 Repairs

507-20 Pier 2 Repairs S07-21 Pier 3 Repairs

Beam Straightening Details

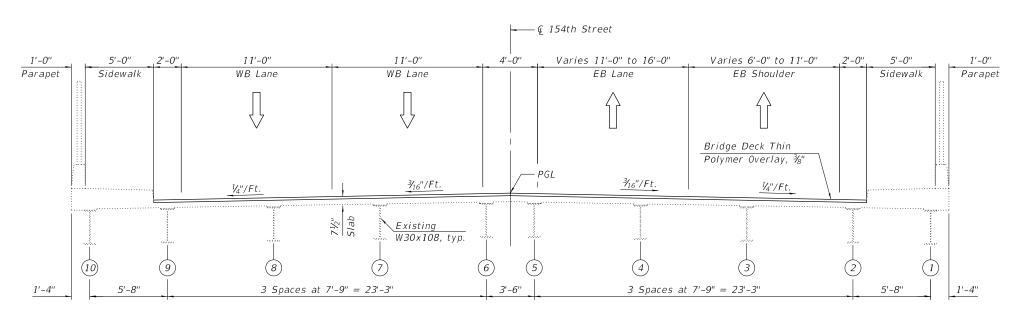
507-23 Slope Wall Details S07-24 Bar Splicer Assembly Details

Existing General Plan and Elevation



12/06/24 3:07:00 F

# $\frac{\textit{STAGE II CONSTRUCTION}}{\textit{(Looking East)}}$



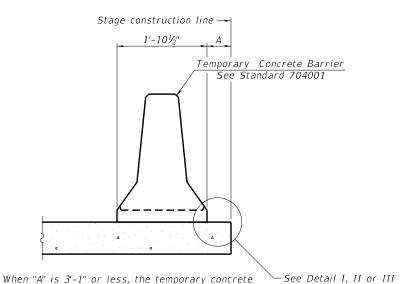
# FINAL DECK CROSS SECTION (Looking East)

### NOTE:

Joint removal and reconstruction not shown but included in the removal and construction limits shown.

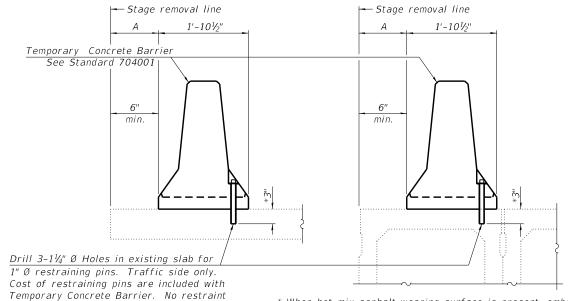
USER NAME =	DESIGNED - JS	REVISED -
	CHECKED - RRD	REVISED -
PLOT SCALE =	DRAWN - SVJ	REVISED -
PLOT DATE =	CHECKED - RRD	REVISED -

F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
94	(42-B-11-1) BR, BJR	24	соок	761	643
			CONTRACT	NO. 6	2W87
		550 H	D. DDOJECT		



barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

### NEW SLAB OR NEW DECK BEAM



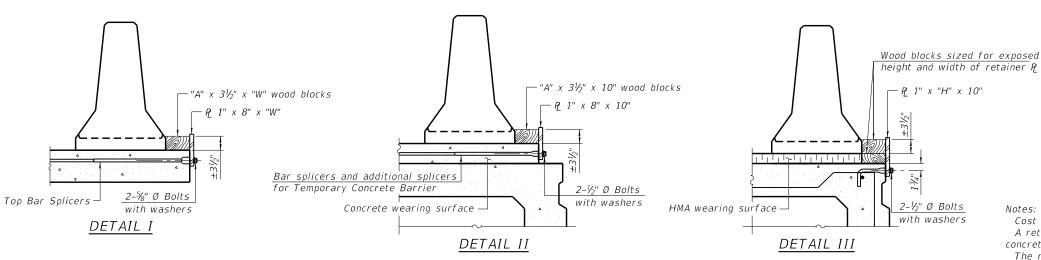
\* When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

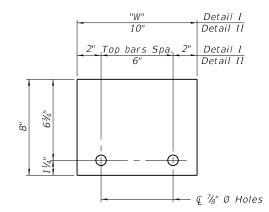
### EXISTING DECK BEAM

### SECTIONS THRU SLAB OR DECK BEAM

is required when "A" is greater than 3'-1".

EXISTING SLAB





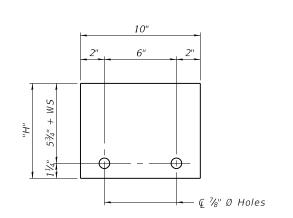
# RAILING CRITERIA

NCHRP 350 Test Level Railing Weight (plf)

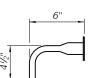
5-15-2023

STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)



STEEL RETAINER P 1" x "H" x 10" (Detail III)



RESTRAINING PIN

### BAR SPLICER FOR #4 BAR - DETAIL III

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate Q of each temporary concrete barrier.

1x8 UNC

US Std. 11/16" I.D. x 21/2" O.D. x approx. 8 gauge thick washer

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

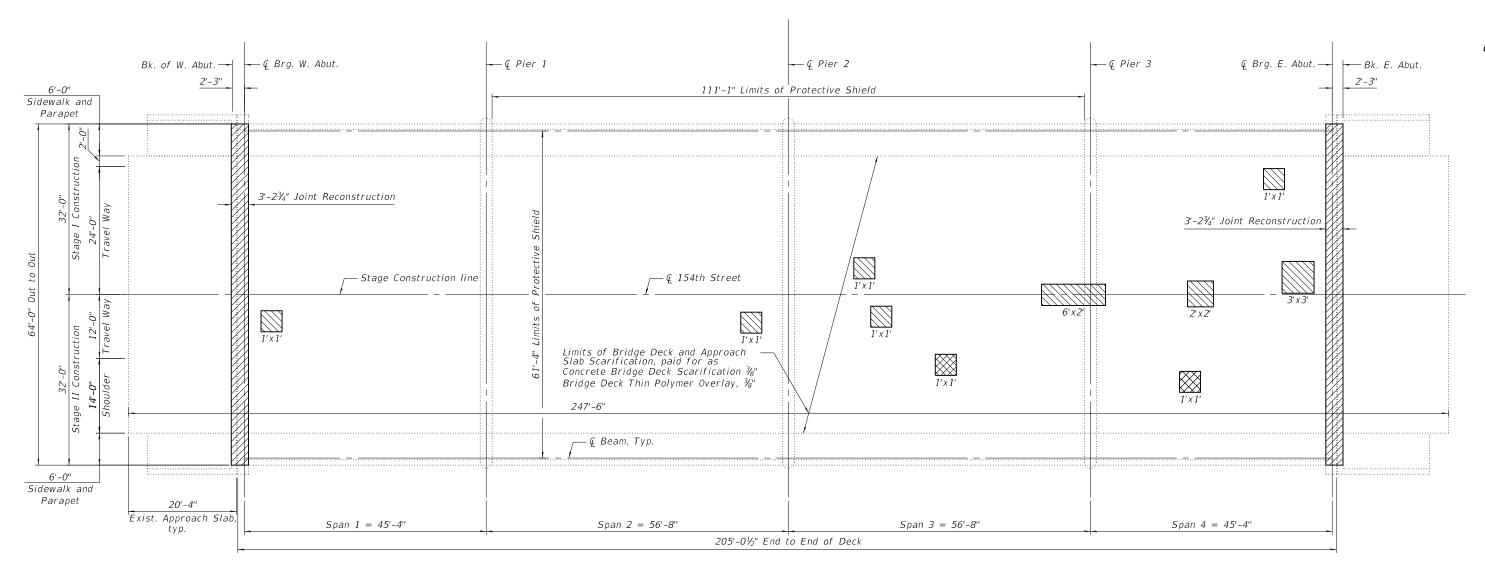
When the 'A' dimension is less than  $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

USER NAME =	DESIGNED -	JS	REVISED -
	CHECKED -	RRD	REVISED -
PLOT SCALE =	DRAWN -	SVJ	REVISED -
PLOT DATE =	CHECKED -	RRD	REVISED -

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION**  TEMPORARY CONCRETE BARRIER **STRUCTURE NO. 016-0914** SHEET S07-05 OF S07-25 SHEETS

А.І. ГЕ.	SEC	TION		COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1)	BR, BJR	24	COOK	761	644
				CONTRACT	NO. 6	2W87



### DECK PLAN

### *NOTES:*

- 1. Areas of deck repair shown are estimated. The Engineer shall determine actual locations of deck repairs at the time of construction.
- 2. For West and East Abutment Expansion Joint Removal and Reconstruction, see Sheets S07-07 thru S07-08.
- 3. Protective coat shall be applied to top and inside face of reconstructed parapets, reconstructed sidewalks, reconstructed transverse joint areas and the surface of the new overlay. Protective coat need not be applied to existing exposed concrete surfaces.
- 4. Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.
- 5. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms. Any damage to the existing steel beams and/or diaphragms caused by the Contractor in the performance of their work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 6. Existing bridge fencing is to be protected during construction. Post location in the removal area for the joint replacement shall be temporally shored during the concrete removal and replacement. Existing concrete anchor shall be replaced in kind and installed into the proposed parapet. Cost included in Concrete Removal.

- 7. Removal and disposal of the existing expansion joints will not be paid for separately, but are included in the cost of Concrete Removal.
- 8. The Contractor must exercise extreme care with the existing conduits in the sections of the parapets to be removed and to protect and support the conduit during construction. The Contractor will be required to repair any damage done to the conduit to the satisfaction of the Engineer. No splicing will be allowed to any cable damaged resulting from this work. The Contractor will be required to repair the entire span of any damaged cable at no additional cost to the Department.
- 9. If the existing name plate falls within the limits of Concrete Removal, it shall be removed and reinstalled in its original locations in accordance with IDOT Standard 515001. Cost included with Concrete Superstructure.
- 10. Adjust Deck Slab Repairs (Full Depth, Type I) limits as required to meet field conditions.

## LEGEND:



Deck Slab Repair (Full Depth, Type I)

Concrete Removal

# BILL OF MATERIAL

ITFM	UNIT	QUANTITY
11 LM	UNII	QUANTITI
Protective Coat	Sq Yd	1,443
Bridge Deck Thin Polymer Overlay, %"	Sq Yd	1,430
Concrete Bridge Deck Scarification ¾"	Sq Yd	1,430
Protective Shield	Sq Yd	758
Jack and Remove Existing Bearings	Each	20
Deck Slab Repair (Full Depth, Type I)	Sq Yd	1
Deck Slab Repair (Partial)	Sq Yd	4

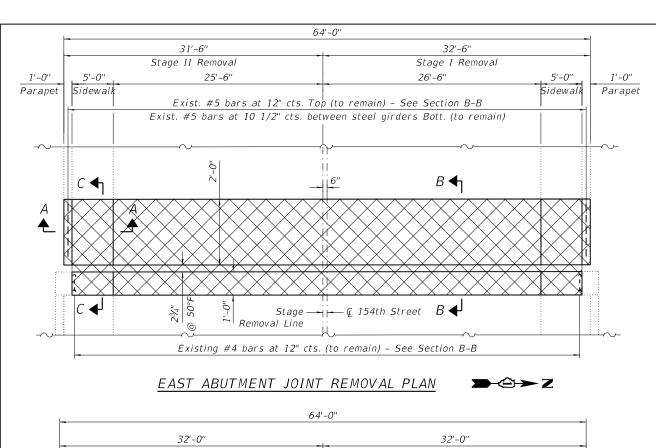


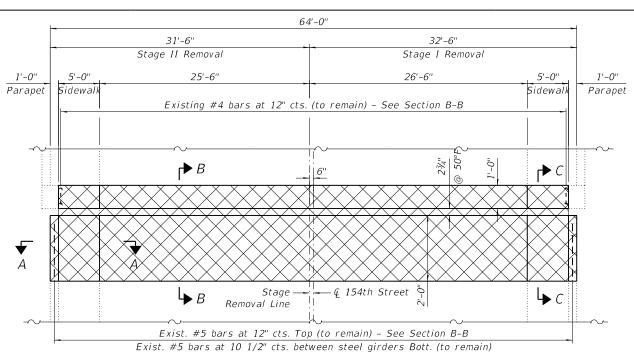
USER NAME =	DESIGNED - JS	REVISED -
	CHECKED - RRD	REVISED -
PLOT SCALE =	DRAWN - SVJ	REVISED -
PLOT DATE =	CHECKED - RRD	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BRIDGE DECK REPAIR PLAN AND DETAILS
STRUCTURE NO. 016-0914
SHEET S07-06 OF S07-25 SHEETS

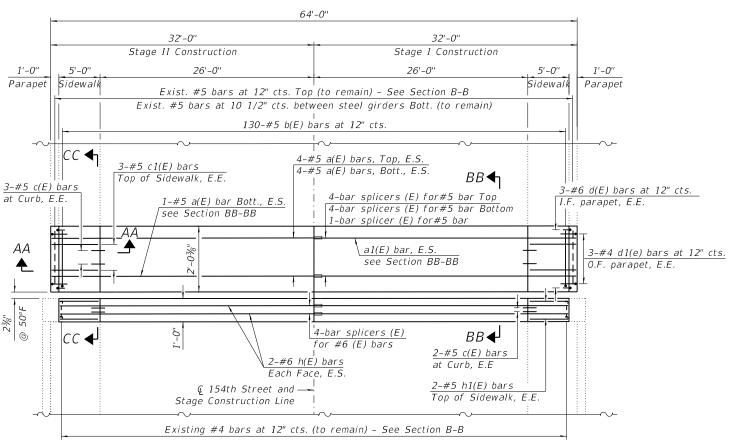
AI. SECTION COUNTY TOTAL SHEETS NO.
94 (42-B-11-1) BR, BJR 24 COOK 761 645
CONTRACT NO. 62W87

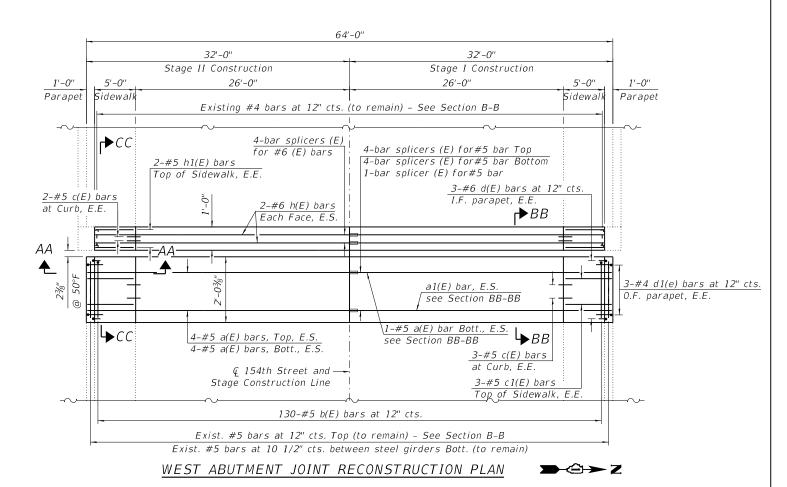




WEST ABUTMENT JOINT REMOVAL PLAN







# EAST ABUTMENT JOINT RECONSTRUCTION PLAN → △ → Z

# NOTES:

1. For Preformed Joint Strip Seal details, see Sheet S07-09 thru S07-11.

2. For Bar Splicer Assembly details, see Sheet S07-24.

- 3. Existing reinforcement bars extending into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system. Cost included with Concrete Removal.
- 4. See Sheet SO-08 for Sections and Bill of Material.

5E3

0.F. = Outside Face

I.F. = Inside Face E.E. = Each End

Concrete Removal

E.S. = Each Side (of stage construction line)

USER NAME =	DESIGNED - JS	R	REVISED	-
	CHECKED - RRD	R	REVISED	-
PLOT SCALE =	DRAWN - SVJ	R	REVISED	-
PLOT DATE =	CHECKED - RRD	R	REVISED	-

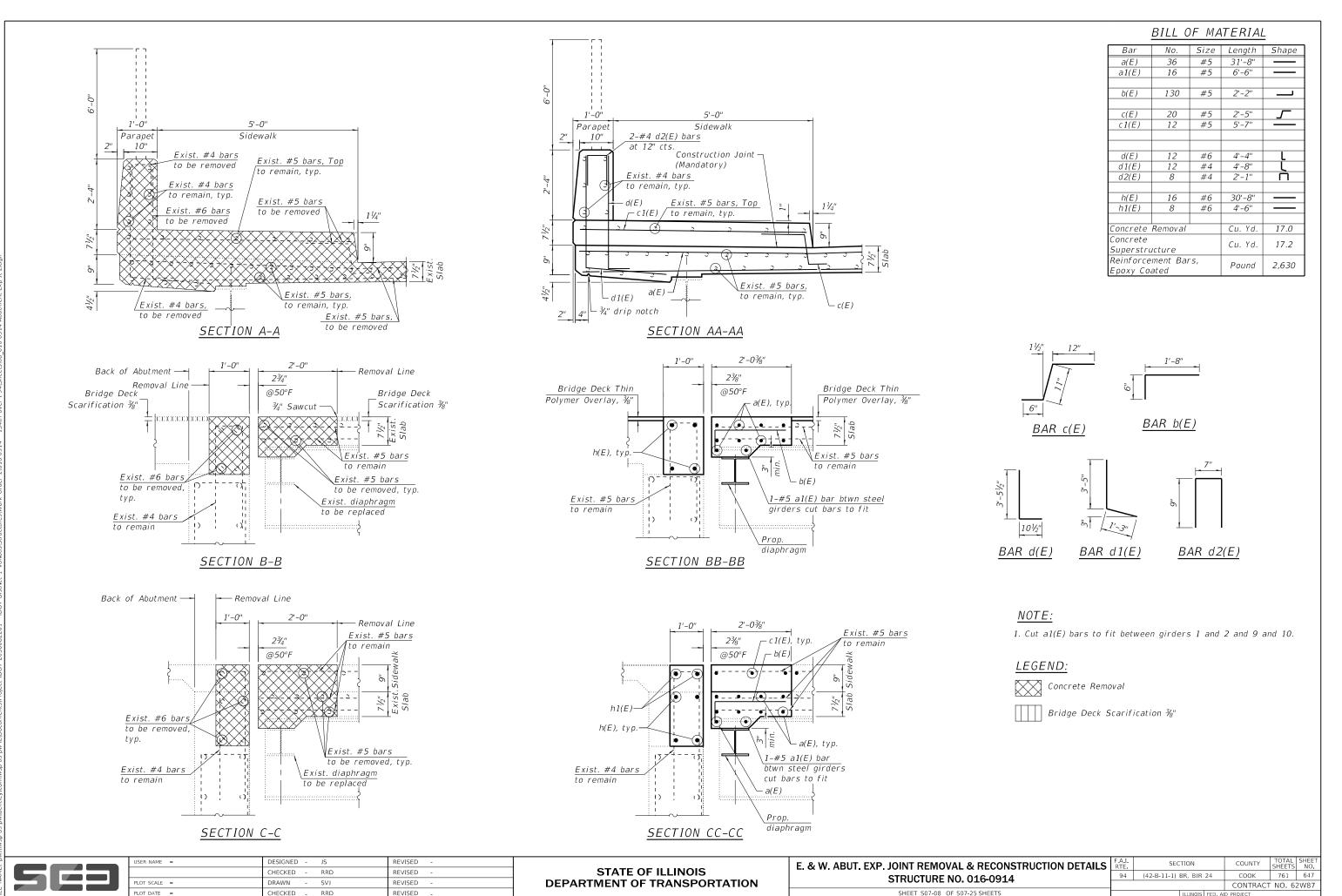
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

E. & W. ABUT. EXP. JOINT REMOVAL & RECONSTRUCTION PLAN	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-0914		(42-B-11-1) BR, BJR 24	COOK	761	646
			CONTRAC	F NO. 6	2W87
SHEET S07-07 OF S07-25 SHEETS		ILLINOIS FED. AI	D PROJECT		

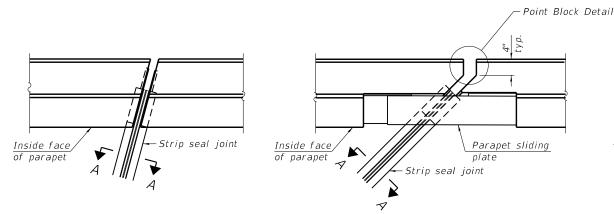
FILE NAME: p

12/06/24 3:07:23 PM

LEGEND:



CHECKED -



FOR SKEWS ≤ 30° FOR SKEWS > 30°

¾" Embedded plate,

full depth

### SECTION B-B

1'-0"

Min. lap

¾" Ø Countersunk bolts

(10 per side 39" parapet) (12 per side 44" parapet)

1/2" Parapet sliding plate

Notes: (8 per side 39" parapet) (10 per side 44" parapet) a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches. applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from

\* ¾" Ø x 6" Studs

° 🖵 ¾" Embedded plate

full depth

1'-0"

Concrete flush with back

face of ¾" plate

<u>Direction</u> of traffic

manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

The strip seal shall be made continuous and shall have

The locking edge rails depicted are configured for typical

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

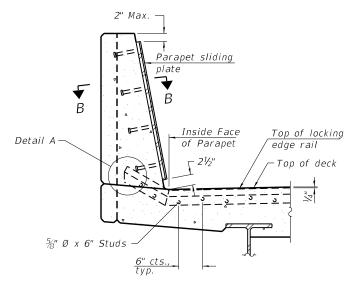
The top surface of sidewalk sliding plates shall have a raised pattern according to ASTM A786.

Cost of parapet sliding plates, sidewalk sliding plates, embedded plates, anchorage studs, and expansion anchors included with Preformed Joint Strip Seal.

39" constant slope barrier shown, 44" constant slope barrier similar as noted.

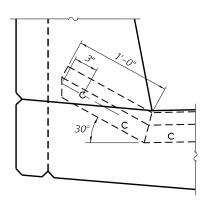
The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

### PLAN AT PARAPET



### SECTION AT PARAPET

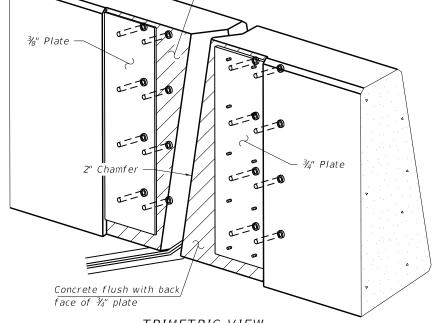
(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)



### DETAIL A

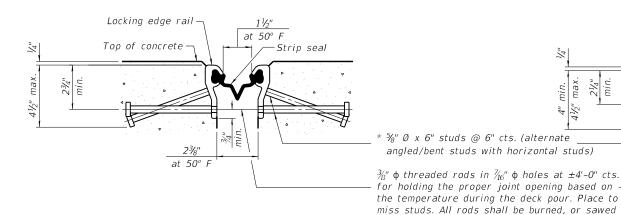
off flush with the plates after concrete is set.

SECTION A-A \* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

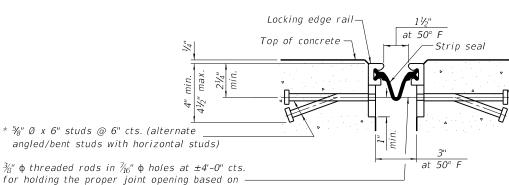


# TRIMETRIC VIEW

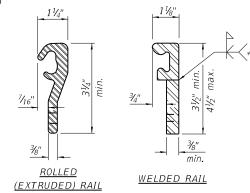
(Showing embedded plates only)



SHOWING ROLLED RAIL JOINT

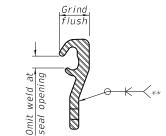


SHOWING WELDED RAIL JOINT



### LOCKING EDGE RAILS

\*\* Back gouge not required if complete joint penetration is verified by mock-up.



### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	124

### EJ-SS-S 11-1-2022



USER NAME =	DESIGNED -	JS	REVISED -
	CHECKED -	RRD	REVISED -
PLOT SCALE =	DRAWN -	SVJ	REVISED -
PLOT DATE =	CHECKED -	RRD	REVISED -

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION** 

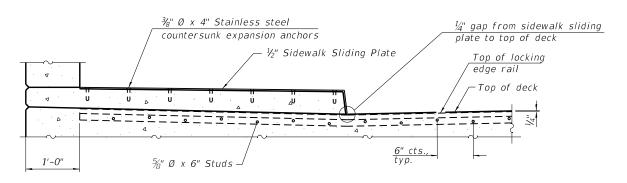
# PREFORMED JOINT STRIP SEAL - SIDEWALK (1 OF 3) **STRUCTURE NO. 016-0914**

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
94	(42-B-11-1) BR, BJR	соок	761	648	
CONTRACT NO. 62W8					
	ILLINOIS	FED AL	D PROJECT		

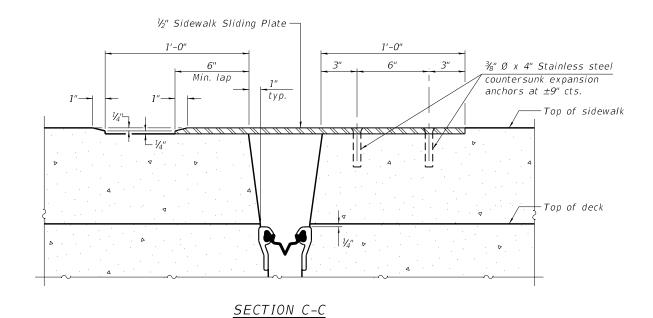
3:07:35 PM

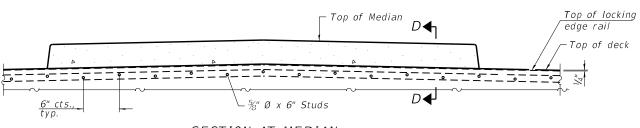
(Sheet 1 of 3)

SHEET S07-09 OF S07-25 SHEETS



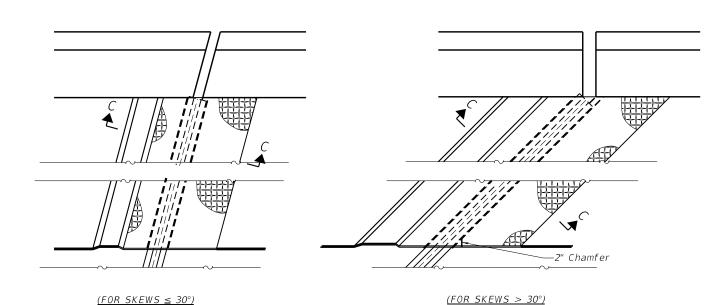
### SECTION AT RAISED SIDEWALK



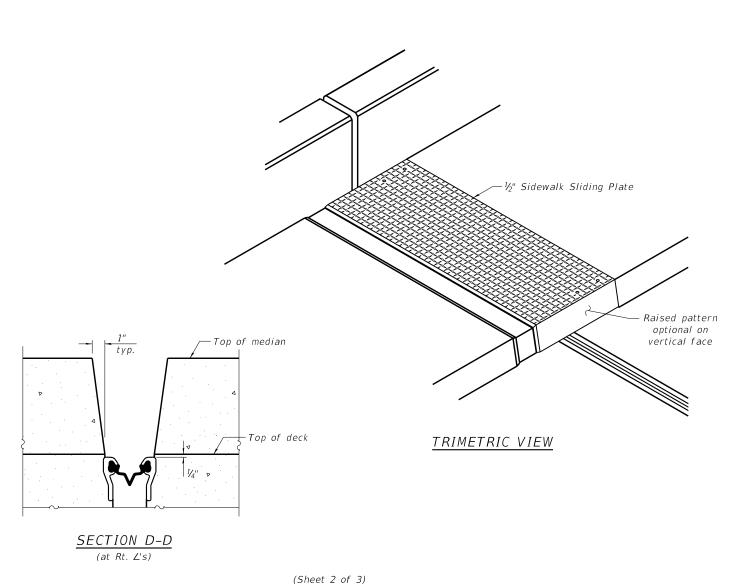


## SECTION AT MEDIAN

For skews > 30°, chamfer acute corners 2" similar to sidewalk.



