

When "A" is 3'-1" or less, the temporary concrete — See Detail I, II or III barrier shall be restrained to the new slab according to Detail I, II or III. No restraint is required when "A" is greater than 3'-1".

├- Stage removal line - Stage removal line 1'-101/2" 1'-101/2" Temporary Concrete Barrier See Standard 704001 min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint is required when "A" is greater than 3'-1".

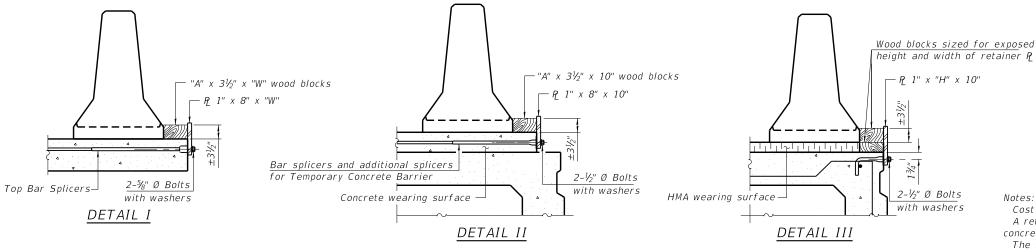
*When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

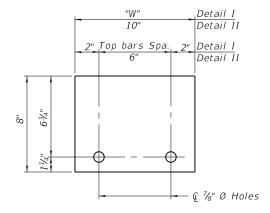
EXISTING DECK BEAM

NEW SLAB OR NEW DECK BEAM

SECTIONS THRU SLAB OR DECK BEAM

EXISTING SLAB





STEEL RETAINER P 1" x 8" x "W" (Detail I and II)

—⊊ ¾" Ø Holes

STEEL RETAINER P 1" x "H" x 10"



Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate Q of each temporary concrete barrier.

BAR SPLICER FOR #4 BAR - DETAIL III

1x8 UNC

1" Ø pin

RESTRAINING PIN

US Std. $1\frac{1}{16}$ " I.D. \times $2\frac{1}{2}$ " O.D. x approx. 8 gauge thick washer

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

RAILING CRITERIA

NCHRP 350 Test Level Railing Weight (plf)

R-27

5-15-2023



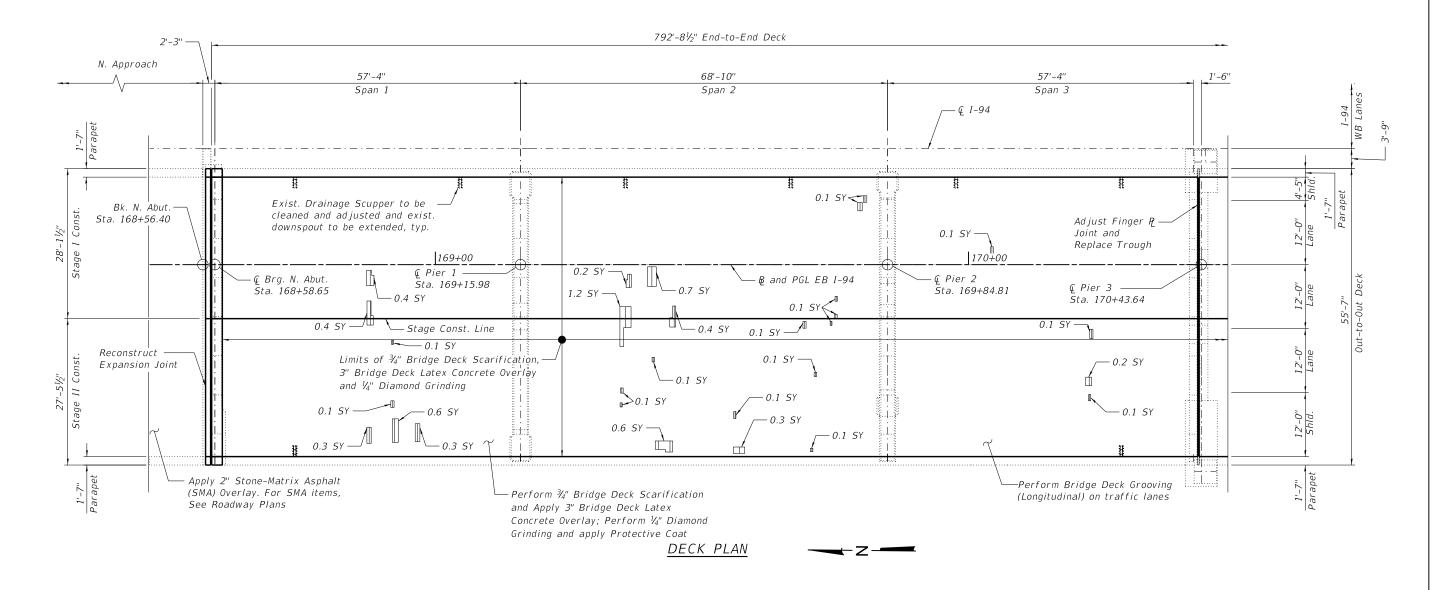
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PLOT DATE	-	12/6/2024	DATE	-	12/9/2024	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** TEMPORARY CONCRETE BARRIER **STRUCTURE NO. 016-0158** SHEET S02-05 OF S02-36 SHEETS

SECTION COUNTY (42-B-11-1) BR. BJR 24 COOK 761 501 CONTRACT NO. 62W87

-7∕₁₆" Ø hole

ITEM	UNIT	QUANTITY
Protective Coat	Sq Yd	1,319
Bridge Deck Grooving (Longitudinal)	Sq Yd	743
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	1,054
Bridge Deck Scarification 3/4"	Sq Yd	1,054
Diamond Grinding (Bridge Section)	Sa Yd	995



NOTES:

- Areas of deck repair shown are estimated. The Engineer shall show actual locations of deck repairs at the time of construction.
- 2. For bridge deck final cross section, see Sheet S02-04.
- 3. Perform $\frac{1}{4}$ " Diamond Grinding to top of bridge deck and abutment hatch block.
- 4. For North Abutment expansion joint removal and reconstruction, see Sheets SO2-11 thru SO2-13.
- . For Pier 3 finger plate joint adjustment and trough replacement details, see Sheets S02-14 and S02-15.
- . Any reinforcement bars that are damaged during concrete removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.

LEGEND



*Deck Slab Repair (Partial Depth)

SY Square Yard

*Areas of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 3"



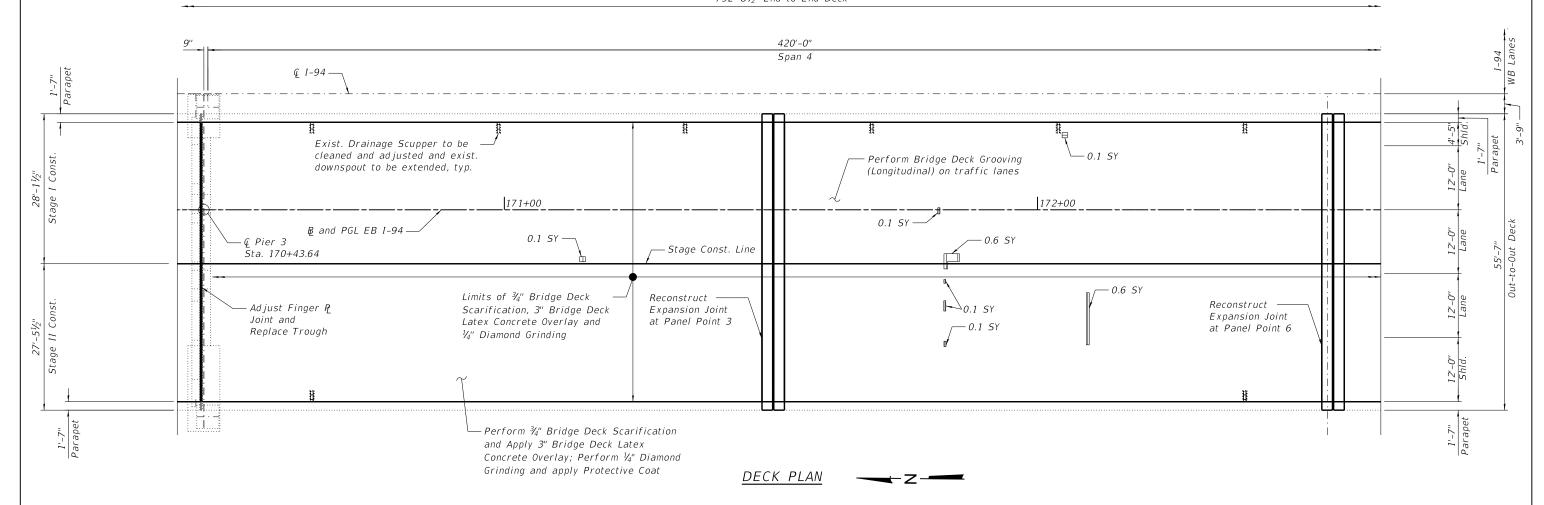
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DECK REPAIR PLAN (SHEET 1 OF 4)
STRUCTURE NO. 016-0158

SHEET 502-06 OF 502-36 SHEETS

A.I. SECTION COUNTY TOTAL SHEETS NO.
94 (42-B-11-1) BR, BJR 24 COOK 761 502

CONTRACT NO. 62W87



NOTES:

- For expansion joint removal and reconstruction at truss Panel Point 3, see Sheets S02-16 and S02-17.
- For expansion joint removal and reconstruction at truss Panel Point 6, see Sheets S02-18 and S02-19.
- For Bill of Material, see Sheet S02-08.
- For additional notes, see Sheet S02-06.
- Any missing or damaged bird screens shall be replaced in accordance with the Bird Screen Detail. Cost included with Protective Shield.

I Gage Stainless Steel Wire Mesh ('z''x 'z'' Openings) 5g'''x *5g'' O.C.	£ z* \$ Stainless Steel botts with stainless steel lock washers (Drill \$ tap holes in truss member)
B	U € Truss Member .
12 DE	\ \
	2° x 3/8° Bor (Typ.)

BIRD SCREEN DATA									
HANDHOLE TYPE	HANDHOL A	E SIZE B	С	0	E				
Α	16"	8"	5"	91/6"	9%"				
В	11"	51/2"	21/2"	580	53/8"				
С	12*	6"	4160	68"	678"				
. D	6'	6*	41/6"	416"	416				

BIRD SCREEN DETAIL

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⊢.	<i>-</i>	_	NI	1)

*Deck Slab Repair (Partial Depth)

Deck Slab Repair (Full Depth, Type I)

SY Square Yard

*Areas of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 3"



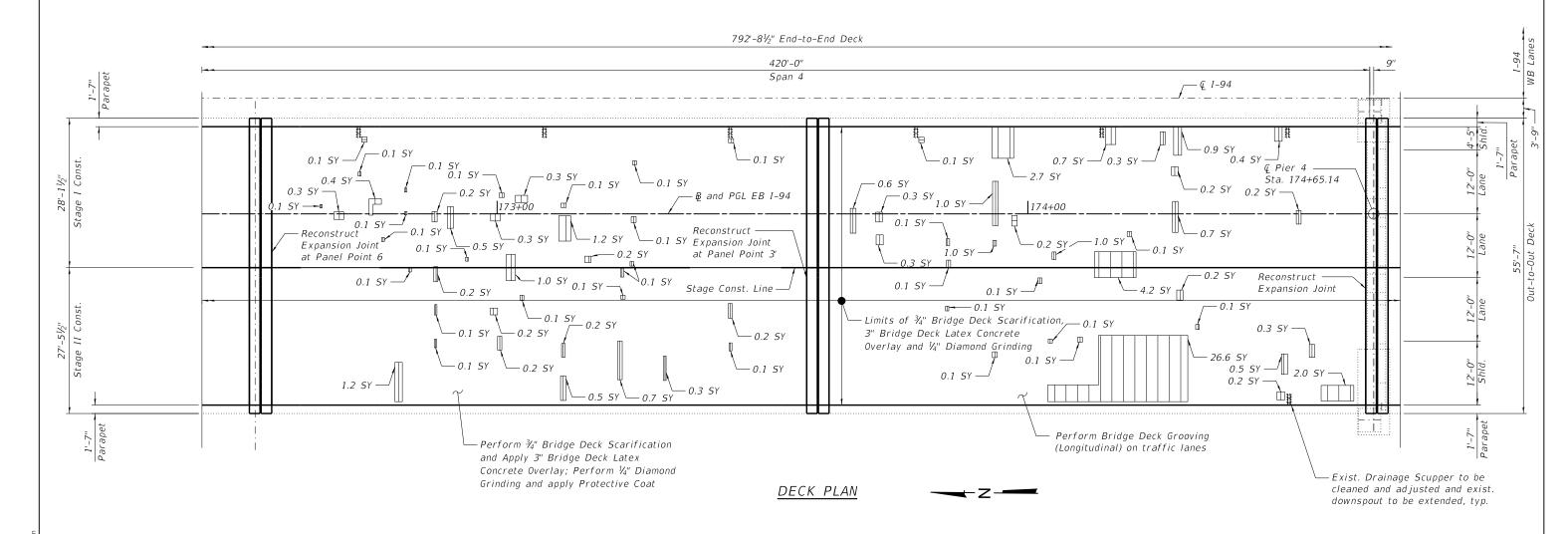
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** DECK REPAIR PLAN (SHEET 2 OF 4) **STRUCTURE NO. 016-0158** SHEET S02-07 OF S02-36 SHEETS

λ.I. ΓΕ.	SEC.	TION		COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1)	BR, BJR	24	COOK	761	503
			CONTRACT	NO. 6	52W87	
ILLINOIS FED. AI				D PROJECT		

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Protective Shield	Sq Yd	2,594
Protective Coat	Sq Yd	2,746
Bridge Deck Grooving (Longitudinal)	Sq Yd	1,686
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	2,364
Bridge Deck Scarification 3/4"	Sq Yd	2,364
Deck Slab Repair (Full Depth, Type I)	Sq Yd	0.5
Diamond Grinding (Bridge Section)	Sa Yd	2,273



NOTES:

- 1. For expansion joint removal and reconstruction at truss Panel Point 6, see Sheets S02-18 and S02-19.
- 2. For expansion joint removal and reconstruction at truss Panel Point 3', see Sheets S02-20 and S02-21.
- 3. For expansion joint removal and reconstruction at Pier 4, see Sheets SO2-22 and SO2-23.
- 4. For additional notes, see Sheet S02-06.

LEGEND

*Deck Slab Repair (Partial Depth)

Deck Slab Repair (Full Depth, Type I)

SY Square Yard

*Areas of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 3"



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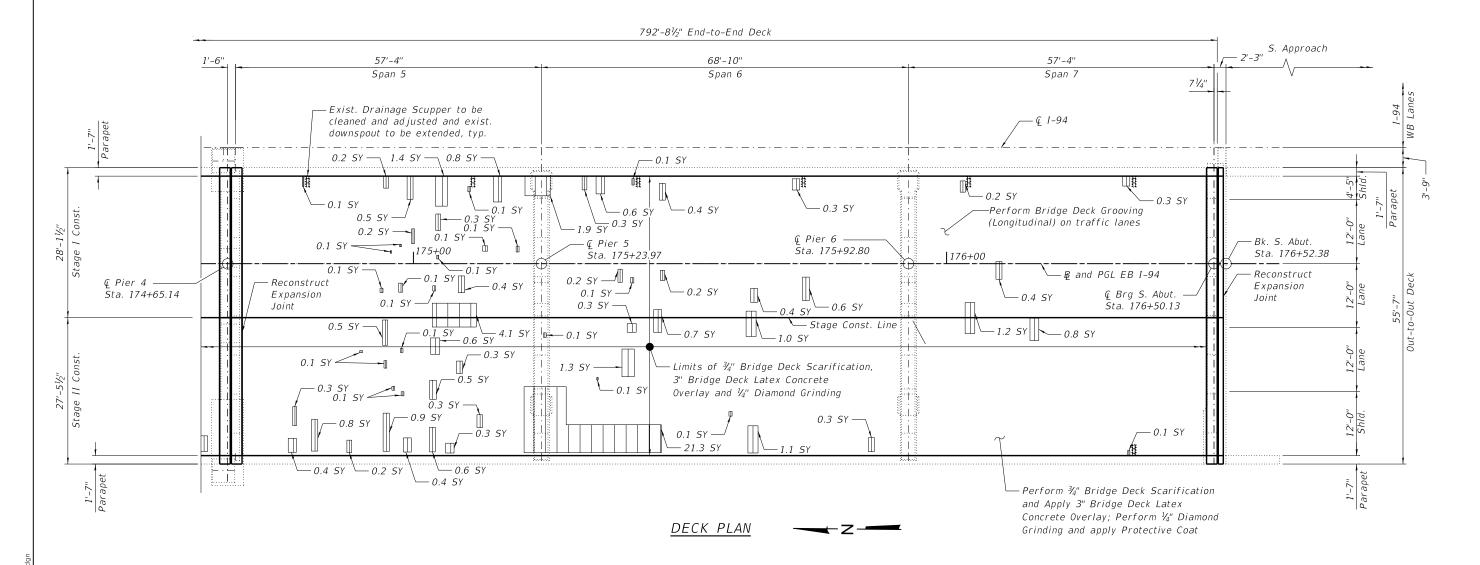
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DECK REPAIR PLAN (SHEET 3 OF 4) STRUCTURE NO. 016-0158 A.I. SECTION COUNTY TOTAL SHEETS NO.
94 (42-B-11-1) BR, BJR 24 COOK 761 504

CONTRACT NO. 62W87

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Protective Coat	Sq Yd	1,319
Bridge Deck Grooving (Longitudinal)	Sq Yd	743
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	1,054
Bridge Deck Scarification 3/4"	Sq Yd	1,054
Diamond Grinding (Bridge Section)	Sa Yd	995



NOTES:

- 1. For expansion joint removal and reconstruction at Pier 4, see Sheets S02-22 and S02-23.
- 2. For South Abutment expansion joint removal and reconstruction, see Sheets S02-24 thru S02-26.
- 3. For additional notes, see Sheet S02-06.

<u>LEGEND</u>

*Deck Slab Repair (Partial Depth)

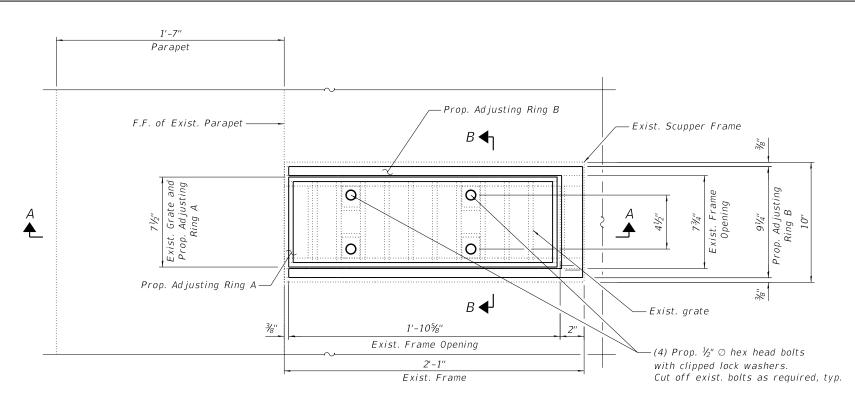
Y Square Yard

*Areas of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 3"

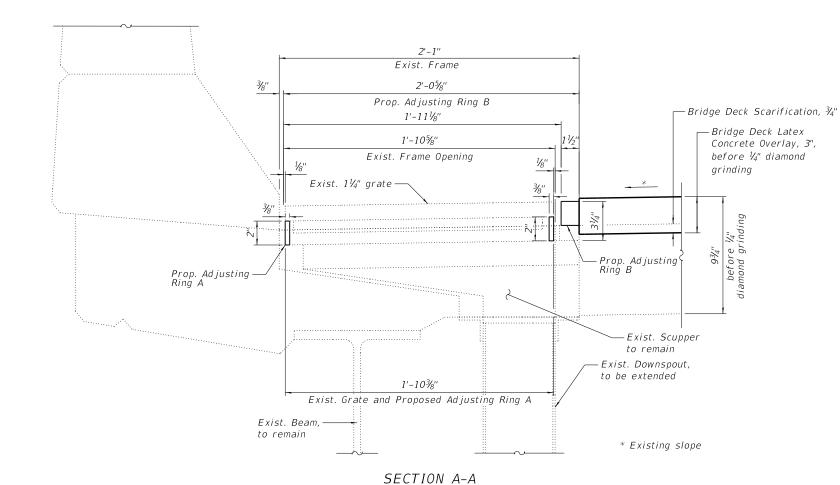


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			CONTRACT	NO.	52W8
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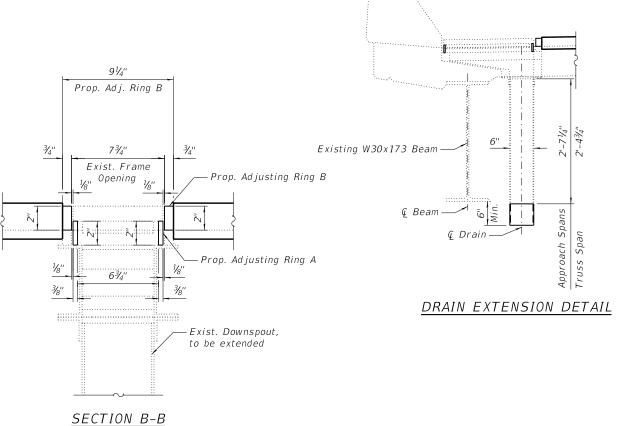


TYPICAL SCUPPER PLAN



BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Deck Drain Extensions	Each	28
Drainage Scuppers To Be Adjusted	Each	28



NOTES:

- 1. The Contractor shall field-verify Existing Dimensions and Details of the Existing Scuppers, grates and downspouts, and make necessary approved adjustments, prior to construction or ordering of material for Drainage Scuppers to be Adjusted and downspout extensions.
- 2. All Cast Iron Parts shall be Grey Iron conforming to the requirements of AASHTO M 105, Class 30.
- 3. Cast Iron Parts shall be unfinished.
- 4. Bolts, washers and nuts shall conform to the requirements of ASTM A307 and be galvanized according to the requirements of AASHTO M232.
- 5. The Contractor shall take appropriate measures to ensure that Protective Coat is not applied to the scuppers.
- Adjusting Rings shall be from Neenah or approved equal. Structural steel weldments or equal sections and of the same configuration may be submitted in place of Cast Iron. Fillet or full penetration welds may be used for weldments. Details shall be submitted to the Engineer for approval.
- 7. Provide 1/8" Fillet Weld around perimeter of new Adjusting Rings to secure to existing Scupper. Electrode shall be compatible with cast iron if existing scupper elements are cast iron construction.
- 3. Cost of all labor and materials necessary to clean all existing floor drains and scuppers, fabricate and install adjusting scupper rings, remove and reinstall grates is included in the cost of Drainage Scuppers to be Adjusted

HBM ENGINEERING GROUP, LLC

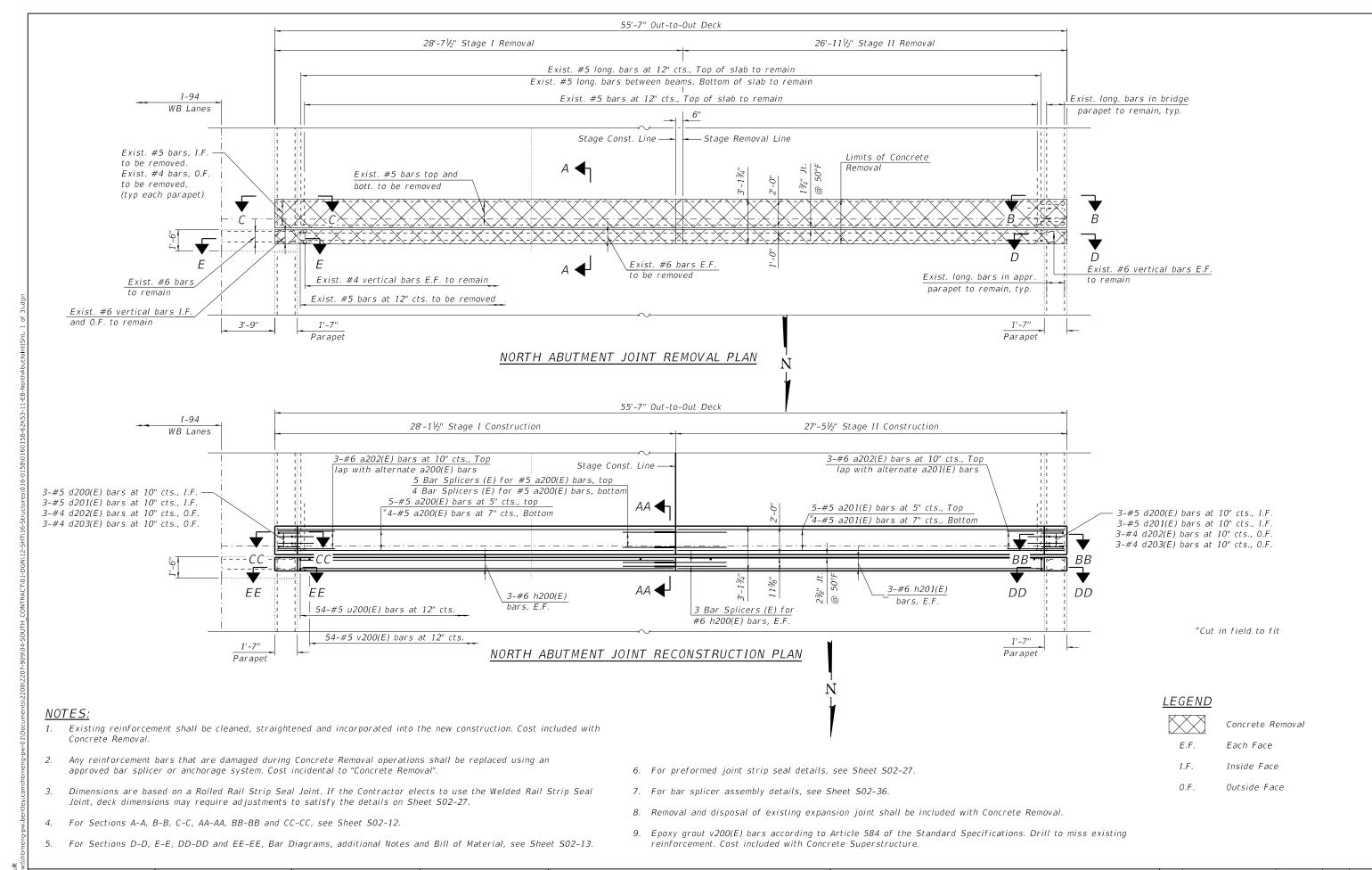
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

 DRAINAGE SCUPPER ADJUSTMENT DETAILS
 F.A.I. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL SHEETS NO.