PLAN AT RAISED SIDEWALK



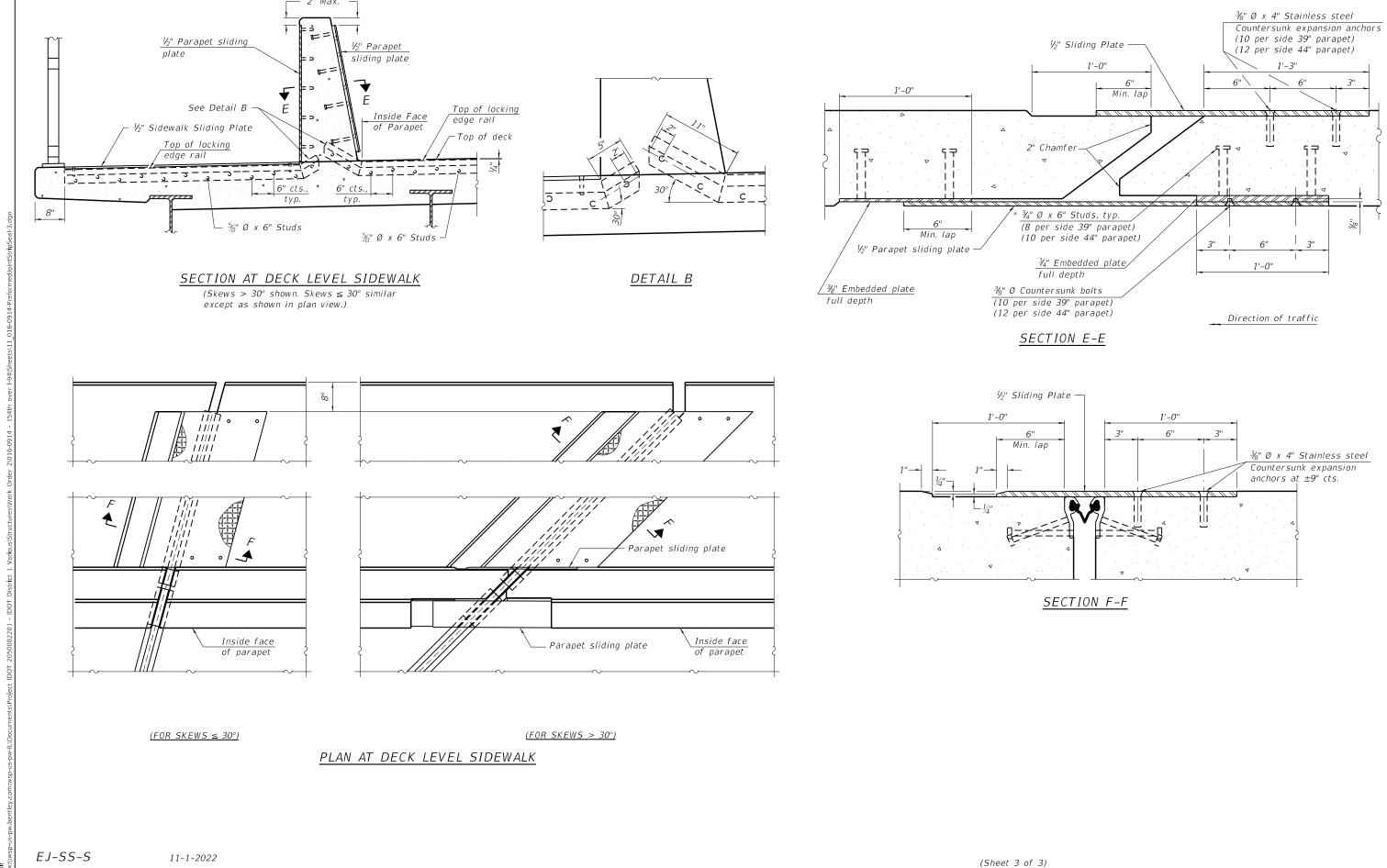
EJ-SS-S

11-1-2022

SER NAME = DESIGNED -REVISED CHECKED -RRD REVISED REVISED CHECKED -REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**  PREFORMED JOINT STRIP SEAL - SIDEWALK (2 OF 3) **STRUCTURE NO. 016-0914** SHEET S07-10 OF S07-25 SHEETS

SECTION 94 (42-B-11-1) BR, BJR 24 COOK 761 649 CONTRACT NO. 62W87



FILE NAME DIV.

207.10.014

SER NAME =

PLOT DATE =

DESIGNED - JS

DRAWN

CHECKED -

CHECKED - RRD

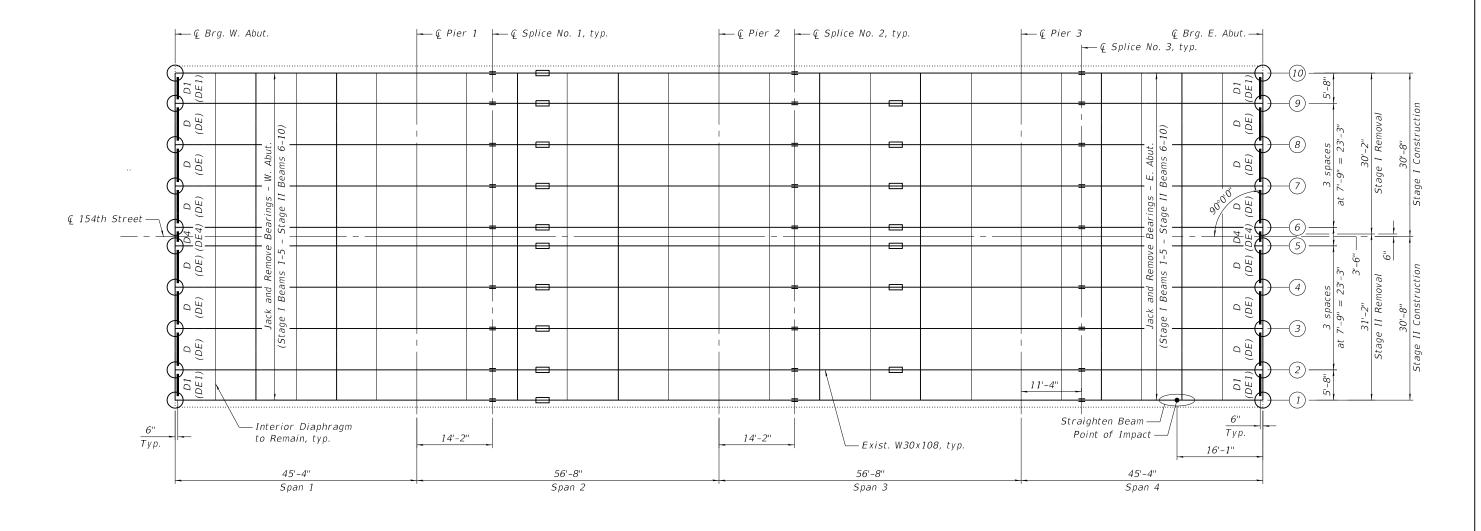
REVISED

REVISED

REVISED

REVISED





### NOTES:

- 1. For Beam End and Mid-Span Repairs, Diaphragm Removal and Replacement Details and Bill of Materials, see Sheet S07-13 thru S07-15.
- 2. DE, DE1 and DE4 are W12x40 existing diaphragms. D, D1 and D4 are proposed W12x40 diaphragms.

# LEGEND

Perform Beam End Plating.
Paid for as Structural Steel Repair.

Remove and Replace Existing Diaphragm.
Paid for as Structural Steel Removal and
Furnishing and Erecting Structural Steel.

Perform Beam Mid-Span Plating. Paid for as Structural Steel Repair.

Proposed
Diaphragm Name
Diaphragm Name

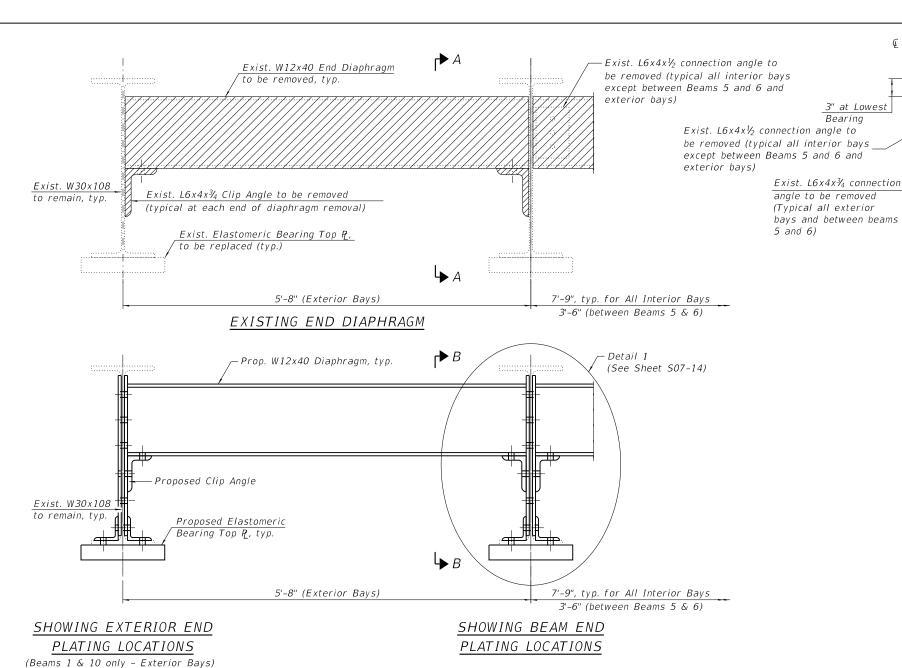
Ę	
NAN	
ILE.	

USER NAME =	DESIGNED - JS	REVISED -
	CHECKED - RRD	REVISED -
PLOT SCALE =	DRAWN - SVJ	REVISED -
PLOT DATE =	CHECKED - RRD	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FRAMING PLAN					
STRUCTURE NO. 016-0914					
SHEET SO7-12	OF SO7-25 SHEETS				

F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.	
94	(42-B-11-1) BR, BJR	COOK	761	651	
			CONTRACT	NO. 6	2W87
	ILLINOIS	FED. Al	D PROJECT		



PROPOSED END DIAPHRAGM

# NOTES:

- 1. For Framing Plan, locations of Diaphragm Removal/Replacement and Beam End and Mid-Span Repairs, see Sheet S07-12.
- 2. All proposed beam end repair plates and bottom flange repair angles shall conform to the requirements of AASHTO M270 Grade 50. All proposed diaphragms and their connection angles shall conform to the requirements of AASHTO M270 Grade 36.
- 3. Diaphragm connection holes shall be  $^{15}\!\!/_{16}$  in. diameter for  $^{3}\!\!/_{4}$  in. diameter A325 bolts. Two hardened washers shall be required at all diaphragm connections and
- 4. Existing Diaphragm and Clip Angle removal shall be paid for as Structural Steel
- 5. All proposed Web Repair Plates, mid-span Strengthening Plates, Flange Repair Angles, Fill Plates and associated bolts and fasteners shall be paid for as Structural Steel Repair. All proposed Diaphragms, Diaphragm Splice Plates, Clip Angles and associated bolts, washers and fasteners shall be paid for as Furnishing and Erecting Structural Steel.
- See Sheet S07-14 for Detail 1.

# END DIAPHRAGM STAGE CONSTRUCTION SEQUENCE

- 1. Order Diaphragm in two sections.
- 2. Attach section (1) of Diaphragm to Beam
- 3. Place Timber Block Posts between section (1) of diaphragm and abutment bearing section.

€ Brg. ~

VIEW A-A

Exist. W12x40 End Diaphragm

Exist. W30x108

Clip Angle

(Beams 1 thru 3

and 8 thru 10)

to remain

Exist. Elastomeric

Bearing, to be replaced

Stage I Construction

W12X40

2'-31/2"

to be removed

- 4. Attach section (2) of diaphragm to both Beam and section (1) of diaphragm during Stage II Construction with splice plates.
- 5. Remove Timber Block Posts.

### → i → i → Φ Φ '-1"x10"x¾" Web ⊕ Splice PL each $\Phi \Phi$ $\Phi$ $\Phi$ ф ф $\phi$ 1'-1"x8"x¾" Web Splice P top and bottom \* Timber block posts 3'-6" (D2)

# PROPOSED END DIAPHRAGM AT STAGE CONSTRUCTION JOINT

(See Proposed End Diaphragm detail for Connection Plate and Angle details.)

\*Cost of Timber Block Posts is included with Furnishing and Erecting Structural Steel

### LEGEND:



### BILL OF MATERIAL

— Prop. W12x40 Diaphragm

Ф

 $\oplus$ 

Ф

11/2"

Φ

Φ

31/2" : 31/2"

10"

Stage II Construction

1'-21/2" € 3/4" Ø

H.S. Bolts

VIEW B-B

Match Existing

- Clip Angle, each side

(Beams 4 thru 7)

¾" ∅ H.S. Bolts

<sup>1</sup>5⁄<sub>16</sub>" ∅ Holes.

(3" at Lowest Bearing)

Proposed Elastomeric Bearing

Exist. W30x108

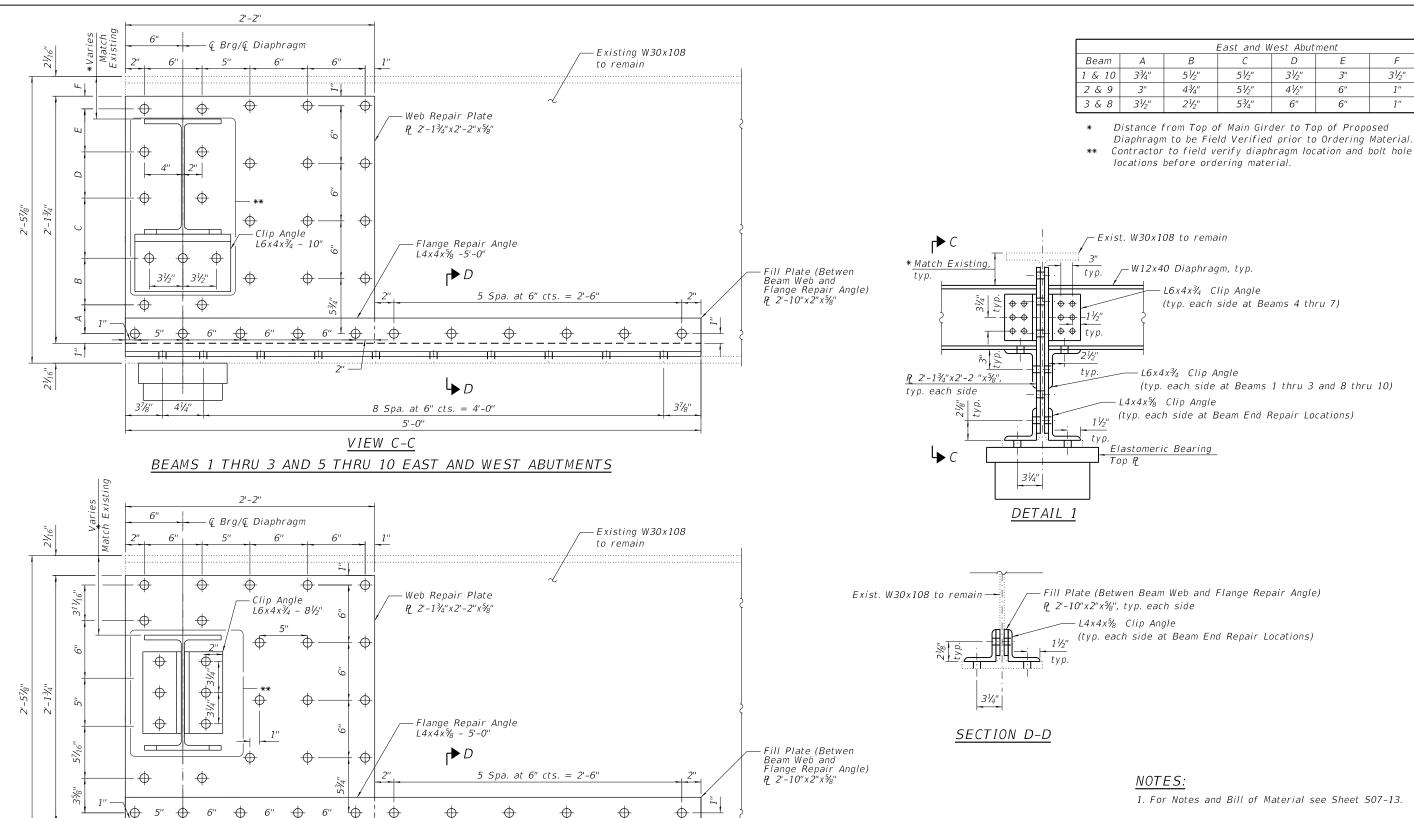
to remain

ITEM	UNIT	QUANTITY
Furnishing and Erecting Structural Steel	Pound	5,830
Structural Steel Removal	Pound	5,430
Structural Steel Repair	Pound	10,650

SER NAME = DESIGNED - JS REVISED CHECKED -RRD REVISED DRAWN REVISED CHECKED -LOT DATE = REVISED

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION**  DIAPHRAGM REPLACEMENT DETAILS **STRUCTURE NO. 016-0914** SHEETS07-13 OF S07-25 SHEETS

SECTION COUNTY 94 (42-B-11-1) BR, BJR 24 COOK 761 652 CONTRACT NO. 62W87



- 1. For Notes and Bill of Material see Sheet S07-13.
- 2. For Locations of Diaphragm Removal/Replacement and Beam End and Mid-Span Repairs, see Sheet S07-12.
- 3. See Sheet S07-13 for Clip Angle lengths.
- 4. Contractor to field verify hole locations before ordering material. Contractor can elect to field drill holes in repair plates.

BEAMS 4 THRU 7 EAST AND WEST ABUTMENTS					VIEW	<u>/ C-C</u>			
BEAMS 4 TIMO / EAST AND WEST ABOTHERTS	<u>BEAMS</u>	4	THRU	7	EAST	AND	WEST	ABUTMENTS	

Proposed Bearing

 $\downarrow D$ 

8 Spa. at 6" cts. = 4'-0"

5'-0"

á	563	USER NAME =	DESIGNED - JS	REVISED -
ME			CHECKED - RRD	REVISED -
ž		PLOT SCALE =	DRAWN - SVJ	REVISED -
∄		PLOT DATE =	CHECKED - RRD	REVISED -

**STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION** 

37/8"

**BEAM END PLATING DETAILS STRUCTURE NO. 016-0914** SHEET S07-14 OF S07-25 SHEETS

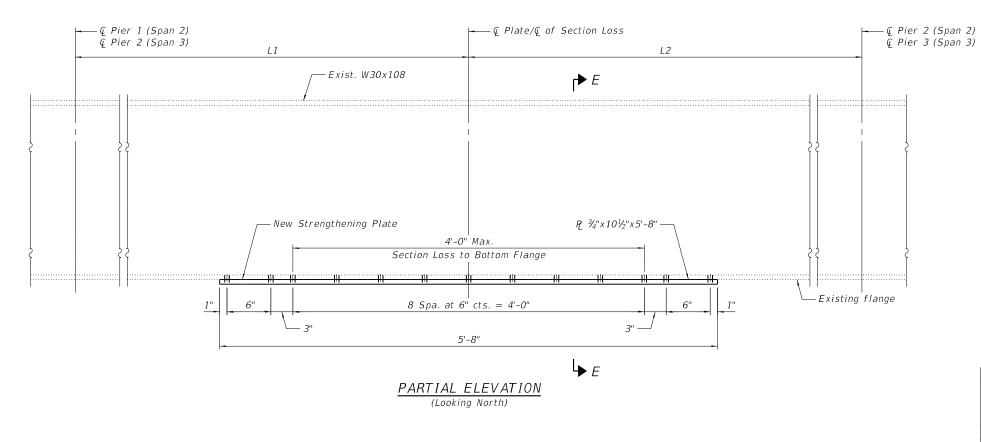
SECTION 94 (42-B-11-1) BR, BJR 24 COOK 761 653 CONTRACT NO. 62W87

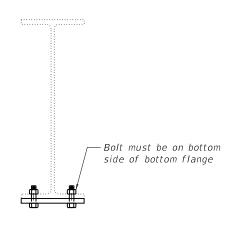
31/2"

6"

37/8"

41/4"





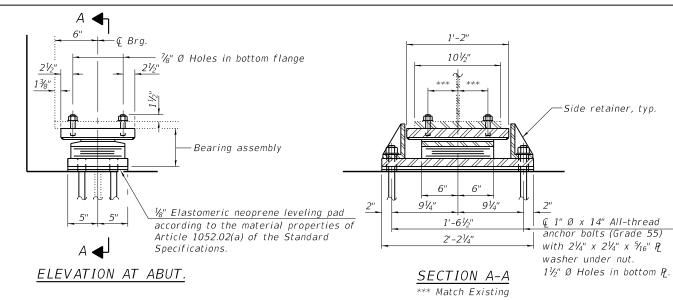
# SECTION E-E

	Mid-Span Repair Detail Table								
Direction of Travel	Beam #	Span Number	Pier Reference		from CL CL Pier				
I-94		, vannoer	rier er erree	L1	L2				
Northbound	2	3	3		32'-0"				
Northbound	4	3	3		32'-0"				
Northbound	5	3	3		32'-0"				
Northbound	6	3	3		32'-0"				
Northbound	7	3	3		32'-0"				
Northbound	8	3	3		32'-0"				
Northbound	9	3	3		32'-0"				
Southbound	1	2	1	27'-10"					
Southbound	2	2	1	31'-0"					
Southbound	3	2	1	31'-0"					
Southbound	4	2	1	31'-0"					
Southbound	5	2	1	29'-0"					
Southbound	6	2	1	29'-0"					
Southbound	7	2	1	31'-0"					
Southbound	8	2	1	25'-0"					
Southbound	9	2	1	25'-0"					
Southbound	10	2	1	28'-0"					

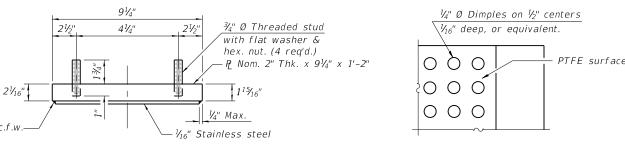
# NOTES:

- 1. For Notes and Bill of Material see Sheet S07-13.
- 2. Contractor to field verify hole locations before ordering material. Contractor can elect to field drill holes in repair plates.

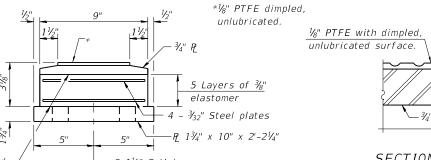
I. E.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1) BR, B.	R 24	соок	761	654
			CONTRACT	NO. 6	2W87
	ILLINC	IS FED. A	ID PROJECT		

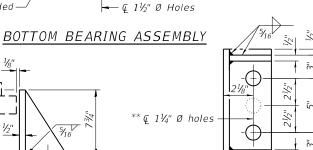


### TYPE II ELASTOMERIC EXP. BRG.



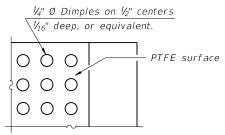
### TOP BEARING ASSEMBLY



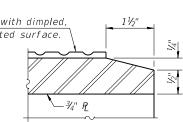


SIDE RETAINER Equivalent rolled angle with stiffeners will be allowed in lieu of welded plates.

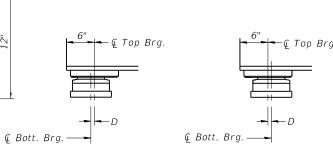
\*\* Center of proposed anchor bolts must be at least 21/4" away from center of existing anchor bolts.



### PLAN-PTFE SURFACE



### SECTION THRU PTFE

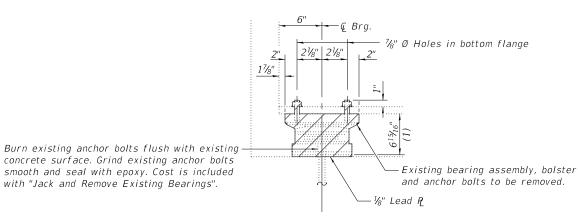


BELOW 50°F.

 $D=\frac{1}{8}$ " per each 100' of expansion for every 15° temp. change from the normal temp. of 50°F.

### EXPANSION BEARING ORIENTATION

The above diagrams are for informational purposes only to show the amount of expected offset "D" for the current temperature in the field.



### EXISTING BEARING REMOVAL

(1) Contractor shall measure in field the distance between bottom of existing bottom flange and top of bearing seat prior to ordering material. Cost is included with "Jack and Remove Existing Bearings".

### GIRDER REACTIONS

Bearing Location	DL (kips)	LL (kips)	IM (kips)	Total (kips)
E. & W. Abut Beam 1 and 10	20.6	47.2	13.9	81.7
E. & W. Abut Beam 5 and 6	20.4	47.2	13.9	81.5
E. & W. Abut Beams 2 thru 4 and 7 thru 9	23.9	47.2	13.9	85.0

Service girder self-weight and deck weight prior to overlay are shown. The Contractor shall design and place jacking system to reposition the specified bearing for the stated reactions and as required in the special provision.

Loads are per Girder.

with "Jack and Remove Existing Bearings".

Side retainers and leveling pad required for the elastomeric bearing assembly shall be included in the cost of Elastomeric Bearing Assembly, Type II.

The  $\frac{1}{8}$ " PTFE sheet shall be bonded directly to the top steel plate with a two-component, medium viscosity epoxy resin, conforming to the requirements of the Federal Specification MMM-A-134, Type I. The bond agent shall be applied on the full area of the contact

Bonding of 1/8" PTFE sheet during vulcanizing process will be permitted provided the process and method of adjusting assembly height is approved by the Engineer.

See Sheet S07-06 for Jack and Remove Existing Bearings Bill of Material and Staging Details.

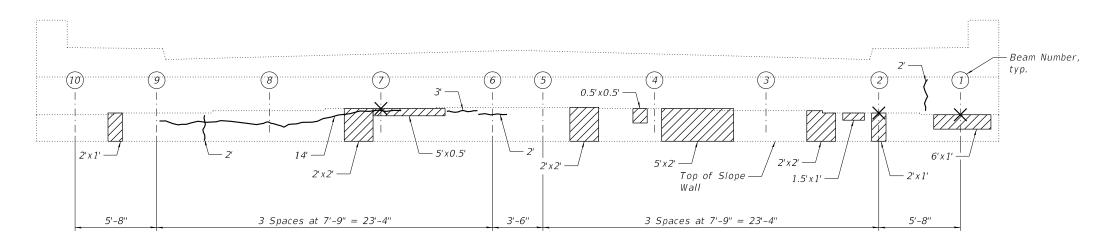
See Sheet S07-23 for existing bearing removal detail and "Jack and Remove Existing Bearings Procedures".

### BILL OF MATERIAL

Item	Unit	Total
Elastomeric Bearing Assembly Type II	Each	20
Anchor Bolts, 1"	Each	80

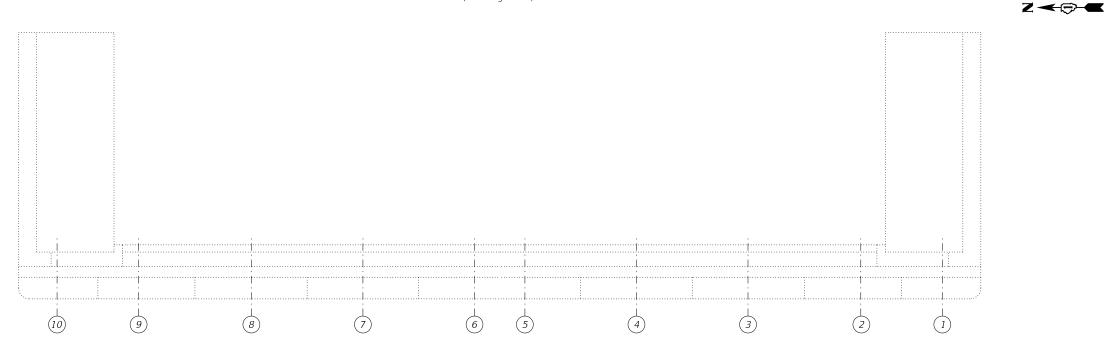
USER NAME =	DESIGNED -	JS	REVISED	-
	CHECKED -	RRD	REVISED	=
PLOT SCALE =	DRAWN -	SVJ	REVISED	=
PLOT DATE =	CHECKED -	RRD	REVISED	-

A I. RTE	SEC	TION		COUNTY	TOTAL SHEETS	SHE
94	(42-B-11-1)	BR, BJR	24	соок	761	65
				CONTRACT	NO. 6	2W8



## **ELEVATION**

(Looking East)



### NOTES:

- 1. Quantities and limits shown are estimates for bidding porposes only. The actual area to be repaired, and the type(s) of repairs to be used, will be determined by the engineer in the field at the time of construction.
- 2. Removal and installation of drainage pipes, electrical cables conduits, or other items attached to the existing structure are included in the cost of Structural Repair of Concrete.
- 3. Concrete repairs directly under Girders shall not start until the Temporary Shoring is installed.

### PLAN

# GIRDER REACTION TABLE

East Abutment	Load (kips)
DL	23.9
LL	47.2
IM	13.9
TOTAL	85.0

Reactions are per girder

QUANTITY

37

BILL OF MATERIAL

Structural Repair Of Concrete

Temporary Shoring and Cribbing

Epoxy Crack Injection

(Depth Equal To Or Less Than 5 In)



Structural Repair of Concrete (Depth Equal to or Less Than 5 inches)

E

Epoxy Crack Injection

X

LEGEND:

Temporary Shoring and Cribbing

SILE NAME: D

 USER NAME
 =
 DESIGNED
 JS
 REVISED

 CHECKED
 RRD
 REVISED

 PLOT SCALE
 =
 DRAWN
 SVJ
 REVISED

 PLOT DATE
 =
 CHECKED
 RRD
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

EAST ABUTMENT REPAIRS STRUCTURE NO. 016-0914 SHEET 507-17 OF 507-25 SHEETS

Sq. Ft.

Foot

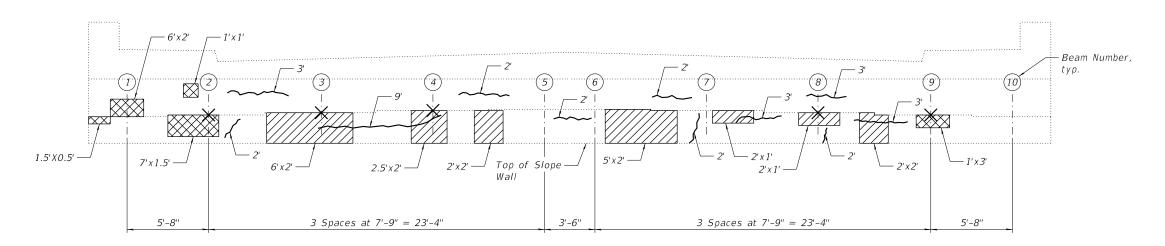
Each

F.A.I. SECTION COUNTY TOTAL SHEETS NO.

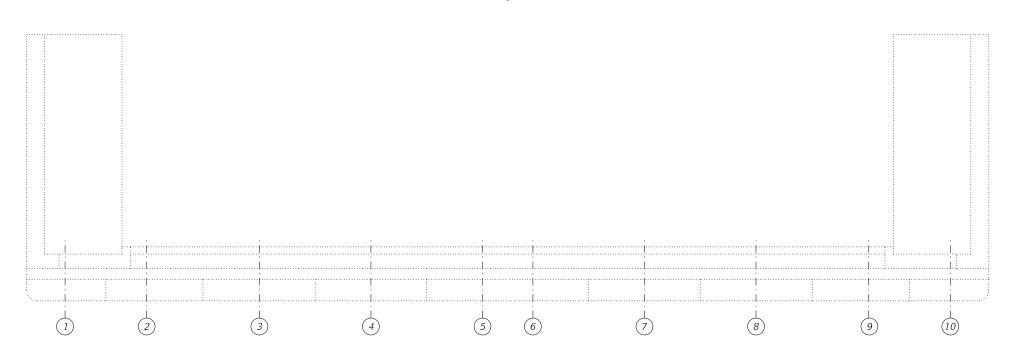
94 (42-B-11-1) BR, BJR 24 COOK 761 656

CONTRACT NO. 62W87

pw.bentley.com.wsp-us-pw-IL\Documents\Project IDOT 2050082201 - IDO



ELEVATION (Looking West)



### <u>NOTES:</u>

- 1. Quantities and limits shown are estimates for bidding porposes only. The actual area to be repaired, and the type(s) of repairs to be used, will be determined by the engineer in the field at the time of construction.
- 2. Removal and installation of drainage pipes, electrical cables conduits, or other items attached to the existing structure are included in the cost of Structural Repair of Concrete.
- 3. Concrete repairs directly under Girders shall not start until the Temporary Shoring is installed.

### PLAN

### GIRDER REACTION TABLE

West Abutment	Load (kips)
DL	23.9
LL	47.2
IM	13.9
TOTAL	85.0

Reactions are per girder

### BILL OF MATERIAL

ITEM	UNIT	QUANTIT
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 In)	Sq. Ft.	42
Structural Repair Of Concrete (Depth Greater Than 5 In)	Sq. Ft.	25
Low Pressure Epoxy Crack Injection	Foot	33
Temporary Shoring and Cribbing	Each	5

### LEGEND:



Structural Repair of Concrete (Depth Equal to or Less Than 5 inches)



Structural Repair of Concrete (Depth Greater than 5 inches)



Epoxy Crack Injection



Temporary Shoring and Cribbing



 USER NAME
 =
 DESIGNED
 JS
 REVISED

 CHECKED
 RRD
 REVISED

 PLOT SCALE
 =
 DRAWN
 SVJ
 REVISED

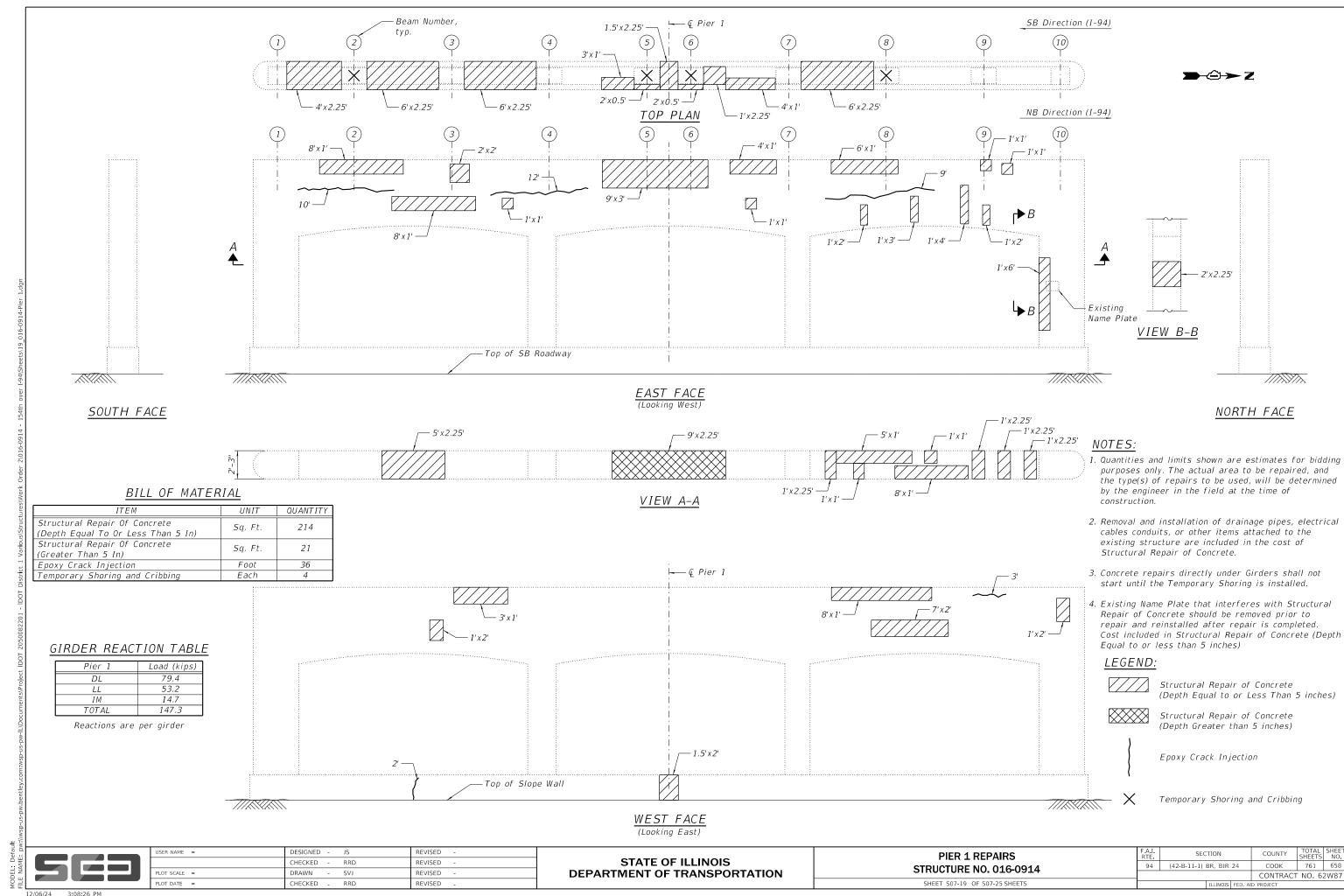
 PLOT DATE
 =
 CHECKED
 RRD
 REVISED

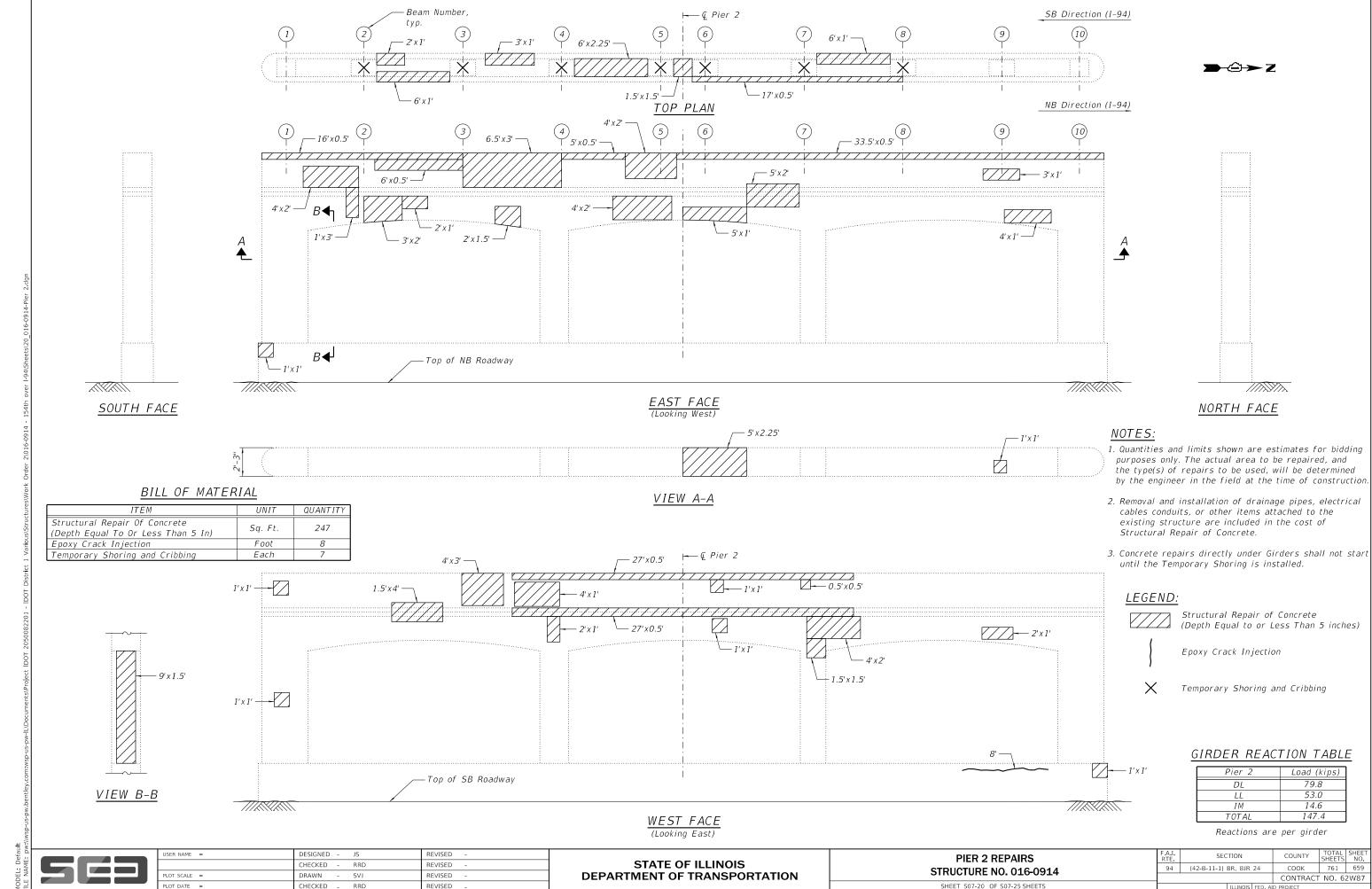
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

WEST ABUTMENT REPAIRS STRUCTURE NO. 016-0914  
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

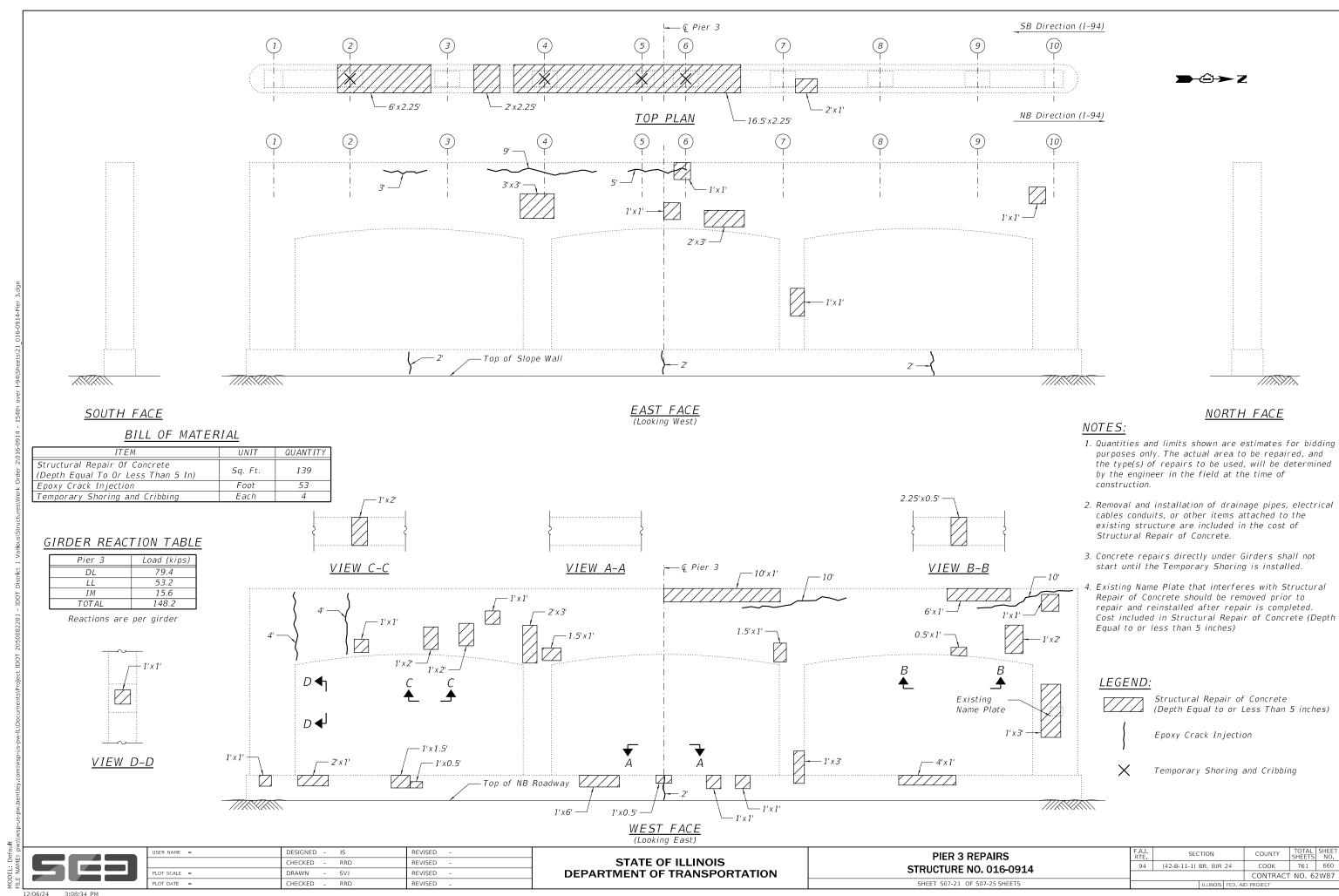
 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 657

 CONTRACT NO. 62W87





E \_\_\_\_\_\_

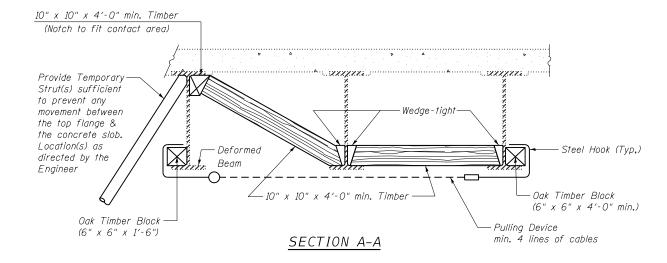


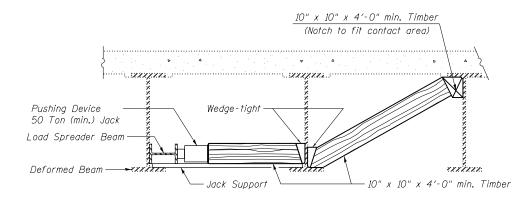
## <u>PARTIAL PLAN</u> SUGGESTED BEAM STRAIGHTENING METHODS

(PUSHING DEVICE)

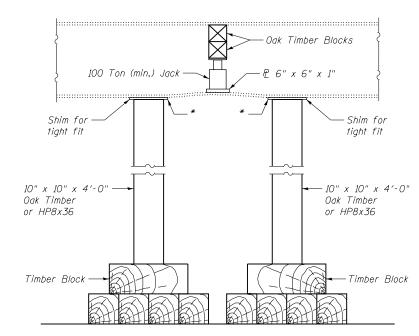
(PULLING DEVICE)

NOTE: Straightening force shall be maintained on all load transfer blocking during beam straightening.





### SECTION B-B



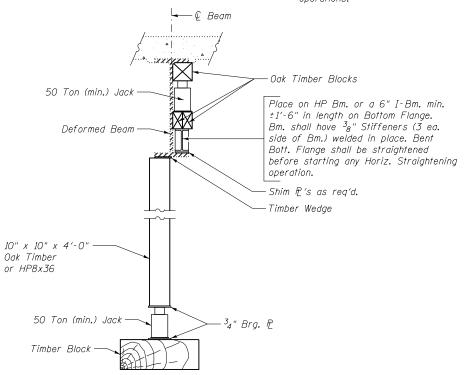
### SUGGESTED VERTICAL STRAIGHTENING DETAIL

(To correct localized vertical flange deformations.)

\* Edge of plate shall line up with edge of deformation.

### NOTE:

Braces and jack assembly shall be placed on same side of web. Bent bottom flange shall be straightened before starting any horizontal straightening operations.



VERTICAL STRAIGHTENING DETAIL



# EXISTING DEFORMATION TO BE STRAIGHTENED

(Looking North) (Approximate max. deflections) Deflected length of beam to be straightened is approximately 4'-0".



CHECKED         -         RRD         REVISED         -           PLOT SCALE         =         DRAWN         -         SVJ         REVISED         -	USER NAME =
PLOT SCALE = DRAWN - SVJ REVISED -	
	PLOT SCALE =
PLOT DATE = CHECKED - RRD REVISED -	PLOT DATE =

BEAM STRAIGHTENING DETAILS STRUCTURE NO. 016-0914
SHEET S07-22 OF S07-25 SHEETS

I.	SEC	TION			COUNTY	TOTAL SHEETS	SHEET NO.
4	(42-B-11-1)	BR, BJR	24		COOK	761	661
					CONTRACT	NO. 6	2W87
		THEMOTE	EED	Α1	D BROJECT		

Notes: **→**②→ Z - @ Brg. - Face of Exist. W. Abutment 1'x2' ±74'-0" WEST SLOPE WALL PLAN ±74'-0" - @ Pier 3 1'x4' —

- Face of Exist.

E. Abutment

REVISED

REVISED

REVISED

REVISED

Repairs shown are based upon observations performed in 2023 and are for bidding purposes only. Actual areas to be repaired shall be determined by the Engineer in the field at the time of construction. Quantities have been adjusted to account for the difference.

Any voids found under the existing slopewall during the Slopewall Removal process are to be filled with Porous Granular Embankment.

> Ex. plate to be removed using the air-arc method and grind smooth all weld material remaining on the bottom flange Remove existing Bearing including Bottom Plate and Ex. Concrete welded Top Plate Slopewall — Bk. of Abutment ±1'-0" Burn existing anchor bolts flush with existing concrete surface. Grind existing

LEGEND:

Slope Wall Removal and Slope Wall 4 Inch

---- Epoxy Crack Injection

anchor bolt smooth and seal with epoxy. Cost is incidental to "Jack and Remove Existing Bearings"

EXISTING BEARING REMOVAL DETAIL (Dimensions at Rt L's)

### JACK AND REMOVE EXISTING BEARINGS PROCEDURES

- 1. Jacking shall be done after existing deck partial removal is completed.
- 2. The Contractor shall submit for approval by the Engineer plans for jacking, prior to commencing any work at the bearings. The maximum dead load reaction with the deck removed (per bearing) at the west and east abutments = 16 kips. The minimum jack capacity at each beam shall be 32 kips at the west and east
- 3. Top of beam elevations shall be measured prior to jacking and shall remain the same after bearings are in place.
- 4. There shall be at least one jack per bearing, and the jack shall be placed close to the bearing. The steel shall be raised a maximum of 1/8 inch and shall be blocked in position until after the completion of the installation of new bearings.
- 5. Burn the existing anchor bolts flush with the concrete surface, grind smooth, and seal with epoxy. The top and bottom plates shall be removed. The top plate shall be removed using the air-arc method. Ground smooth all weld material remaining on the bottom flange. Cost of removing anchor, bolts, top plates, and bottom plates shall be included with "Jack and Remove Existing Bearings".
- 6. The new elastomeric bearings shall be in place and the jacks lowered before the new concrete deck is poured.
- 7. See sheet S07-16 for bearing bill of materials.

### BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Porous Granular Embankment	Cu Yd	2
Slope Wall Removal	Sq Yd	3
Slope Wall 4 Inch	Sq Yd	3
Epoxy Crack Injection	Foot	106

# EAST SLOPE WALL PLAN

DESIGNED - JS

DRAWN

CHECKED -

CHECKED - RRD

SER NAME =

— @ Brg.

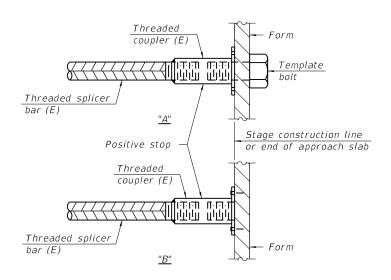
### STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

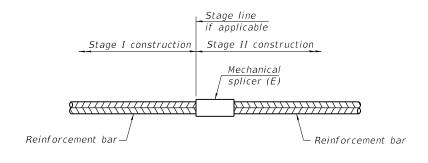
Location	Bar	No. assemblies	Minimum
Location	size	required	lap length
W. Abutment	#5	9	3'-9"
W. Abutment	#6	4	4'-0"
E. Abutment	#5	9	3'-9"
E. Abutment	#6	4	4'-0"



### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for

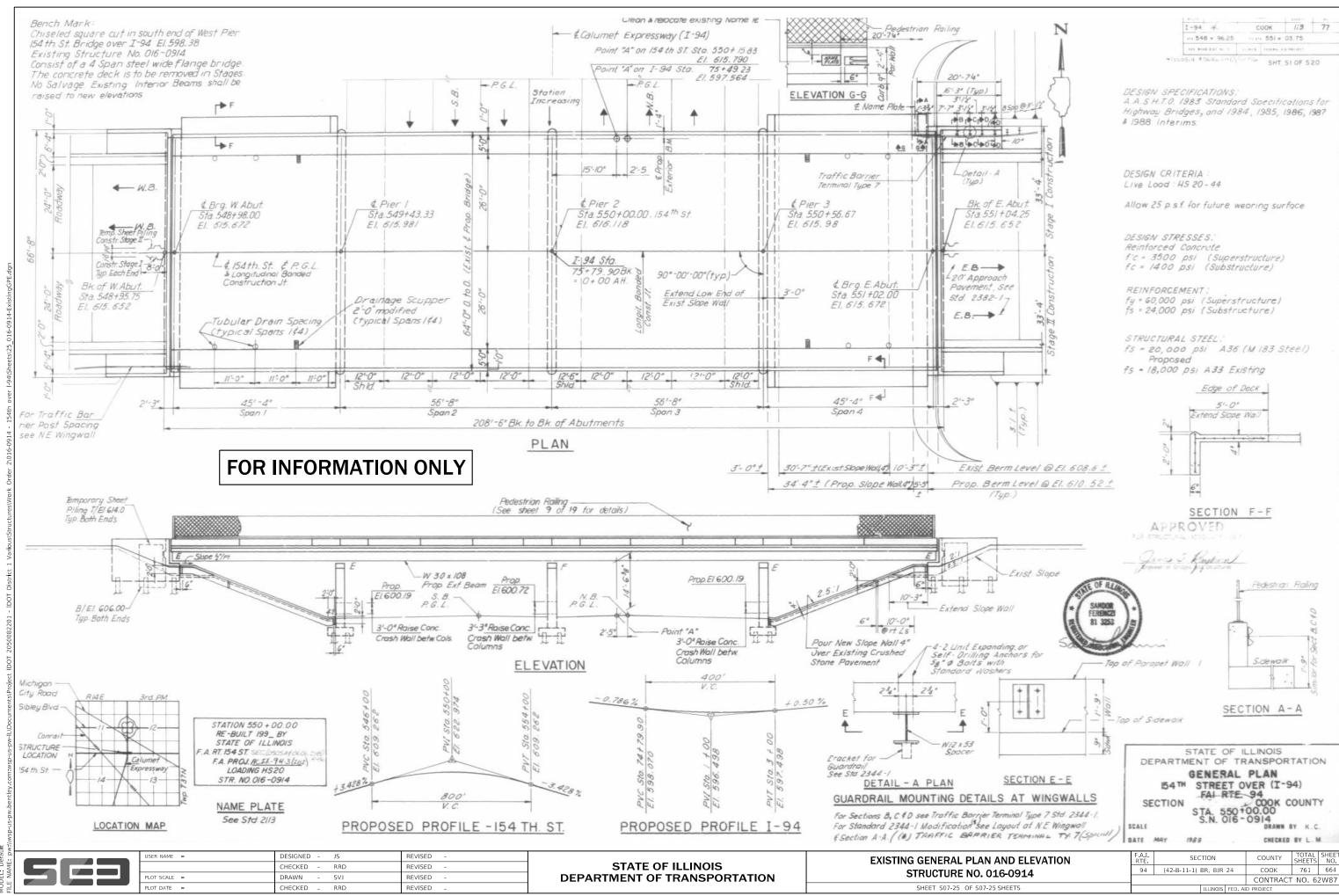
See approved list of bar splicer assemblies and mechanical splicer alternatives.

BSD-1

-1 5-15-2023



٦	USER NAME =	DESIGNED -	JS	REVISED -
		CHECKED -	RRD	REVISED -
	PLOT SCALE =	DRAWN -	SVJ	REVISED -
	PLOT DATE =	CHECKED -	RRD	REVISED -



"X" scribed in chiseled box on top of concrete barrier wall at the southeast corner of bridge structure for Westbound Bench Mark: FAI-94 over the Greenwood Avenue Elev. 627.12 (Assumed Local Datum) Existing Structure: S.N. 016-0162 originally constructed in 1947 as a 4 span structure. In 1981, the structure was widened with a new deck and new beam lines on both sides of the superstructure. All superstructure was cleaned and painted. In 2009, the approach slabs, deck joints at abutments and deck overlay were replaced. The deck joint between the center parapets was resealed. Abutment bearings were replaced with elastomeric bearings. The steel beam ends and end diaphragms at abutments were cleaned and painted. 68'-4" 68'-4" 85'-6" 85'-6" Brg. Abut Brg. Abut ΘĺΩ. 2'-6" 12'-0"12'-0" 2'-6" Existing Approach Existing Approach Curb and Curb and Slab Slab Gutter Gutter Exist. Perform Slopewall Perform Concrete -Ground Repairs Abutment Repairs Perform Concrete @ Greenwood Ave. Pier Repairs, Typ. Existing Slopewall to Remain, Typ. ELEVATION  $312'-10^{3}/_{4}''$  Bk. to Bk. abutments

85'-6"

Limits of Protective Shield

\_\_\_\_\_

į Pier -

. Sta. 570+03.95

Greenwood Ave

PLAN

28°37'30":

typ.

85'-6"

Stage Construction

Line

Sta. 570+89.44

— Stage Construction

Bridge Deck Scarification ¾" &

Bridge Deck Thin Polymer Overlay 3/8"

I-94 Sta. 570+46.70

Greenwood Ave.