 STRUCTURE NO. 016-0158
 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 506

 SHEET 502-10
 OF 502-36
 SHEETS
 SHEETS
 ILLINOIS FED AD PROJECT
 62W87

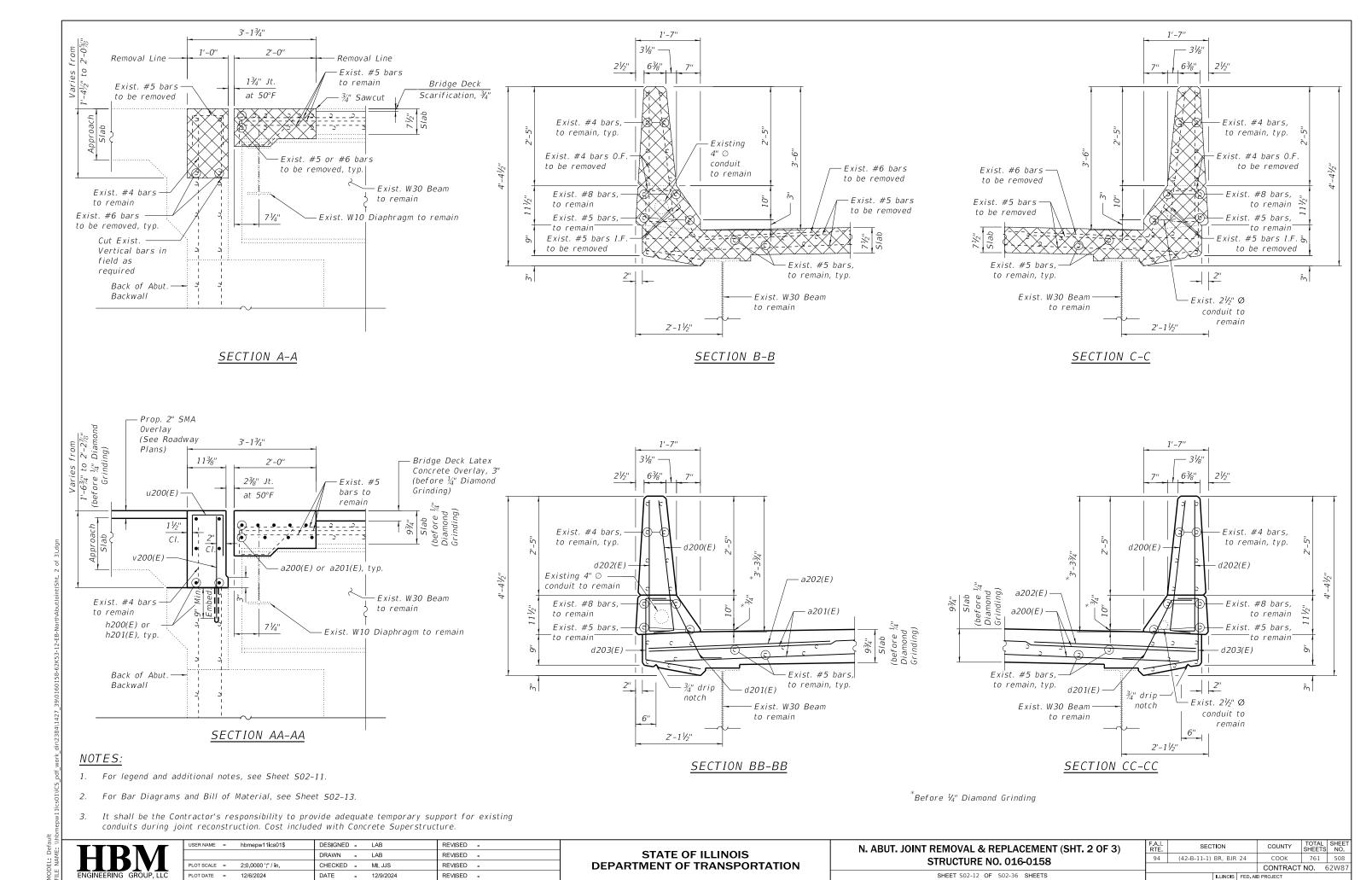


ENGINEERING GROUP, LLC

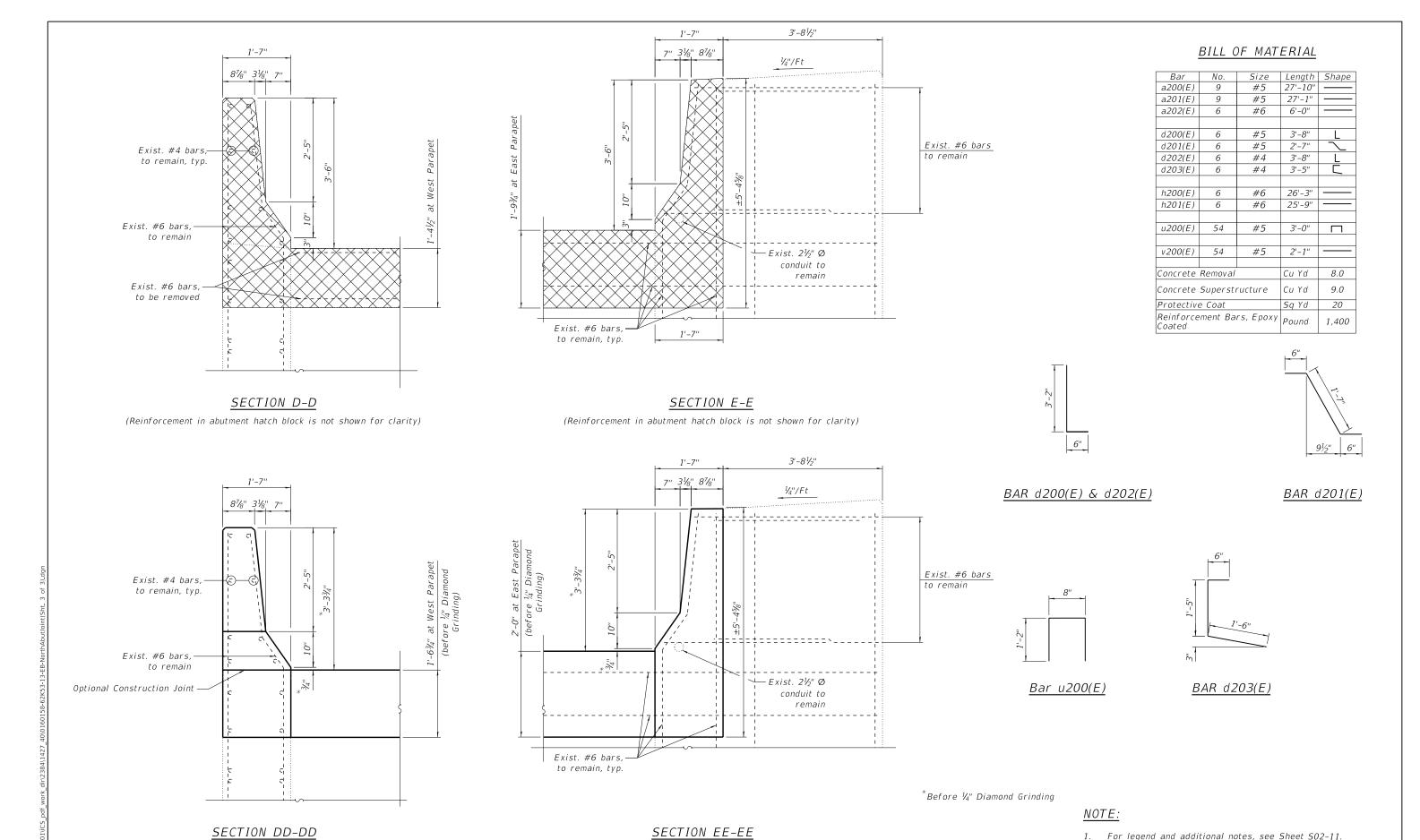
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

N. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 1 OF 3)
STRUCTURE NO. 016-0158

SHEET S02-11 OF S02-36 SHEETS



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HBM

(Reinforcement in abutment hatch block is not shown for clarity)

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

(Reinforcement in abutment hatch block is not shown for clarity)

N. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 3 OF 3)
STRUCTURE NO. 016-0158

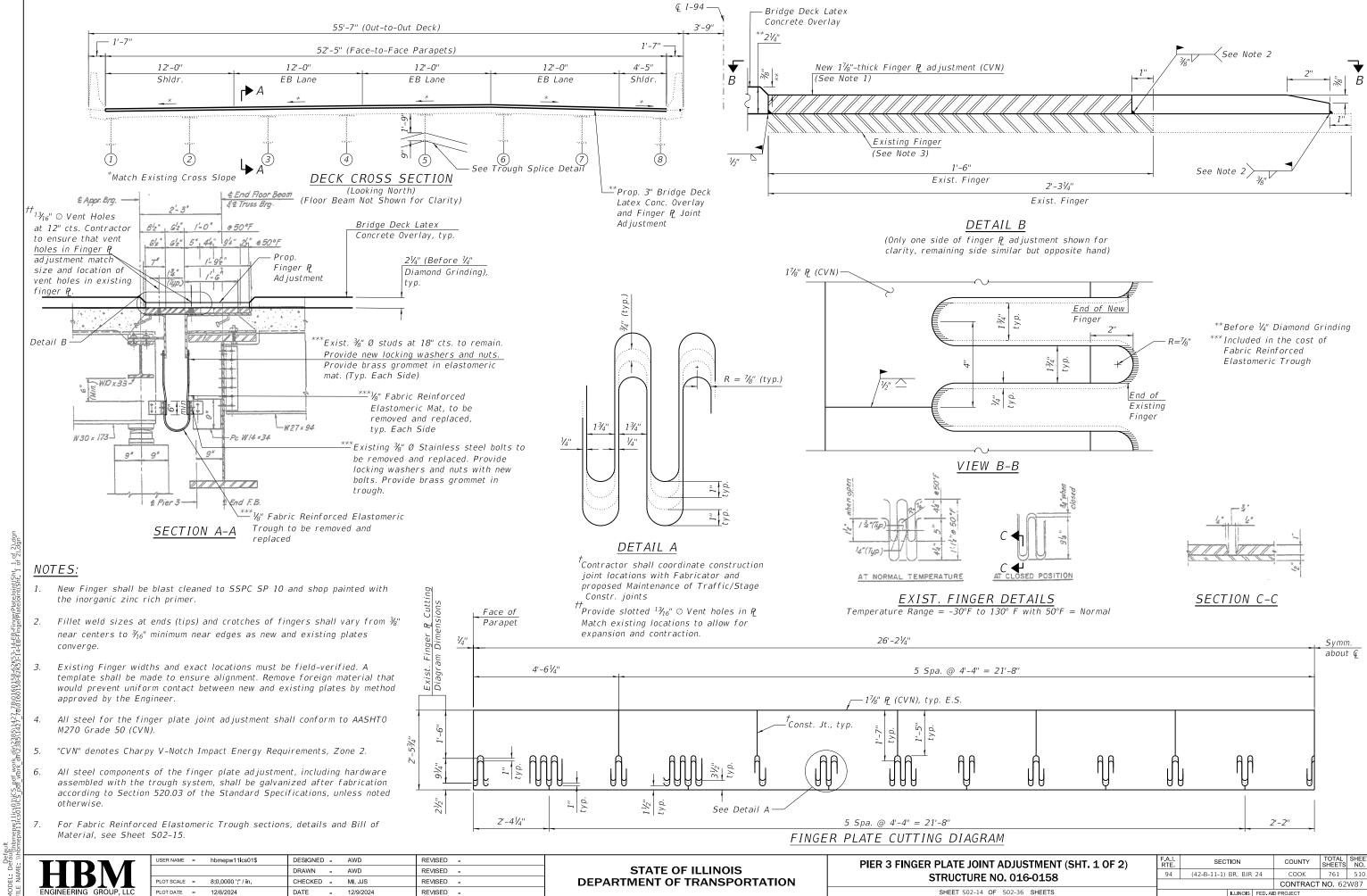
SHEET S02-13 OF S02-36 SHEETS

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 COUNTY
 TOTAL SHEETS NO.
 SHEET SNO.