### DESIGN SPECIFICATIONS

2002 AASHTO Standard Specifications for Highway Bridges, 17th Edition

### DESIGN STRESSES

FIELD UNITS (New Construction) f'c = 4,000 psi (Superstructure)

f'c = 3,500 psi (Substructure)fy = 60,000 psi (Reinforcement)

fy = 50,000 psi (M270 Grade 50) FIELD UNITS (2009 Repairs)

f'c = 3,500 psi (Concrete - Deck Slab) fy = 60,000 psi (Reinforcement - Deck Slab) fs = 36,000 psi (M270 Grade 36)

### FIELD UNITS (1980 Rehab)

f'c = 3,500 psi (Concrete - Deck Slab)

fc = 1,400 psi (Substructure)

fy = 60,000 psi (Reinforcement - Deck Slab) fs = 20,000 psi (Reinforcement- Substructure)

fs = 20,000 psi (Structural Steel) (M183 Grade 36) FIELD UNITS (1946 Original Construction)

fs = 18,000 psi (A7 Struct. Steel) f'c = 1,200 psi (Superstructure)

fy = 20,000 psi (Reinforcement) f'c = 800 psi (Concrete - Substructure)

### LOADING HL-93

Replace Expansion Joint with Preformed Joint Strip Seal, typ.

Dimensioned

30'-0"

Approach Slab

Bk. of S. Abut.

Sta. 572+45.90

PG EB Lanes

along Ç I-94

PG WB Lanes

2'-73/8"

Bk. of S.Abut.

---

Wingwall, typ.

68'-4"

G Brg. S. Abut.

₹ Pier 3

<u>Sta. 572+43.29</u>

-Sta. 571+74.95

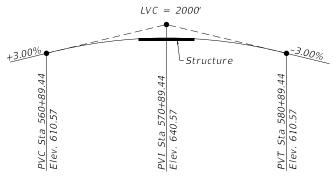
⊊ Brg.-

\_\_\_\_\_

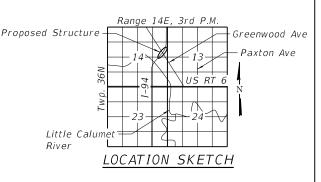
S. Abut.

### SCOPE OF WORK

- Remove the existing bridge deck overlay.
- Repair deck with partial and full depth patches.
- Repair of bridge approach slabs.
- 4. Removal and replacement of expansion joints at the abutments.
- 5. Install a  $\frac{3}{6}$ " thin polymer overlay.
  - Substructure and slopewall repairs.
- . Repair steel beams at corroded deck drain connections and beam ends.



# EXISTING PROFILE GRADE FAI-94



GENERAL PLAN AND ELEVATION

I-94 OVER GREENWOOD AVE

FAI 94 SEC. 1975-079-BR

COOK COUNTY

STATION 570+46.70

SN 016-0162

William P. Malinowski S.E. Licensed Structural Engineer State of Illinois No. 081–006059 Registration Expires 11/30/2026

SHEET S08-01 OF S08-19 SHEETS

WILLIAM P. MALINOWS

wsp

2'-73/8"

Bk. of N.Abut

30'-0"

Approach Slab

 $\langle \Box$ 

 $\triangleleft$ 

 $\Rightarrow$ 

 $\Rightarrow$ 

 $\Rightarrow$ 

Deck Drain, typ.—

68'-4"

G Brg. N. Abut.

Sta. 569+35.61

Brg.

N. Abut

-Bk. of N. Abut.

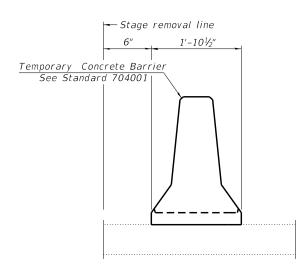
Sta. 569+33.00

USER NAME =	DESIGNED -	BJD	REVISED -		
	CHECKED -	MGH	REVISED -		
PLOT SCALE =	DRAWN -	BJD	REVISED -		
PLOT DATE =	CHECKED -	MGH	REVISED -		

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

### GENERAL NOTES

- 1. No field welding is permitted except as specified in the contract documents.
- 2. Reinforcement bars designated (E) shall be epoxy coated.
- 3. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding  $\frac{3}{4}$  inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 4. Plan dimensions and details relative to the existing structure have been taken from existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 5. Existing reinforcement shall be cleaned, straightened and incorporated into the new construction. Cost included with Concrete Removal.
- 6. Fasteners shall be ASTM F 3125 Grade A325 Type 1. Fastener shall be hot dip galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel." Bolts  $\frac{3}{4}$  in. Ø holes  $\frac{13}{16}$  in. Ø, unless otherwise noted.
- 7. All new structural steel shall be galvanized. See Special Provision for "Hot Dip Galvanizing for Structural Steel."
- 8. Existing structural steel that will be in contact with new structural steel shall be cleaned and painted prior to erection as required by the Special Provision "Cleaning and Painting Contact Surface Areas of Existing Steel Structures."
- 9. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to address the presence of lead on this project.
- 10. Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost incidental to "Concrete Removal".
- 11. Calculated weight of Structural Steel = 2,008 lbs.
- 12. Cleaning and field painting of the existing structural steel shall be done under a separate painting contract.
- 13. Cost of removal and re-installation of all members necessary to complete the work as detailed on the plans and as specified in the Special Provisions shall be included with Structural Steel Repair.



### EXISTING SLAB

### SECTIONS THRU SLAB OR DECK BEAM

### USER NAME = DESIGNED - BJD REVISED -CHECKED - MGH REVISED -DRAWN - BJD REVISED PLOT DATE = CHECKED - MGH REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

### **GENERAL DATA STRUCTURE NO. 016-0162** SHEET S08-02 OF S08-19 SHEETS

### SECTION COUNTY 94 (42-B-11-1) BR. BJR 24 COOK 761 666 CONTRACT NO. 62W87

# INDEX OF SHEETS

508-01	Generai	Plan	and	Elevation
508-02	General	Data		

S08-03 Construction Staging - 1

S08-04 Construction Staging - 2

S08-05 Deck Repair Plan

508-06 North Abutment Expansion Joint Reconstruction Plan S08-07 South Abutment Expansion Joint Reconstruction Plan S08-08 Abutment Expansion Joint Reconstruction Details - 1

S08-09 Abutment Expansion Joint Reconstruction Details - 2

S08-10 Preformed Joint Strip Seal

SO8-11 Framing Plan

508-12 Steel Beam Repair Details

S08-13 North Abutment Repairs

S08-14 South Abutment Repairs S08-15 South Slopewall Repairs

508-16 Pier 1 Repairs

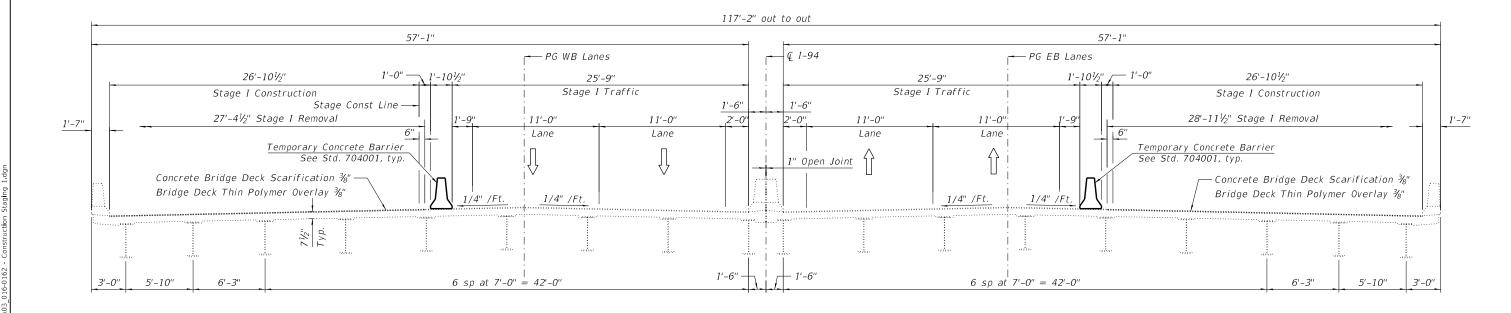
508-17 Pier 2 Repairs

508-18 Pier 3 Repairs

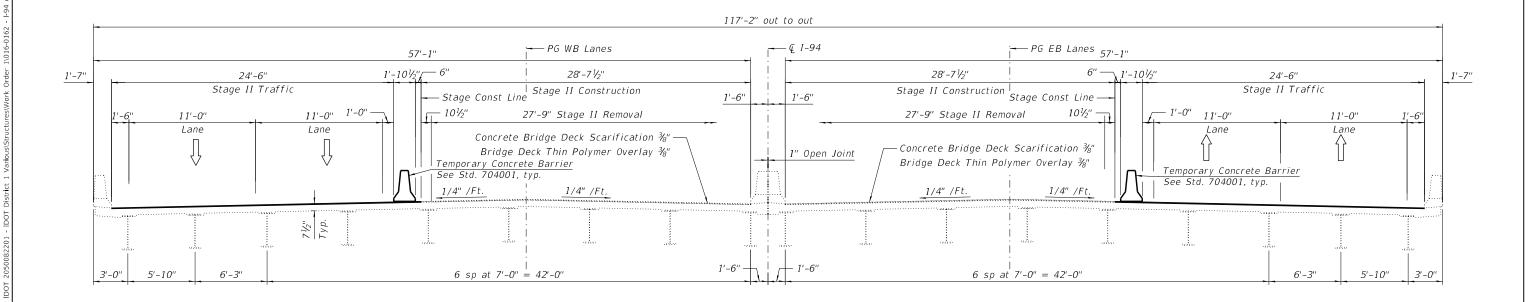
S08-19 Bar Splicer Assembly Details

### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER- STRUCTURE	SUB- STRUCTURE	TOTAL
Porous Granular Embankment	Cu. Yd.	-	211	211
Concrete Removal	Cu. Yd.	23	9	32
Slope Wall Removal	Sq Yd.	-	159	159
Protective Shield	Sq Yd.	1,114	-	1,114
Concrete Structures	Cu. Yd.	-	9.0	9.0
Concrete Superstructure	Cu. Yd.	23.0	-	23.0
Protective Coat	Sq Yd.	10	-	10
Reinforcement Bars, Epoxy Coated	Pound	4,730	1,220	5,950
Bar Splicers	Each	52	12	64
Slope Wall 4 Inch	Sq Yd.	_	159	159
Preformed Joint Strip Seal	Foot	267	-	267
Epoxy Crack Injection	Foot	_	217	217
Approach Slab Repair (Full Depth)	Sq. Yd.	7	-	7
Structural Steel Repair	Pound	2,010	-	2,010
Cleaning Bridge Seats	Sq. Ft.	-	1,566	1,566
Concrete Bridge Deck Scarification 3/8"	Sq. Yd.	3,860	_	3,860
Bridge Deck Thin Polymer Overlay 3/8"	Sq. Yd.	3,860	_	3,860
Structural Repair of Concrete (Depth Equal to	Sg. Ft.	_	872	872
or Less Than 5 Inches)	34.11.	_	0/2	0/2
Deck Slab Repair (Full Depth - Type I)	Sq. Yd.		-	6
Deck Slab Repair (Full Depth - Type II)	Sq. Yd.	158	-	158
Deck Slab Repair (Partial)	Sq. Yd.	255	-	255
Expansion Joint (Special)	Foot	313	-	313



# STAGE I CONSTRUCTION (Looking South)

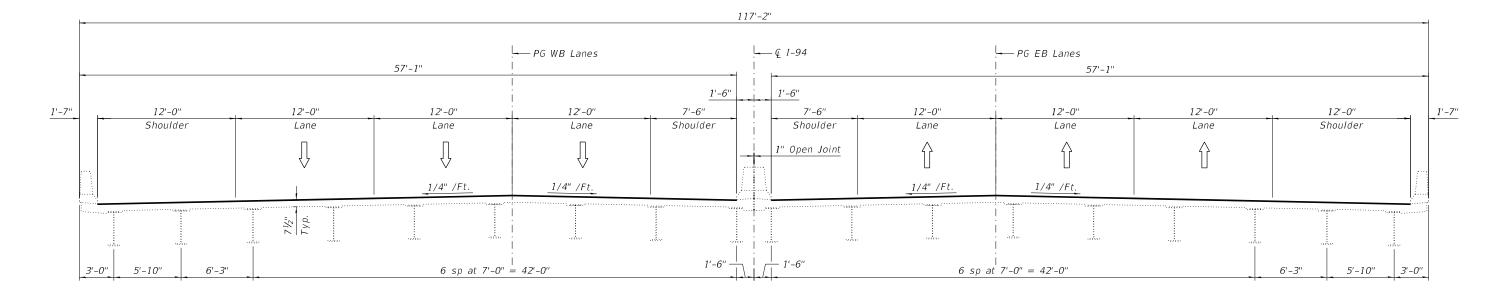


# STAGE II CONSTRUCTION (Looking South)

WSD
-----

CONSTRUCTION STAGING - 1					
STRUCTURE NO. 016-0162					
CHEET COURS OF COURSE OF					

A.I TE.			COUNTY	TOTAL SHEETS	SHEET NO.			
94	4 (42-B-11-1) BR, BJR 24		соок	761	667			
·			CONTRACT	NO. 62	W87			
ILLINOIS FED. A			D PROJECT					



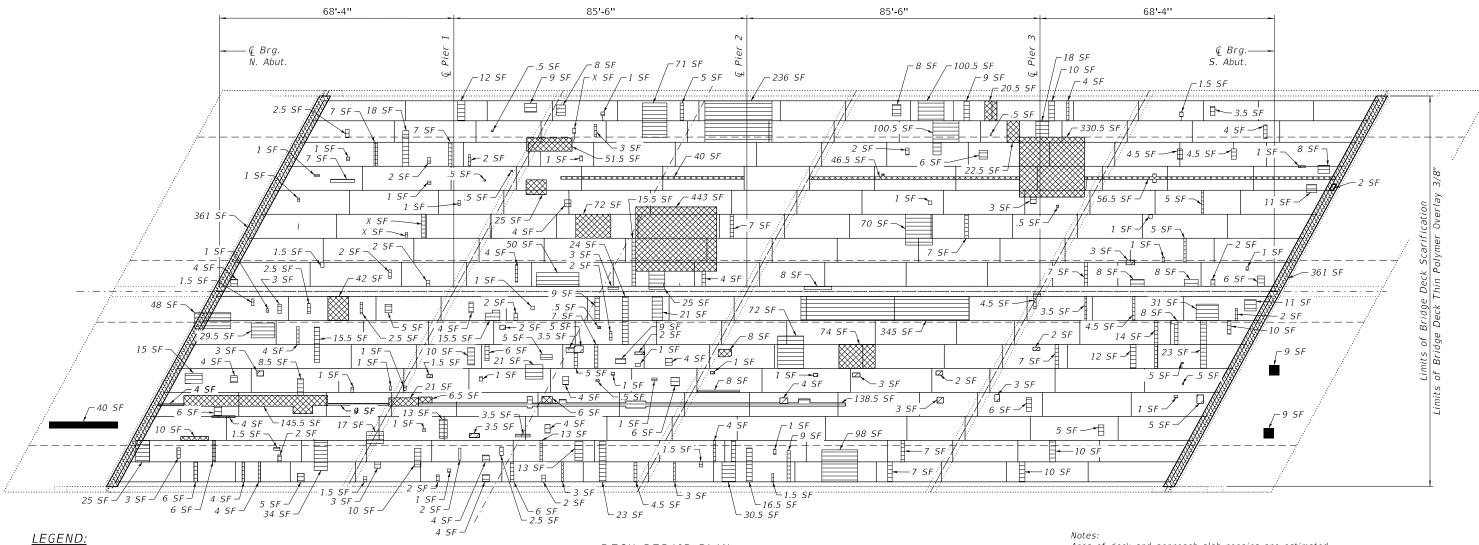
FINAL CROSS SECTION

(Looking South)

115	)

12/9/2024 7:17:23 AM





# DECK REPAIR PLAN

Concrete Removal for Joint Replacement (see Sheets S08-07 to S08-10)

Approach Slab Repair (Full Depth)

Deck Slab Repair (Full Depth, Type I)

Deck Slab Repair (Full Depth, Type II)

Deck Slab Repair (Partial)

Area of deck and approach slab repairs are estimated. Actual type, location, and dimensions are to be determined by the Engineer during construction and documented on as-built plans.

### BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	QUANTITY
Concrete Removal	Cu. Yd.	23	9	32
Approach Slab Repair (Full Depth)	Sq. Yd.	7	-	7
Deck Slab Repair (Full Depth, Type I)	Sq. Yd.	6	-	6
Deck Slab Repair (Full Depth, Type II)	Sq. Yd.	158	_	158
Deck Slab Repair (Partial Depth)	Sq. Yd.	255	-	255

wsp

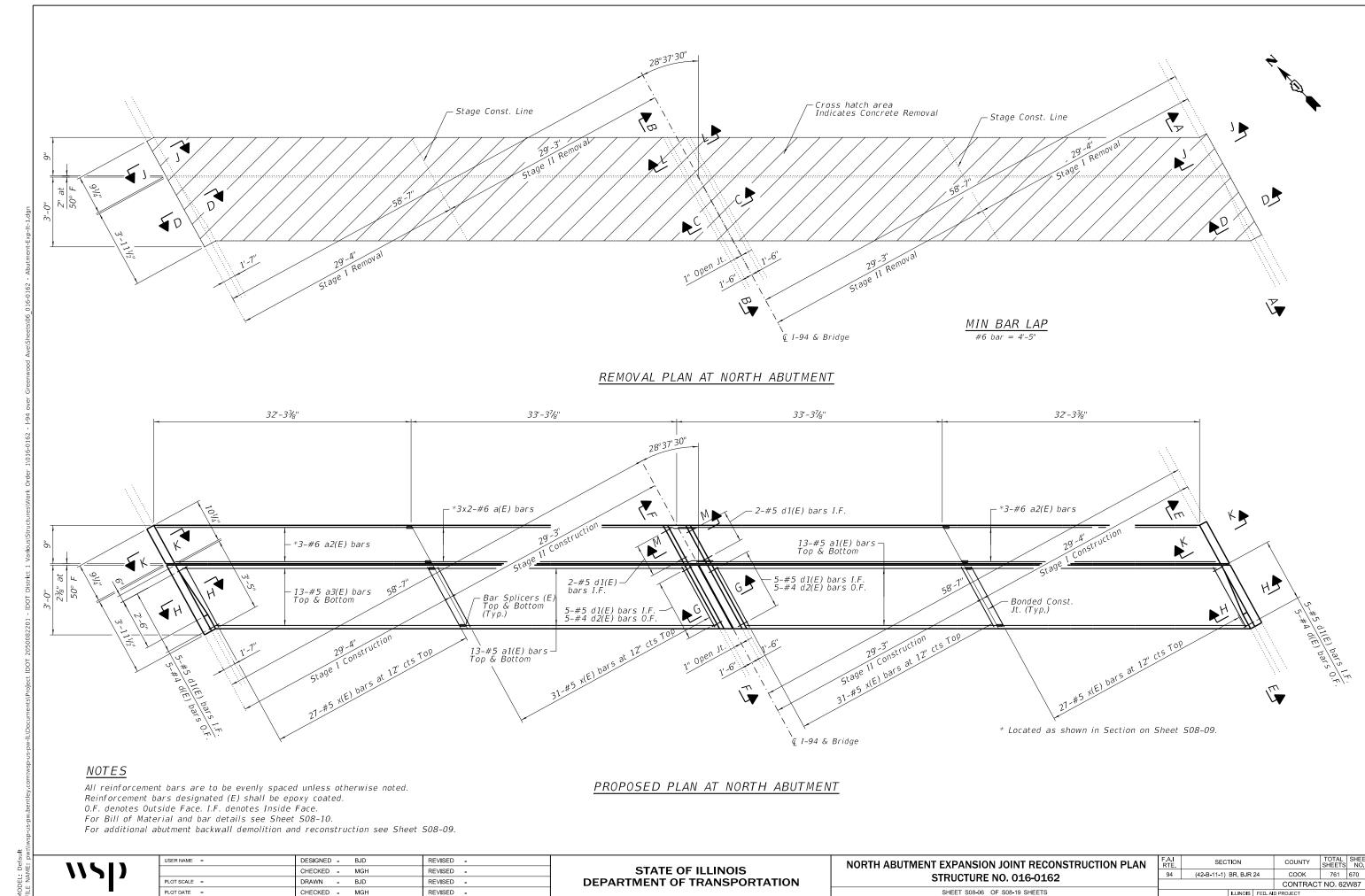
USER NAME =	DESIGNED -	BJD	REVISED -
	CHECKED -	MGH	REVISED -
PLOT SCALE =	DRAWN -	BJD	REVISED -
PLOT DATE =	CHECKED -	MGH	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

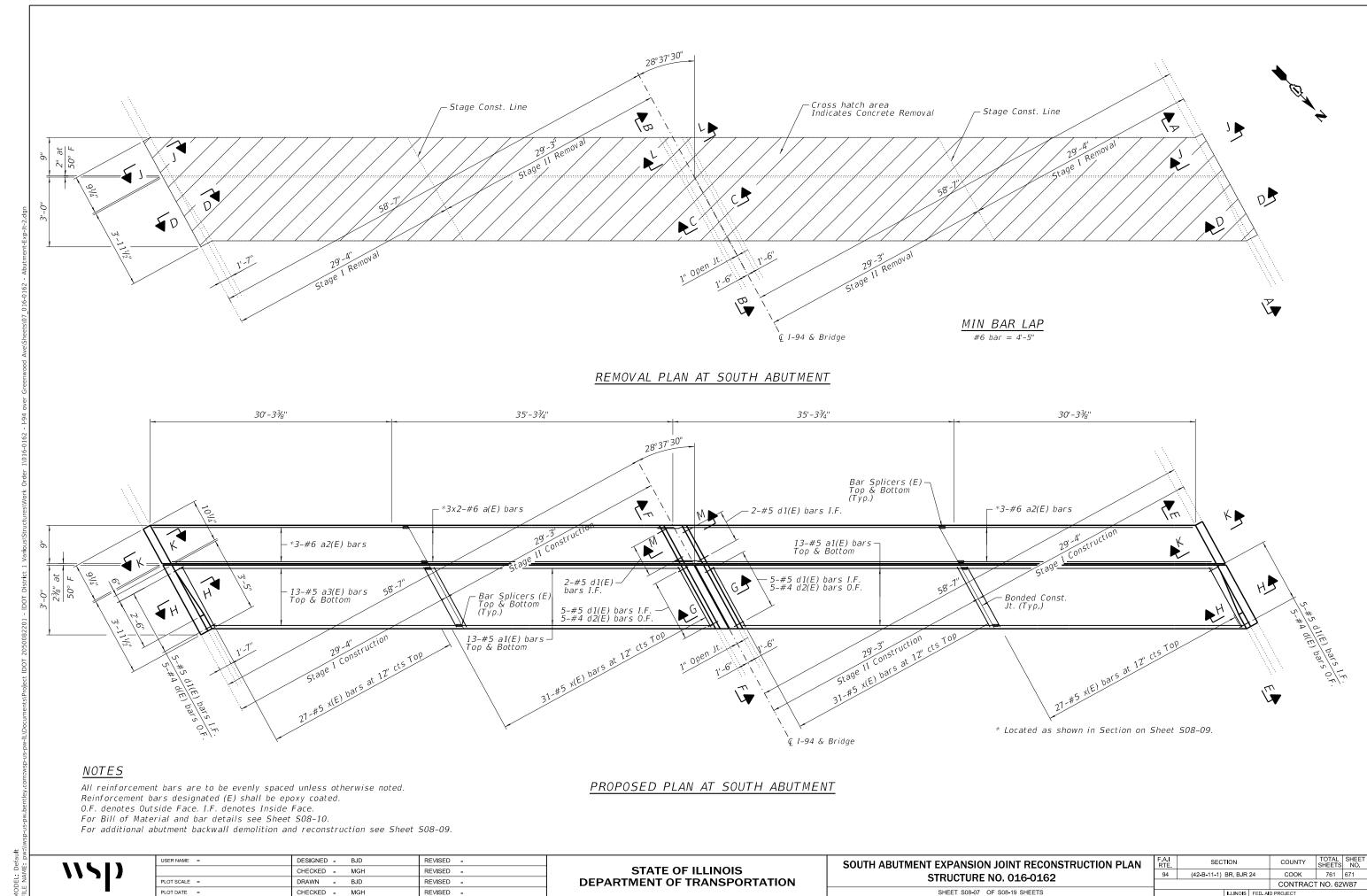
DECK REPAIR PLAN
STRUCTURE NO. 016-0162
SHEET S08-05 OF S08-19 SHEETS

F.A.I SECTION COUNTY TOTAL SHEE SHEETS NO.
94 (42-B-11-1) BR, BJR 24 COOK 761 669

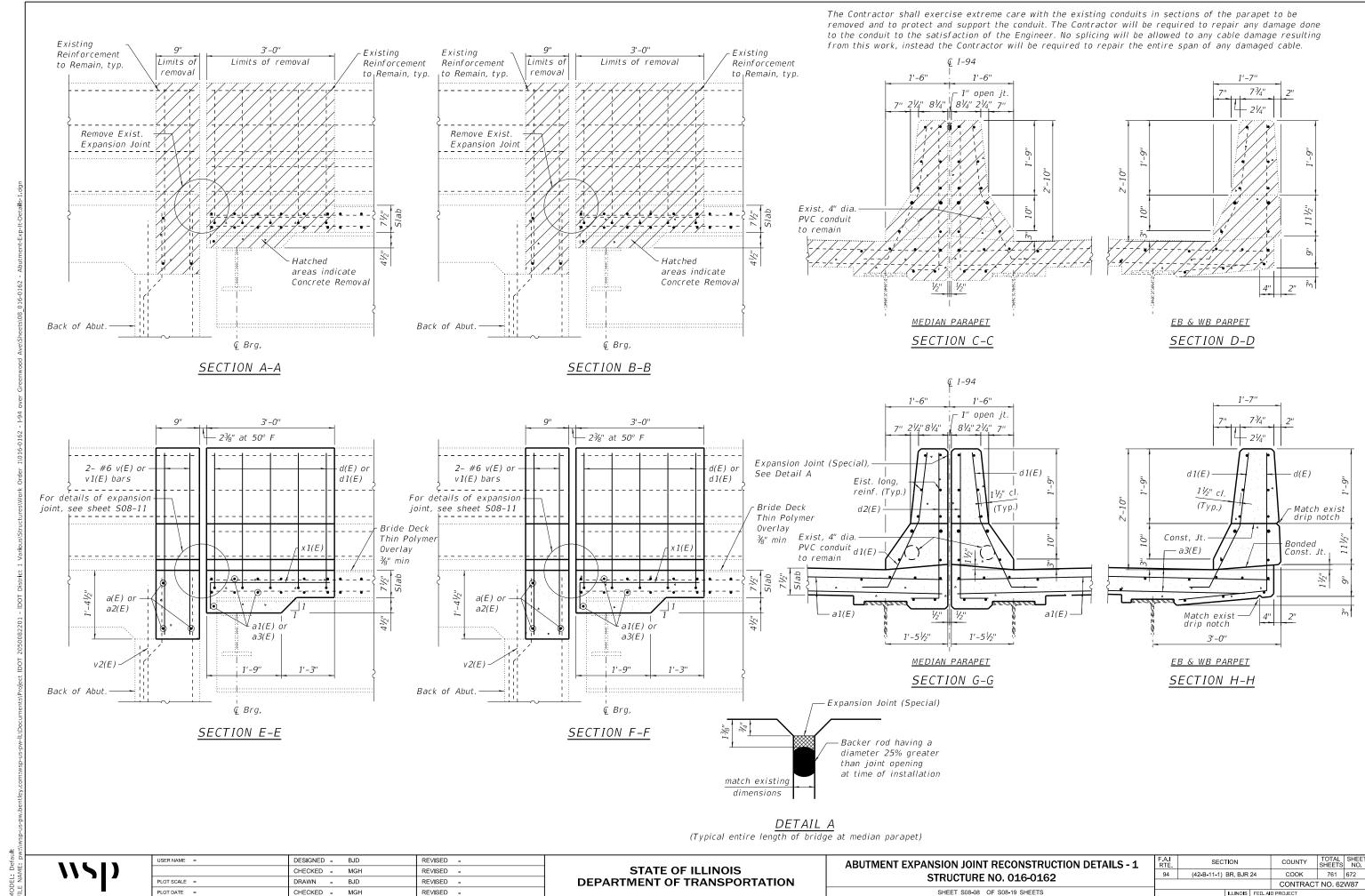
CONTRACT NO. 62W87

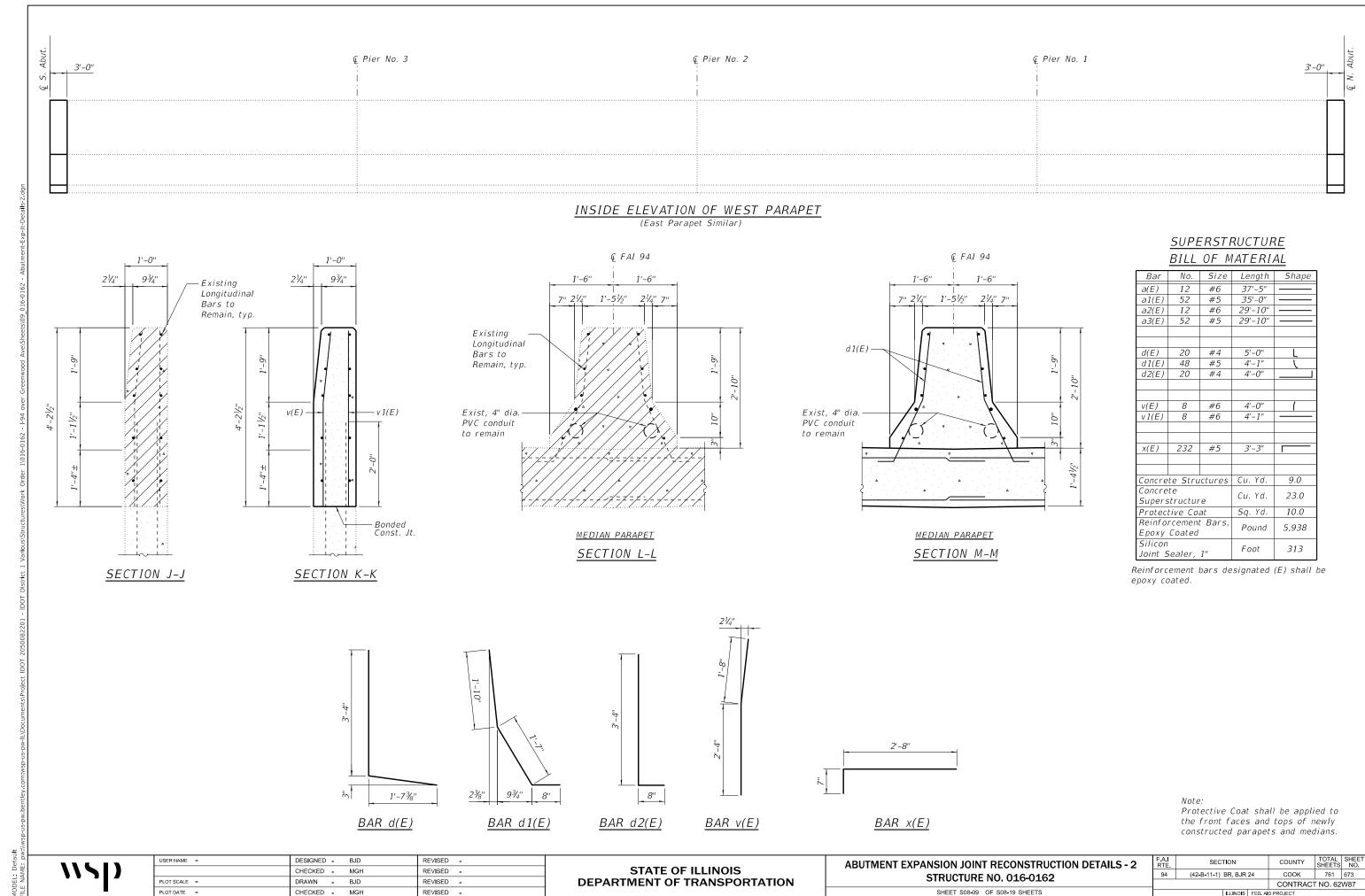


12/9/2024 7:17:31 AM

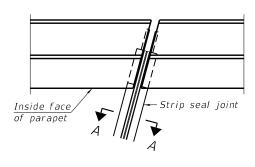


12/9/2024 7:17:35 AM

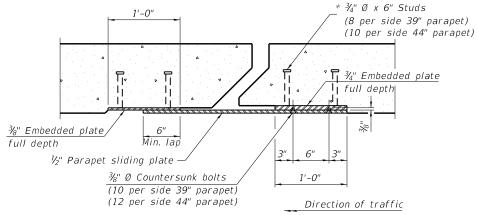




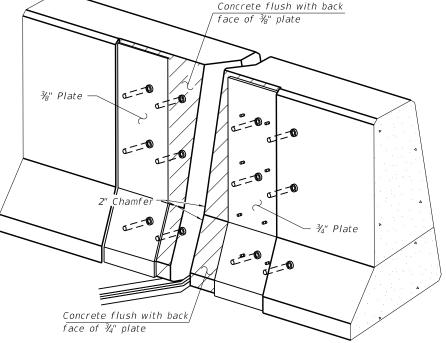
12/9/2024 7:17:43 AM



### PLAN AT PARAPET



### SECTION B-B



are not permitted. The gland shall be sized for a maximum rated movement of 4 inches. The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application

The strip seal shall be made continuous and shall have a minimum thickness of  $\frac{1}{4}$ ". The configuration of the strip

seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations

and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

Notes:

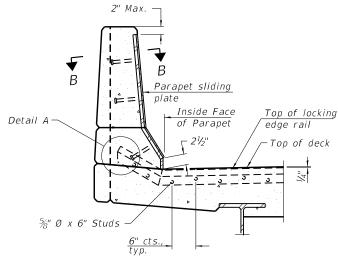
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

39" constant slope barrier shown, 44" constant slope barrier similar as noted.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.



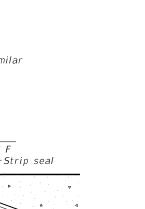
### SECTION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

at 50° F

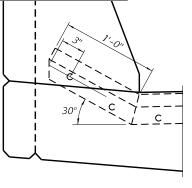
Locking edge rail-

Top of concrete

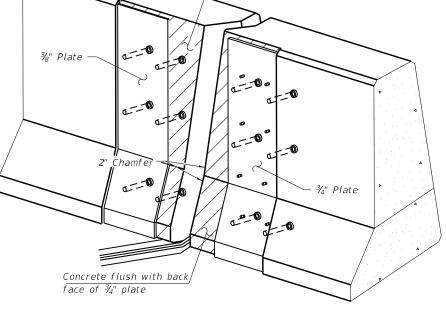


SHOWING ROLLED RAIL JOINT

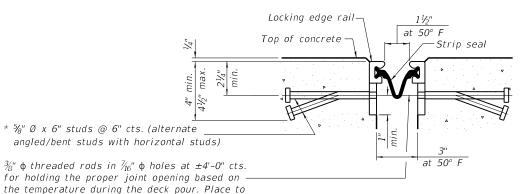
at 50° F



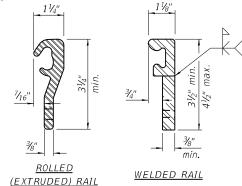
DETAIL A



### TRIMETRIC VIEW (Showing embedded plates only)

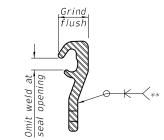


SHOWING WELDED RAIL JOINT



### LOCKING EDGE RAILS

\*\* Back gouge not required if complete joint penetration is verified by mock-up.



### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	267

### SECTION A-A

\* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

miss studs. All rods shall be burned, or sawed

off flush with the plates after concrete is set.

USER NAME =	DESIGNED -	BJD	REVISED -	
	CHECKED -	MGH	REVISED -	
PLOT SCALE =	DRAWN -	BJD	REVISED -	
PLOT DATE =	CHECKED -	MGH	REVISED -	

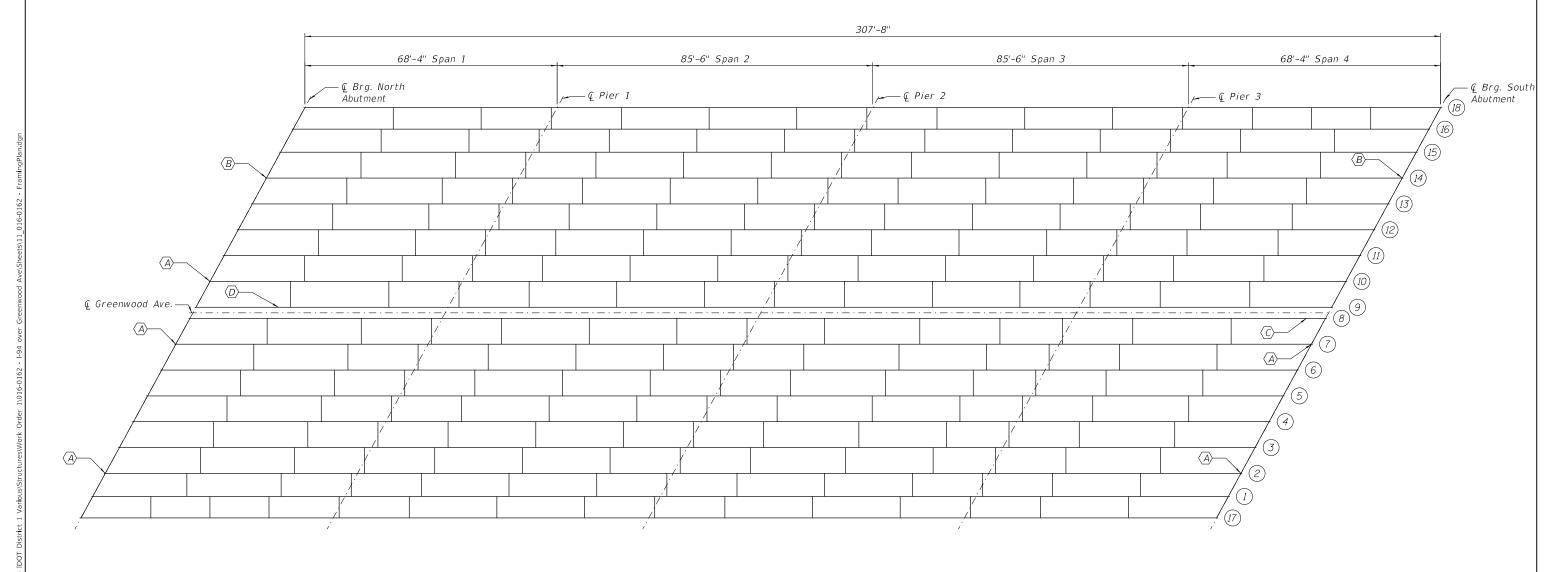
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

	JOINT STRIP SEAL NO. 016-0162
SHEET S08-10	OF S08-19 SHEETS

E.	SECT	ION		COUNTY	TOTAL SHEETS	SHEET NO.
4	(42-B-11-1)	BR, BJR	24	соок	761	674
				CONTRAC	Γ NO. 62	W87
		ILLINOIS	FFD. Al	D PROJECT		

12/9/2024 7:17:46 AM





### FRAMING PLAN

- (A)- Beam End Repairs, 40" (5 Locations). See Sheet S08-12 for details.
- (B)- Beam End Repairs, 20" (2 Locations). See Sheet S08-12 for details.
- ©- Beam 8 Top and Bottom Flange Repairs. See Sheet S08-12 for details.
- D- Beam 9 Top and Bottom Flange and Web Repairs (Includes repair to the web at corroded deck drain). See Sheet SO8-12 for details.

wsp	
-----	--

USER NAME =	DESIGNED -	BJD	REVISED -
	CHECKED -	MGH	REVISED -
PLOT SCALE =	DRAWN -	BJD	REVISED -
PLOT DATE =	CHECKED -	MGH	REVISED -

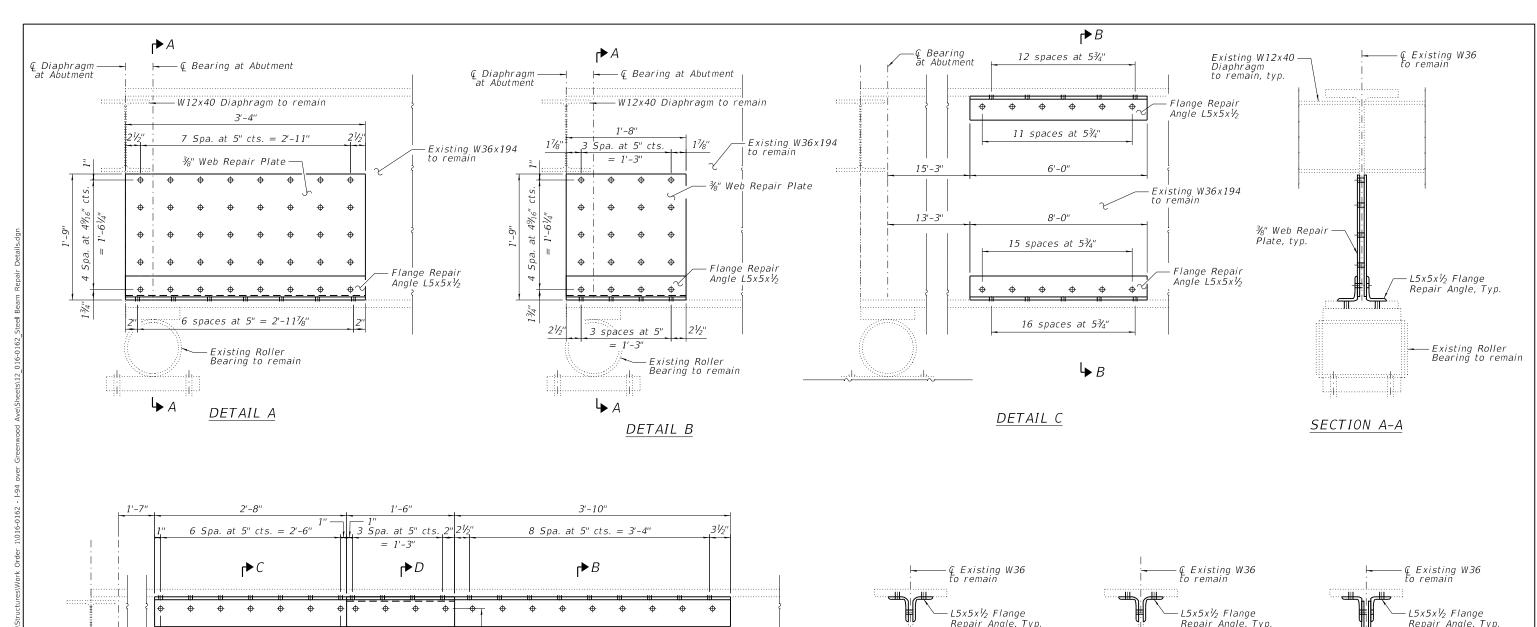
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

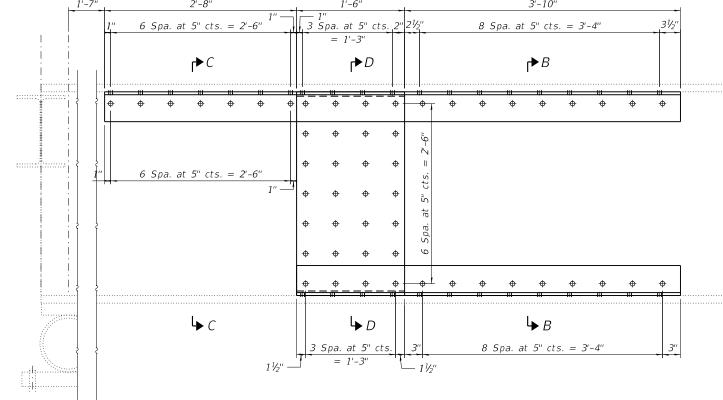
FRAMING PLAN STRUCTURE NO. 016-0162		SECTION
		(42-B-11-1) BR, BJ
3111001011L 110. 010-0102		
SHEET S08-11 OF S08-19 SHEETS		ILLINOI

A.I SECTION COUNTY SHEETS NO.

94 (42-B-11-1) BR, BJR 24 COOK 761 675

CONTRACT NO. 62W87





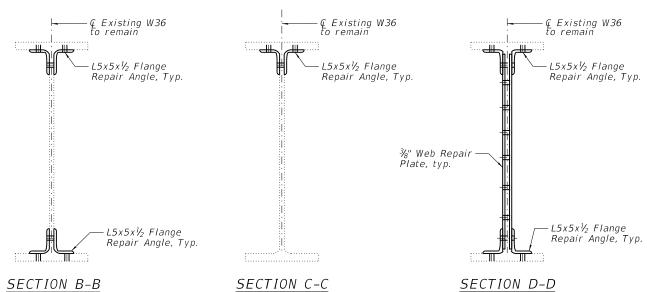
DETAIL D

REVISED -

REVISED -

DRAWN

CHECKED - MGH



## NOTES:

**STRUCTURE NO. 016-0162** 

SHEET S08-12 OF S08-19 SHEETS

- Distance from top of main girder to top of diaphrgam to be field verified prior to ordering material.
- 2. Contractor to field verify diaphragm location and bolt hole locations before ordering material.
- 3. No welds are to be installed on the back/end of the beam. All welds are included in the cost of Structural Steel Repair.
- 4. Contractor to field verify hole locations before ordering material. Contractor can elect to field drill holes in repair plates.

<u>BILL OF M</u>	<u>IATERIAL</u>	
ITEM	UNIT	QUANTITY
Structural Steel Repair	Pound	2.010

<u> </u>		
ITEM	UNIT	QUANTITY
Structural Steel Repair	Pound	2,010

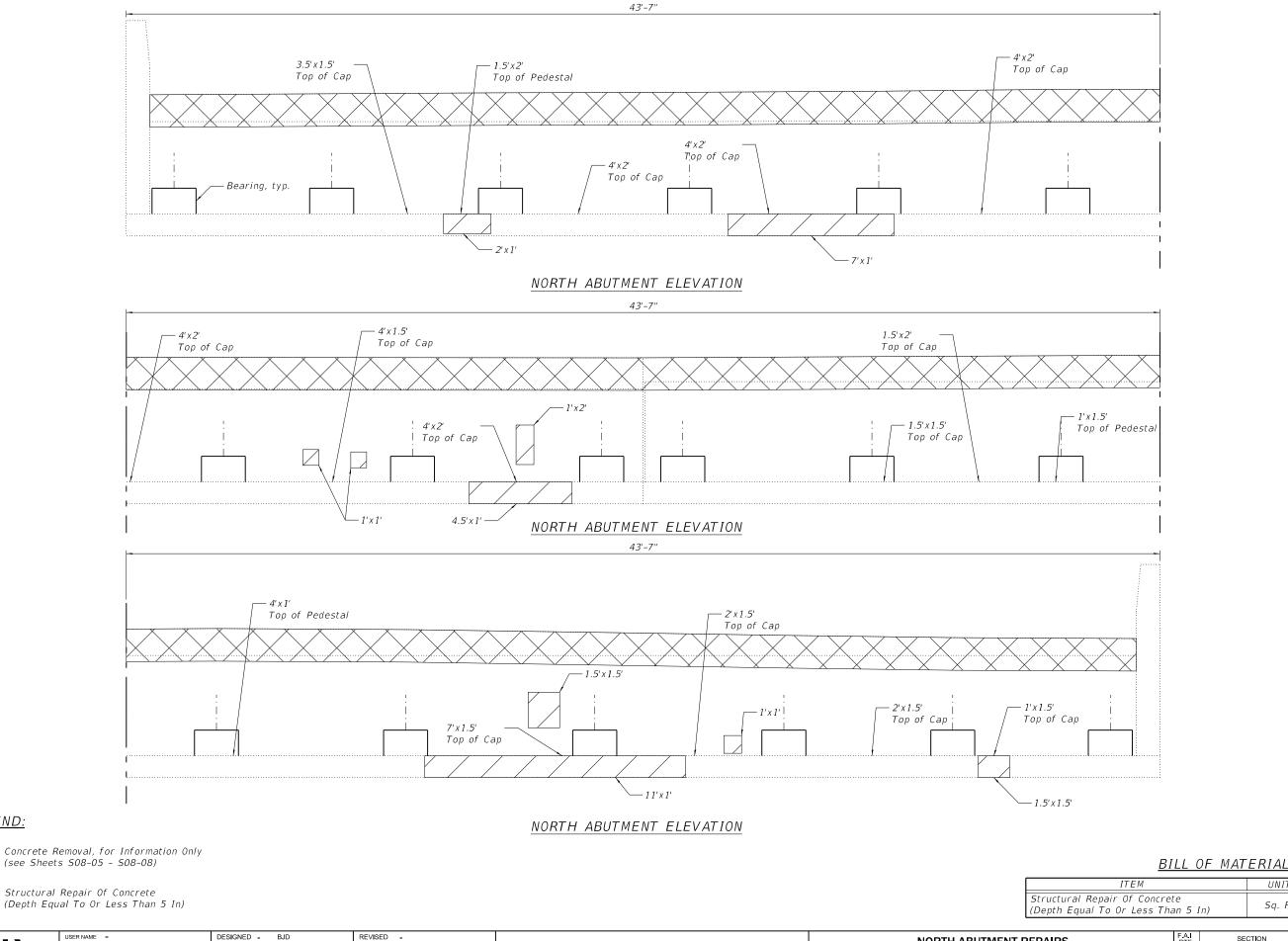
DESIGNED - BJD REVISED -USER NAME = SECTION STEEL BEAM REPAIR DETAILS **STATE OF ILLINOIS** CHECKED - MGH REVISED -

**DEPARTMENT OF TRANSPORTATION** 

1/23/2025 12:23:08 PM

PLOT DATE =

COUNTY (42-B-11-1) BR. BJR 24 соок 761 676 CONTRACT NO. 62W87



STATE OF ILLINOIS

**DEPARTMENT OF TRANSPORTATION** 

12/9/2024 7:17:56 AM

LEGEND:

PLOT DATE =

CHECKED - MGH

DRAWN - BJD

CHECKED - MGH

REVISED -

REVISED -

REVISED -

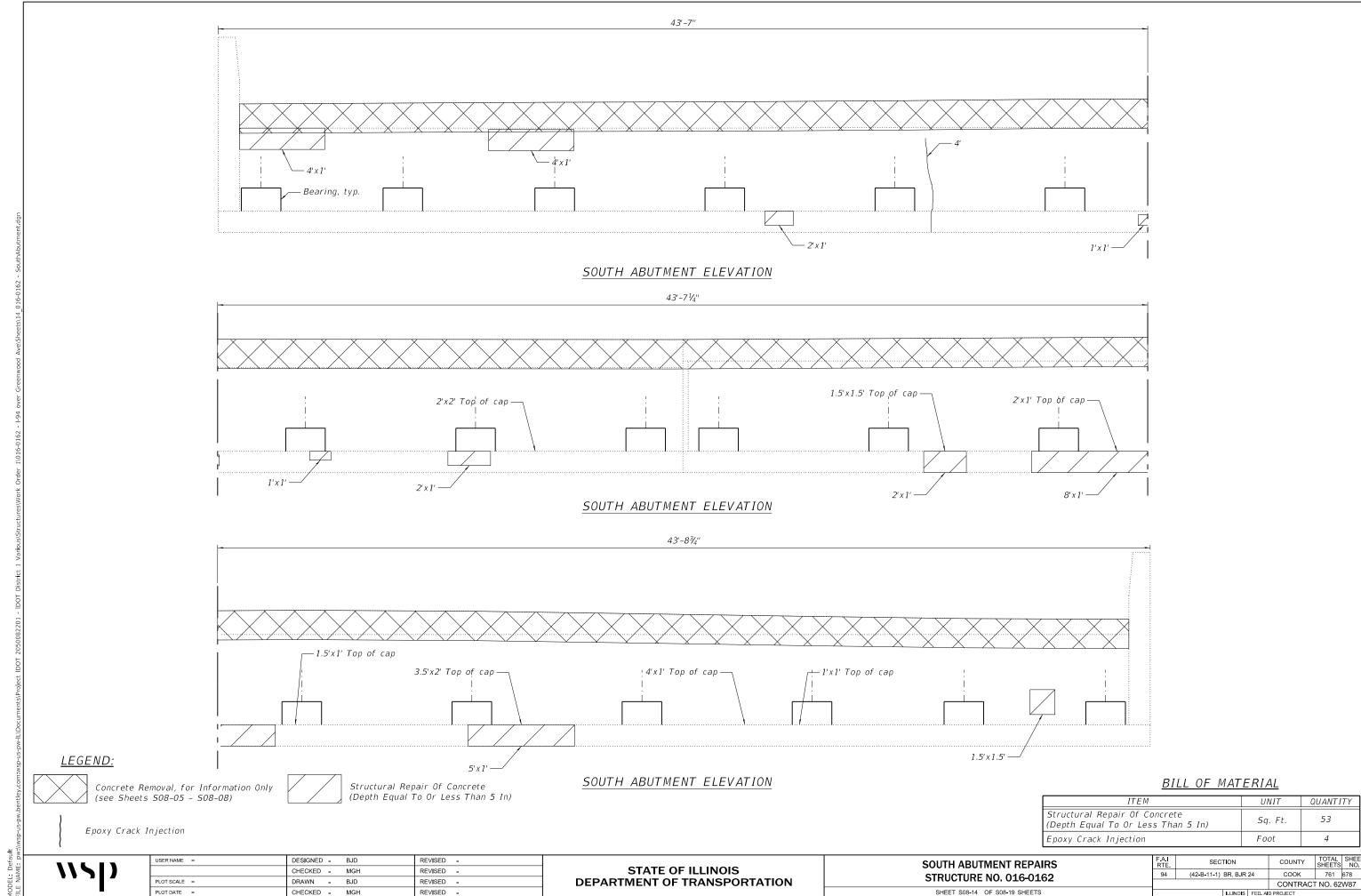
SECTION COUNTY NORTH ABUTMENT REPAIRS (42-B-11-1) BR, BJR 24 COOK 761 677 STRUCTURE NO. 016-0162 CONTRACT NO. 62W87 SHEET S08-13 OF S08-19 SHEETS

UNIT

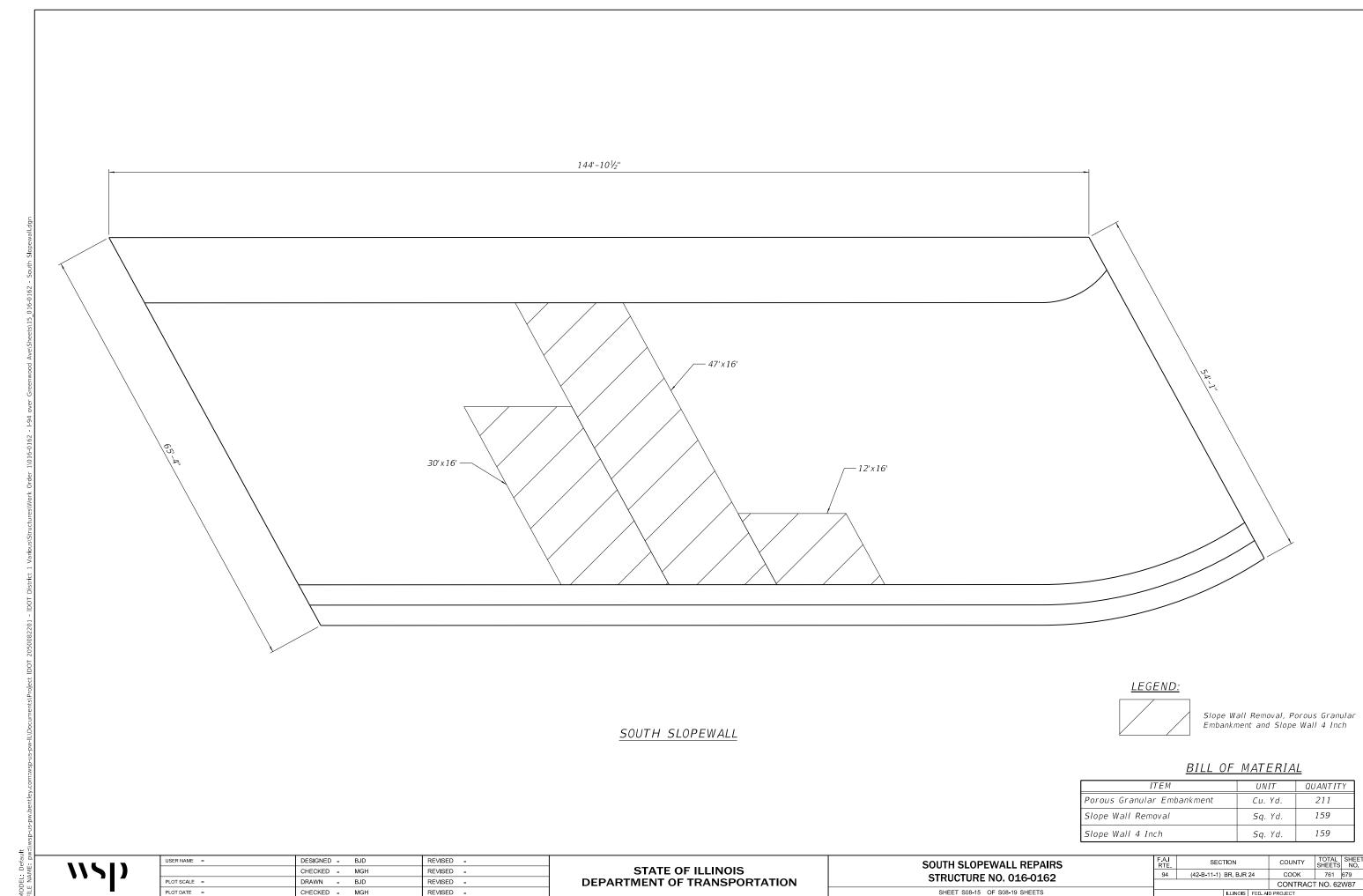
Sq. Ft.