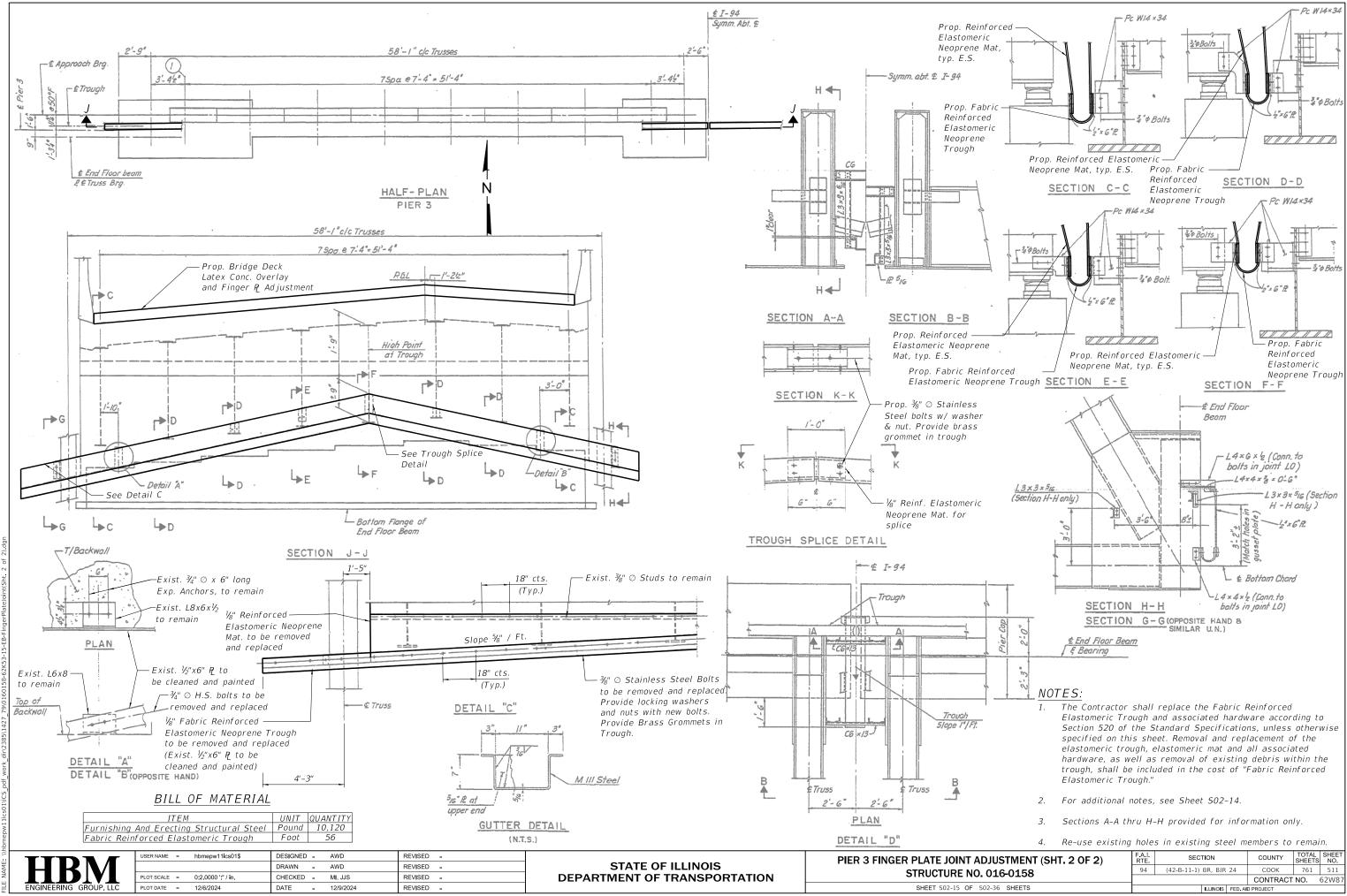
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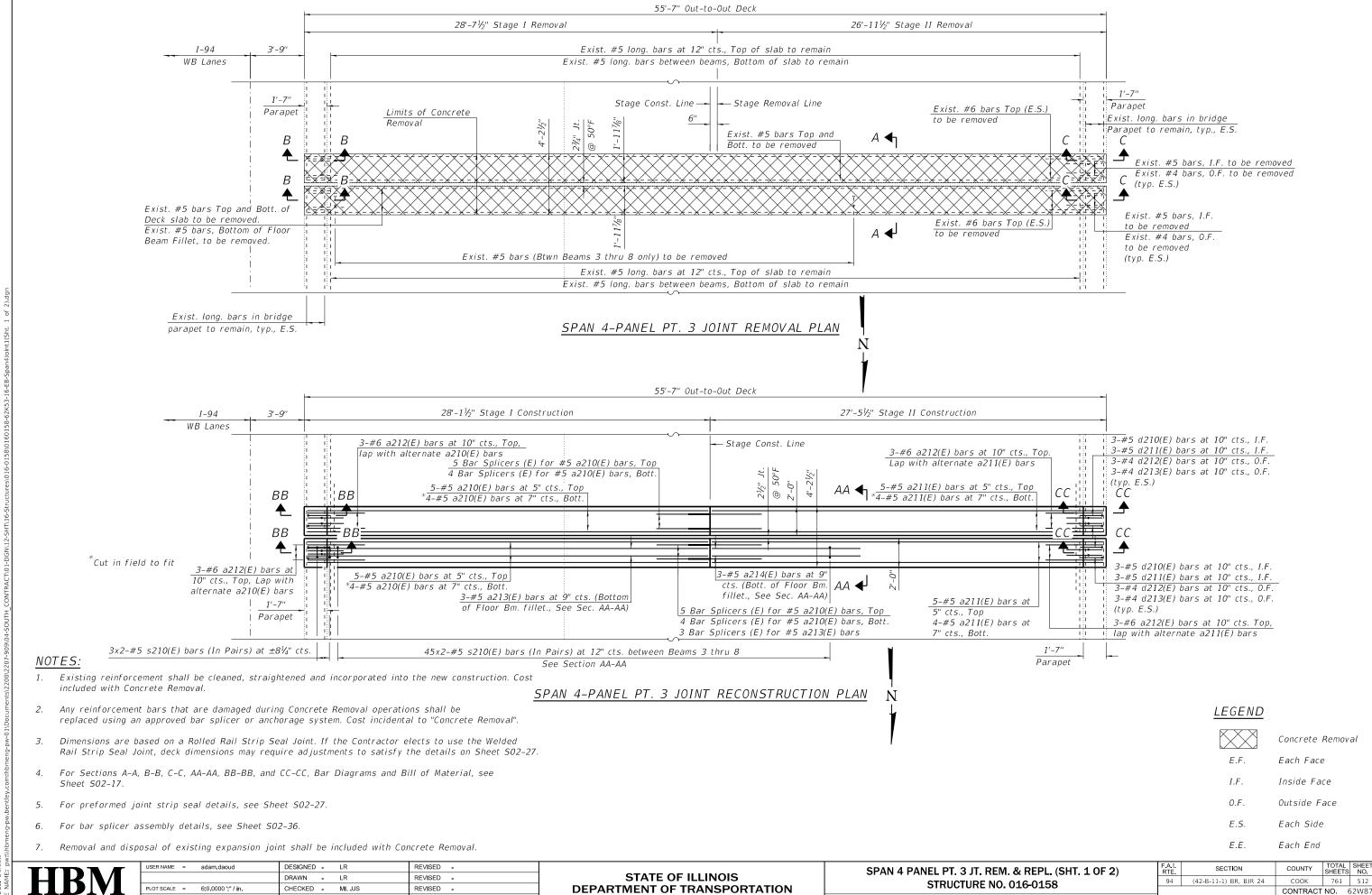
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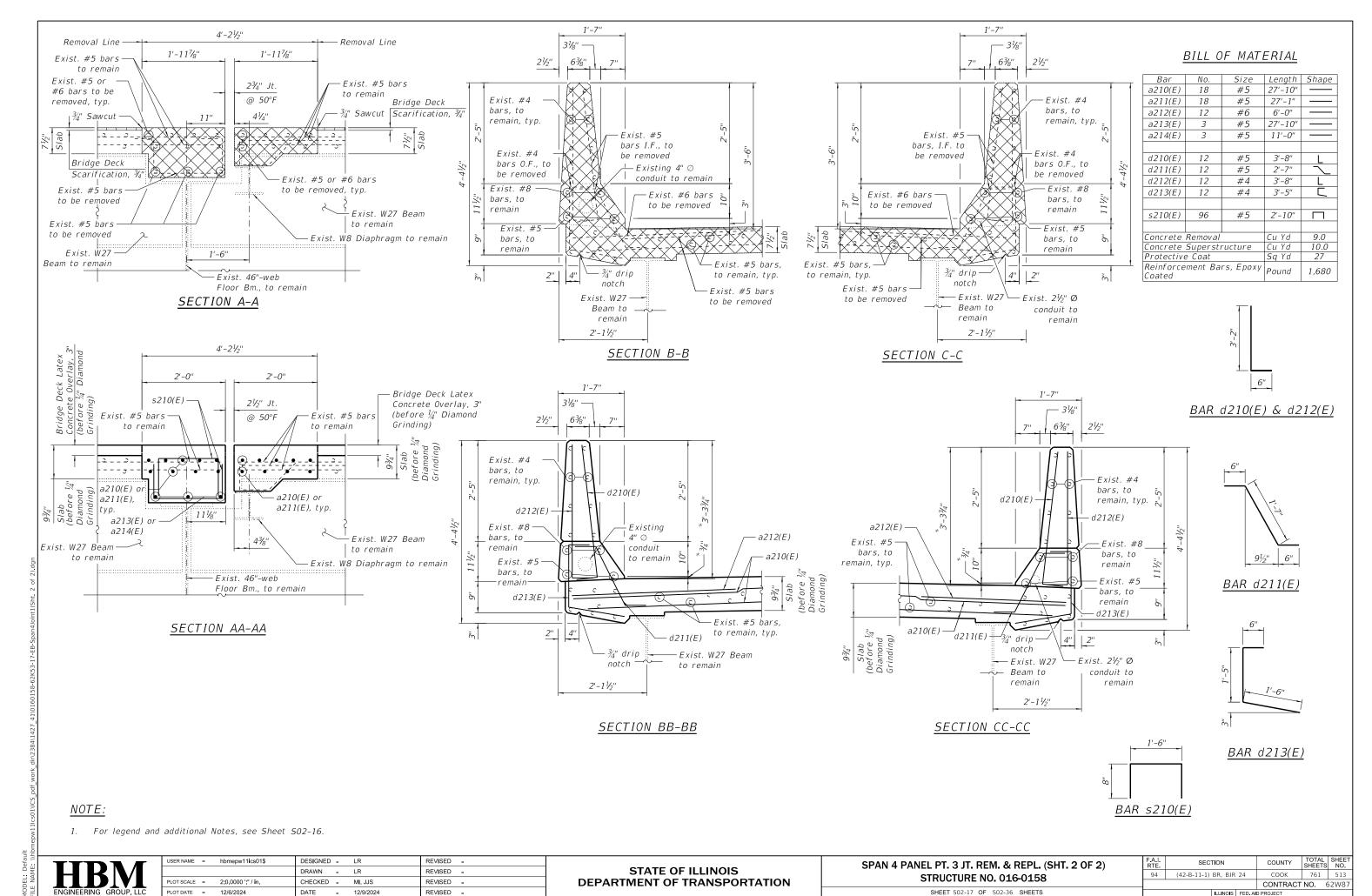
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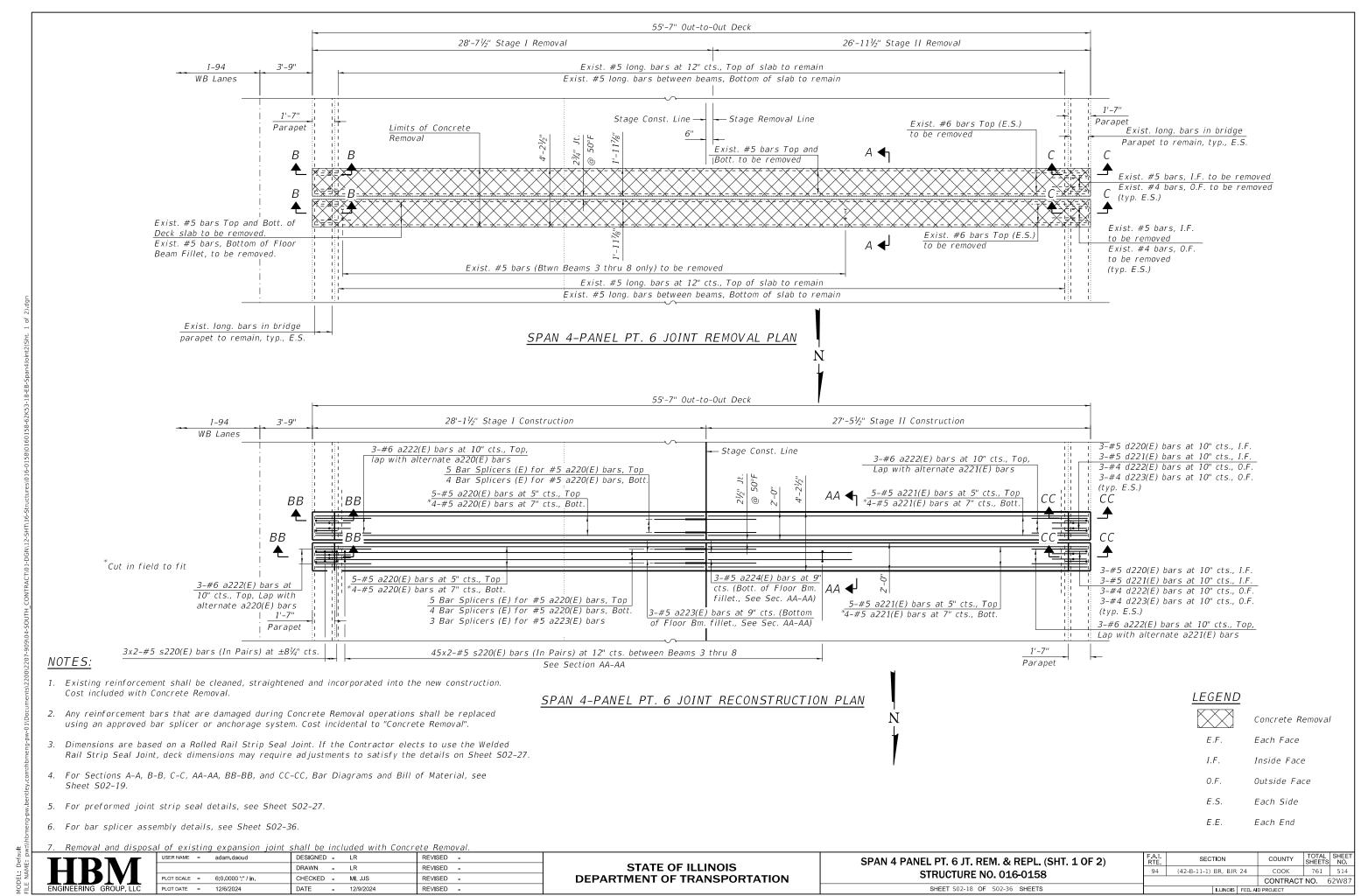
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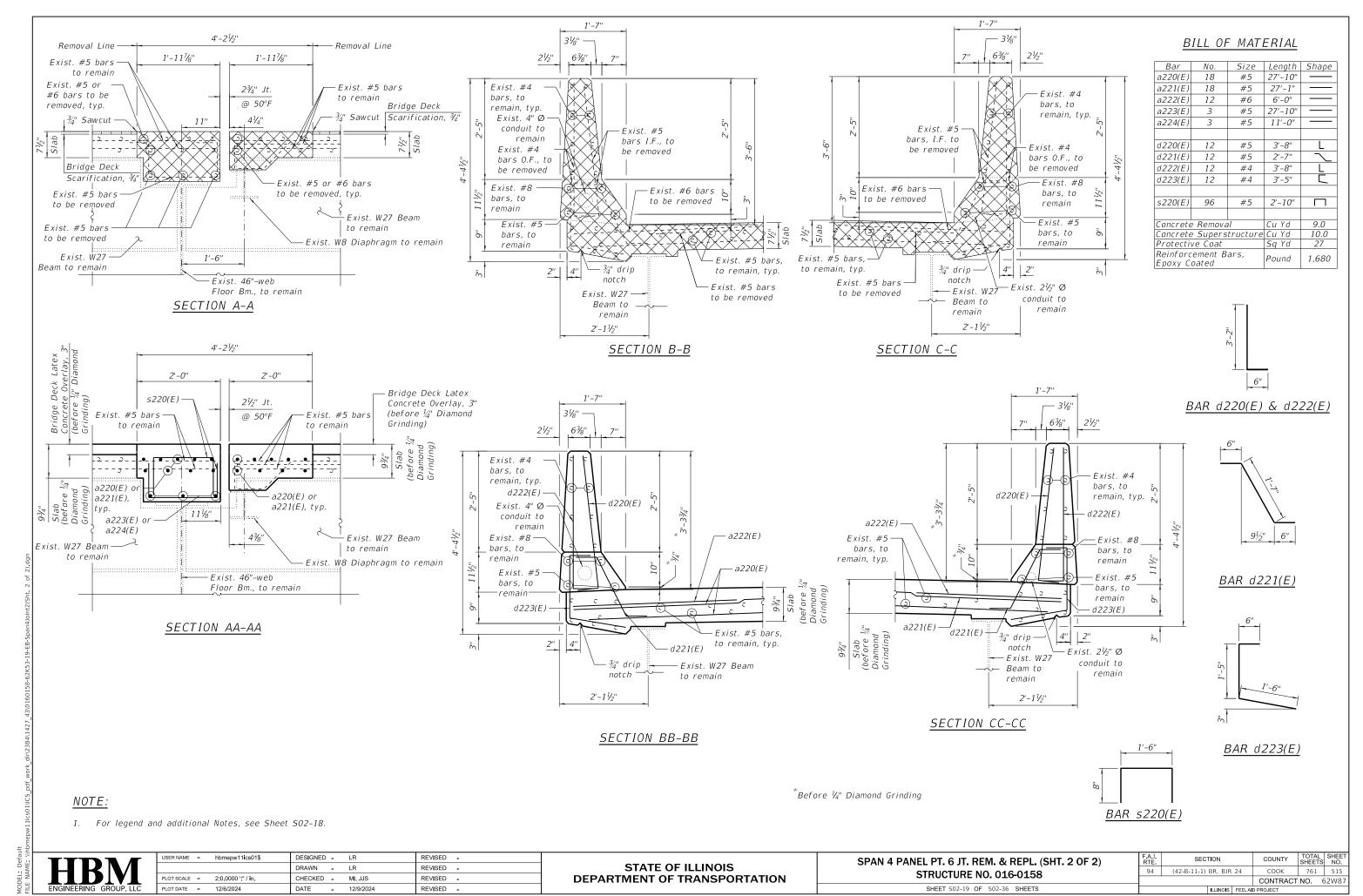
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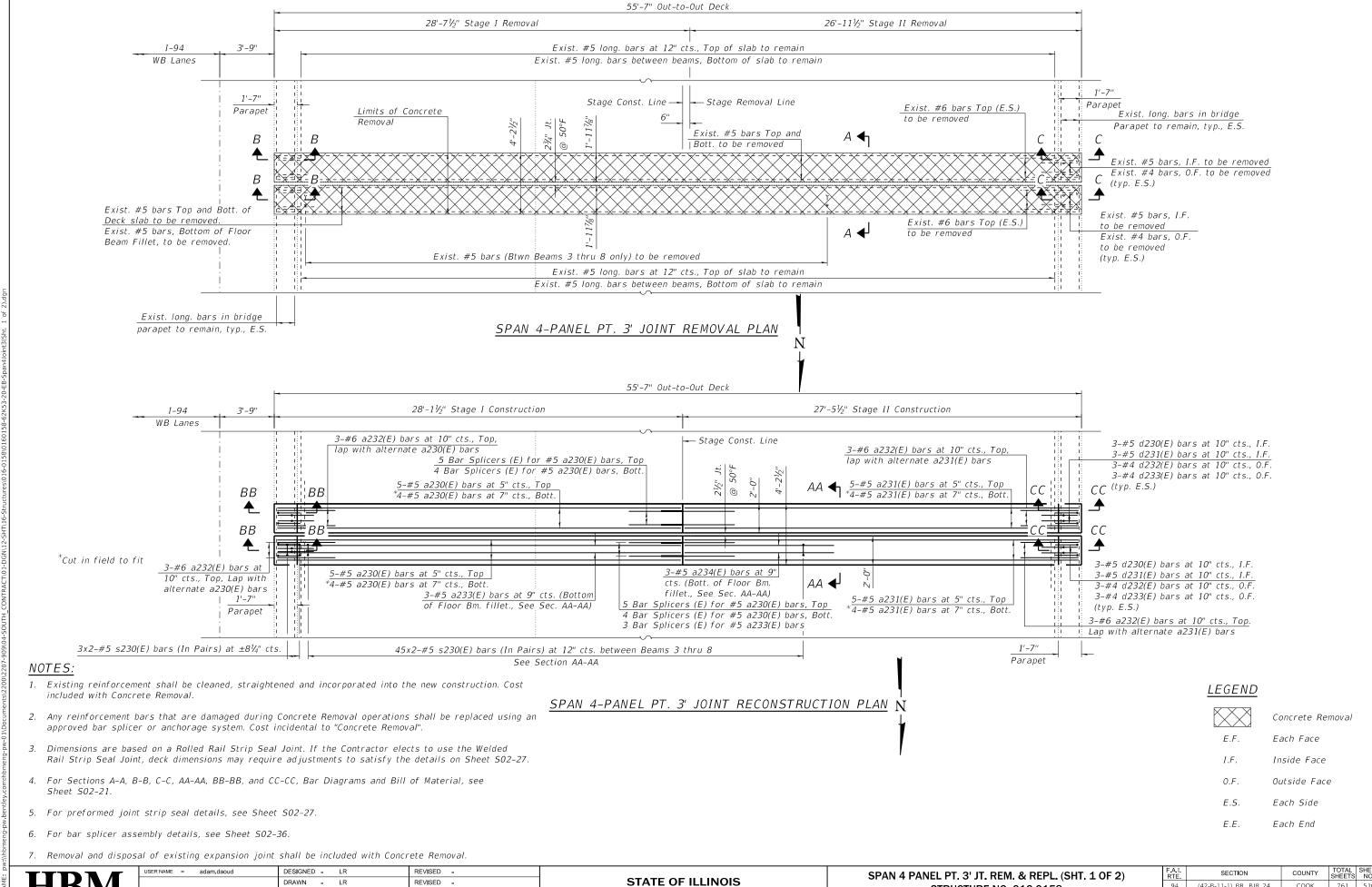
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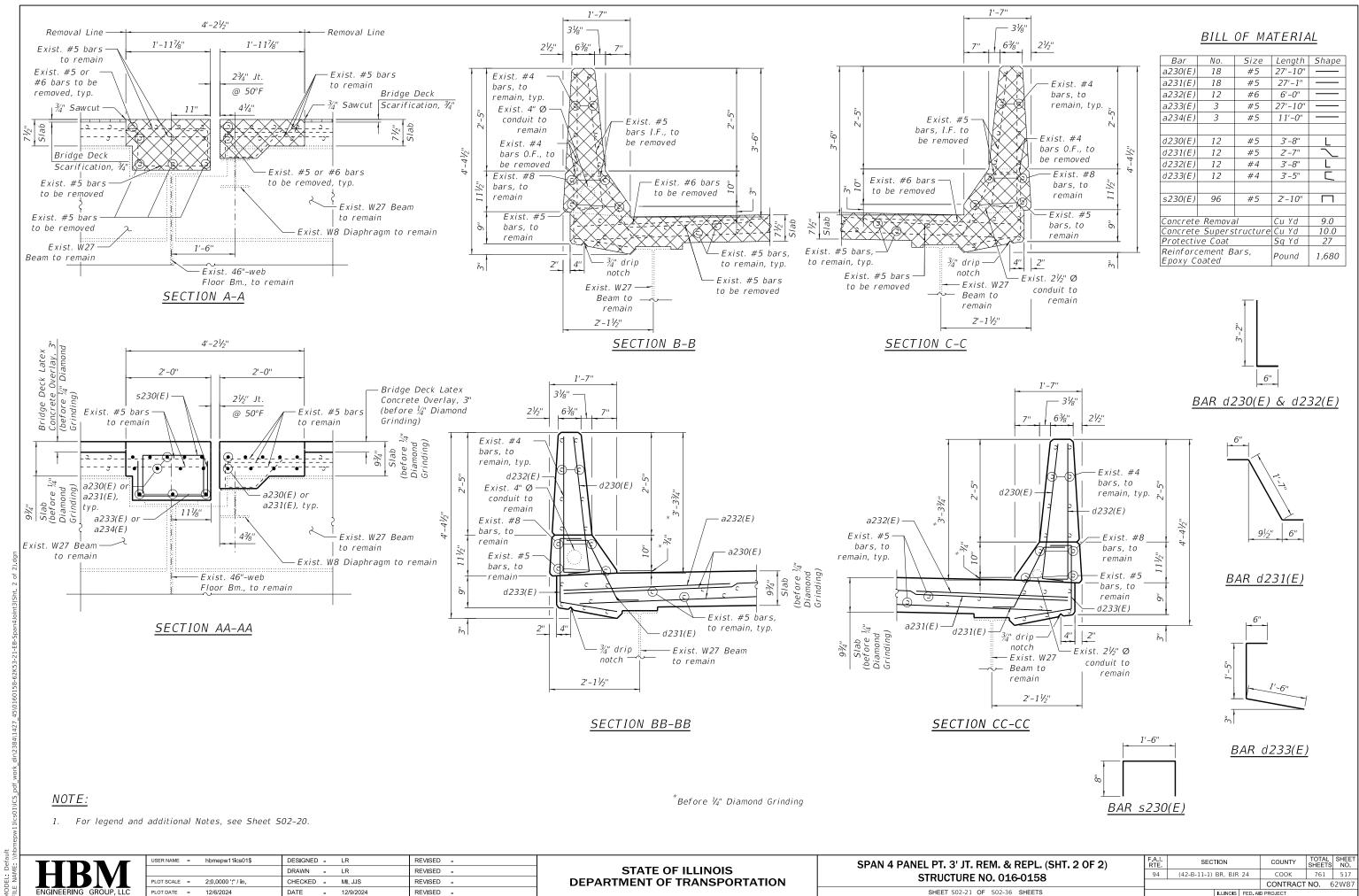
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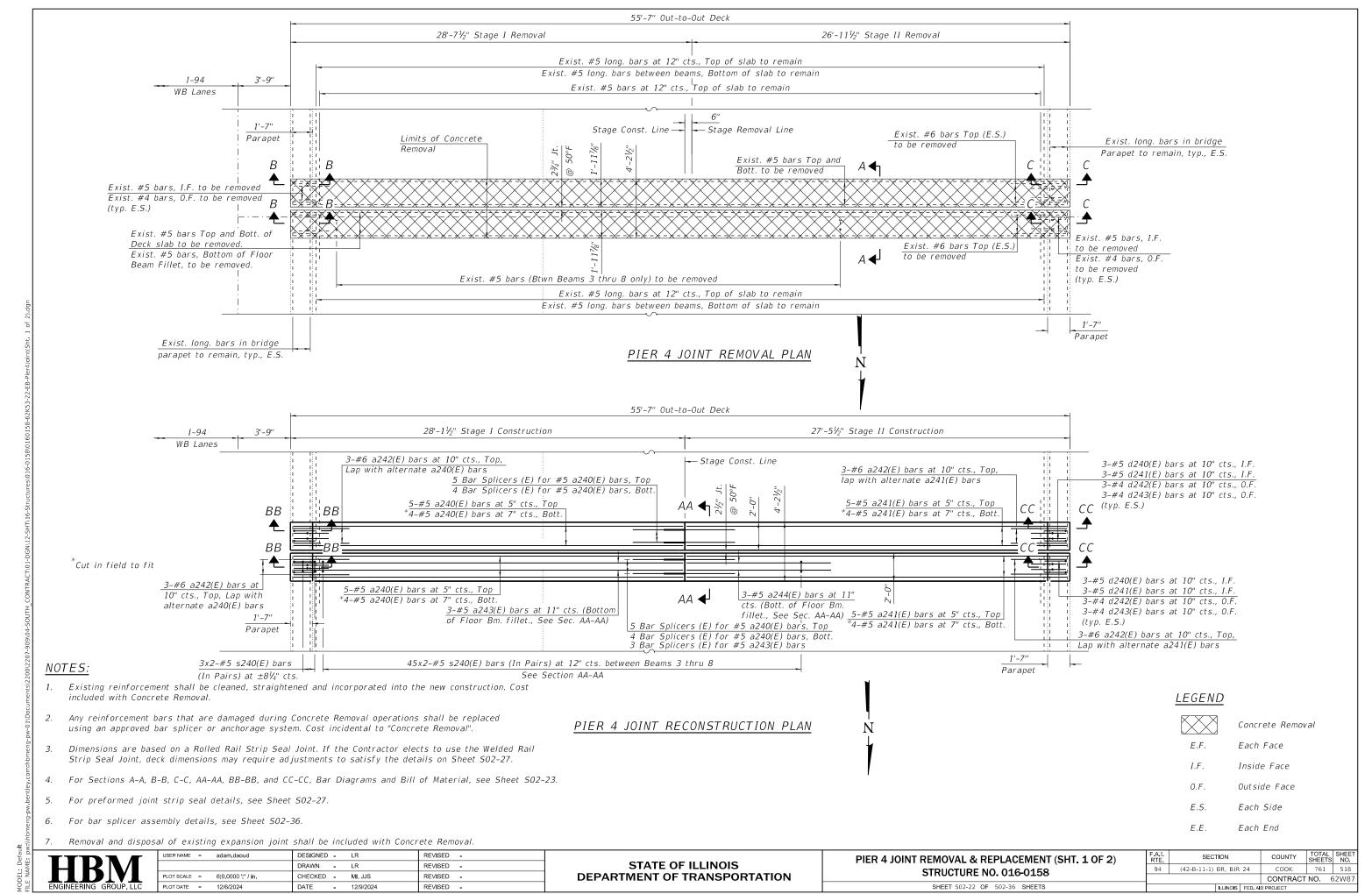
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **STRUCTURE NO. 016-0158** SHEET S02-20 OF S02-36 SHEETS

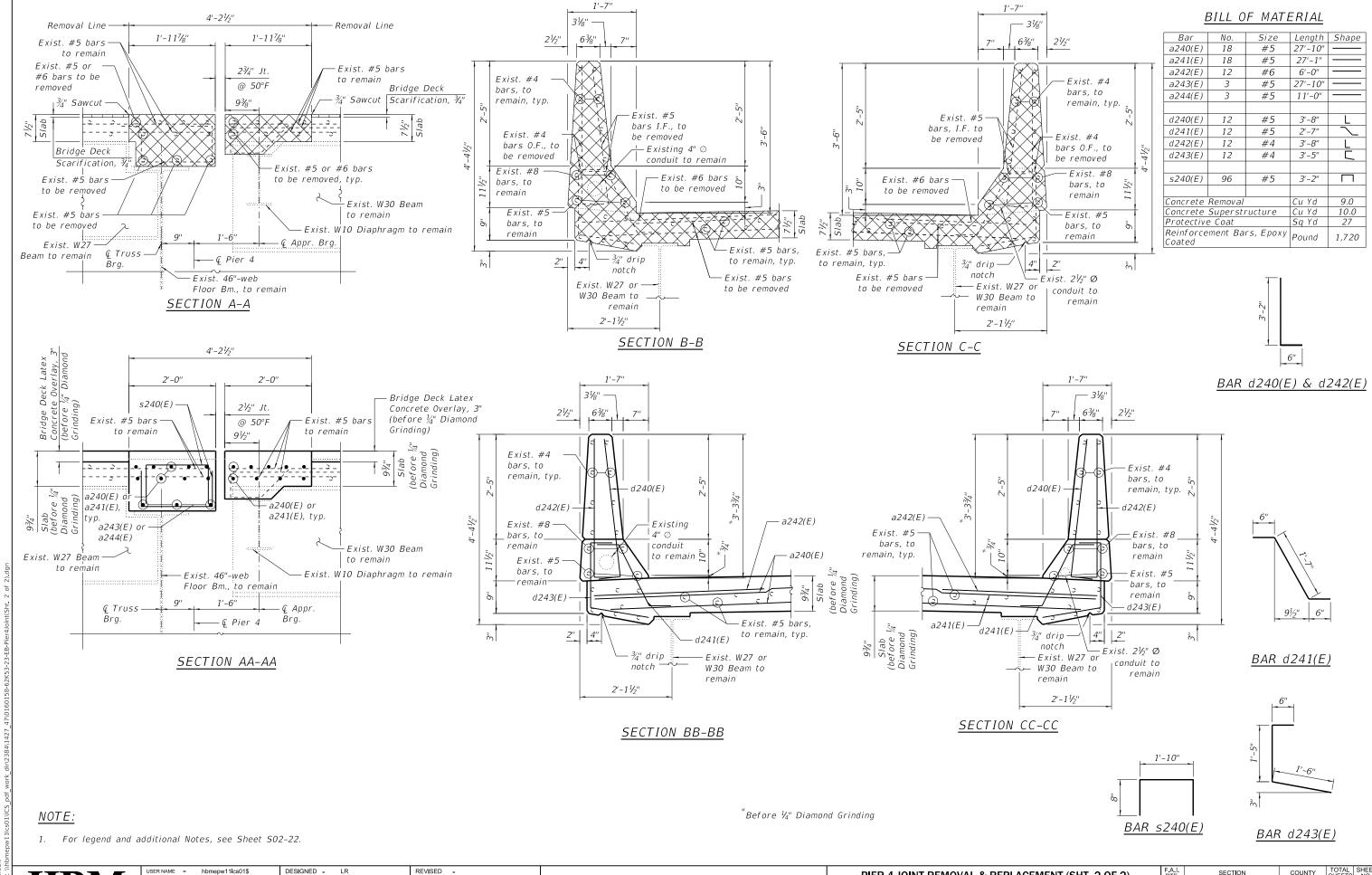
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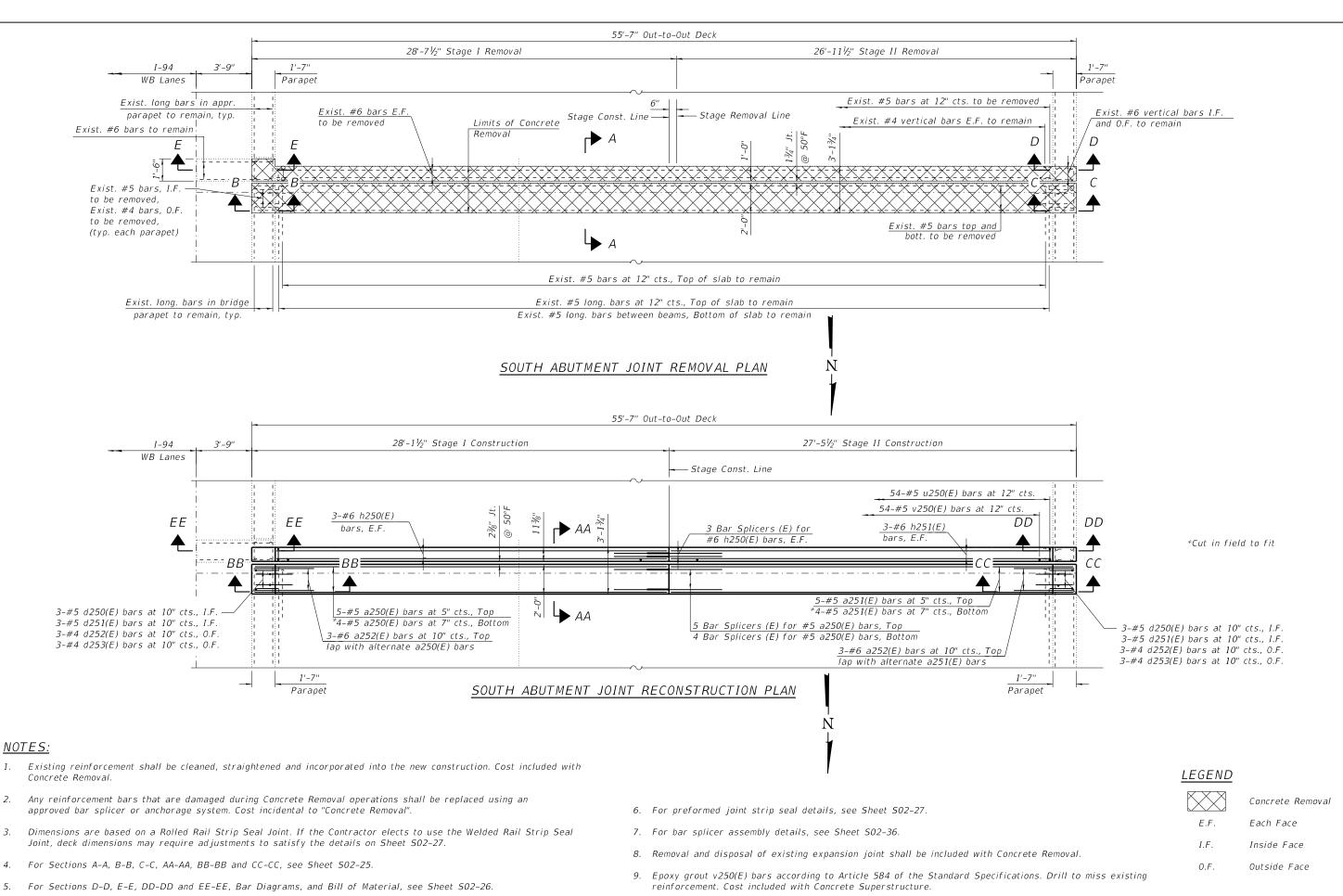


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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PIER 4 JOINT REMOVAL & REPLACEMENT (SHT. 2 OF 2)
STRUCTURE NO. 016-0158

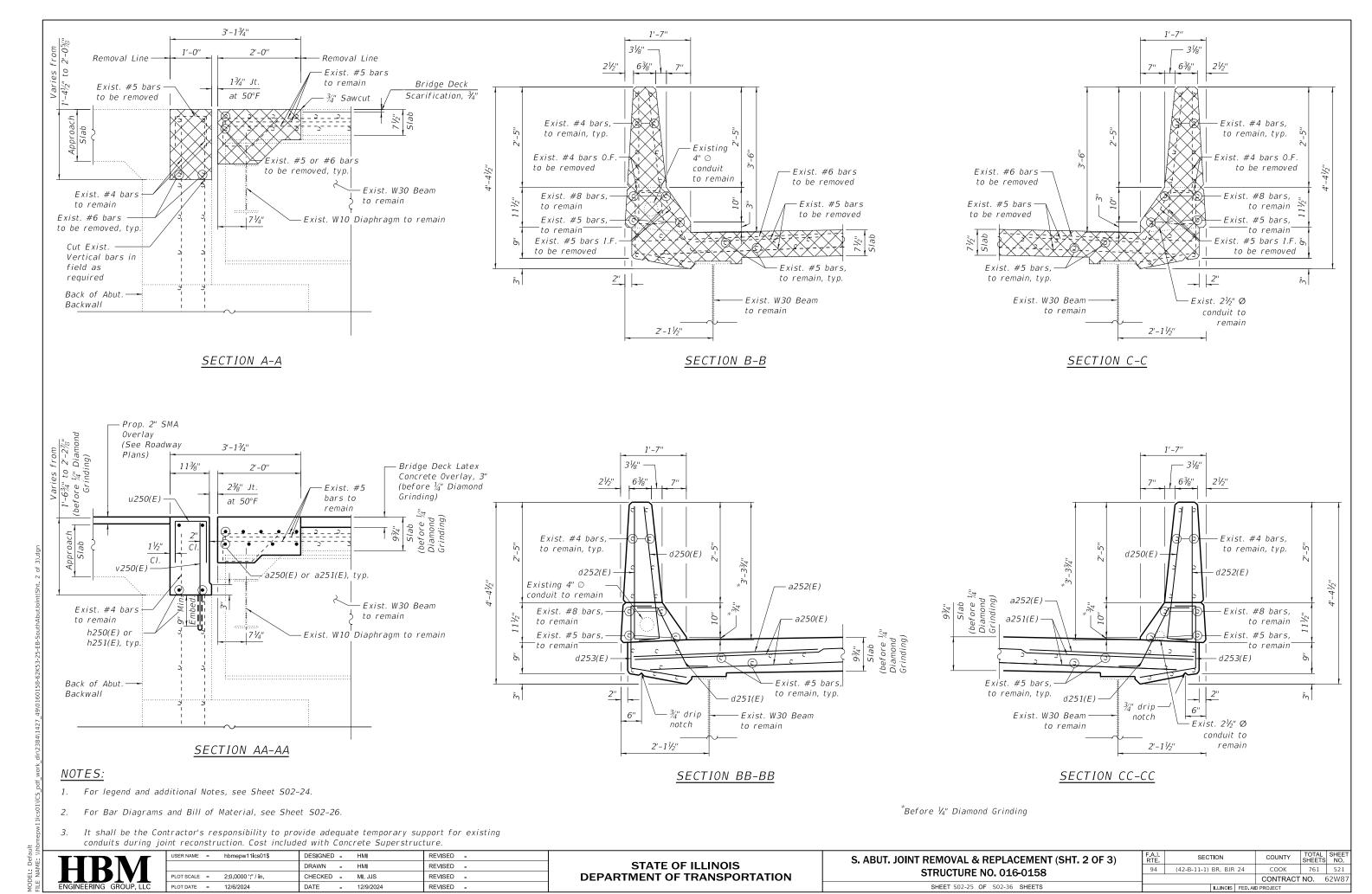
SHEET S02-23 OF S02-36 SHEETS



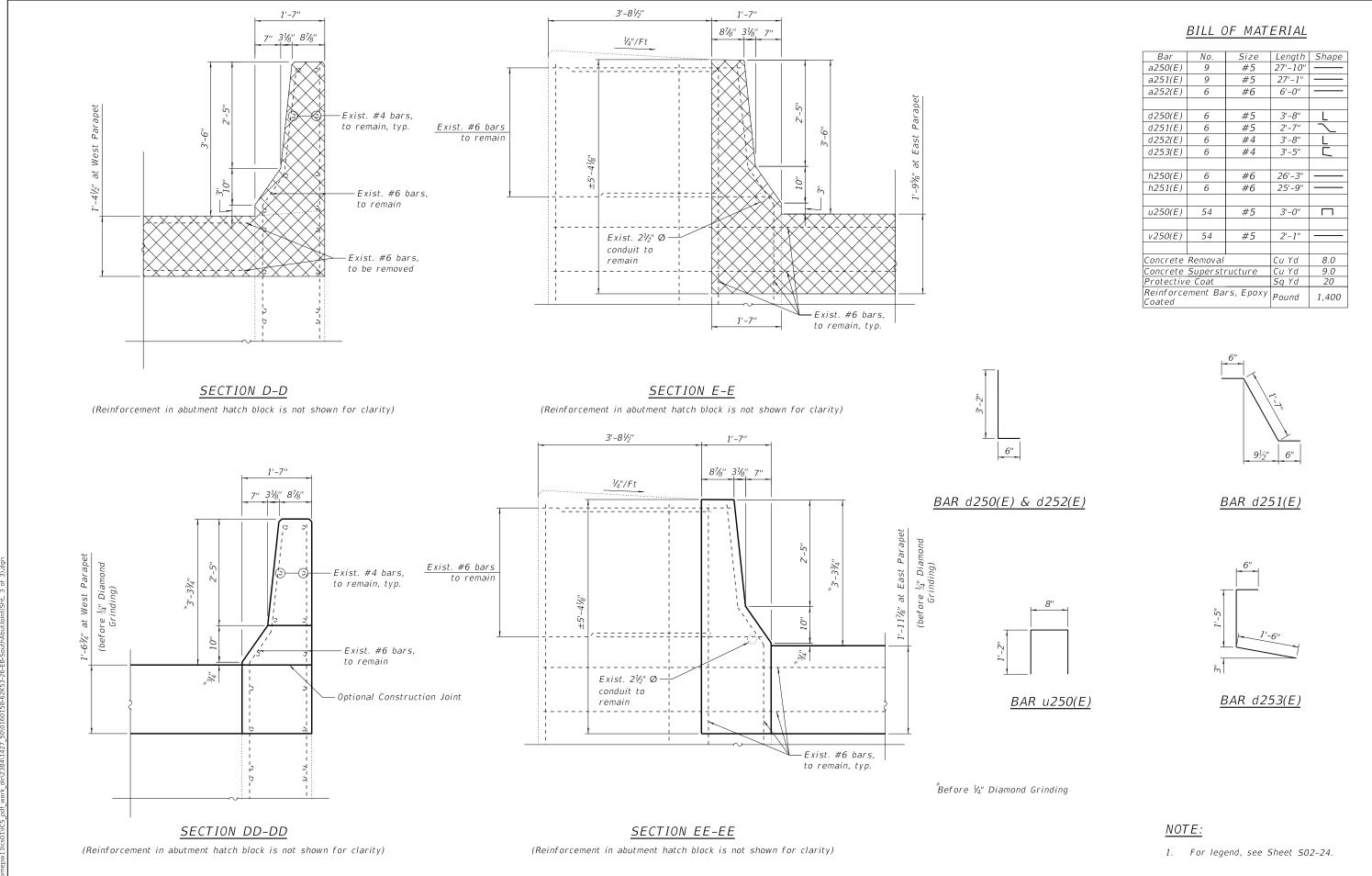
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION S. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 1 OF 3) **STRUCTURE NO. 016-0158** SHEET S02-24 OF S02-36 SHEETS

SECTION (42-B-11-1) BR, BJR 24 COOK 761 520 CONTRACT NO. 62W87



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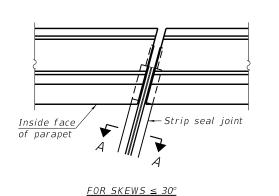
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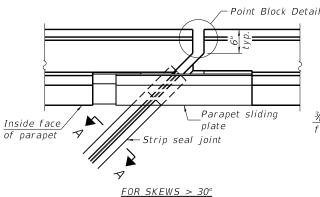
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

S. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 3 OF 3)
STRUCTURE NO. 016-0158

SHEET S02-26 OF S02-36 SHEETS

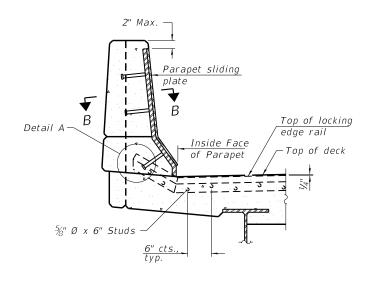
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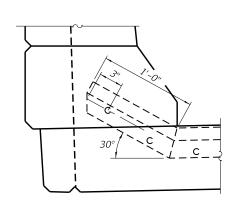




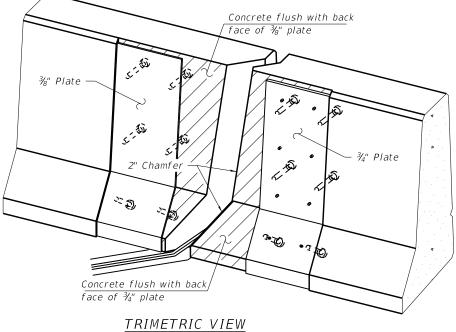
* ¾" Ø x 6" Studs (6 per side 34" parapet) (8 per side 42" parapet) ∏ ¾" Embedded plate | full depth ¾" Embedded plate, full depth 1/2" Parapet sliding plate ¾" Ø Countersunk bolts 1'-0" (8 per side 34" parapet) (10 per side 42" parapet) ____Direction of traffic

SECTION B-B





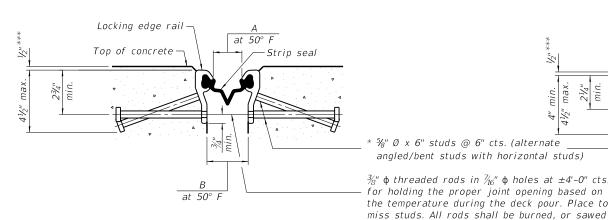
DETAIL A

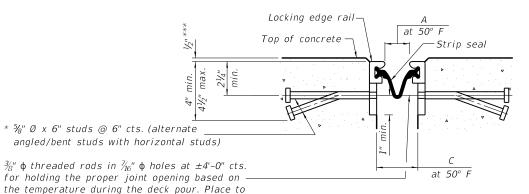


(Showing embedded plates only)

ELEVATION AT PARAPET

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)





LOCKING EDGE RAILS

WELDED RAIL

<u>ROLLED</u>

(EXTRUDED) RAIL

** Back gouge not required if complete joint penetration is verified by mock-up.

The strip seal shall be made continuous and shall have a

minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4

inches. The locking edge rails depicted are configured for

configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided

they fit the application and meet the minimum anchorage

shown. Flanged edge rails, however, will not be allowed.

provided the anchorage system is revised according to the

space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail

recommended installation methods shall be followed. All steel

components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications. The Maximum

splice detail. Cost of parapet sliding plates, embedded plates,

strip seal will vary based on the locking edge rail chosen by

the Contractor. Deck and parapet lengths shown elsewhere in

the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail.

If the Contractor elects to use a different locking edge rail,

dimensional adjustments may be required. One exception to

this would be the strip seal joint at the end of the precast

bridge approach slab. For these cases, the pavement connector length shall be adjusted, not the length of the

bridge approach slab.

anchorage studs, and expansion anchors included with Preformed Joint Strip Seal. 34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the

Locking edge rails may exceed the $4\frac{1}{2}$ " maximum depth

manufacturer's recommendation. The manufacturer's

typical applications and are conceptual only. The actual

LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Preformed Joint Strip Seal	Foot	330

SHOWING ROLLED RAIL JOINT

Joint	Α	В	С
North Abut.	11/2"	23/8"	3"
Span 4 P.P. 3 Jt.	15/8"	21/2"	31/8"
Span 4 P.P. 6 Jt.	15/8"	21/2"	31/8"
Span 4 P.P. 3' Jt.	15/8"	21/2"	31/8"
Pier 4	15/8"	21/2"	31/8"
South Abut.	11/2"	23/8"	3"

SECTION A-A * Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

off flush with the plates after concrete is set.

 *** Before V_4 " Diamond Grinding



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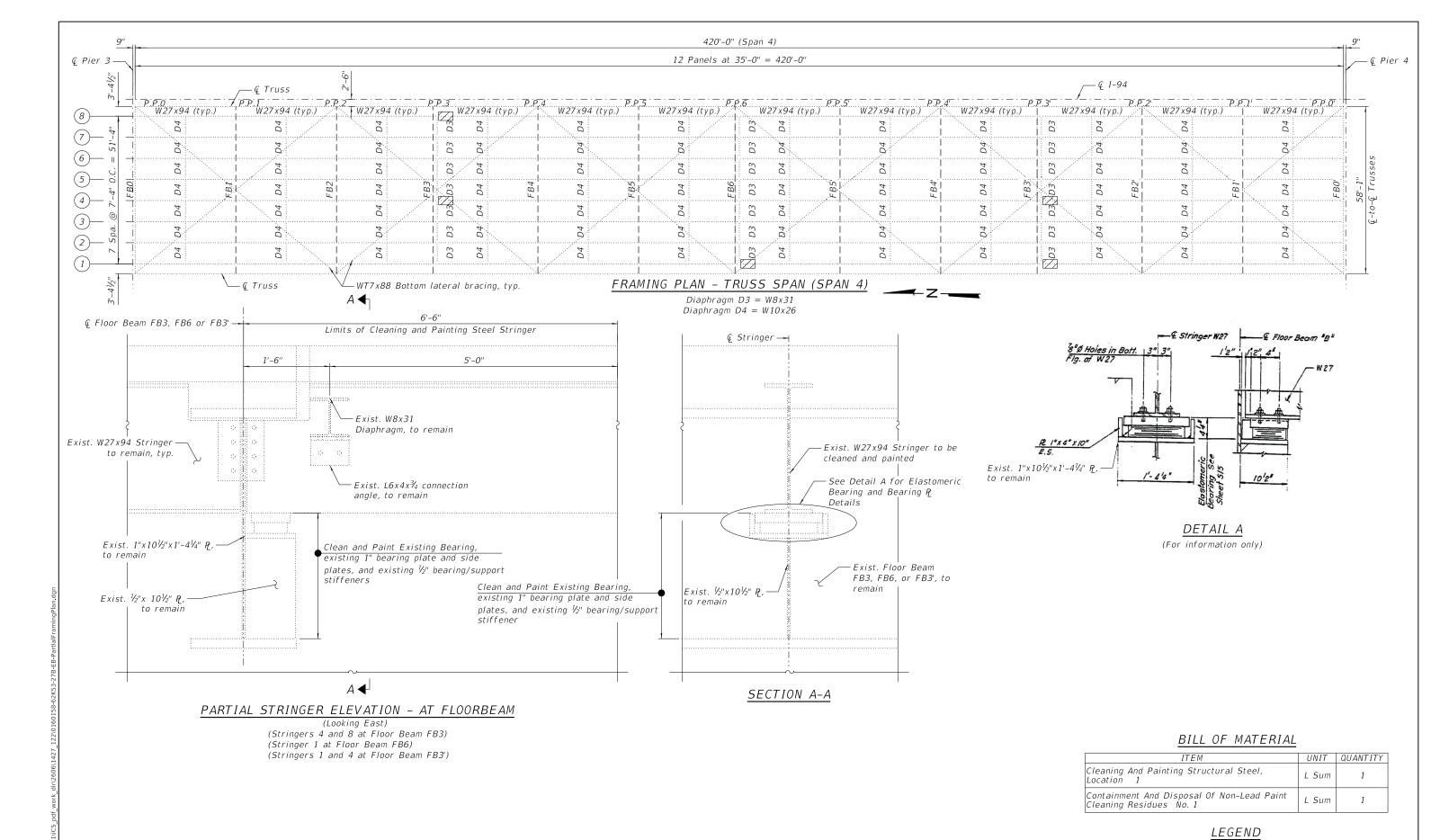
PLAN AT PARAPET

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SHOWING WELDED RAIL JOINT

PREFORMED JOINT STRIP SEAL **STRUCTURE NO. 016-0158** SHEET S02-27 OF S02-36 SHEETS

λ.I. ΓΕ.	SECTI	ON		COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1) B	R, BJR	24	COOK	761	523
				CONTRACT	NO. 6	52W87
	11	LINOIS	EED AI	D DDO IECT		



NOTE:

For Paint Notes, see Sheet S02-02.

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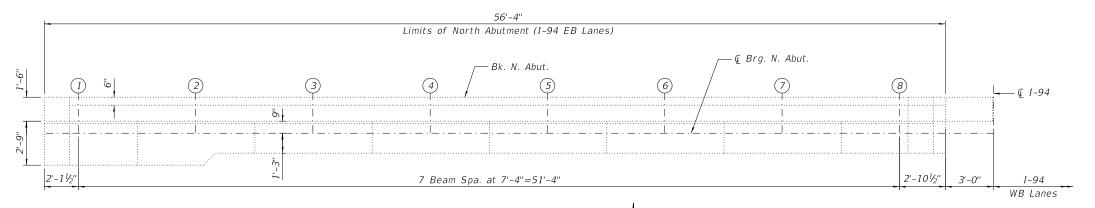
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** PARTIAL FRAMING PLAN & STEEL PAINTING DETAILS **STRUCTURE NO. 016-0158**

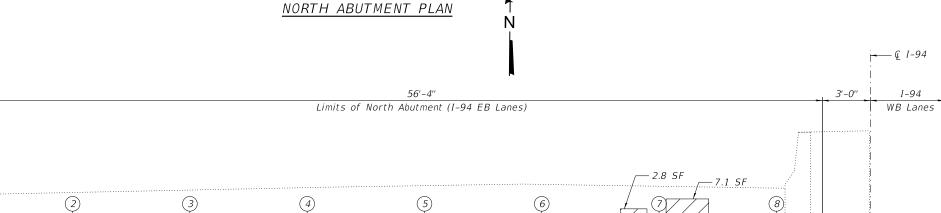
94 (42-B-11-1) BR, BJR 24 COOK 761 523A CONTRACT NO. 62W8

Cleaning and Painting Structural Steel

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	232
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	11



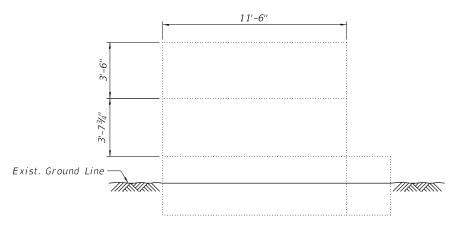


1.0 SF

Exist. Ground Line — 7//X

NORTH ABUTMENT ELEVATION

(Looking North)



<u>LEGEND</u>

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Square Foot

NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Concrete Sealer is to be applied to the abutment seats and the bottom 2 ft. of the abutment backwall.