QUANTITY

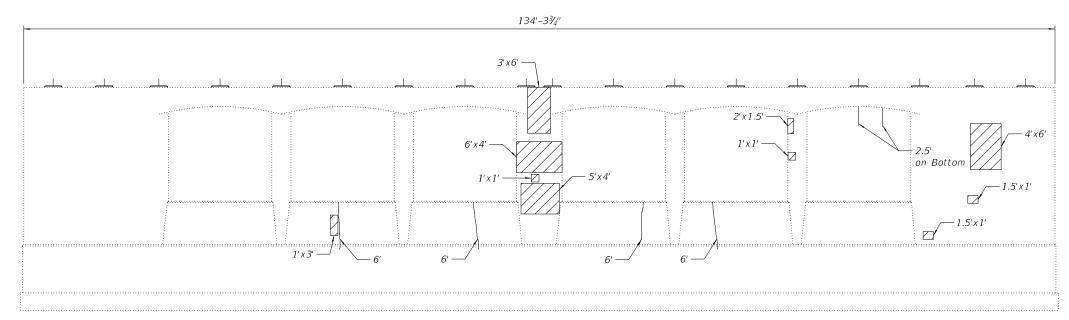
115



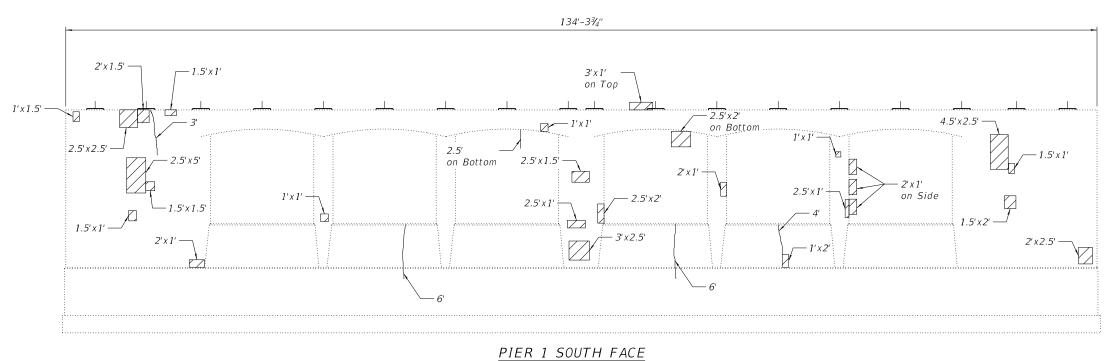
12/9/2024 7:18:00 AM



12/9/2024 7:18:03 AM



PIER 1 NORTH FACE



# <u>LEGEND:</u>

Structural Repair Of Concrete (Depth Equal To Or Less Than 5 In)

Epoxy Crack Injection

### <u>BILL OF MATERIAL</u>

ITEM	UNIT	QUANTITY
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 In)	Sq. Ft.	190.5
Epoxy Crack Injection	Foot	50.5



USER NAME =	DESIGNED -	BJD	REVISED -	
	CHECKED -	MGH	REVISED -	
PLOT SCALE =	DRAWN -	BJD	REVISED -	
PLOT DATE =	CHECKED -	MGH	REVISED -	

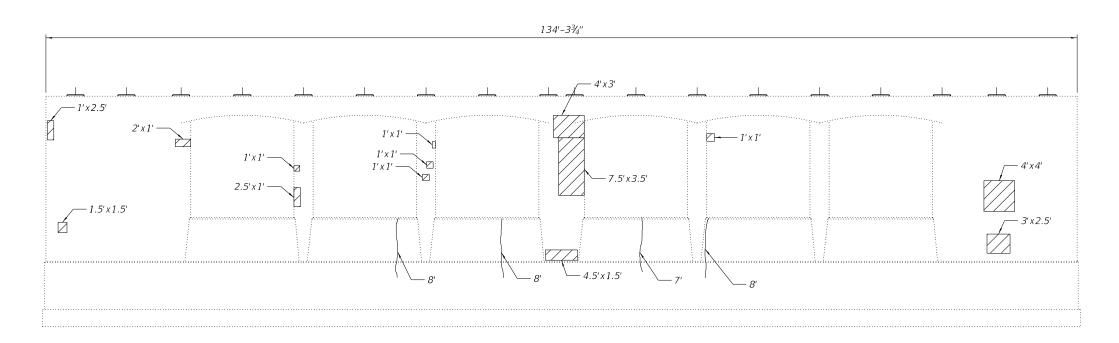
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PIER 1 REPAIRS
STRUCTURE NO. 016-0162

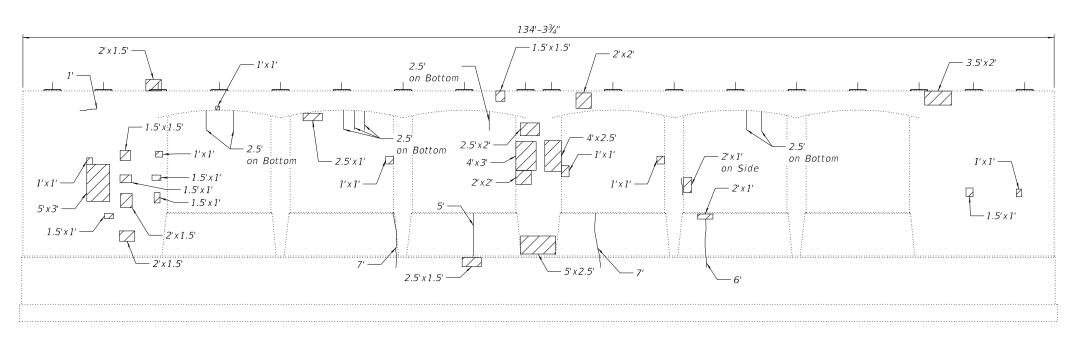
SHEET S08-16 OF S08-19 SHEETS

A.I SECTION COUNTY TOTAL SHEETS NO.
94 (42-B-11-1) BR, BJR 24 COOK 761 680

CONTRACT NO. 62W87



### PIER 2 NORTH FACE



### <u>LEGEND:</u>



Epoxy Crack Injection

### PIER 2 SOUTH FACE

### BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 In)	Sq. Ft.	190.5
Epoxy Crack Injection	Foot	77



USER NAME =	DESIGNED -	BJD	REVISED -
	CHECKED -	MGH	REVISED -
PLOT SCALE =	DRAWN -	BJD	REVISED -
PLOT DATE =	CHECKED -	MGH	REVISED -

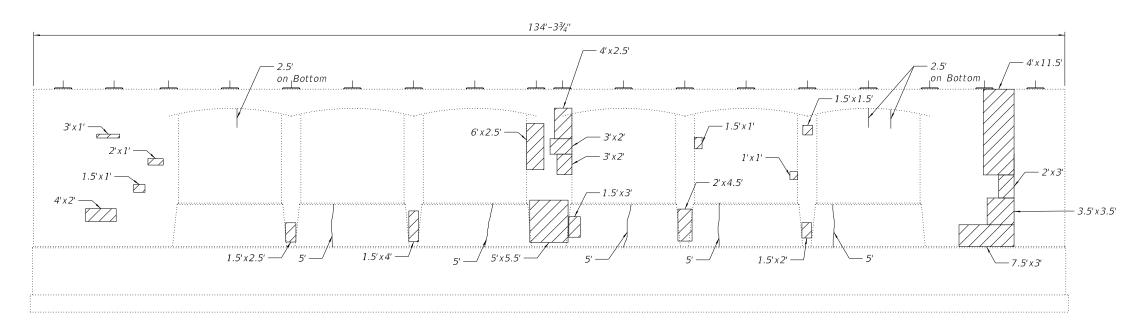
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PIER 2 REPAIRS
STRUCTURE NO. 016-0162

SHEET S08-17 OF S08-19 SHEETS

12/9/2024 7:18:10 AM

### PIER 3 NORTH FACE



### PIER 3 SOUTH FACE

### LEGEND:



Structural Repair Of Concrete (Depth Equal To Or Less Than 5 In)

Epoxy Crack Injection

# <u>BILL OF MATERIAL</u>

ITEM	UNIT	QUANTITY
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 In)	Sq. Ft.	322.5
Epoxy Crack Injection	Foot	85.5



USER NAME =	DESIGNED - BJD	REVISED -
	CHECKED - MGH	REVISED -
PLOT SCALE =	DRAWN - BJD	REVISED -
PLOT DATE =	CHECKED - MGH	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PIER 3 REPAIRS
STRUCTURE NO. 016-0162
SHEET S08-18 OF S08-19 SHEETS

12/9/2024 7:18:13 AM

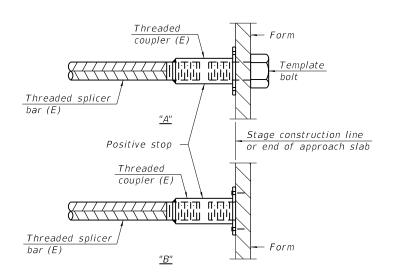
### STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

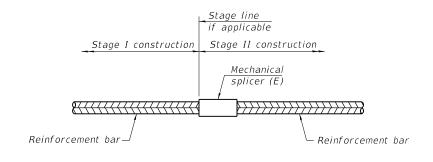
Location	Bar size	No. assemblies required	Minimum Iap length
Abutment	#6	12	4'-5"
Deck	#5	52	3'-1"



### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

SHEET S08-19 OF S08-19 SHEETS

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for

alternatives.

BSD-1

2-1-2023



USER NAME =	DESIGNED -	BJD	REVISED -
	CHECKED -	MGH	REVISED -
PLOT SCALE =	DRAWN -	BJD	REVISED -
PLOT DATE =	CHECKED -	MGH	REVISED -

Bench Mark: "X" scribed in chiseled box on top of concrete barrier wall at the southeast corner of bridge structure for Westbound FAI-94 over the Little Calumet River Elev. 606.96 (Assumed Local Datum)

Existing Structure: S.N. 016-0163 originally constructed in 1947 as a 3 span structure. In 1981, the structure was widened with a new deck and new beam lines on both sides of the superstructure. All superstructure was cleaned and painted. In 2009, the approach slabs, deck joints at abutments and deck overlay were replaced. The deck joint between the center parapets was resealed. Abutment bearings were replaced with elastomeric bearings. The steel beam ends and end diaphragms at abutments were cleaned and painted.

Traffic to be maintained utilizing staged construction.

81'-0"

G Brg. N. Abut.

Sta 580+02.50

— Plug\_Existing

Floor Drains

Elev.

--

-Bk. of N. Abut

Sta 580+00.40

Elev.

Plug Existing

Floor Drain

30'-0"

Approacl

Slah

Plug Existing-

21/3" Deck Scarification and

2¾" Latex Concrete Overlay.

and apply Protective Coat.

Perform 1/4" Diamond Grinding

Floor Drain

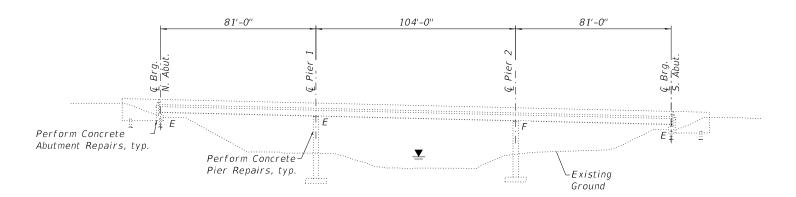
 $\leftarrow$ 

 $\triangleleft$ 

 $\Rightarrow$ 

 $\Longrightarrow$ 

2'-11/4"



### ELEVATION

270'-21/3" Back to Back Abutments

104'-0"

Limits of Protective Shield

18°0′0″/

Stage Construction

typ.

### DESIGN SPECIFICATIONS

2002 AASHTO Standard Specifications for Highway Bridges, 17th Edition

### DESIGN STRESSES

### FIELD UNITS (New Construction)

f'c = 4,000 psi (Superstructure)f'c = 3,500 psi (Substructure)

fy = 60,000 psi (Reinforcement)fy = 50,000 psi (M270 Grade 50)

### FIELD UNITS (2009 Repairs)

f'c = 3,500 psi (Concrete - Deck Slab) fv = 60.000 psi (Reinforcement - Deck Slab)

fs = 36,000 psi (M270 Grade 36)

### FIELD UNITS (1980 Rehab)

f'c = 3,500 psi (Concrete - Deck Slab)

fc = 1,400 psi (Substructure)

fy = 60,000 psi (Reinforcement - Deck Slab)

fs = 20,000 psi (Reinforcement- Substructure)fs = 20,000 psi (Structural Steel) (M183 Grade 36)

### FIELD UNITS (1946 Original Construction)

fs = 18,000 psi (A7 Struct. Steel) f'c = 1,200 psi (Superstructure)

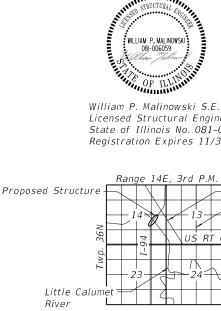
fy = 20,000 psi (Reinforcement)

f'c = 800 psi (Concrete - Substructure)

### LOADING HL-93

### SCOPE OF WORK

- Scarify 21/2" of the existing bridge deck.
- Repair deck with partial and full depth patches.
- Repair of bridge approach slabs.
- Removal and replacement of expansion joints at the abutments.
- Clean the existing floor drains and plug the floor drains within 10 feet of abutments.
- Install enclosed drainage system.
- Install a 2¾" latex concrete overlay.
- Perform 1/4" Diamond Grinding to the top of bridge deck and abutment hatch block.
- Perform Bridge Deck Grooving (Longitudinal) on traffic lanes.
- Apply Protective Coat to the top of the reconstructed transverse joints, top of new Latex Concrete Overlay, and top and inside face of parapets.
- 11. Repair steel beams at North Abutment.
- 12. Perform Substructure repairs.



Licensed Structural Engineer State of Illinois No. 081-006059 Registration Expires 11/30/2026



US RT 6

Greenwood Ave

Paxton Ave

I-94 OVER LITTLE CALUMET RIVER FAI 94 SEC. 1975-079-BR COOK COUNTY STATION 581+35.50 SN 016-0163

Sta 560+89.44 '. 610.57 PVT Sta 580+89.44 Elev. 610.57 EXISTING PROFILE GRADE FAI-94

SHEET S09-01 OF S09-15 SHEETS

∟Structure

The Profile Grade shows the final grade after grinding.

LVC = 2000'

USER NAME =	DESIGNED -	BJD	REVISED -
	CHECKED -	MGH	REVISED -
PLOT SCALE =	DRAWN -	BJD	REVISED -
PLOT DATE =	CHECKED -	MGH	REVISED -

-Q Pier 1

Line

Flev.

-Sta 580+83.50

Stage Construction-

PLAN

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

30'-0"

Approach

Slab

2'-11/4"

81'-0"

Plug Existing-

Floor Drain

€ Brg. S. Abut.

Sta 582+68.51

Plua Existina

Floor Drains

Elev.

∽@ Pier 2

Elev.

Sta 581+87.50

Elev.

Bk. of S. Abut.-

Sta 582+70.61

... Plug Existing-

Floor Drain 3'-2"

Тур.

Dimensioned along G I-94

PG WB Lanes

+3.00%

-€ I-94

-PG EB Lanes

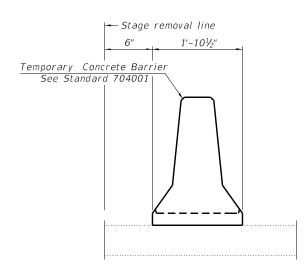
SECTION COUNTY (42-B-11-1) BR, BJR 24 COOK 761 684 CONTRACT NO. 62W87

3/10/2025 1:14:07 PM

18'-( Media

### GENERAL NOTES

- 1. No field welding is permitted except as specified in the contract documents.
- 2. Reinforcement bars designated (E) shall be epoxy coated.
- 3. Plan dimensions and details relative to the existing structure have been taken from existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 4. Existing reinforcement shall be cleaned, straightened and incorporated into the new construction. Cost included with Concrete Removal.
- 5. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to address the presence of lead on this project.
- 6. Cleaning and painting of the existing structural steel shall be done under a separate painting contract.
- 7. Any reinforcement bars that are damaged during concrete removal operations shall be repaired or replaced using an approved bar splicer or anchorage system. Cost incidental to "Concrete Removal".
- 8. Joint openings shall be adjusted according to Article 520.04 of the Standard Specifications when the joint concrete is poured at an ambient temperature other than 50°F.
- 9. All exposed concrete edges shall have a  $\frac{3}{4}$ " x 45° chamfer except where shown otherwise.
- 10. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding ¾ inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 11. Up to  $\frac{1}{4}$  inch to be ground off the bridge deck and the bridge approach slabs. The Profile Grade shows the final grade after grinding.



### EXISTING SLAB

### SECTIONS THRU SLAB OR DECK BEAM

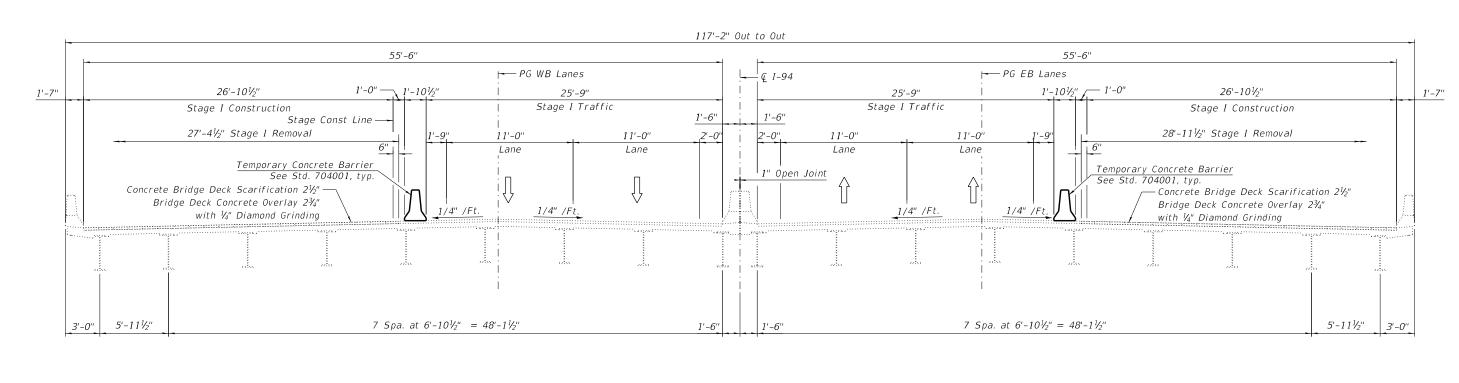
### INDEX OF SHEETS

509-01	General Plan and Elevation
<i>509-02</i>	General Data
S09-03	Construction Staging – 1
509-04	Construction Staging – 2
S09-05	Deck Repair Plan
509-06	North Abutment Expansion Joint Reconstruction Plan
<i>509-07</i>	South Abutment Expansion Joint Reconstruction Plan
<i>509-08</i>	Abutment Expansion Joint Reconstruction Details - 1
<i>509-09</i>	Abutment Expansion Joint Reconstruction Details - 2
509-10	Preformed Joint Strip Seal
509-10A	Framing Plan
S09-10B	Steel Beam Repair Details
S09-11	Abutment Repairs
<i>509-12</i>	Pier 1 Repairs
S09-13	Pier 2 Repairs
509-14	Enclosed Drainage System
S09-15	Bar Splicer Assembly and Mechanical Splicer Details

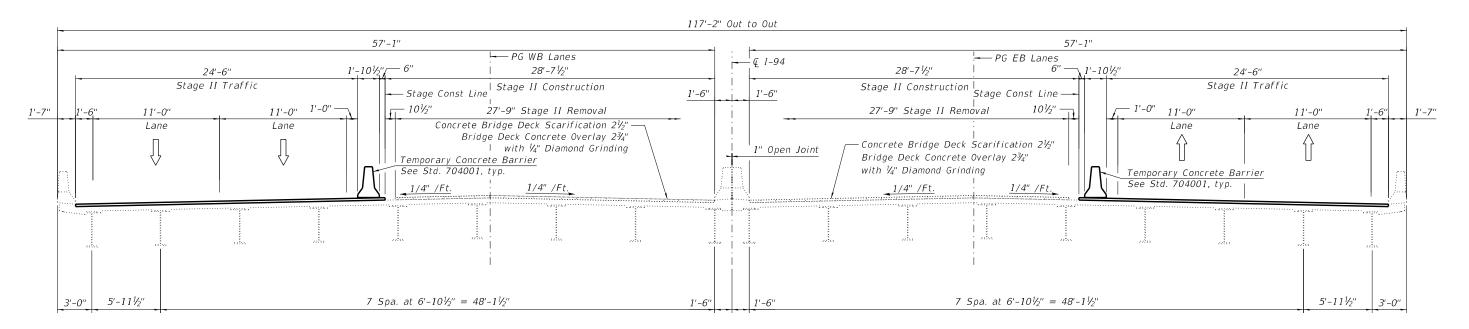
### TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER- STRUCTURE	SUB- STRUCTURE	TOTAL
Concrete Removal	Cu. Yd.	23	9	32
Protective Shield	Sq. Yd.	1,354		1,354
Concrete Structures	Cu. Yd.		9.0	9.0
Concrete Superstructure	Cu. Yd.	23.0		23.0
Protective Coat	Sq. Yd.	4,085		4,085
Reinforcement Bars, Epoxy Coated	Pound	4,460	1,130	5,590
Bar Splicers	Each	52	12	64
Preformed Joint Strip Seal	Foot	247		247
Epoxy Crack Injection	Foot		38	38
Plug Existing Floor Drains	Each	8		8
Bridge Deck Grooving (Longitudinal)	Sq. Yd.	2,972		2,972
Bridge Drainage System	Each	1		1
Approach Slab Repair (Full Depth)	Sq. Yd.	1		1
Approach Slab Repair (Partial Depth)	Sq. Yd.	53		53
Structural Steel Repair	Pound	300		300
Bridge Latex Concrete Overlay 2 3/4"	Sq. Yd.	3,333		3,333
Cleaning Bridge Seats	Sq. Ft.		1,346	1,346
Clean Existing Inlets	Each	40		40
Bridge Deck Scarification 2 1/2"	Sq. Yd.	3,333		3,333
Structural Repair of Concrete (Depth Equal to	C F4		C.F.	C.F.
or Less Than 5 Inches)	Sq. Ft.		65	65
Deck Slab Repair (Full Depth - Type I)	Sq. Yd.	59		59
Deck Slab Repair (Full Depth - Type II)	Sq. Yd.	95		95
Expansion Joint (Special)	Foot	271		271
Diamond Grinding (Bridge Section)	Sq. Yd.	3,303		3,303

USER NAME =	DESIGNED -	BJD	REVISED -	Γ
	CHECKED -	MGH	REVISED -	
PLOT SCALE =	DRAWN -	BJD	REVISED -	
PLOT DATE =	CHECKED -	MGH	REVISED -	



# STAGE I CONSTRUCTION (Looking South)

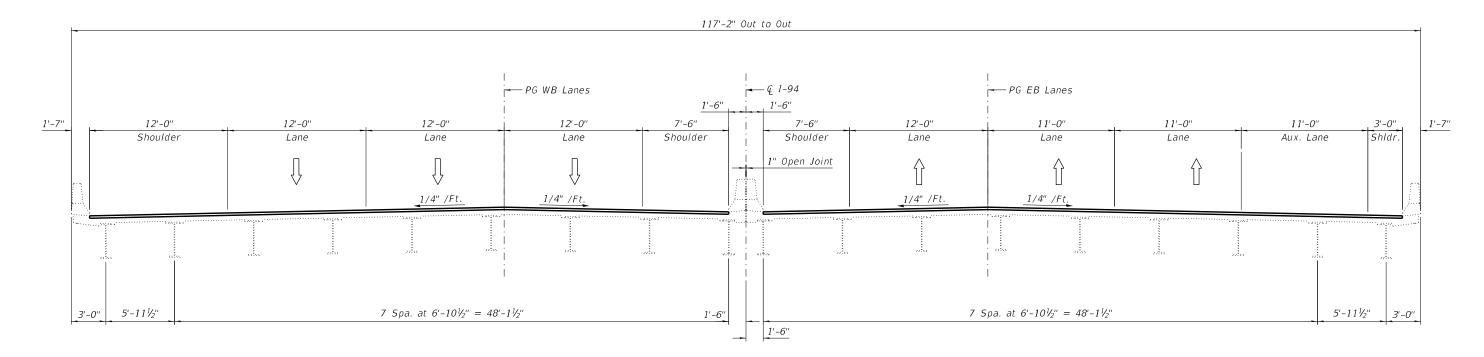


# STAGE II CONSTRUCTION (Looking South)

WSD
-----

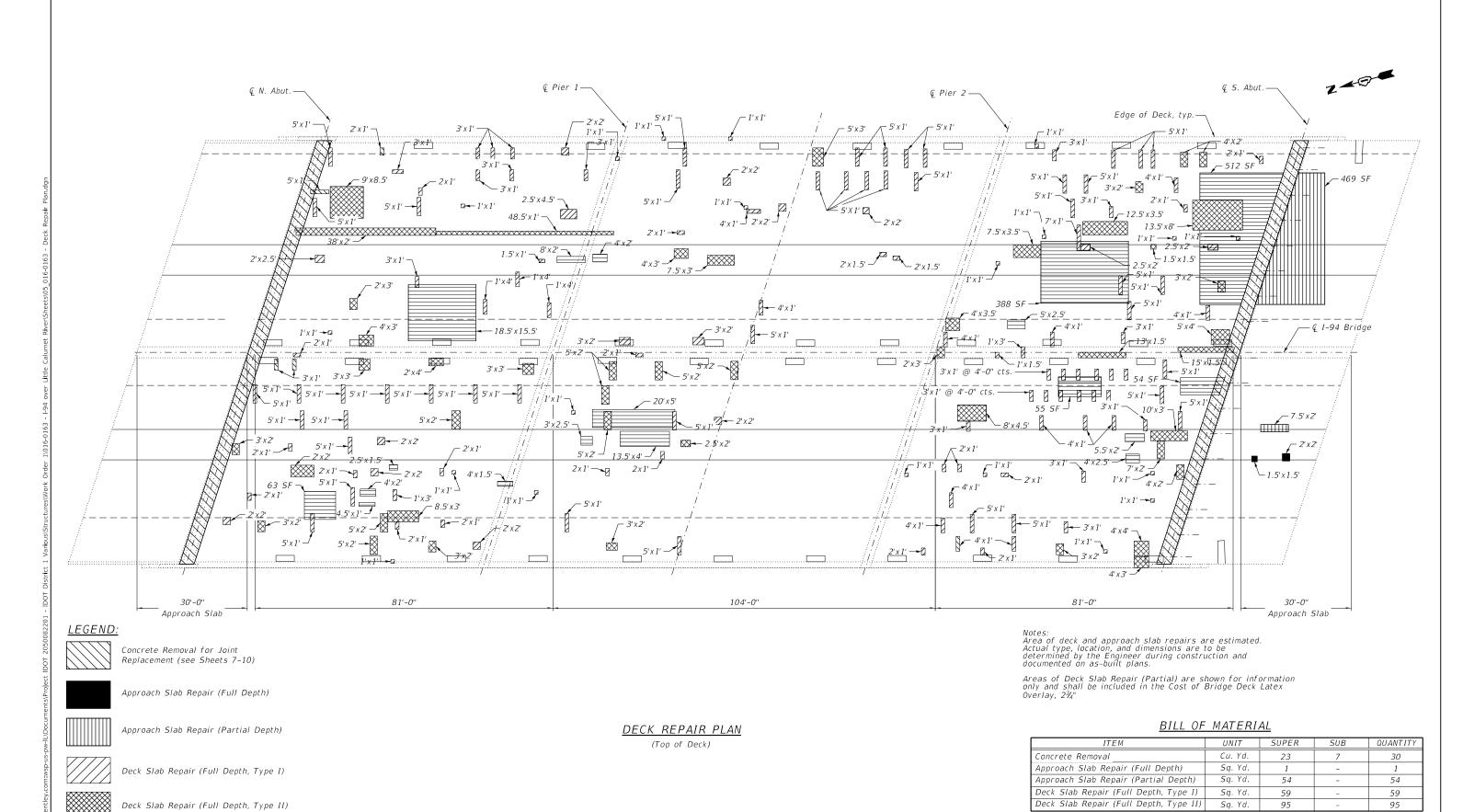
CONSTRUCTION STAGING - 1 STRUCTURE NO. 016-0163	
SHEET S09-03 OF S09-15 SHEETS	_