NORTHWEST WING WALL ELEVATION (Looking East)

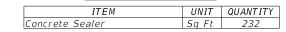
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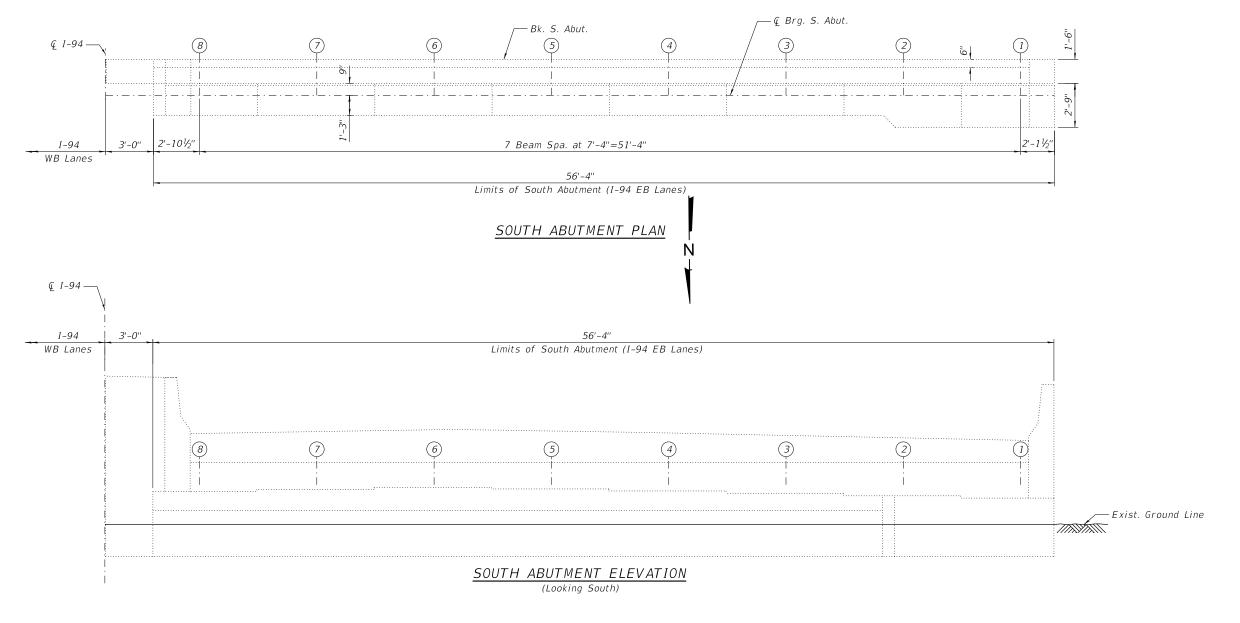
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

NORTH ABUTMENT REPAIRS	F.A.I. RTE	SECTION
STRUCTURE NO. 016-0158	94	(42-B-11-1) BR, E
311(00101)L 140: 010-0130		
PHEET COR SO OF COR SO PHEETS		

COUNTY , BJR 24 COOK 761 524 CONTRACT NO. 62W87

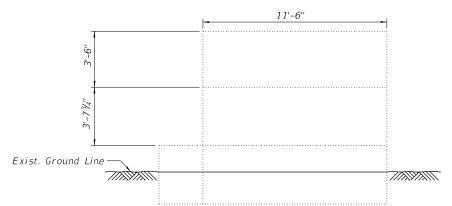
BILL OF MATERIAL





<u>NOTES</u>:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Concrete Sealer is to be applied to the abutment seats and the bottom 2 ft. of the abutment backwall.



SOUTHWEST WING WALL ELEVATION

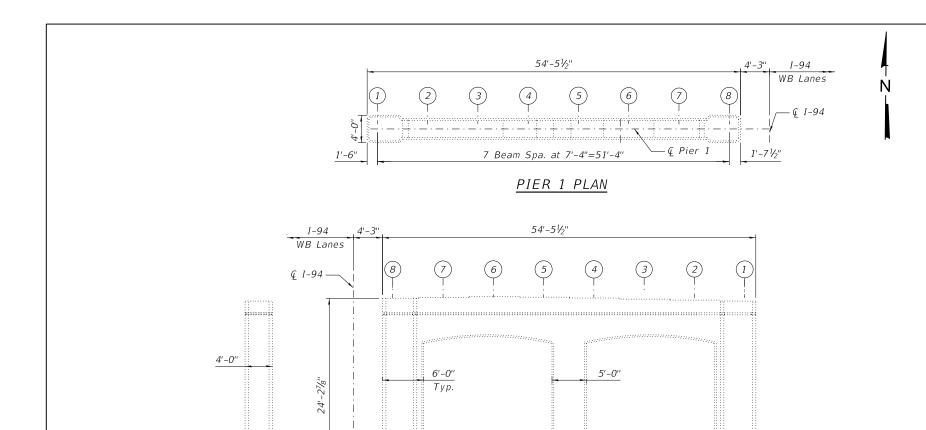
(Looking East)

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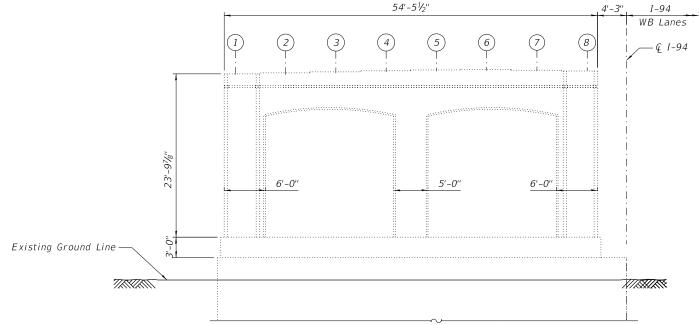
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SOUTH ABUTMENT REPAIRS	F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 016-0158	94	(42-B-11-1) BR, BJR	24	COOK	761	525
311(00101)L 1(0: 010-0130				CONTRACT	NO. 6	52W87
SHEET S02-29 OF S02-36 SHEETS		ILLINOIS	FED. AII	D PROJECT		



END VIEW

PIER 1 ELEVATION
(Looking South)



PIER 1 ELEVATION (Looking North)

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PIER 1 REPAIRS STRUCTURE NO. 016-0158 SHEET 502-30 OF 502-36 SHEETS

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

HBM

Existing Ground Line —

///XV/X

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NOTES:

1. Quantitie

-Existing Ground Line

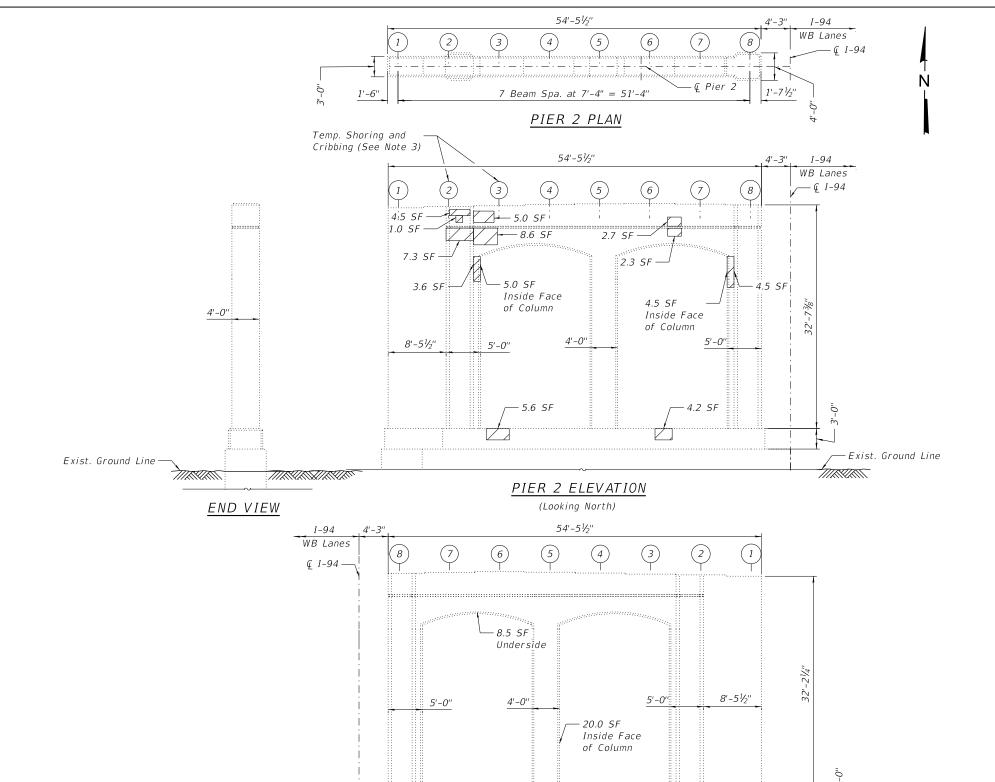
 Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

2. The Contractor is responsible to remove, support, and reinstall all existing utilities interfering with the work. Cost shall be included with Structural Repair of Concrete (Depth Equal To or Less Than 5").

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

SF Square Foot



BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	88
Temporary Shoring And Cribbing	Each	2

*SUMMARY OF REACTIONS					
Pier 2, Beams 2 & 3					
R DL	(k)	91.0			
R LL	(k)	54.0			
R IM	(k)	14.6			
R Total	(k)	159.6			

^{*}Taken from Existing Plans

NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. The Contractor is responsible to remove, support, and reinstall all existing utilities interfering with the work. Cost shall be included with Structural Repair of Concrete (Depth Equal To or Less Than 5").
- 3. Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

LEGEND

- Exist. Ground Line

///



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

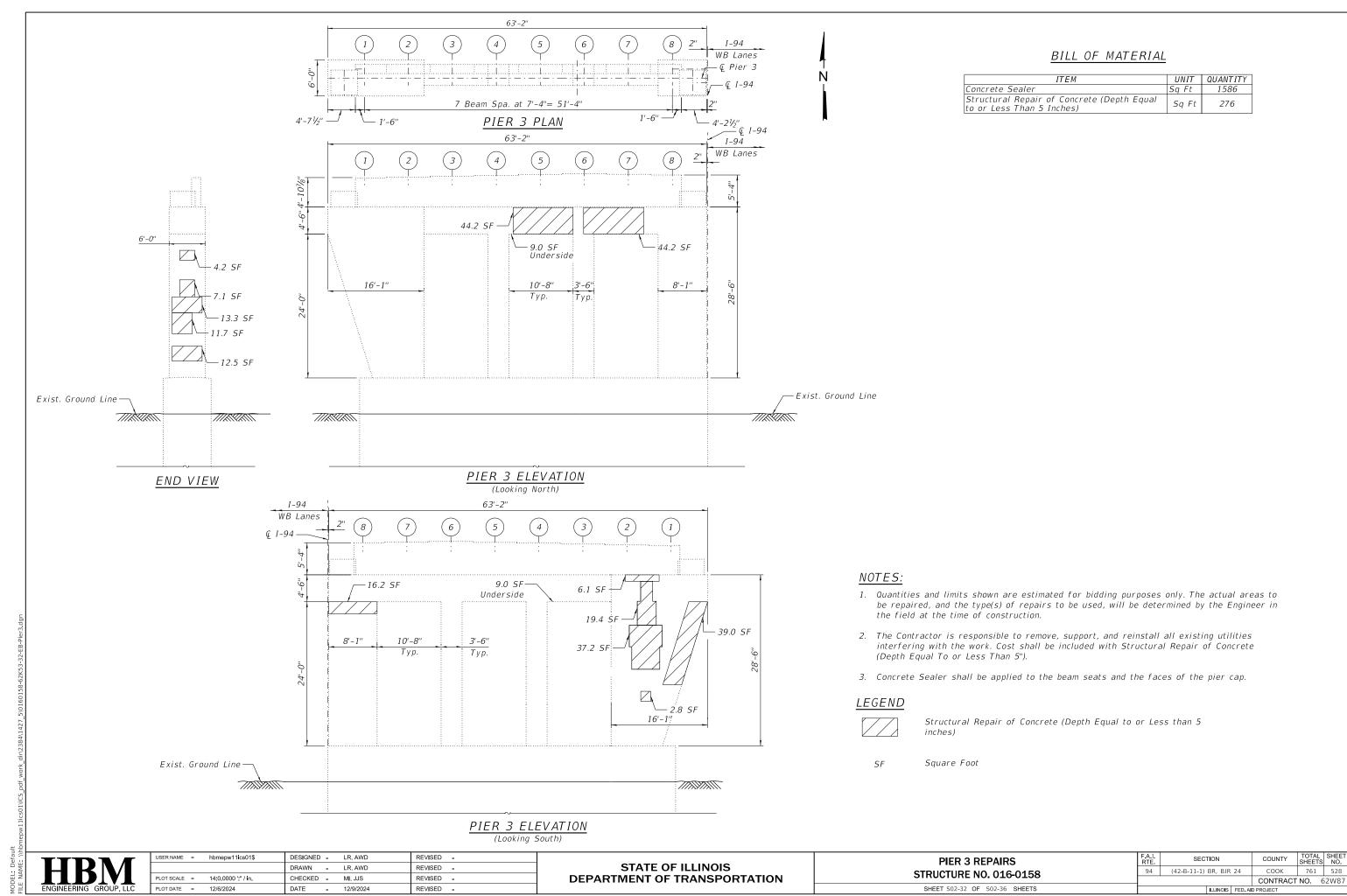
SF Square Foot

PIER 2 ELEVATION

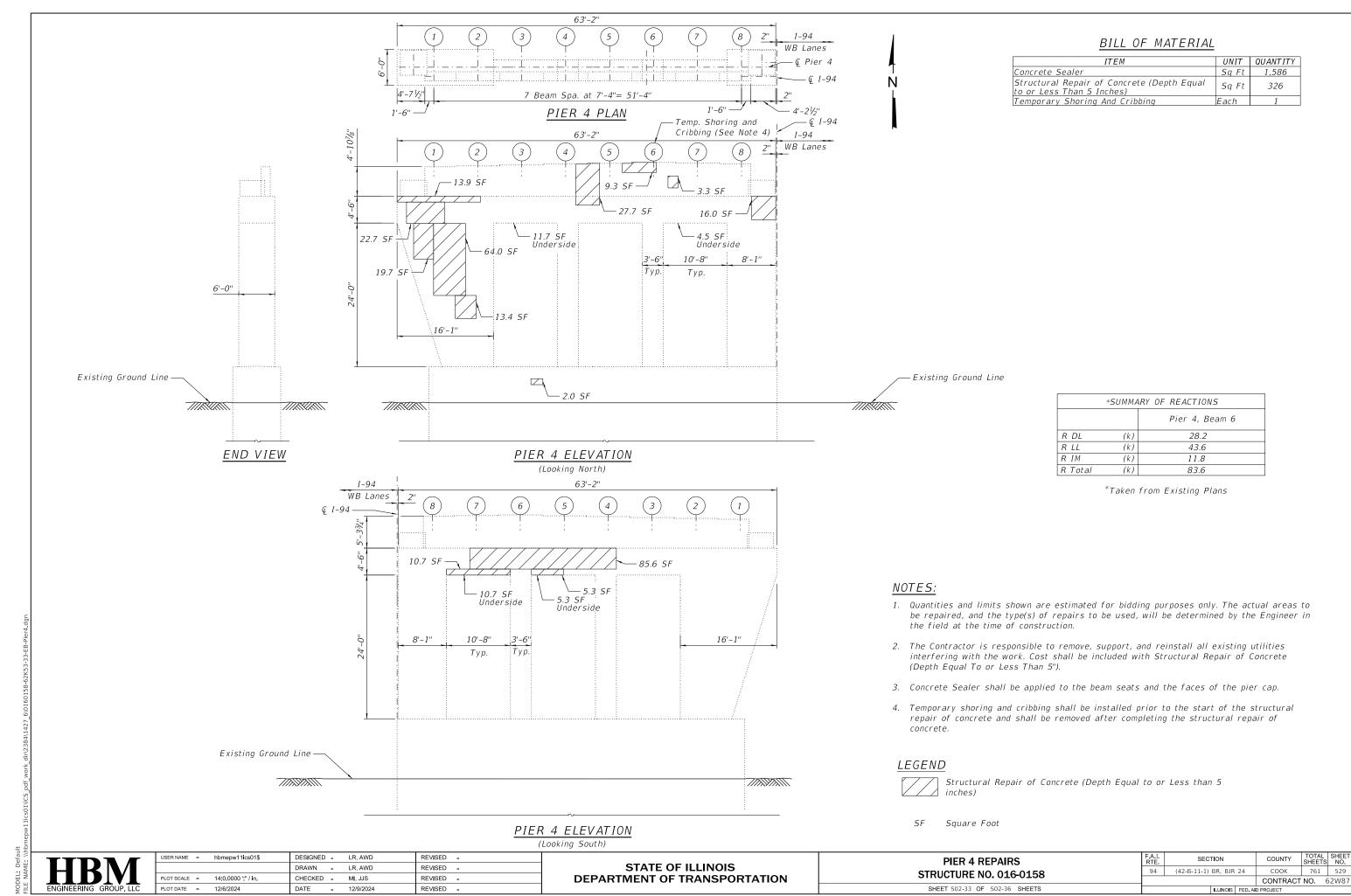
(Looking South)



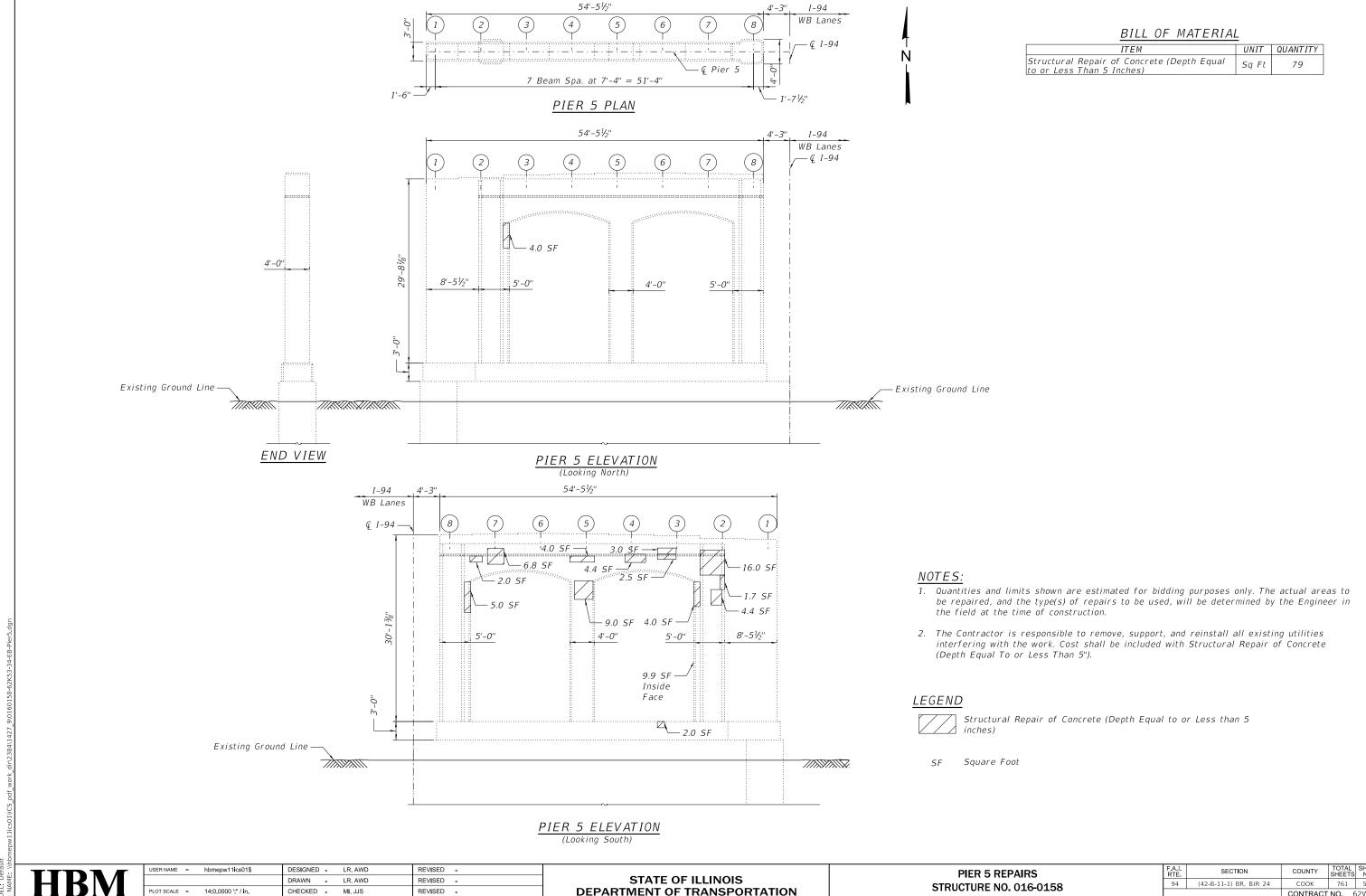
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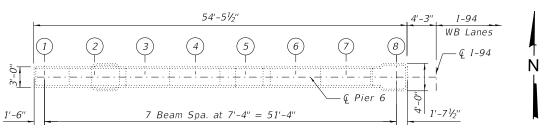


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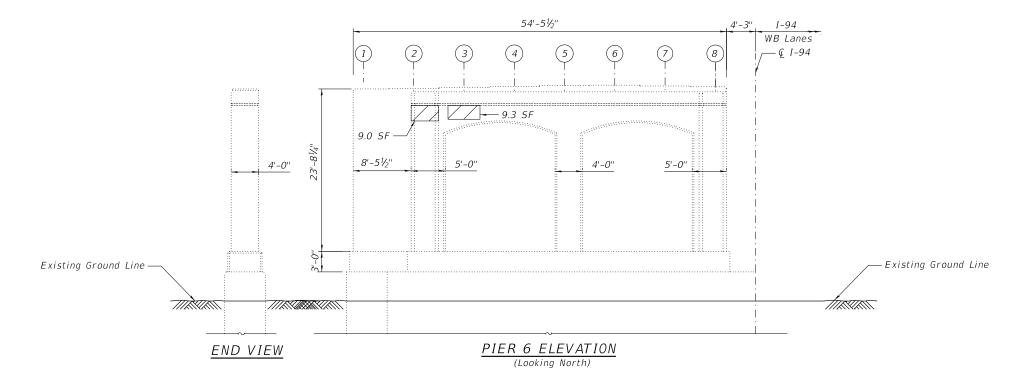
DEPARTMENT OF TRANSPORTATION

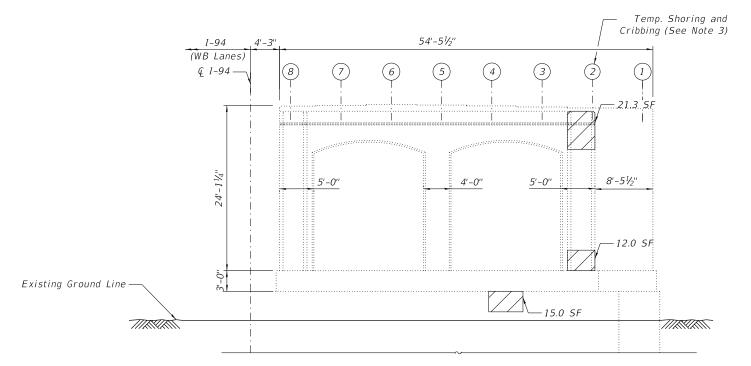
STRUCTURE NO. 016-0158 SHEET S02-34 OF S02-36 SHEETS

COOK 761 530 CONTRACT NO. 62W87



PIER 6 PLAN





PIER 6 ELEVATION (Looking South)

DIONED LD AWD

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Structural Repair of Concrete (Depth Equal to or Less Than 5 Inches)	Sq Ft	67
Temporary Shoring And Cribbing	Each	1

	*SUMMARY OF REACTIONS					
		Pier 6, Beam 2				
R DL	(k)	91.0				
R LL	(k)	54.0				
R IM	(k)	14.6				
R Total	(k)	159.6				

*Taken from Existing Plans

NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. The Contractor is responsible to remove, support, and reinstall all existing utilities interfering with the work. Cost shall be included with Structural Repair of Concrete (Depth Equal To or Less Than 5").
- Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

SF Square Foot

HBM ENGINEERING GROUP, LLC

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

PIER 6 STRUCTURE			-
SHEET S02-35	OF	S02-36	SHEETS

 F.A.I. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL NO.
 SHEET NO.

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 CONTRACT NO.
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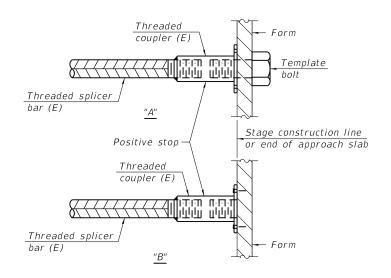
STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

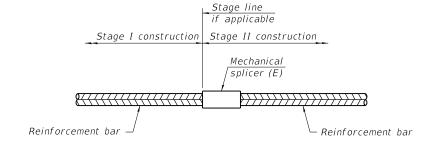
Location	Bar Size	No. assemblies required	Minimum Iap length
North Abut.	#5	9	3'-6"
	#6	6	5'-6"
Span 4 P.P. 3 Jt.	#5	21	3'-6"
Span 4 P.P. 6 Jt.	#5	21	3'-6"
Span 4 P.P. 3' Jt.	#5	21	3'-6"
Pier 4	#5	21	3'-6"
South Abut.	#5	9	3'-6"
	#6	6	5'-6"



INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.

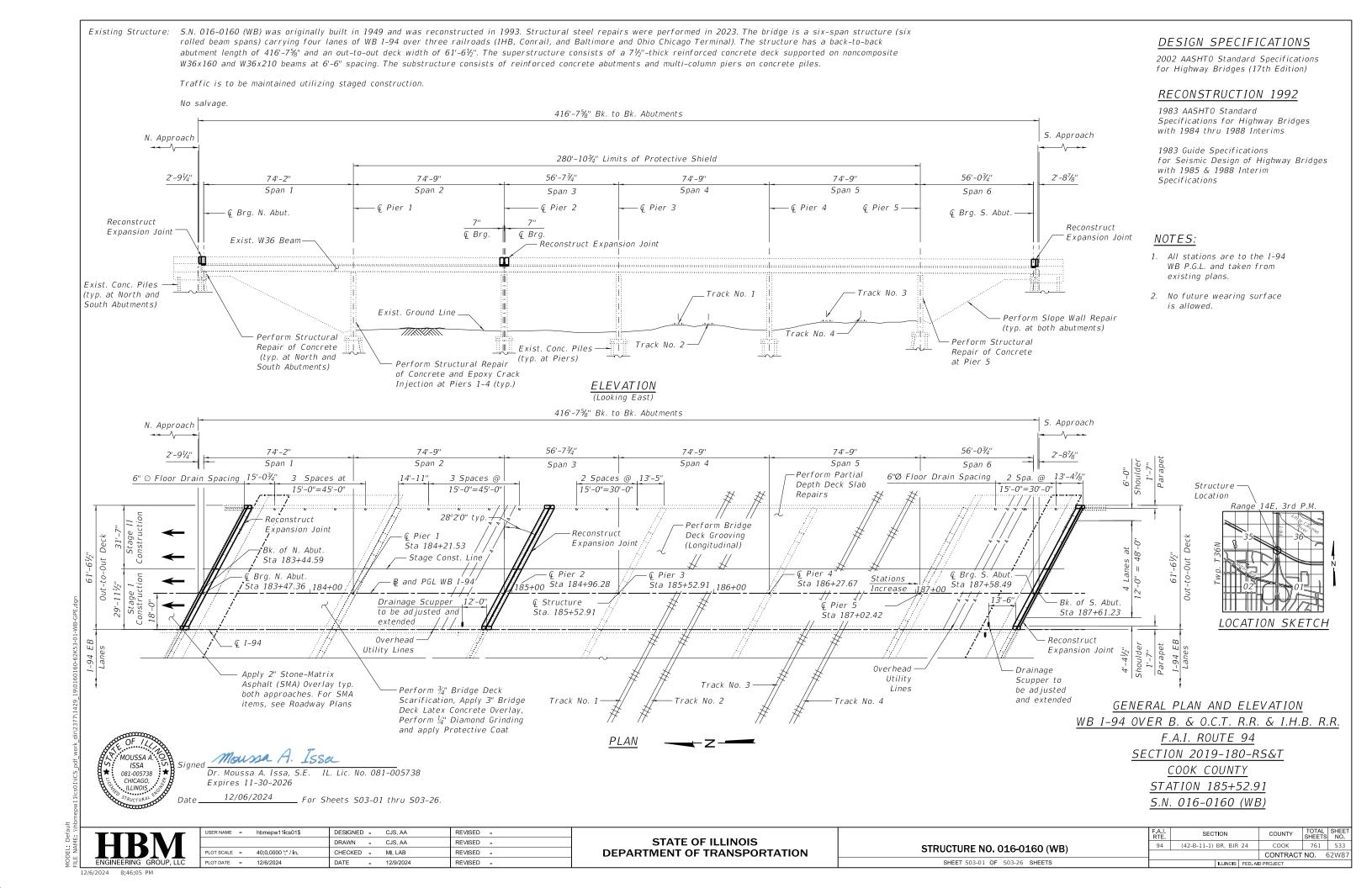
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- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Plan dimensions and details relative to the existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall field-verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not cause for additional compensation for a change in the scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 3. The Contractor may request copies of existing construction plans that are currently on file with the Illinois Department of Transportation (IDOT). The request shall be in writing with the understanding that any reproduction cost will be the Contractors expense and at no additional cost to the Department.
- 4. All exposed concrete edges shall have a ¾" x 45" chamfer except where shown
- 5. Protective coat shall be applied to the top of reconstructed transverse joint areas, top of new latex concrete overlay, and top and inside faces of parapets.
- 6. Joint openings shall be adjusted according to Article 520.04 of the standard specifications when the deck is poured at an ambient temperature other than 50°F.
- 7. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose detrimental foreign material shall be removed from the surfaces in contact with concrete (SSPC-SP3 standards). Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be paid for according to Article 109.04 of the Standard Specifications. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding V_4 inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 8. The Contractor shall take all necessary precautions for the protection of passing vehicles and pedestrians from falling objects and/or materials until completion of the
- 9. It shall be the Contractors responsibility to locate and protect any utilities or facilities on, within or under the bridge deck including but not limited to under deck lighting, traffic signals or signs attached to the structure. Any damage to existing utilities caused by the Contractor in the performance of the work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 10. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to existing elements to remain cause by the Contractor in the performance of the work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 11. Cleaning and field painting of structural steel shall be done under a separate painting contract
- 12. For SMA overlay on Approach Slab, see Civil Sheets.
- 13. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 14. Adjacent I-94 EB bridge is not shown throughout the plans for clarity.
- 15. The Contractor is responsible to protect the existing conduit and junction box embedded in the parapet during removal and construction. Any damage to the existing conduit and junction box shall be repaired by the Contractor at no additional cost to the Department.
- 16. Concrete Sealer shall be applied to the designated areas of the abutments and piers.
- 17. Prior to the application of the Concrete Sealer, the Contractor shall clean all existing debris from the abutment seats. The method of debris removal shall not damage the existing concrete and shall be approved by the Engineer. See special provision for Debris Removal.
- 18. The Engineer shall show actual locations and size of deck repairs on As-built Plans.

INDEX OF SHEETS

503-01 503-02	General Plan and Elevation General Notes, Index of Sheets & TBOM
503-03	Stage Construction (Sheet 1 of 2)
503-04	Stage Construction (Sheet 2 of 2)
<i>S03-05</i>	Temporary Concrete Barrier
<i>503-06</i>	Deck Repair Plan (Sheet 1 of 2)
<i>S03-07</i>	Deck Repair Plan (Sheet 2 of 2)
503-08	Drainage Scupper Adjustment Details
503-09	N. Abut. Joint Removal & Replacement (Sht. 1 of 3)
503-10	N. Abut. Joint Removal & Replacement (Sht. 2 of 3)
S03-11	N. Abut. Joint Removal & Replacement (Sht. 3 of 3)
S03-12	Pier 2 Joint Removal & Replacement (Sht. 1 of 2)
S03-13	Pier 2 Joint Removal & Replacement (Sht. 2 of 2)
S03-14	S. Abut. Joint Removal & Replacement (Sht. 1 of 3)
S03-15	S. Abut. Joint Removal & Replacement (Sht. 2 of 3)
S03-16	S. Abut. Joint Removal & Replacement (Sht. 3 of 3)
S03-17	Preformed Joint Strip Seal
S03-18	North Abutment Repairs
503-19	South Abutment Repairs
503-20	Pier 1 Repairs
503-21	Pier 2 Repairs
503-22	,
503-23	Pier 4 Repairs

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	Cu Yd	-	1	1
Concrete Removal	Cu Yd	33.2	-	33.2
Protective Shield	Sq Yd	1,921	-	1,921
Concrete Superstructure	Cu Yd	37.3	-	37.3
Protective Coat	Sq Yd	3,099	-	3,099
Reinforcement Bars, Epoxy Coated	Pound	5,020	-	5,020
Bar Splicers	Each	48	-	48
Preformed Joint Strip Seal	Foot	210	-	210
Concrete Sealer	Sq Ft	-	4,189	4,189
Epoxy Crack Injection	Foot	-	16	16
Chain Link Fence, 5'	Foot	-	5	5
Bridge Deck Grooving (Longitudinal)	Sq Yd	2,217	-	2,21
Slope Wall Crack Sealing	Foot	-	84	84
Deck Drain Extensions	Each	16	-	16
Drainage Scuppers To Be Adjusted	Each	2	-	2
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	2,617	-	2,61
Bridge Deck Scarification 3/4"	Sq Yd	2,617	-	2,61
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	-	160	160
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	-	19	19
Diamond Grinding (Bridge Section)	Sq Yd	2,511	-	2,51
Temporary Shoring And Cribbing	Each	-	2	2

PROPOSED SCOPE OF WORK

S03-24 Pier 5 Repairs S03-25 Slope Wall Repairs

1. Provide Protective shield within limits indicated on the plans.

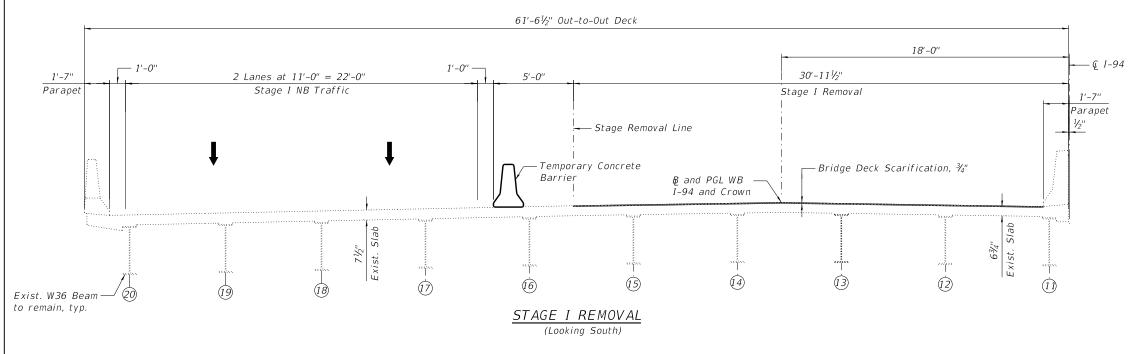
S03-26 Bar Splicer Assembly & Mechanical Splicer Detail

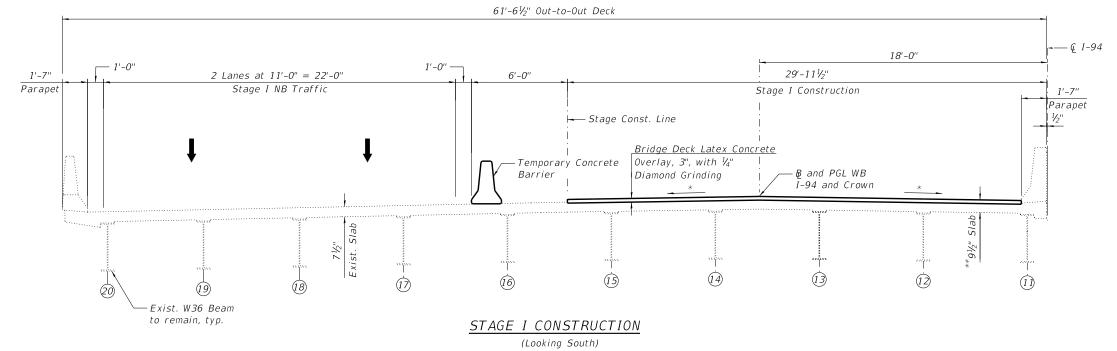
- Perform Deck Slab Repairs and adjust and extend existing scuppers and floor drains as required.
- 3. Perform ¾" Bridge Deck Scarification.
- Reconstruct Expansion Joints at the North and South abutments and Pier 2 and install new preformed joint strip seals.
- 5. Apply 3" Bridge Deck Latex Concrete Overlay on Bridge Deck.
- 6. Perform $\frac{1}{4}$ " Diamond Grinding to top of bridge deck and abutment hatch block.
- 7. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes.
- Apply protective coat to the top of reconstructed transverse joint areas, top of new latex concrete overlay and top and inside faces of parapets.
- Perform structural repair of concrete to all spalled and delaminated areas, and perform low pressure epoxy injection to all open cracks ($\frac{1}{18}$ "-wide and wider), for the abutments and piers as noted on the plans.
- 10. Perform Slope Wall repairs.

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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

GENERAL NOTES, INDEX OF SHEETS & TBOM	F.A.I. RTE.	SECTION
STRUCTURE NO. 016-0160 (WB)		(42-B-11-1) BI
STRUCTURE NO. 010-0100 (WD)		
SHEET S03-02 OF S03-26 SHEETS		п





STAGE I REMOVAL

- 1. Install temporary concrete barrier as shown to locate traffic on the west side of the existing structure.
- 2. Perform ¾" bridge deck scarification.
- 3. Remove portions of bridge deck/approach slab adjacent to expansion joints at the North and South Abutments, and remove portions of bridge deck slab adjacent to expansion joint at Pier 2

STAGE I CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct transverse expansion joints and install new preformed joint strip seals within the limits of Stage I Construction.
- 3. Adjust existing drainage scuppers per the details shown in the plans.
- 4. Perform structural repair of concrete and epoxy crack injection for the abutments and piers.
- 5. Apply 3" bridge deck latex concrete overlay.
- 6. Perform ¼" diamond grinding to bridge deck and abutment hatch block.
- 7. Perform Bridge Deck Grooving (Longitudinal) for the 3" bridge deck latex concrete overlay and reconstructed expansion joint areas.
- 8. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
- 9. Apply protective coat to top and inside faces of parapets, reconstructed transverse expansion joint areas and to the surface of the new overlay.
- 10. Perform slope wall repairs as shown on the plans.

NOTES:

- 1. For Temporary Concrete Barrier details, see Sheet S03-05.
- 2. For quantity of Temporary Concrete Barrier, see Roadway Plans.

*Match Existing Cross-slopes

**After grinding

HBM ENGINEERING GROUP, LLC

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

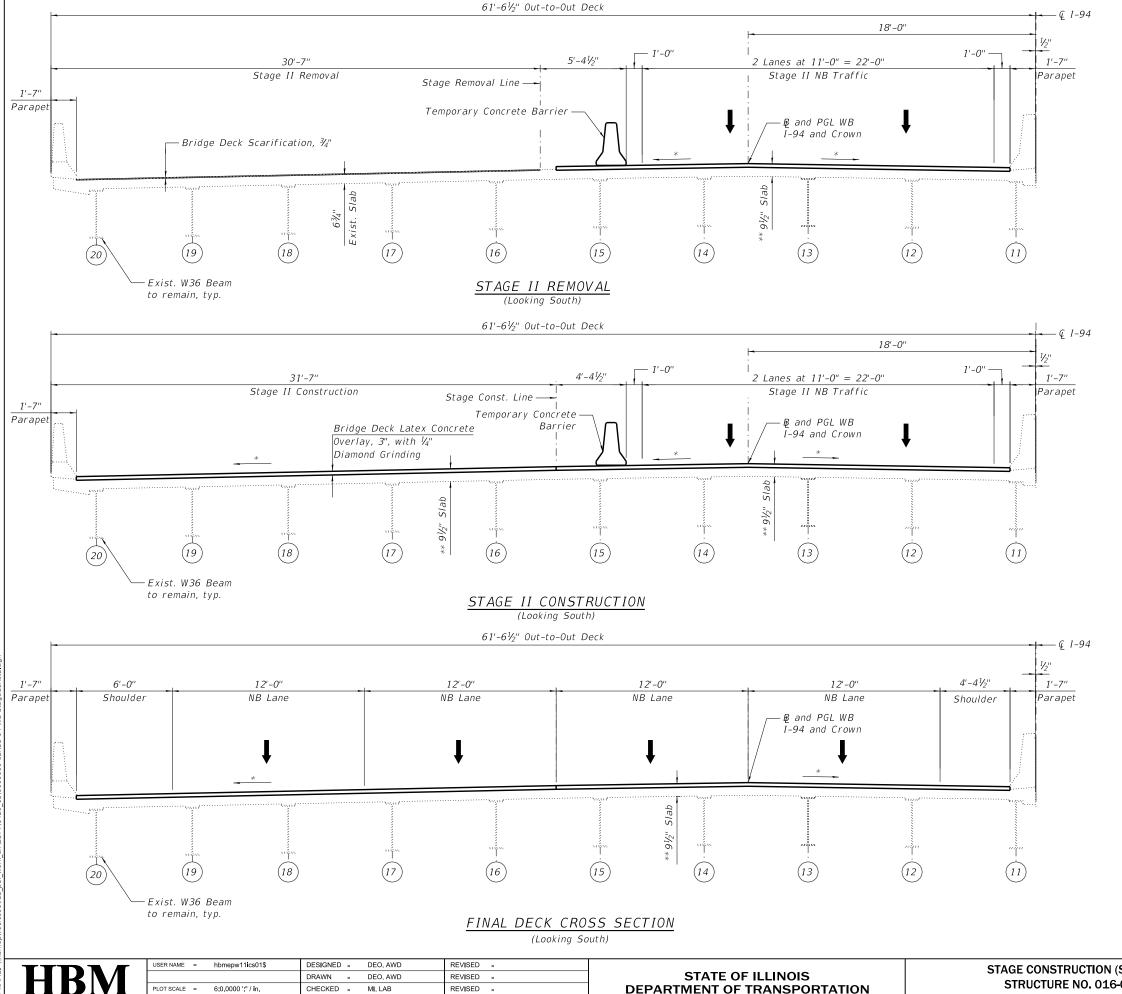
STAGE CONSTRUCTION (SHEET 1 OF 2)
STRUCTURE NO. 016-0160 (WB)

SHEET 503-03 OF 503-26 SHEETS

 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEET NO.

 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 535

 CONTRACT NO.
 62W87



STAGE II REMOVAL

- 1. Relocate temporary concrete barrier as shown to locate traffic on the west side of the existing structure.
- 2. Perform ¾" bridge deck scarification.
- 3. Remove portions of bridge deck/approach slab adjacent to expansion joints at the North and South Abutments, and remove portions of bridge deck slab adjacent to expansion joint at Pier 2.

STAGE II CONSTRUCTION

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct transverse expansion joints and install new preformed joint strip seals within the limits of Stage II Construction.
- 3. Perform structural repair of concrete and epoxy crack injection for the abutments and piers.
- 4. Apply 3" bridge deck latex concrete overlay.
- 5. Perform $\frac{1}{4}$ " diamond grinding to bridge deck and abutment hatch block.
- 6. Perform Bridge Deck Grooving (Longitudinal) for the 3" bridge deck latex concrete overlay and reconstructed expansion joint areas.
- 7. Apply 2" Stone-Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
- 8. Apply protective coat to top and inside faces of parapets, reconstructed transverse expansion joint areas, and to the surface of the new overlay.
- 9. Perform slope wall repairs as shown on the plans.

NOTES:

- 1. For Temporary Concrete Barrier details, see Sheet S03-05.
- 2. For quantity of Temporary concrete Barrier, see Roadway Plans.

*Match Existing Cross-slopes

**After grinding

STAGE CONSTRUCTION (SHEET 2 OF 2) STRUCTURE NO. 016-0160 (WB) SHEET S03-04 OF S03-26 SHEETS

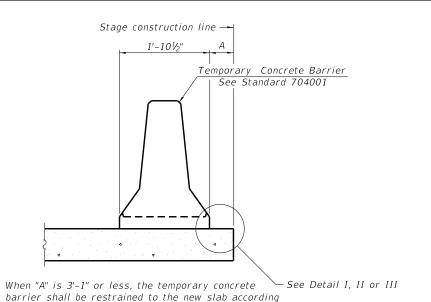
SECTION (42-B-11-1) BR, BJR 24 соок 761 536 CONTRACT NO. 62W87

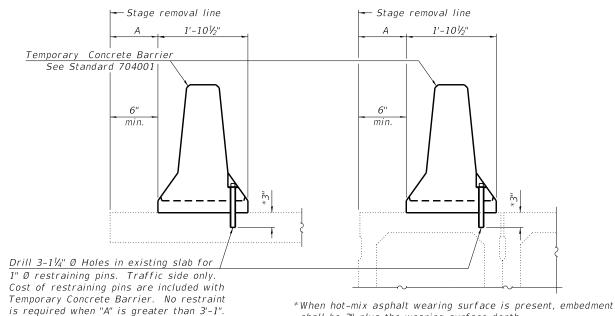
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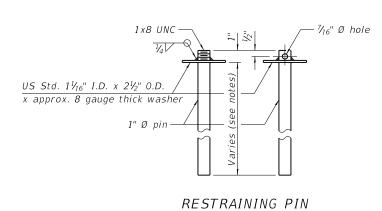
DATE

- 12/9/2024

REVISED -







NEW SLAB OR NEW DECK BEAM

to Detail I, II or III. No restraint is required

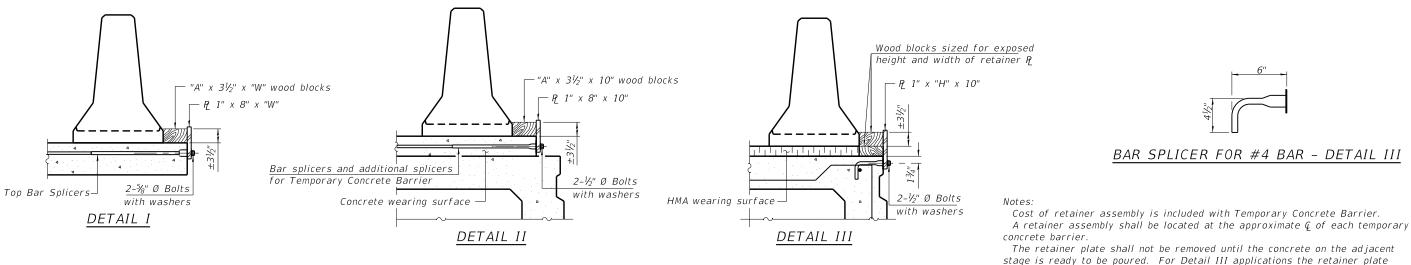
when "A" is greater than 3'-1".