LI E.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
4	(42-B-11-1) BR, BJR 24		соок	761	686
			CONTRACT	NO. 62	W87
	ILLINOIS FED AID PROJECT				



FINAL CROSS SECTION (Looking South)

12/9/2024 7:20:28 AM



**STATE OF ILLINOIS** 

**DEPARTMENT OF TRANSPORTATION** 

SECTION

(42-B-11-1) BR, BJR 24

**DECK REPAIR PLAN** 

**STRUCTURE NO. 016-0163** 

SHEET S09-05 OF S09-15 SHEETS

COUNTY

COOK

761 688

CONTRACT NO. 62W87

MODEL: Default

12/9/2024 7:20:32 AM

Deck Slab Repair (Partial), For Information Only

USER NAME =

LOT SCALE =

DESIGNED - BJD

CHECKED - MGH

BJD

DRAWN

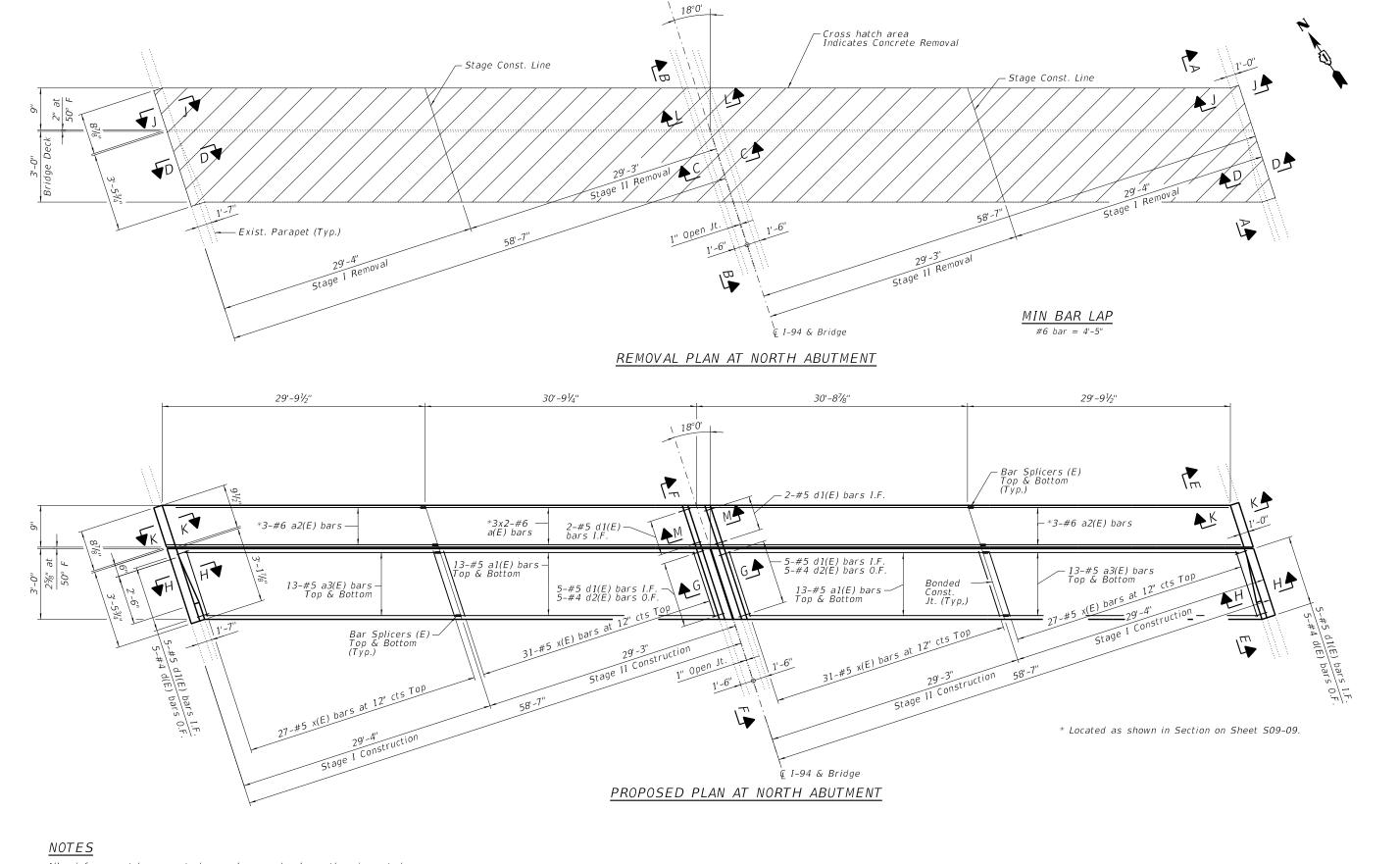
CHECKED -

REVISED -

REVISED -

REVISED -

REVISED -



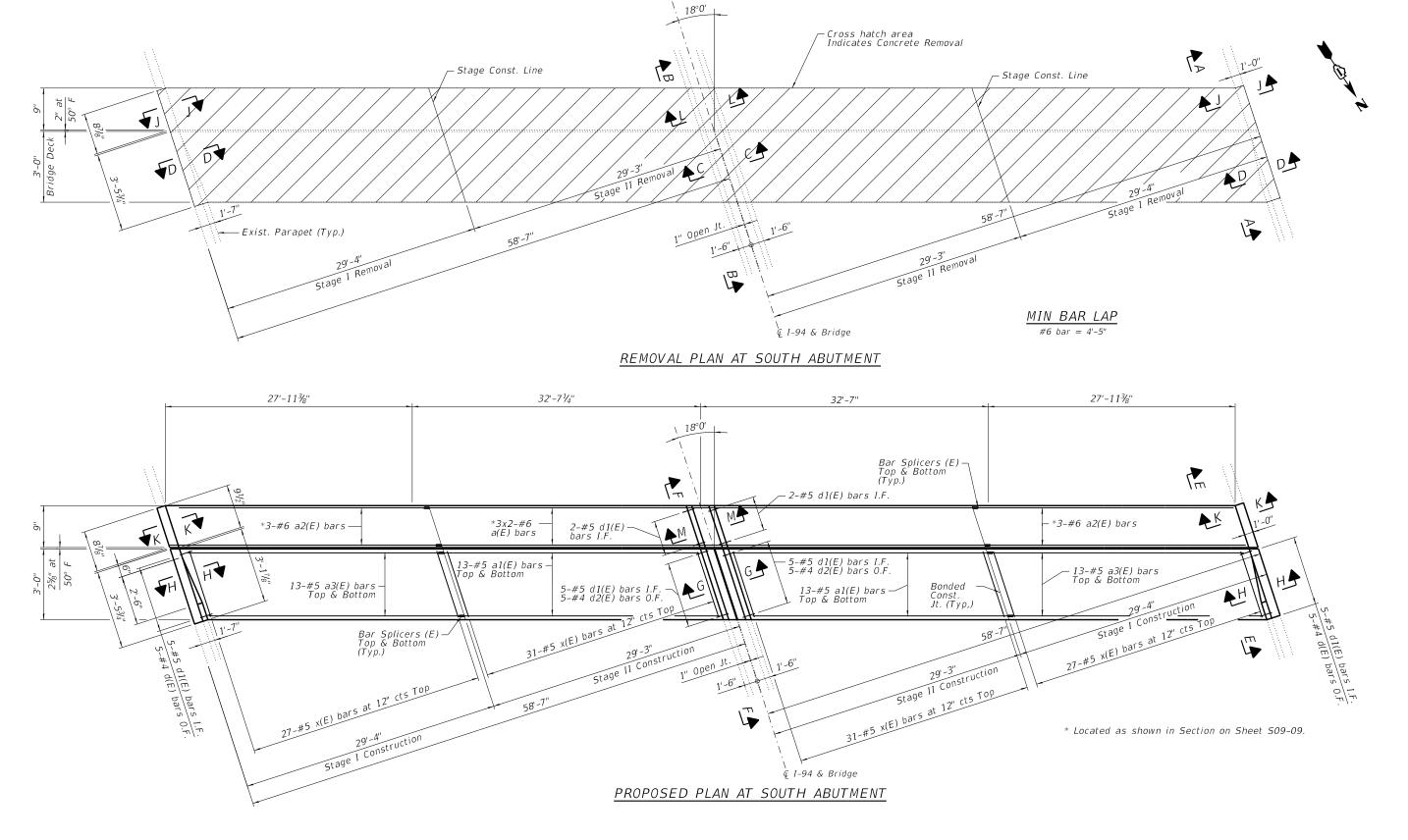
All reinforcement bars are to be evenly spaced unless otherwise noted. Reinforcement bars designated (E) shall be epoxy coated.

O.F. denotes Outside Face. I.F. denotes Inside Face.

For Bill of Material and bar details see Sheet S09-10.

For additional abutment backwall demolition and reconstruction see Sheet S09-09.

NORTH ABUTMENT EXPANSION JOINT RECONSTRUCTION PLAN	F.A.I RTE	SECTION
STRUCTURE NO. 016-0163	94	(42-B-11-1) BR, BJI
STRUCTURE NO. 010 0100		
CHEET COLOR OF COLORES		



### NOTES

All reinforcement bars are to be evenly spaced unless otherwise noted. Reinforcement bars designated (E) shall be epoxy coated.

O.F. denotes Outside Face. I.F. denotes Inside Face.

For Bill of Material and bar details see Sheet S09-10.

For additional abutment backwall demolition and reconstruction see Sheet S09-09.

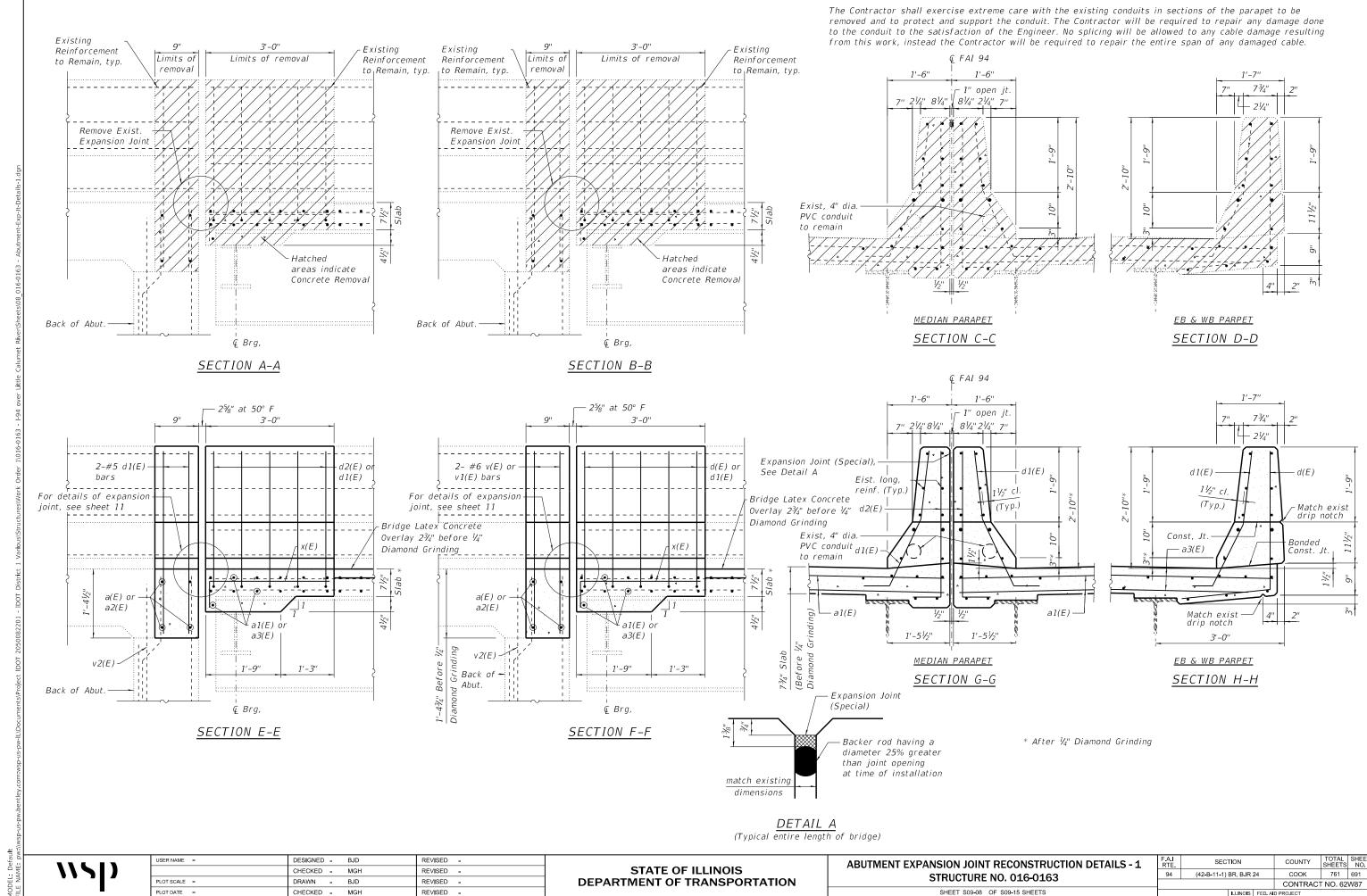
USER NAME =	DESIGNED - BJD	REVISED -
	CHECKED - MGH	REVISED -
PLOT SCALE =	DRAWN - BJD	REVISED -
PLOT DATE =	CHECKED - MGH	REVISED -

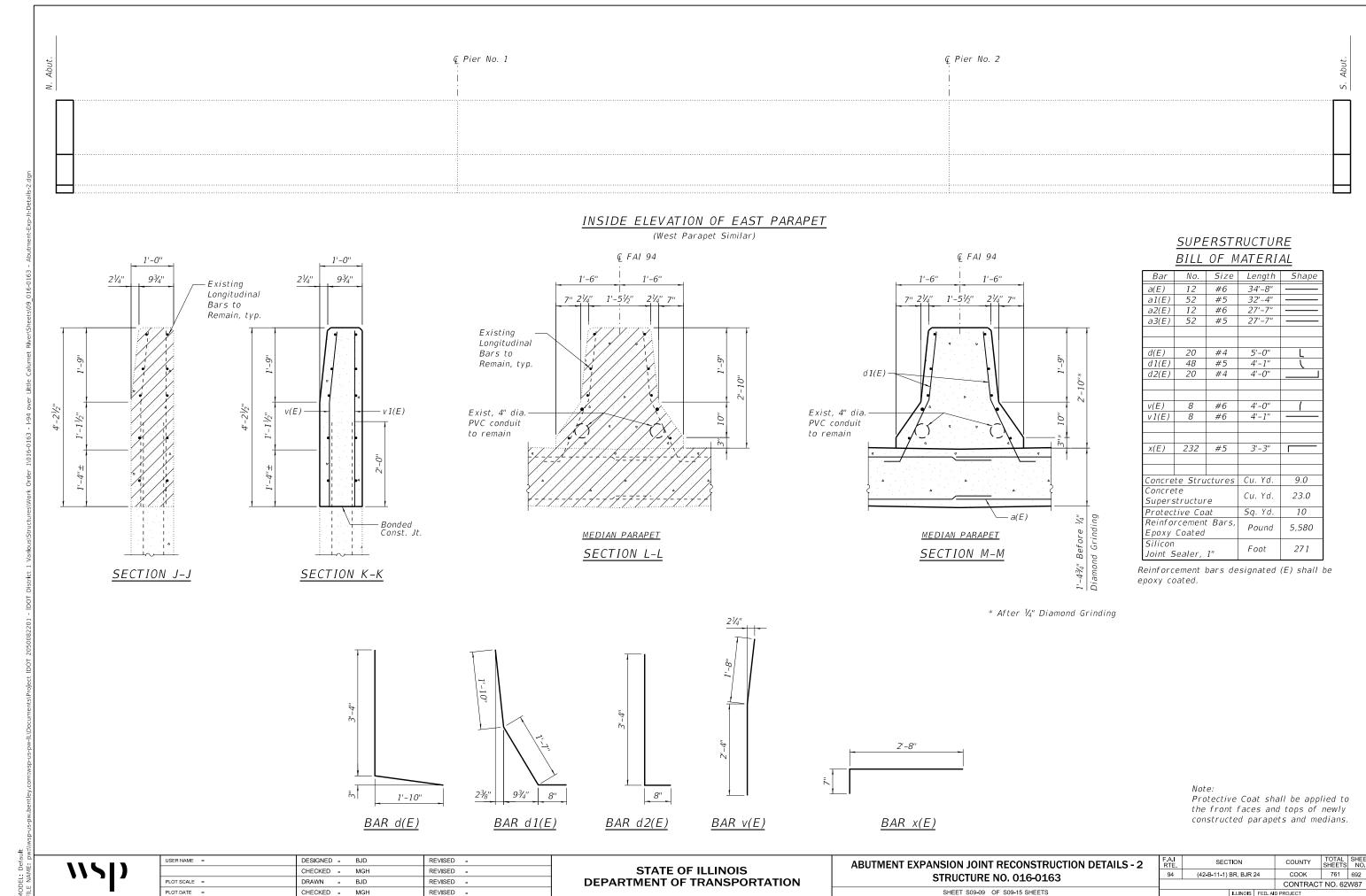
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SOUTH ABUTMENT EXPANSION JOINT RECONSTRUCTION PLAN				
STRUCTURE NO. 016-0163				
SHEET S09-07 OF S09-15 SHEETS	H			

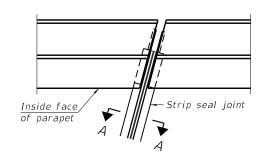
SECTION COUNTY (42-B-11-1) BR, BJR 24 COOK 761 690 CONTRACT NO. 62W87

12/9/2024 7:20:39 AM

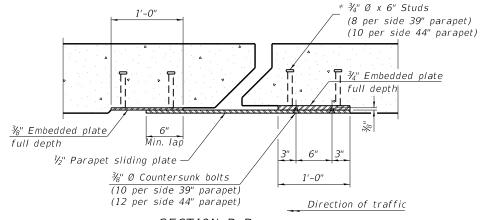




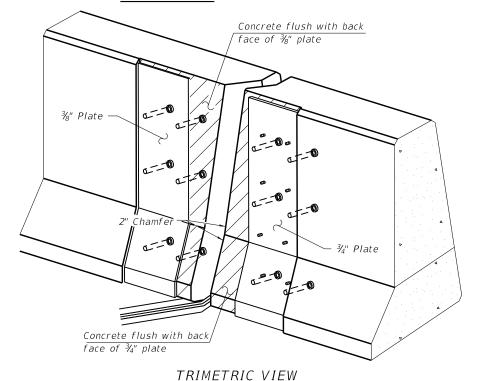
12/9/2024 7:20:46 AM



### PLAN AT PARAPET



### SECTION B-B



### Notes:

The strip seal shall be made continuous and shall have a minimum thickness of ½". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

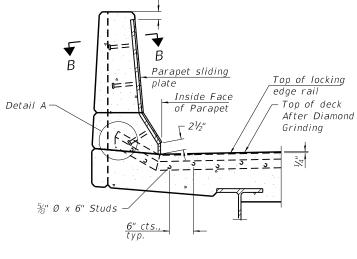
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be  $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage study included with Preformed Joint Strip Seal.

39" constant slope barrier shown, 44" constant slope barrier similar as noted.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

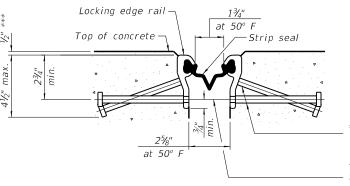


2" Max. -

### SECTION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

### DETAIL A



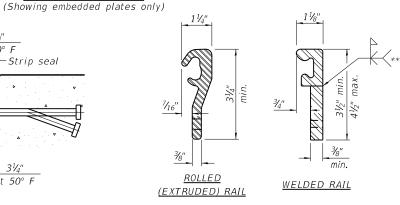
SHOWING ROLLED RAIL JOINT

# Locking edge rail Top of concrete \* $\frac{134''}{at 50^{\circ}}$ F Strip seal \* $\frac{18}{8}$ Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs) \* $\frac{134''}{at 50^{\circ}}$ F \* $\frac{134''}{at 50^{\circ}}$ F

%''  $\phi$  threaded rods in  $\%''_6$   $\phi$  holes at  $\pm 4'$ -0" cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed

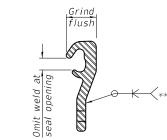
off flush with the plates after concrete is set.

### SHOWING WELDED RAIL JOINT



### LOCKING EDGE RAILS

\*\* Back gouge not required if complete joint penetration is verified by mock-up.



### LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

### BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	247

### SECTION A-A

- \* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.
- \*\*\* Before 1/4" Diamond Grinding

wsp

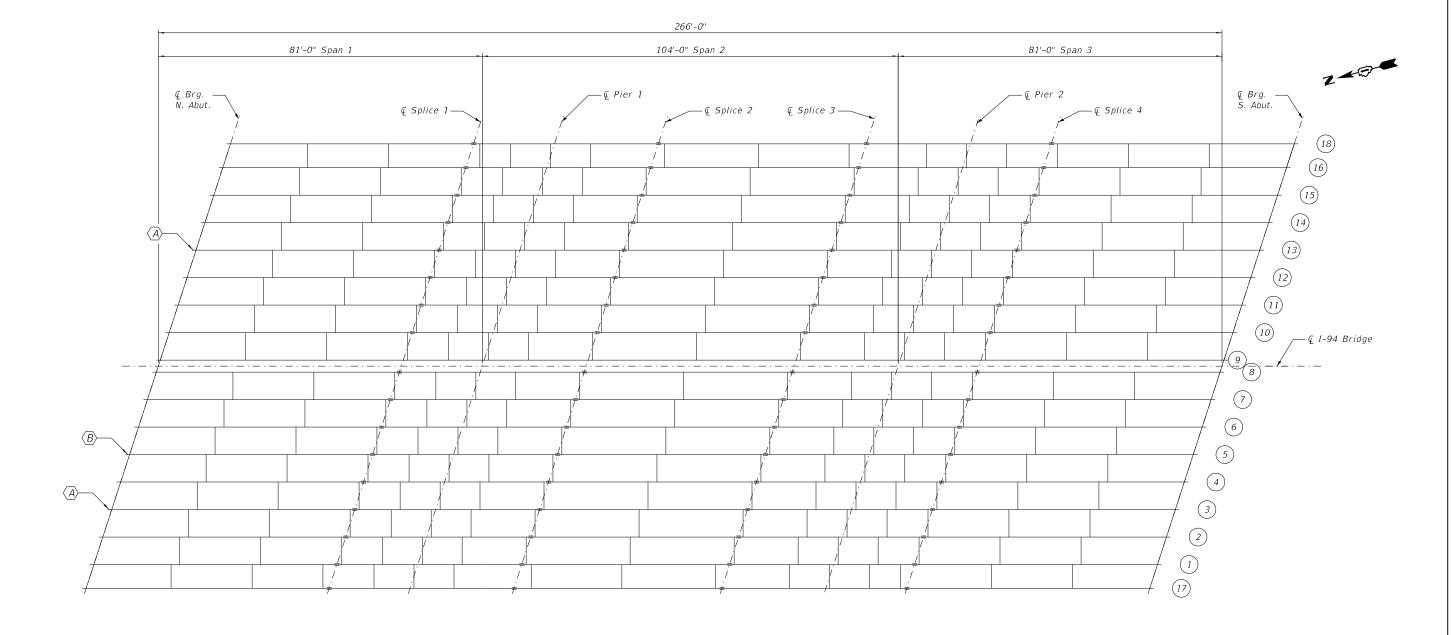
USER NAME =	DESIGNED -	BJD	REVISED -
	CHECKED -	MGH	REVISED -
PLOT SCALE =	DRAWN -	BJD	REVISED -
PLOT DATE =	CHECKED -	MGH	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

MODIFIED PREFORMED JOINT STRIP S STRUCTURE NO. 016-0163	SEAL
SHEET S09-10 OF S09-15 SHEETS	

.I E.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
4	(42-B-11-1) BR, BJR 24		соок	761	693
CONTRACT NO				Γ NO. 62	W87
	III INOIC	EED M	D BBO JECT		

12/9/2024 7:20:49 AM

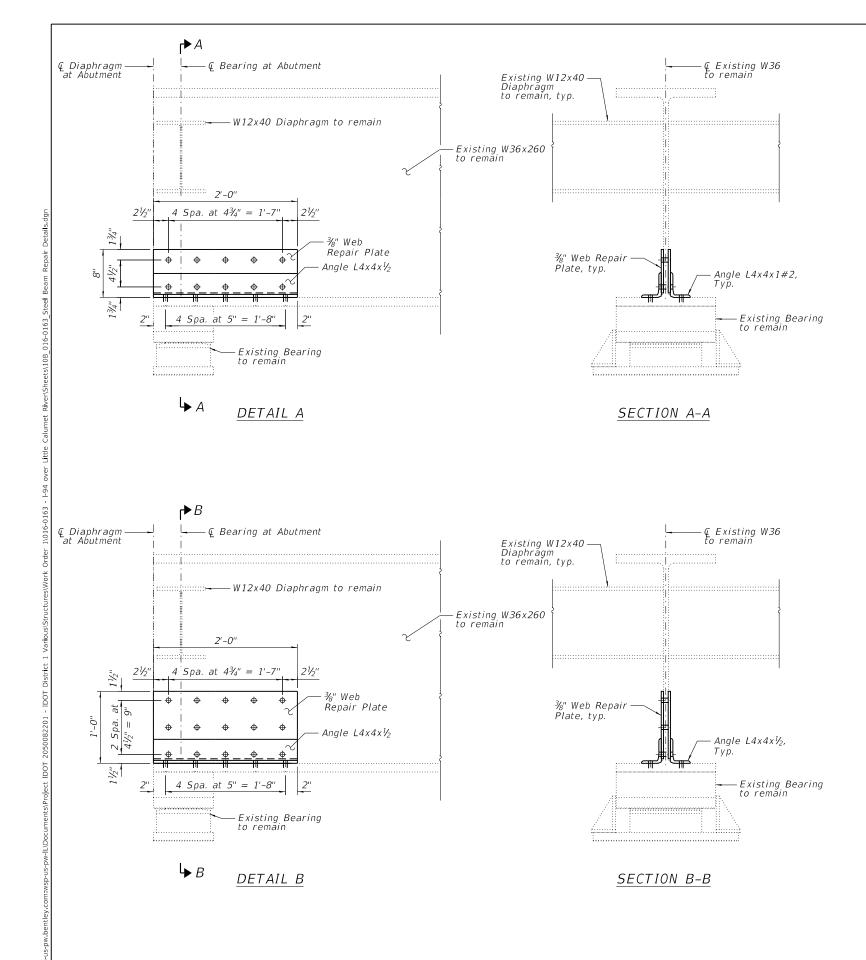


### FRAMING PLAN

- (A)- Beam End Repairs, 8"X24" (2 Locations). See Sheet S09-10A for details.
- $\langle B \rangle$  Beam End Repairs, 12"X24" (1 Location). See Sheet S09-10A for details.