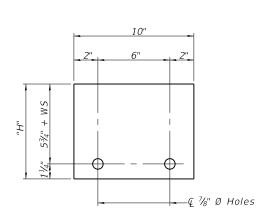
EXISTING SLAB

EXISTING DECK BEAM

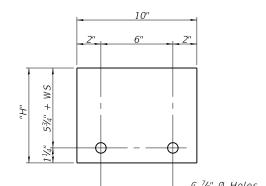
shall be 3" plus the wearing surface depth.

SECTIONS THRU SLAB OR DECK BEAM





STEEL RETAINER P 1" x "H" x 10"



surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.

shall not be removed until just prior to placing the adjacent beam.

Detail I - Installation for a new bridge deck or bridge slab.

When the 'A' dimension is less than 1½", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate

Detail II - Installation for a new deck beam with an initial concrete wearing

BAR SPLICER FOR #4 BAR - DETAIL III

the shear key clamping device.

Detail III - Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

		D 0 C 0111 1
	10"	Detail II
	2" Top bars Spa. 2" 6"	<u>Detail I</u> Detail II
8""		
11/4"		
		î %" Ø Holes

Detail I

RAILING CRITERIA

NCHRP 350 Test Level 440 Railing Weight (plf)

5-15-2023

STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)

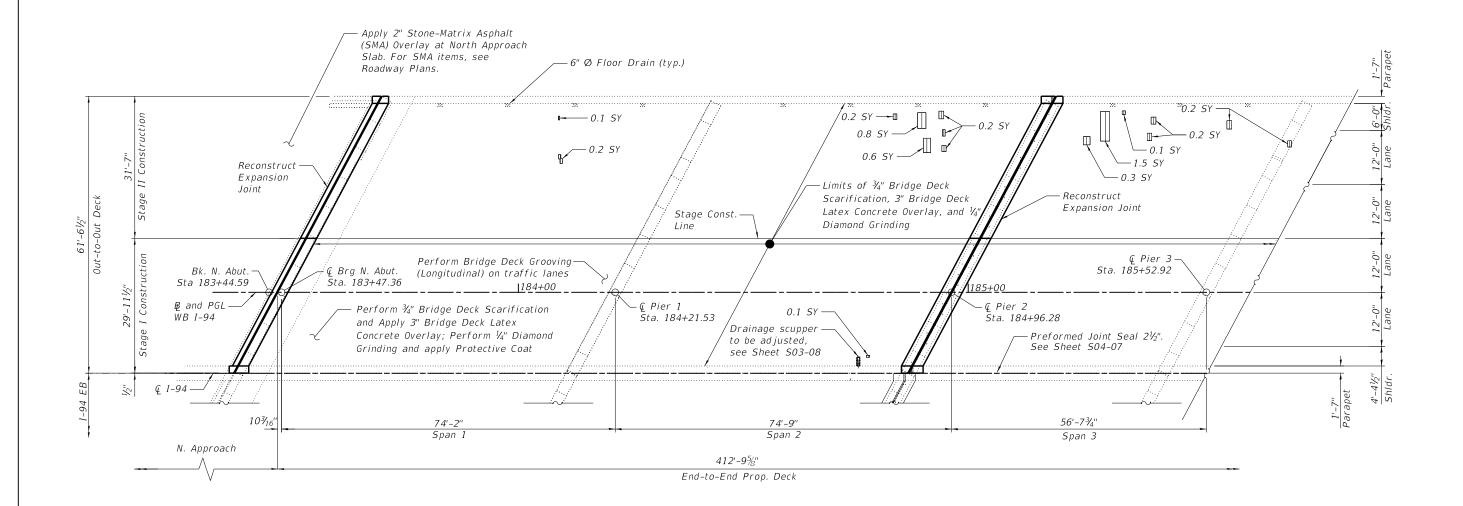
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			DRAWN	-	DEO	REVISED	-
USER NAME	-	hbmepw11ics01\$	DESIGNED	-	DEO	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION COUNTY TEMPORARY CONCRETE BARRIER (42-B-11-1) BR. BJR 24 STRUCTURE NO. 016-0160 (WB)

SHEET S03-05 OF S03-26 SHEETS

COOK 761 537 CONTRACT NO. 62W87



DECK PLAN — Z-

*Areas of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 3"

<u>LEGEND</u>

Deck Slab Repair (Partial)*

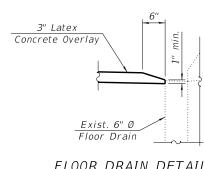
SY Square Yard

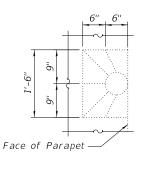
<u>NOTE:</u>

1. For Notes and Bill of Material, see Sheet S03-07.

HBM	
ENGINEERING GROUP, LLC	

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			DRAWN	-	DEO, AWD	REVISED -
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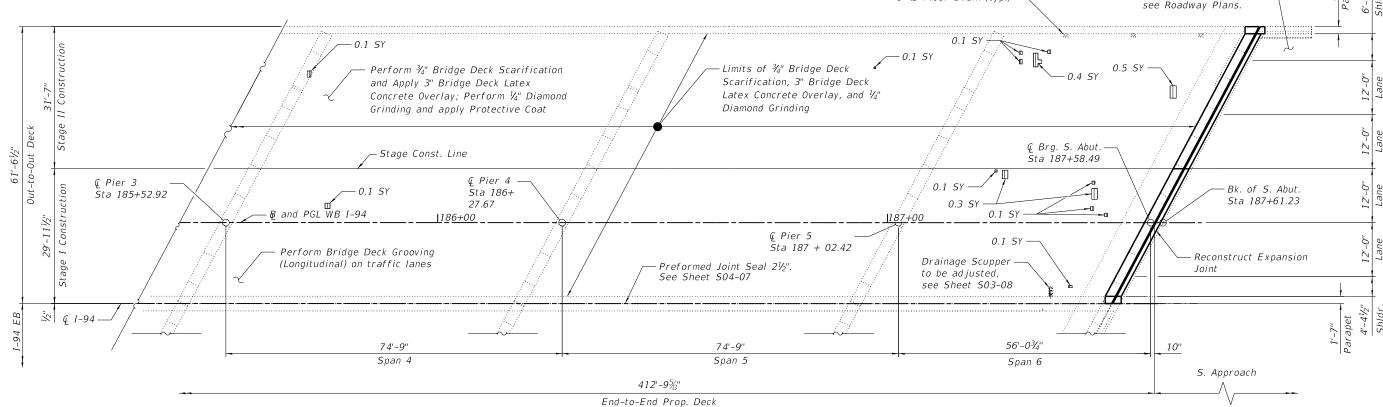
BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Protective Coat	Sq Yd	3,019
Bridge Deck Grooving (Longitudinal)	Sq Yd	2,217
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	2,617
Bridge Deck Scarification 3/4"	Sq Yd	2,617
Diamond Grinding (Bridge Section)	Sq Yd	2,511

FLOOR DRAIN DETAIL

TOP	PL	ΑN
, 0,	, _	/ 1/ V

Apply 2" Stone-Matrix -Asphalt (SMA) Overlay at South Approach Slab. For SMA items, 6" Ø Floor Drain (typ.) – see Roadway Plans. 0.1 SY = -0.1 SY



DECK PLAN

NOTES:

- 1. Areas of deck repair shown are estimated. The Engineer shall show actual locations of deck repairs at the time of construction.
- 2. For bridge deck final cross section, see Sheet S03-04.
- For North Abutment, Pier 2, and South Abutment transverse joint removal and reconstruction, see Sheets 503-09 thru 503-16.
- Perform 1/4" Diamond Grinding to top of bridge deck and abutment hatch block.
- Any reinforcement bars that are damaged during concrete removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.

*Areas of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 3"

LEGEND



Deck Slab Repair (Partial)*

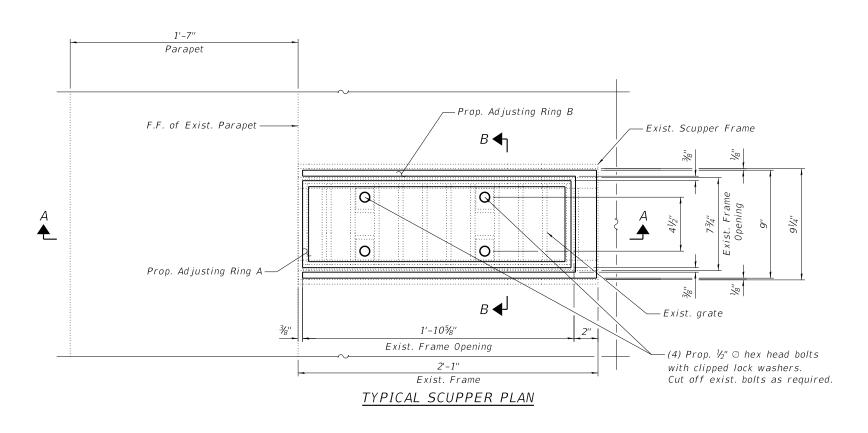
SY Square Yard

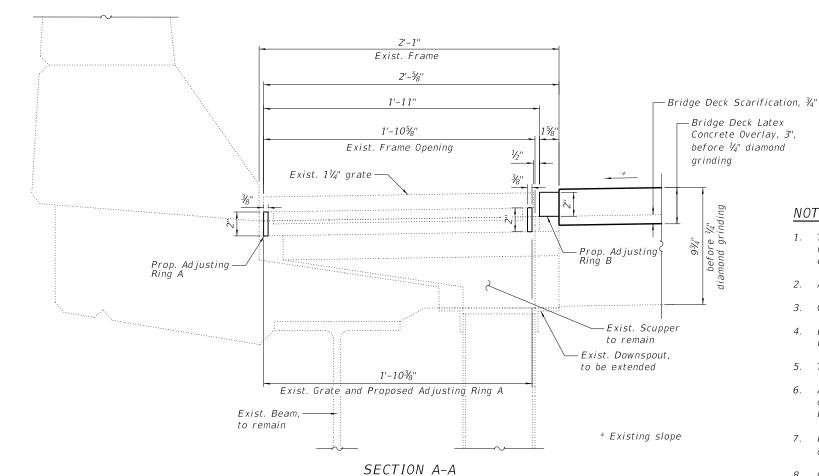
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ENGINEERING GROUP, LLC	

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.I. E.	SECTION		COUNTY	TOTAL SHEETS	SHEE'
4	(42-B-11-1) BR, BJR 24		COOK	761	539
			CONTRACT	NO.	62W8
	ILLINOIS FE	D. A	D PROJECT		

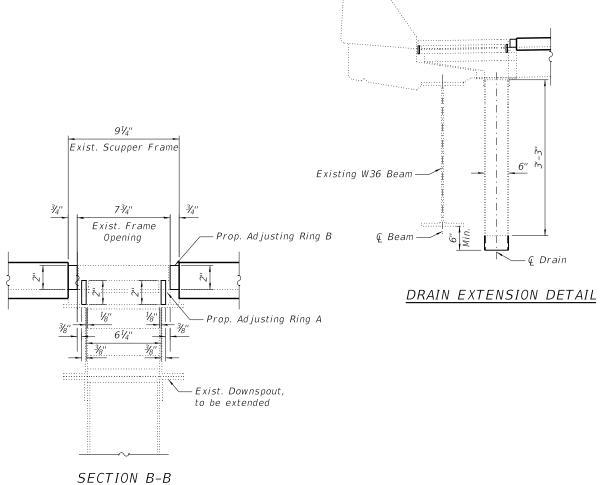
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BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Deck Drain Extensions	Each	16
Drainage Scuppers To Be Adjusted	Each	2

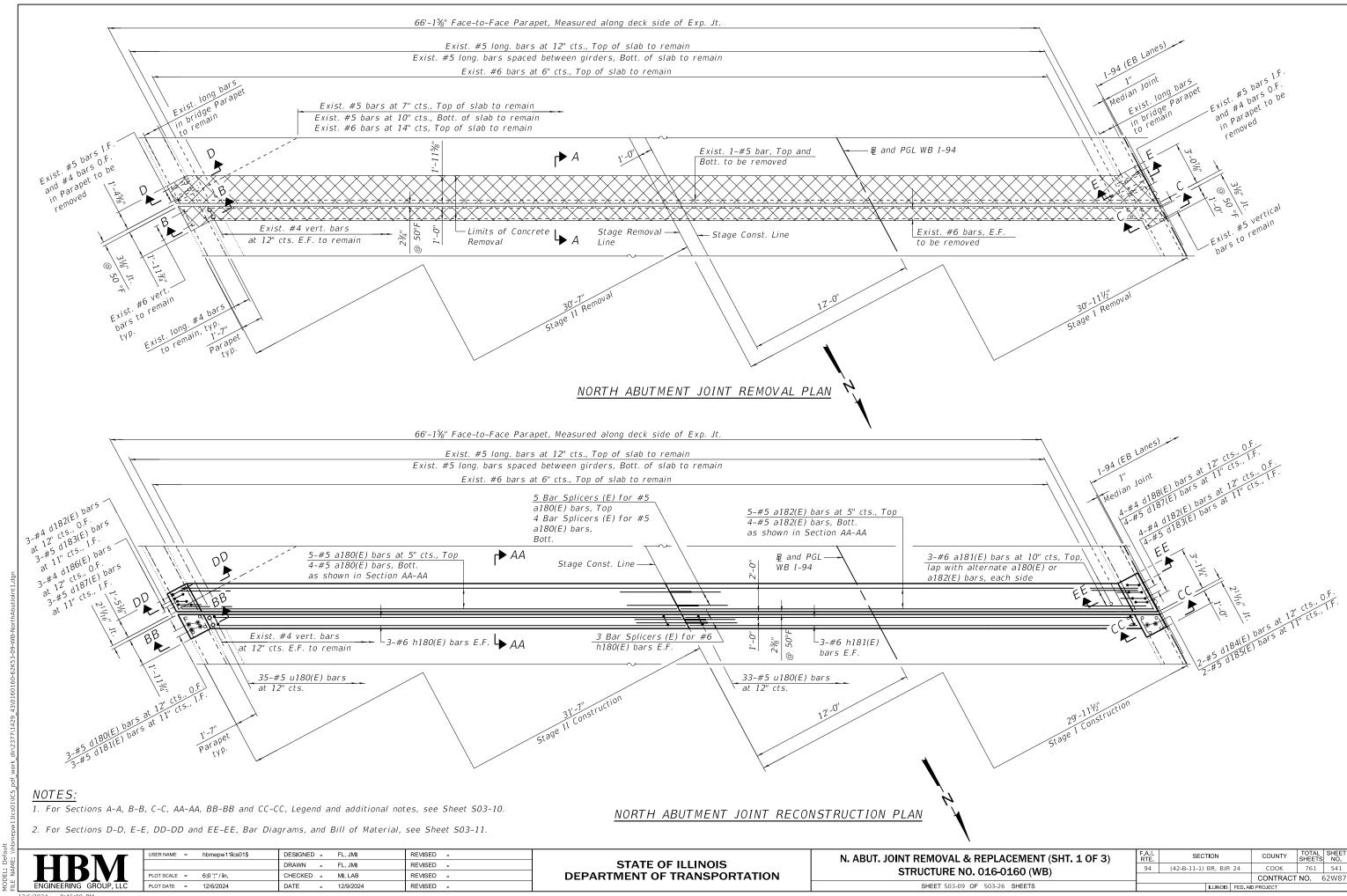


NOTES:

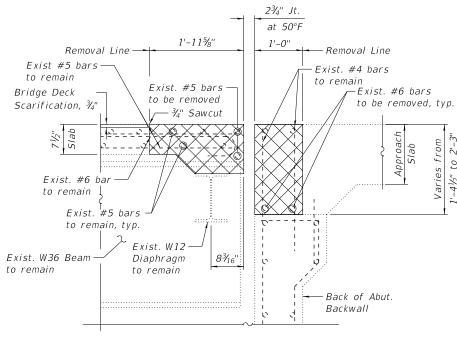
- 1. The Contractor shall field-verify Existing Dimensions and Details of the Existing Scuppers and downspouts and make necessary approved adjustments prior to construction or ordering of material for Adjusting Drainage Scuppers and extending downspouts.
- 2. All Cast Iron Parts shall be Grey Iron conforming to the requirements of AASHTO M 105, Class 35B.
- Cast Iron Parts shall be unfinished.
- Bolts, washers and nuts shall conform to the requirements of ASTM A307 and be galvanized according to the requirements of AASHTO M232.
- 5. The Contractor shall take appropriate measures to ensure that Protective Coat is not applied to the scuppers.
- Adjusting Rings shall be from Neenah or approved equal. Structural steel weldments or equal sections and of the same configuration may be submitted in place of Cast Iron. Fillet or full penetration welds may be used for weldments. Details shall be submitted to the Engineer for approval.
- 7. Provide 1/8" Fillet Weld around perimeter of new Adjusting Rings to secure to existing Scupper. Electrode shall be compatible with cast iron if existing scupper elements are cast iron construction.
- Cost of all labor and materials necessary to clean all existing floor drains and scuppers, fabricate and install adjusting scupper rings, remove and reinstall grates is included in the cost of Drainage Scuppers to be Adjusted.

HBM
ENGINEERING GROUP, LLC

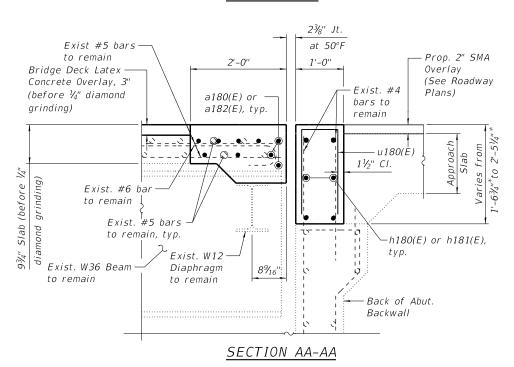
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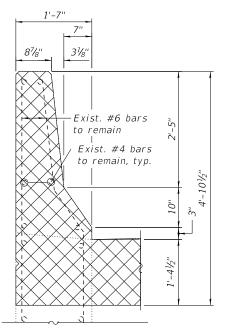


SECTION A-A



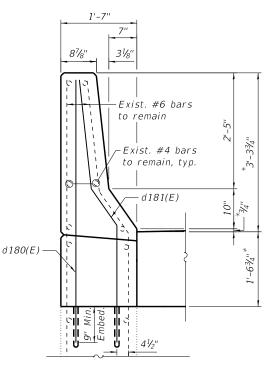
NOTES:

- 1. For preformed joint strip seal details, see Sheet SO3-17.
- 2. For bar splicer assembly details, see Sheet S03-26.
- 3. Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 4. Epoxy grout d180(E), d181(E), d184(E) and d185(E) bars according to Article 584 of the Standard Specifications. Drill to mis included with Concrete Superstructure.
- 5. Existing reinforcement bars extending into the removal area shall be cleaned, straightened, and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system at the contractor's expense.
- 6. Any reinforcement bars that are damaged during Concrete Removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to "Concrete Removal".
- 7. Dimensions are based on Roller Rail Strip Seal joint. If the Contractor elects to use the Welded Rail Strip Seal joint, deck dimensions may require adjustments to satisfy the details on Sheet S03-17.



SECTION B-B (Reinforcement in the pour strip

not shown for clarity)



SECTION CC-CC

4½"

(Reinforcement in the pour strip not shown for clarity)

Median Jt.

31/8"

87/8"

Exist. #5 bars

Exist. #4 barsto remain, typ.

Exist. #5 bars

Exist. #4 bars

to remain, typ.

d185(E,

to remain

to remain

I-94

(EB Lanes)

Exist. 4" Ø conduit to remain

Median Jt.

I-94

(EB Lanes)

- Exist. 4" Ø conduit to remain

d184(E)

Prop. PJF

SECTION C-C

(Reinforcement in the pour strip

not shown for clarity)

31/8"

8⁷/8''

LEGEND

Concrete Removal E.F. Each Face

I.F. Inside Face

0.F. Outside Face

SECTION COUNTY 94 (42-B-11-1) BR, BJR 24 COOK 761 542 CONTRACT NO. 62W87

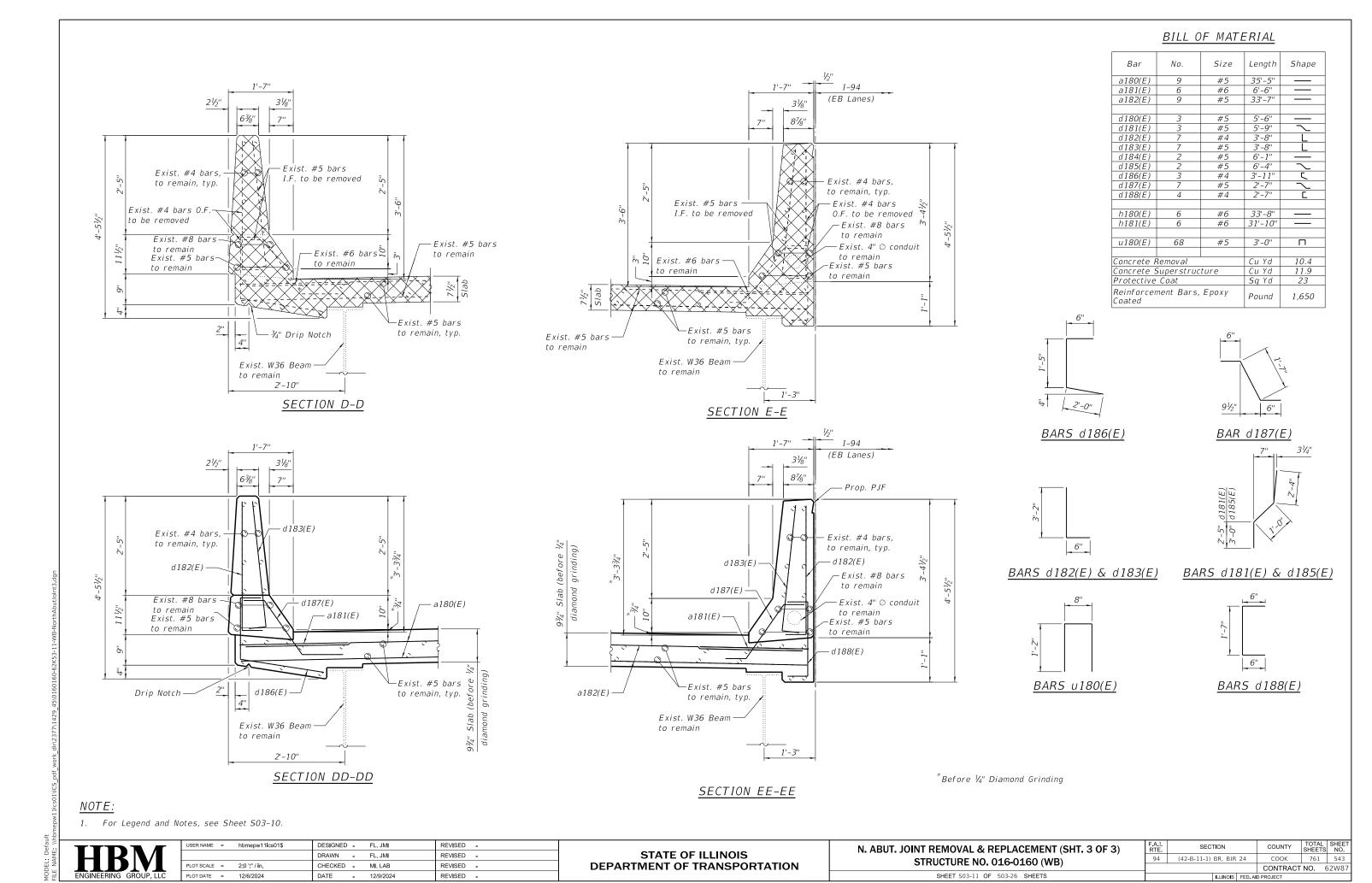
(E)	}		1'-6¾"*
	41/2"	-	<u> </u>
<u>SECTION</u>	BB-BB		
(Reinforcement in not shown fo		rip	
ss existing reinforc	ement. Cost		