USER NAME =	DESIGNED - BJD	REVISED -
	CHECKED - MGH	REVISED -
PLOT SCALE =	DRAWN - BJD	REVISED -
PLOT DATE =	CHECKED - MGH	REVISED -

FRAMING PLAN	F.A.I RTE	SE
STRUCTURE NO. 016-0163	94	(42-B-11-
911001011E 110: 010-0103		



### BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Structural Steel Repair	Pound	300

### NOTES:

- 1. Contractor to field verify diaphragm location and bolt hole locations before ordering material.
- No welds are to be installed on the back/end of the beam. All welds are included in the cost of Structural Steel Repair.
- 3. Contractor to field verify hole locations before ordering material. Contractor can elect to field drill holes in repair plates.

wsp

USER NAME =	DESIGNED -	BJD	REVISED -
	CHECKED -	MGH	REVISED -
PLOT SCALE =	DRAWN -	BJD	REVISED -
PLOT DATE =	CHECKED -	MGH	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

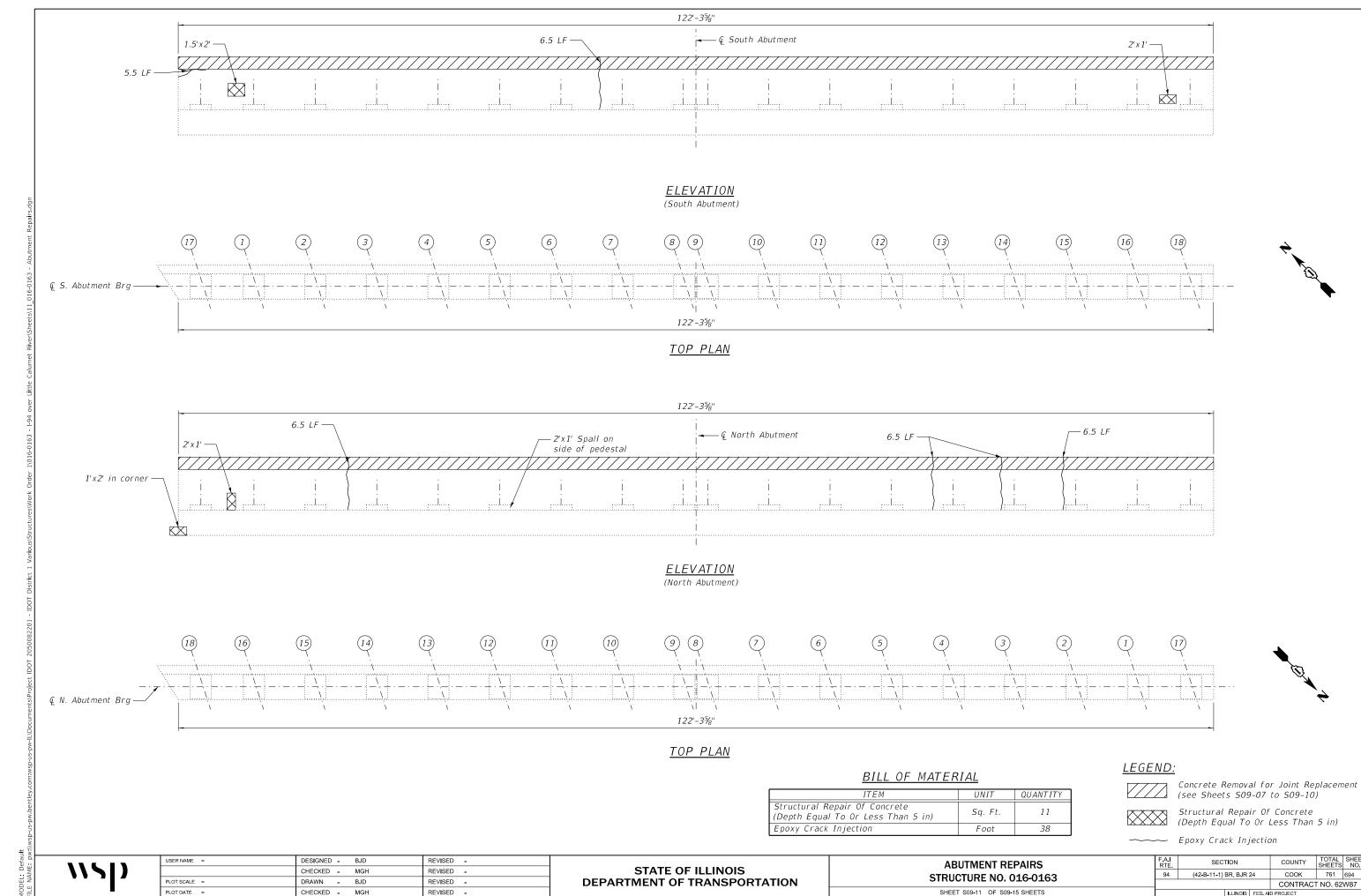
STEEL BEAM REPAIR DETAILS STRUCTURE NO. 016-0163						
SHEET S09-10B OF S09-15 SHEETS						

 
 F.A.I RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 NO.

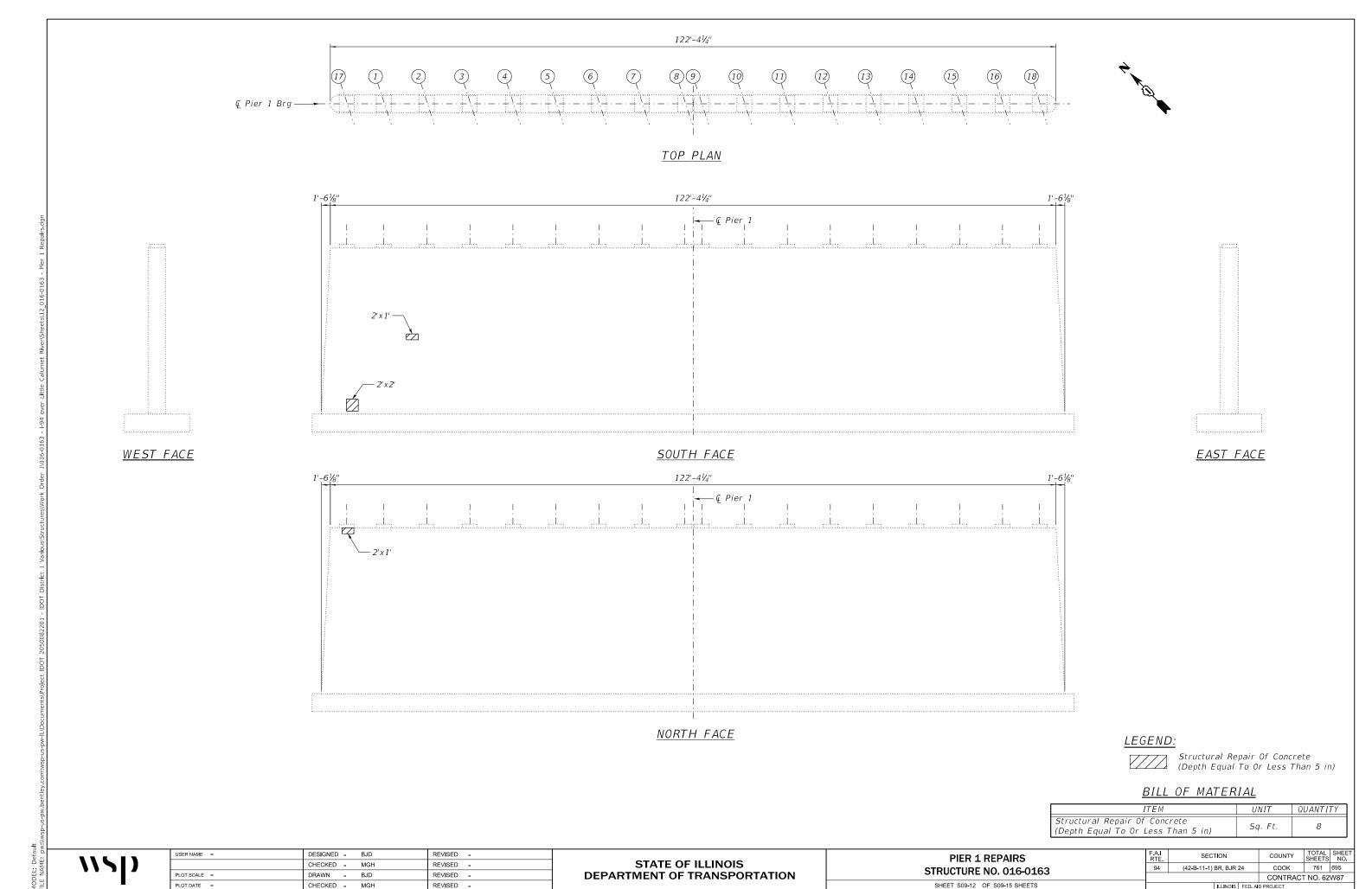
 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 693B

 CONTRACT NO. 62W87

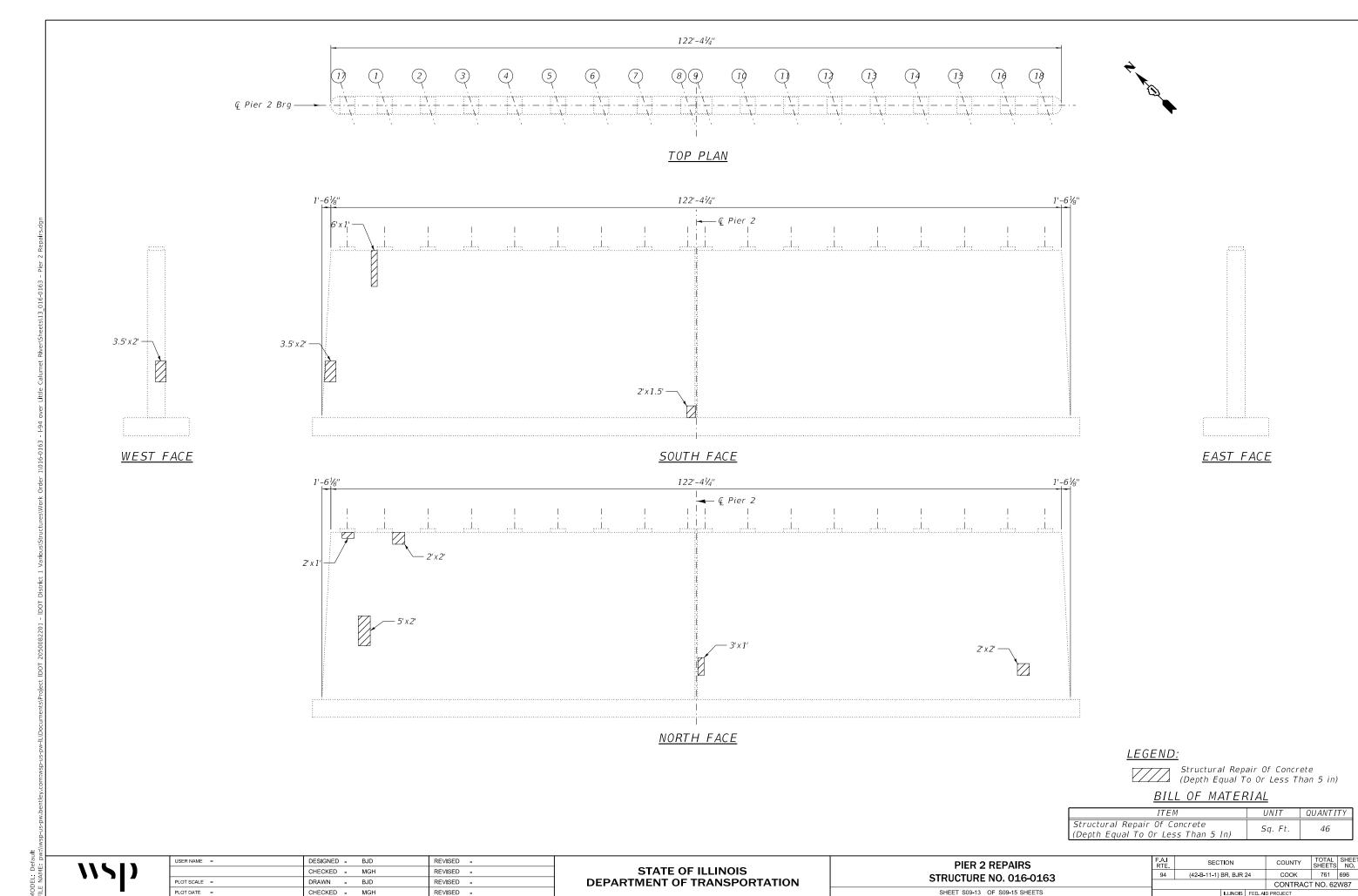
3/10/2025 1:14:17 PM



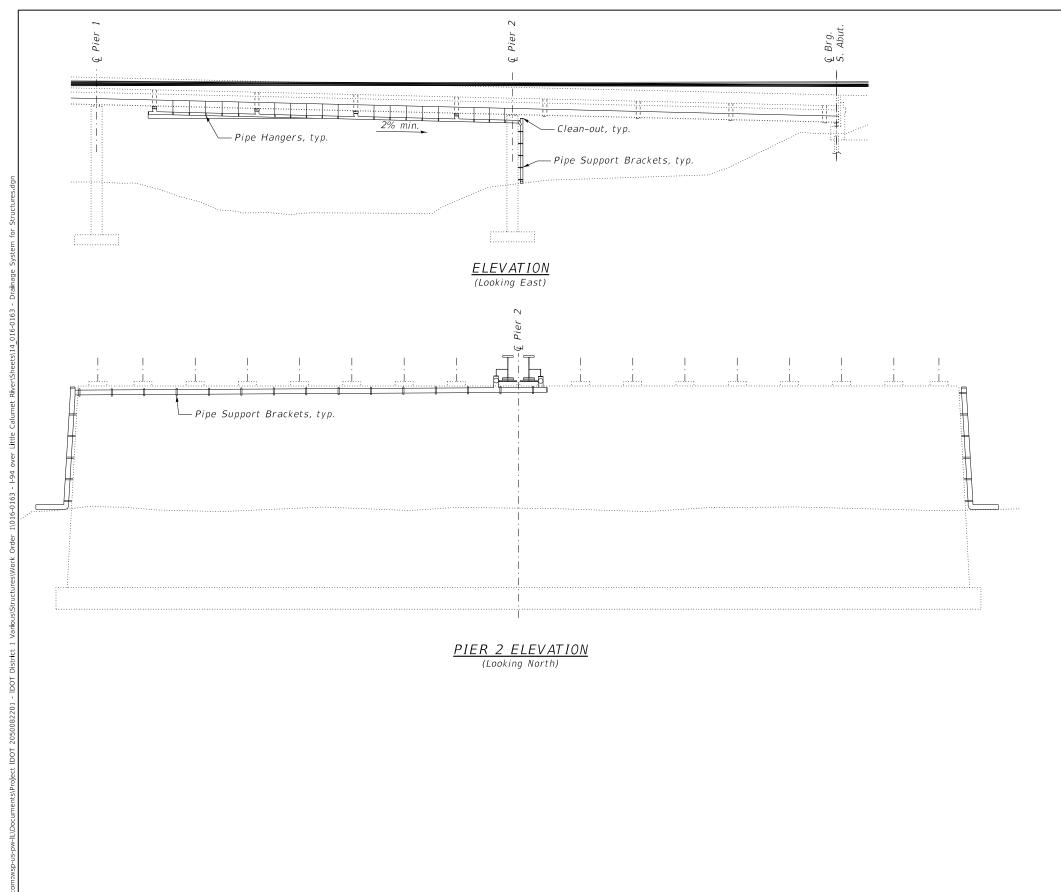
12/9/2024 7:20:52 AM



12/9/2024 7:20:55 AM



12/9/2024 7:20:58 AM



### BILL OF MATERIAL

Г	ITEM	UNIT	QUANTITY
	Drainage System For Structures	L. Sum	1

### SCUPPER LOCATIONS

Station	0ffset
580+79.48	57.00' Rt.
581+05.10	57.00' Rt.
581+29.85	57.00' Rt.
581+55.00	57.00' Rt.
580+98.36	1.50' Rt.
581+23.31	1.50' Rt.
581+49.27	1.50' Rt.
581+74.26	1.50' Rt.
581+02.94	1.50' Lt.
581+27.36	1.50' Lt.
581+52.23	1.50' Lt.
581+75.09	1.50' Lt.
581+17.39	57.00' Lt.
581+42.41	57.00' Lt.
581+67.68	57.00' Lt.
581+92.40	57.00' Lt.

### Notes:

- All drain pipes and fittings shall be 8" 

   Reinforced
   Thermosetting Resin Pipe (RTRP) in accordance with
   Article 523.02 of the Standard Specifications.
- All pipe hangers, supports and hardware shall be galvanized by the hot-dip process. The zinc coatings shall conform to the requirements of AASHTO M2332.
- Pipe hangers/supports shall be provided on all horizontal/vertical pipes at each tee, elbow or change in direction and at intermediate spacings not to exceed those recommended by the manufacturer.
- 4. Hanger dimensions shall be adjusted in the field by the Engineer to fit existing conditions and to maximize slope.
- 5. Details shown are schematic only. Contractor to determine required fittings, provisions for expansion/contraction and routing of piping as required to pass diaphragms and maintain minimum slopes.
- 6. All cost for the components and work required for the drainage system will be included in the pay item Drainage System for Structures (L. Sum.).

wsp

 USER NAME
 =
 DESIGNED
 BJD
 REVISED

 CHECKED
 MGH
 REVISED

 PLOT SCALE
 =
 DRAWN
 BJD
 REVISED

 PLOT DATE
 =
 CHECKED
 MGH
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ENCLOSED DRAINAGE SYSTEM STRUCTURE NO. 016-0163 | TOTAL | SHEETS | NO. | SHEETS | NO

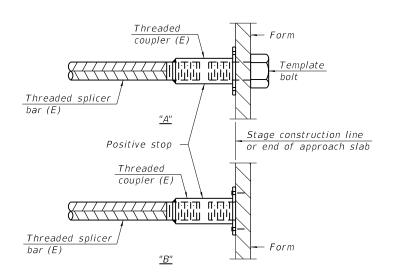
### STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length +  $1\frac{1}{2}$ " + thread length

\* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

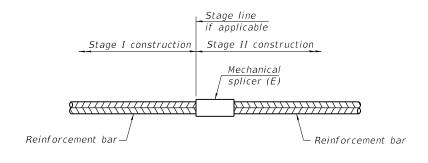
Location	Bar size	No. assemblies required	Minimum Iap length
Abutment	#6	12	4'-5"
Deck	#5	52	3'-1"



### INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt "B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



### STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

SHEET S09-15 OF S09-15 SHEETS

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for

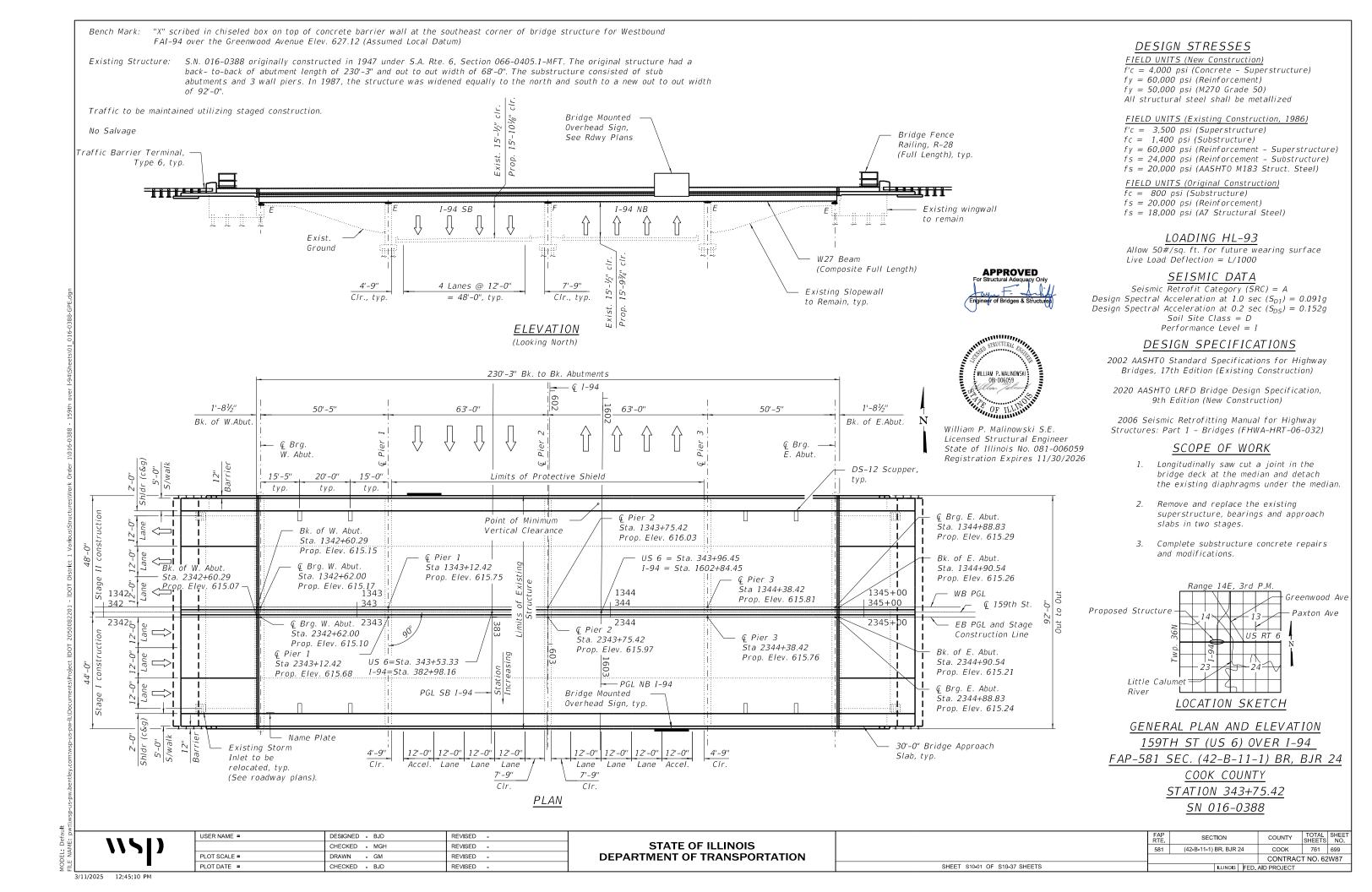
See approved list of bar splicer assemble alternatives.

BSD-1

2-1-2023

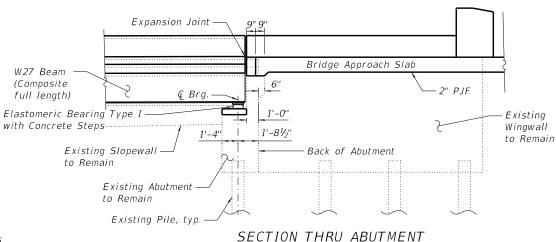


USER NAME =	DESIGNED -	BJD	REVISED -
	CHECKED -	MGH	REVISED -
PLOT SCALE =	DRAWN -	BJD	REVISED -
PLOT DATE =	CHECKED -	MGH	REVISED -



### GENERAL NOTES

- 1. Fasteners shall be ASTM F 3125 Grade A325 Type 1, hot dip galvanized bolts in metallized areas. Bolts % in. diameter, holes  $^{15}\!\!/_{\!6}$  in. diameter, unless otherwise noted. See Special Provision for "Metallizing of Structural Steel"
- 2. Calculated weight of Structural Steel = 36,610 (Grade 36) Calculated weight of Structural Steel = 623,700 (Grade 50)
- 3. All structural steel shall be metalized. See Special Provision for "Metallizing of Structural Steel."
- 4. Reinforcement bars designated (E) shall be epoxy coated.
- 5. The finishing machine rails shall be placed on the top of the top flange of the exterior beams within the deck. Beam blocks shall be placed between beams at all tie locations in each bay for the full width of the deck pour.
- 6. Slipforming of the parapets is not allowed.
- 7. Bearing seat surfaces shall be constructed or adjusted to the designated elevations within a tolerance of  $\frac{1}{2}$  in. (0.01 ft.). Adjustment shall be made either by grinding the surface or by shimming the bearings.
- 8. Plan dimensions and details relative to the existing structure have been taken from existing plans are subject to nominal construction variations. The Contactor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 9. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to address the presence of lead on this project.
- 10. All steel rail elements of the Bridge Fence Railing shall be galvanized. See Sht. S10-19 of S10-37.



STA. 344+75.42 RE-BUILT 202X BY STATE OF ILLINOIS F.A.P. Rt. 581 Sec. (42-B-11-1) BR. BJR 24 LOADING HL-93 STR. NO. 016-0388

### NAME PLATE

See Std. 515001. Existing Name Plate shall be cleaned and relocated next to new Name Plate. Cost included in Name Plates.

### INDEX OF SHEETS

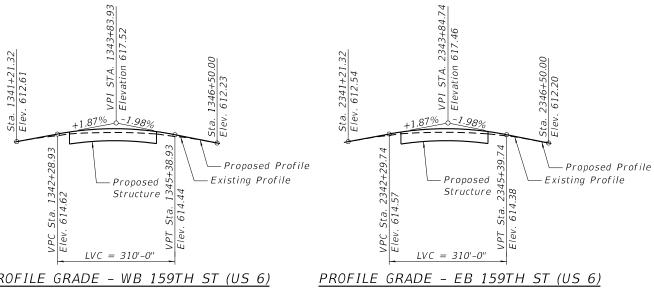
S10-01 General Plan and Elevation 510-02 General Data 510-03 Stage Construction Details - 1 *510-04* Stage Construction Details - 2 Temporary Concrete Barrier For Stage Construction S10-05 S10-06 Top of Deck Slab Elevation Layout S10-07 Top of Deck Slab Elevations - 1 510-08 Top of Deck Slab Elevations - 2 510-09 Top of Deck Slab Elevations - 3 S10-10 Top of West Approach Slab Elevations S10-11 Top of East Approach Slab Elevations S10-12 Superstructure - 1 S10-13 Superstructure - 2 Superstructure Details S10-14 S10-15 Superstructure Details S10-16 Bridge Approach Slab Details - 1 S10-17 Bridge Approach Slab Details - 2 S10-18 Bridge Fence Railing *510-19* Bridge Fence Railing S10-20 Preformed Joint Strip Seal Preformed Joint Strip Seal - Sidewalk 510-21 S10-22 Drainage Scupper Details 510-23 Framing Plan S10-24 Structural Steel S10-25 Structural Steel Details S10-26 Bearing Details S10-27 West Abutment Removal and Repairs S10-28 East Abutment Removal and Repairs 510-29 West Abutment Modifications S10-30 East Abutment Modifications S10-30A Abutment Modifications Details - 1 S10-30B Abutment Modifications Details - 2 S10-31 Pier 1 Removal and Repairs S10-32 Pier 2 Removal and Repairs 510-33 Pier 3 Removal and Repairs S10-34 Pier 1 Modifications S10-35 Pier 2 Modifications

S10-37 Bar Splicer Assembly and Mechanical Splicer Details

### TOTAL BILL OF MATERIAL

S10-36 Pier 3 Modifications

ITEM	UNIT	SUPER- STRUCTURE	SUB- STRUCTURE	TOTA
Removal of Existing Superstructures	Each	1	-	1
Concrete Removal	Cu Yd	-	20.7	20.
Protective Shield	Sq Yd	1,288	-	1,28
Concrete Structures	Cu Yd	-	127.6	127
Concrete Superstructure	Cu Yd	952.6	-	952
Bridge Deck Grooving	Sq Yd	2,368	-	2,30
Protective Coat	Sq Yd	3,184	-	3,18
Concrete Superstructure (Approach Slab)	Cu Yd	253.8	_	253
Furnishing and Erecting Structural Steel	L Sum	1	_	1
Stud Shear Connectors	Each	14,574	-	14,5
Reinforcement Bars, Epoxy Coated	Pound	361,430	29,920	391,
Bar Splicers	Each	896	154	1,0
Bridge Fence Railing	Foot	524	-	52
Name Plates	Each	2	-	2
Preformed Joint Strip Seal	Foot	184	-	18
Elastomeric Bearing Assembly, Type I	Each	56	-	56
Anchor Bolts, 1"	Each	112	-	11
Anchor Bolts, 1 1/4"	Each	28	-	28
Concrete Sealer	Sq Ft	-	1,625	1,6
Epoxy Crack Injection	Foot	-	147	14
Bar Terminators	Each	156	-	15
Structural Repair of Concrete (Depth Equal To or Less Than 5 Inches)	Sq Ft	-	207	20
Drainage Scuppers, DS-12	Each	8	_	8



PROFILE GRADE - WB 159TH ST (US 6)

VPI Sta. 1600+98.60 VPI Sta. 1603+27.37 Elev. 596.78 Elev. 595.99 VPI Sta. 1602+10.04 Elev. 596.22

> PROFILE GRADE -I-94 NB (BISHOP FORD)

VPI Sta. 381+84.63 VPI Sta. 383+70.00 Elev. 596.41 Elev. 596.08 -0.48% -0.30% 70.0% VPI STA. 382+95.33 Elevation 596.08 PROFILE GRADE -I-94 SB (BISHOP FORD)

	USER NAME =	DESIGNED - BJD	REVISED -
		CHECKED - MGH	REVISED -
	PLOT SCALE =	DRAWN - GM	REVISED -
	PLOT DATE =	CHECKED - BJD	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** 

SECTION COUNTY **GENERAL DATA** 581 (42-B-11-1) BR. BJR 24 COOK 761 700 **STRUCTURE NO. 016-0388** CONTRACT NO. 62W87 SHEET S10-02 OF S10-37 SHEETS