*Before ¼" Diamond Grinding

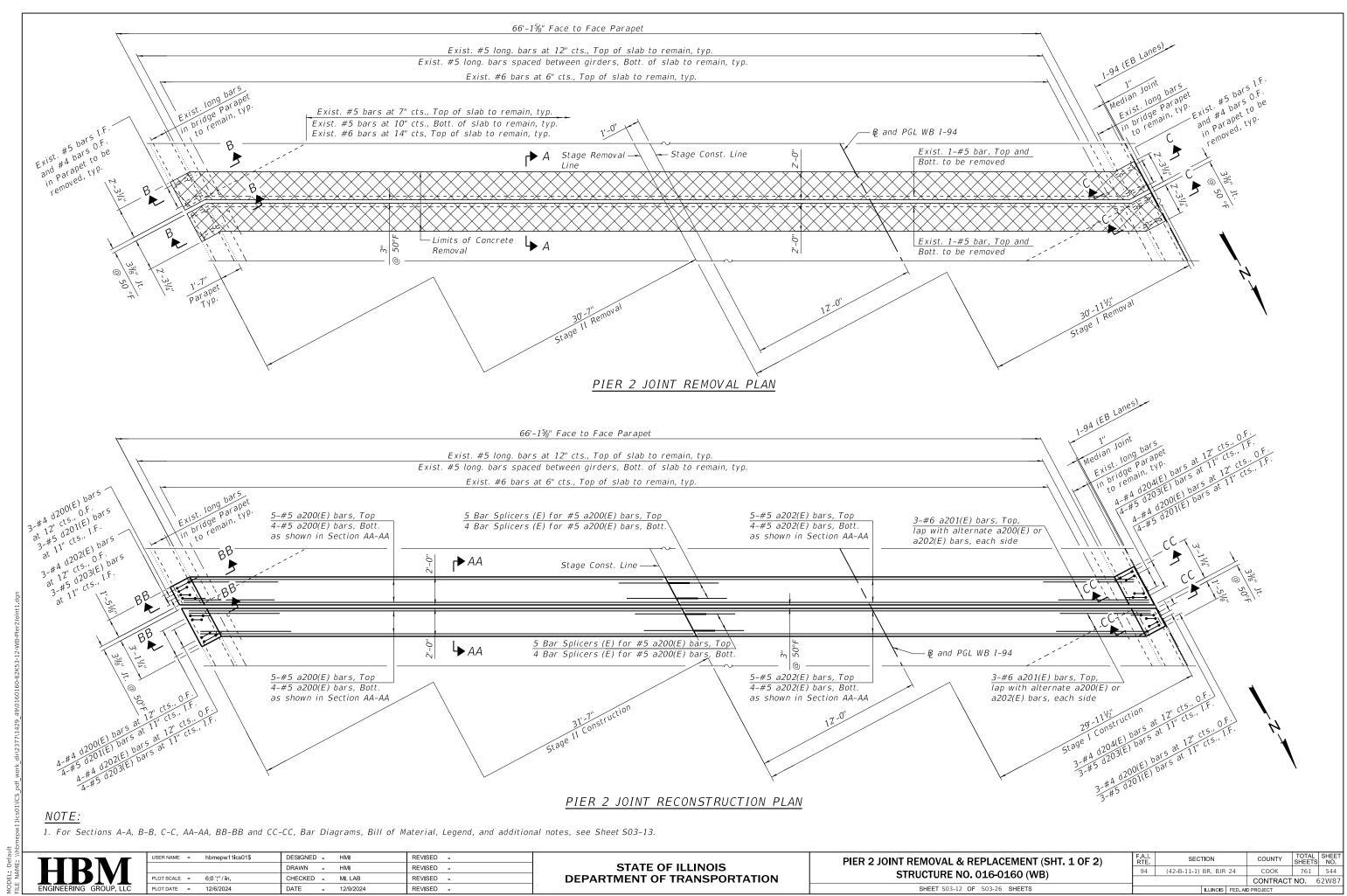
N. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 2 OF 3) STRUCTURE NO. 016-0160 (WB) SHEET S03-10 OF S03-26 SHEETS

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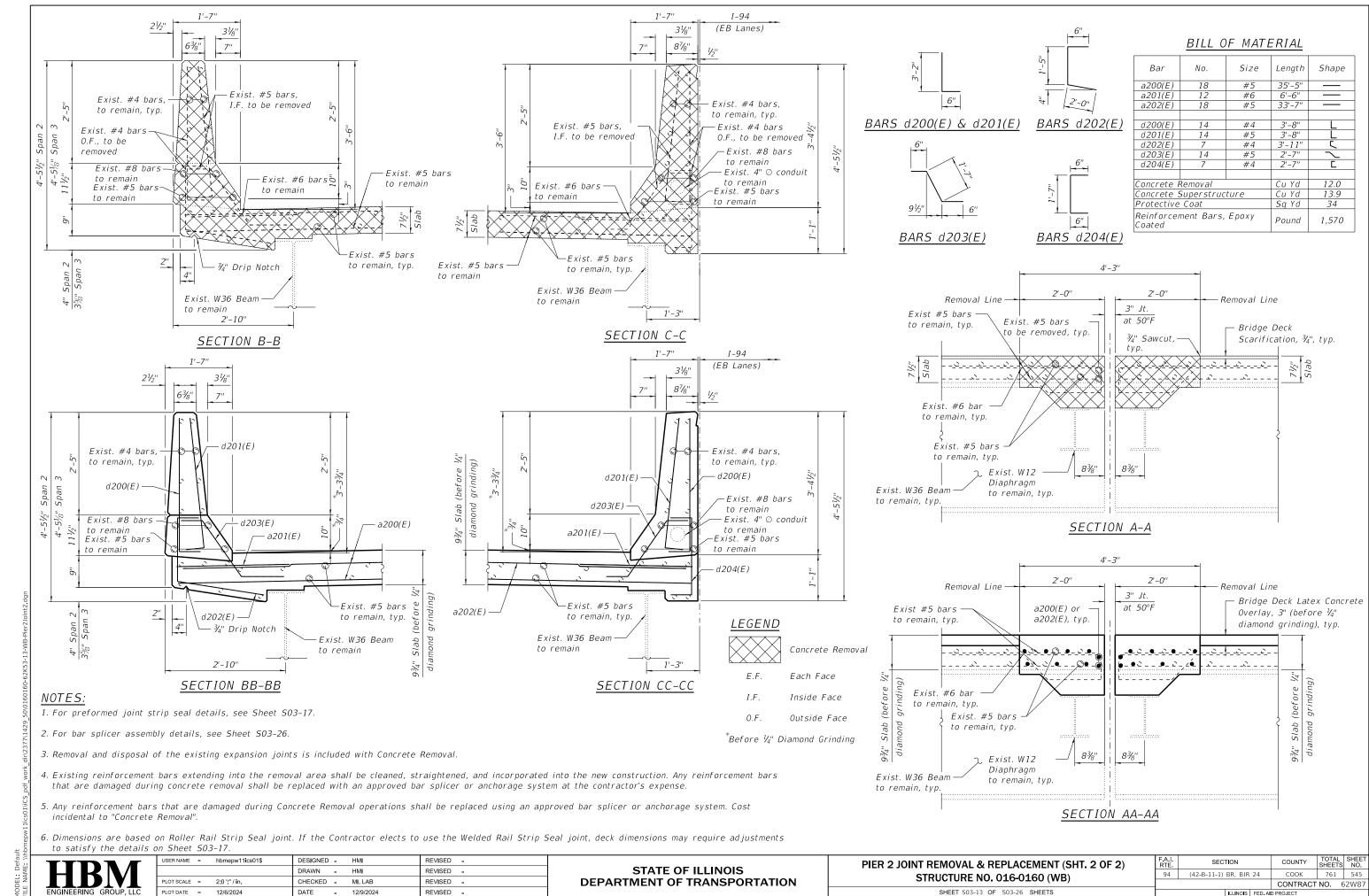
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION



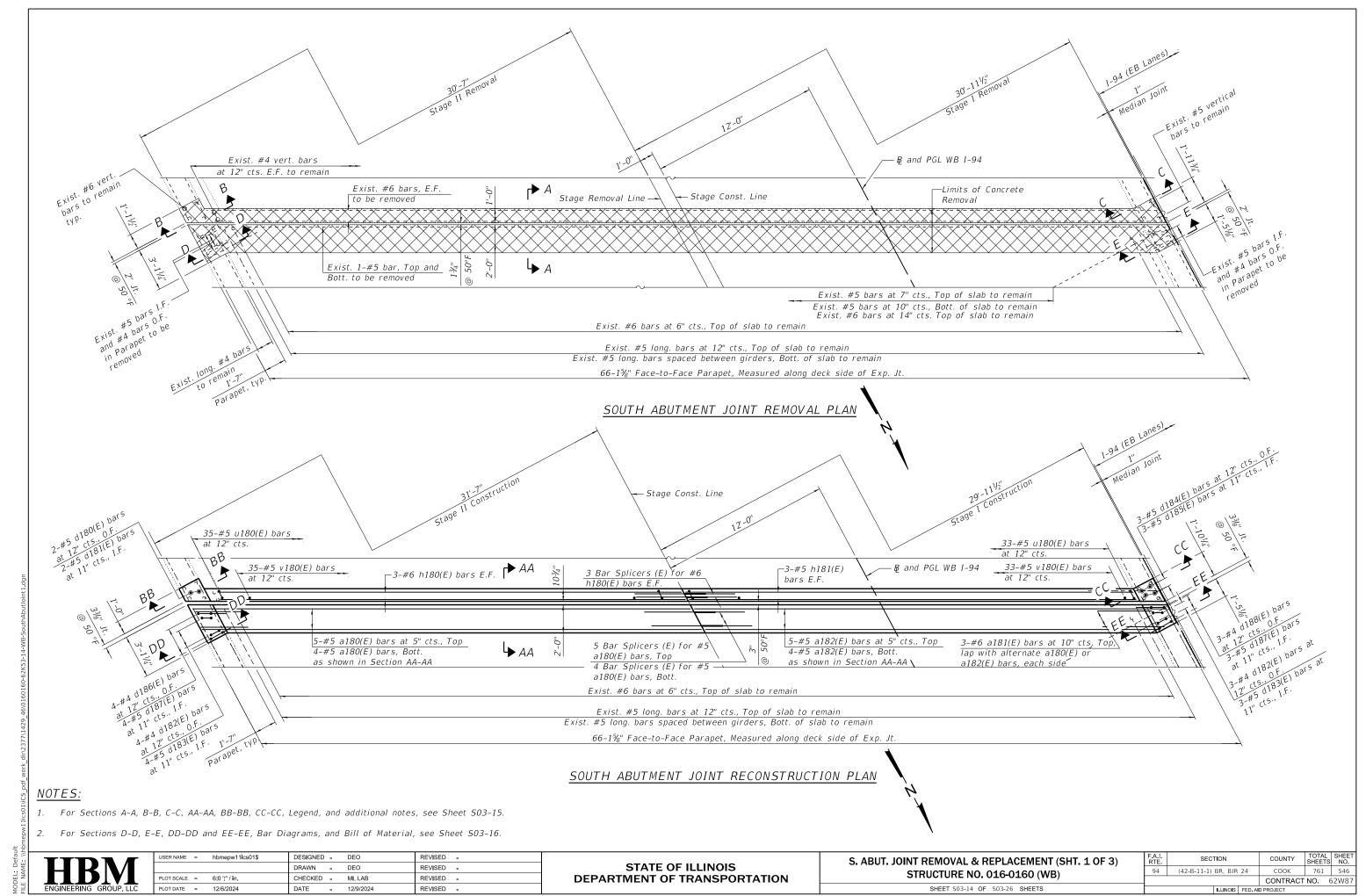
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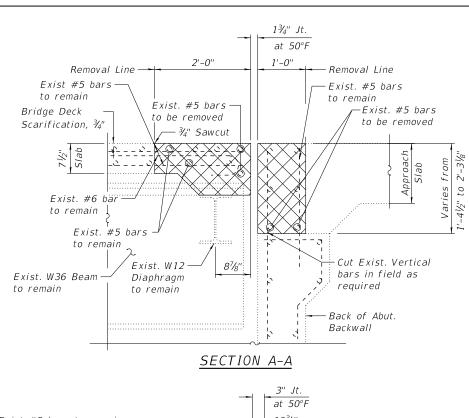
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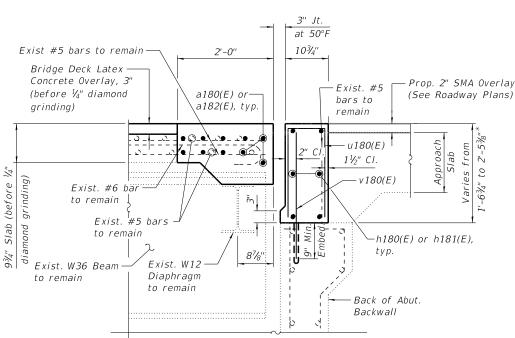


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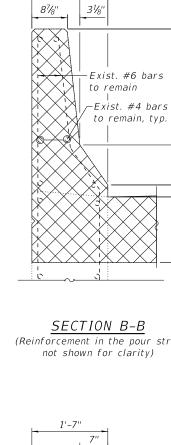




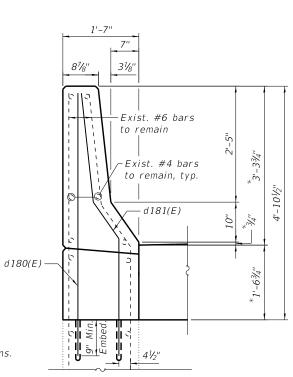
- 1. For preformed joint strip seal details, see Sheet S03-17.
- 2. For bar splicer assembly details, see Sheet S03-26.
- 3. Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 4. Epoxy grout d180(E), d181(E), d184(E), d185(E) and v180(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.
- 5. Existing reinforcement bars extending into the removal area shall be cleaned, straightened, and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system at the contractor's expense.

SECTION AA-AA

- 6. Any reinforcement bars that are damaged during Concrete Removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to "Concrete Removal".
- 7. Dimensions are based on a Roller Rail Strip Seal joint. If the Contractor elects to use the Welded Rail Strip Seal Joint, deck dimensions may require adjustments to satisfy the details on Sheet S03-17.



(Reinforcement in the pour strip



SECTION BB-BB (Reinforcement in the pour strip not shown for clarity)

(Reinforcement in the pour strip *Before ¼" Diamond Grinding

(EB Lanes) 31/8" 8⁷/8'' - Prop. PJF Exist. #5 bars to remain Exist. #4 bars to remain, typ. d185(E, Exist. 4" Ø conduit to remain -d184(E)

Median Jt.

31/8" 87/8"

Exist. #5 bars to remain

Exist. #4 bars to remain, typ.

5'-63''

I-94 (EB Lanes)

Exist. 4"

Ø conduit

to remain

Median Jt.

I-94

SECTION C-C

(Reinforcement in the pour strip

not shown for clarity)

SECTION CC-CC

41/2"

not shown for clarity)

Concrete Removal

Each Face

I.F. Inside Face

0.F. Outside Face

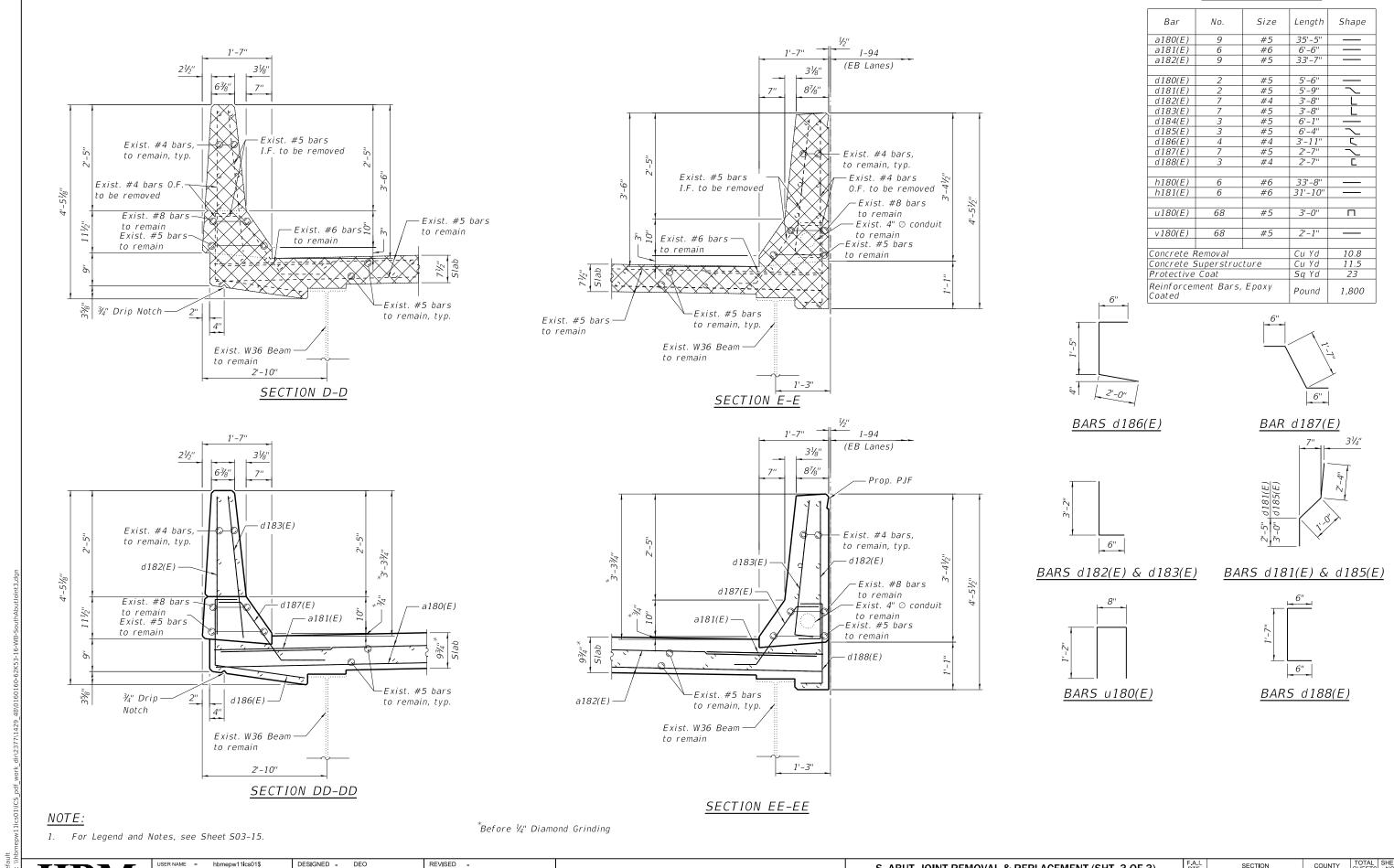
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION S. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 2 OF 3) STRUCTURE NO. 016-0160 (WB) SHEET S03-15 OF S03-26 SHEETS

SECTION COUNTY 94 (42-B-11-1) BR, BJR 24 COOK 761 547 CONTRACT NO. 62W87

LEGEND

E.F.



FII F NAME

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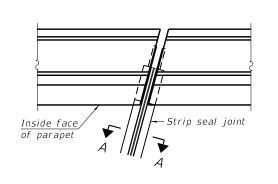
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

S. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 3 OF 3)
STRUCTURE NO. 016-0160 (WB)

SHEET S03-16 OF S03-26 SHEETS

BILL OF MATERIAL



FOR SKEWS ≤ 30°

plate

Parapet sliding

Inside Face

of Parapet

2" Max.

В

5/8" Ø x 6" Studs -

Detail A

Point Block Detail /\//// Parapet sliding Inside face/ 7 of parapet -Strip seal joint FOR SKEWS > 30°

* ¾" Ø x 6" Studs (6 per side 34" parapet) (8 per side 42" parapet) °∏ ¾" Embedded plate full depth ¾" Embedded plate, full depth 1/2" Parapet sliding plate ¾" Ø Countersunk bolts 1'-0" (8 per side 34" parapet) (10 per side 42" parapet) <u>Direct</u>ion of traffic

PLAN AT PARAPET

Top of locking

edge rail

SECTION B-B

face of 3/4" plate

(Showing embedded plates only)

Concrete flush with back face of ¾" plate ¾" Plate · 5- 30 .; Đ D. . Concrete flush with back

included with Preformed Joint Strip Seal. 39" constant slope barrier shown, 44" constant slope barrier similar as noted.

The strip seal shall be made continuous and shall have

a minimum thickness of 1/4". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations

are not permitted. The gland shall be sized for a maximum

The locking edge rails depicted are configured for typical

applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from

manufacturer to manufacturer provided they fit the application

however, will not be allowed. Locking edge rails may exceed the

4½" maximum depth provided the anchorage system is revised

All steel components shall be galvanized after fabrication

curb or parapet shall be welded as shown in the locking edge

The top surface of sidewalk sliding plates shall have a

Cost of parapet sliding plates, sidewalk sliding plates,

embedded plates, anchorage studs, and expansion anchors

according to Article 520.03 of the Standard Specifications. The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the

and meet the minimum anchorage shown. Flanged edge rails,

The manufacturer's recommended installation methods

according to the manufacturer's recommendation.

raised pattern according to ASTM A786.

Notes:

rated movement of 4 inches.

shall be followed.

rail splice detail.

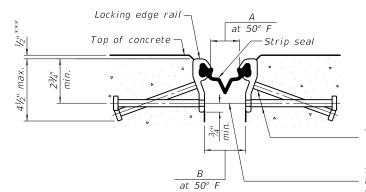
The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

ELEVATION AT PARAPET

6" cts., typ.

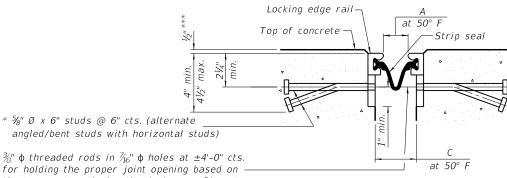
(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

TRIMETRIC VIEW



SHOWING ROLLED RAIL JOINT

Joint	Α	В	С
North Abut.	11/2"	23/8"	3"
Pier 2	21/8"	3"	35%"
South Abut.	11/2"	2¾"	3"

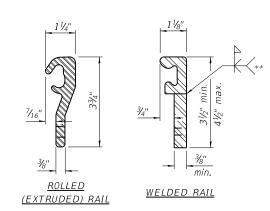


for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

DETAIL A

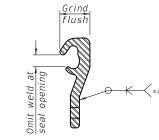
SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	01111	Total
Preformed Joint Strip Seal	Foot	210



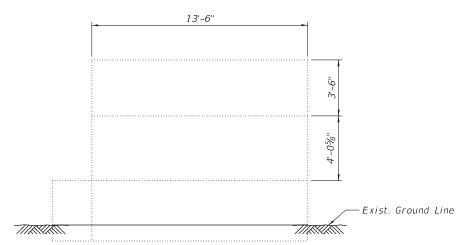
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SHOWING WELDED RAIL JOINT

PREFORMED JOINT STRIP SEAL STRUCTURE NO. 016-0160 (WB) SHEET S03-17 OF S03-26 SHEETS

L. E.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
4	(42-B-11-1) BR, BJR	24	COOK	761	549
			CONTRACT	NO. 6	52W87



<u>-</u>		
ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	294
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	46

NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Concrete Sealer is to be applied to the abutment seats and the bottom 2 feet of the abutment backwall.

NODTHEACT WINCWALL FLEWATION

<u>NORTHEAST</u>	WINGWALL	ELEVATION
	(Looking West)	_

NORTH ABUTMENT REPAIRS	
STRUCTURE NO. 016-0160 (WB)	

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1) BR, BJR 24	соок	761	550
		CONTRACT	NO. 6	52W87
	ILLINOIS FED. A	D PROJECT		

(Depth Equal to or Less than 5 inches)

Structural Repair of Concrete

Square Foot

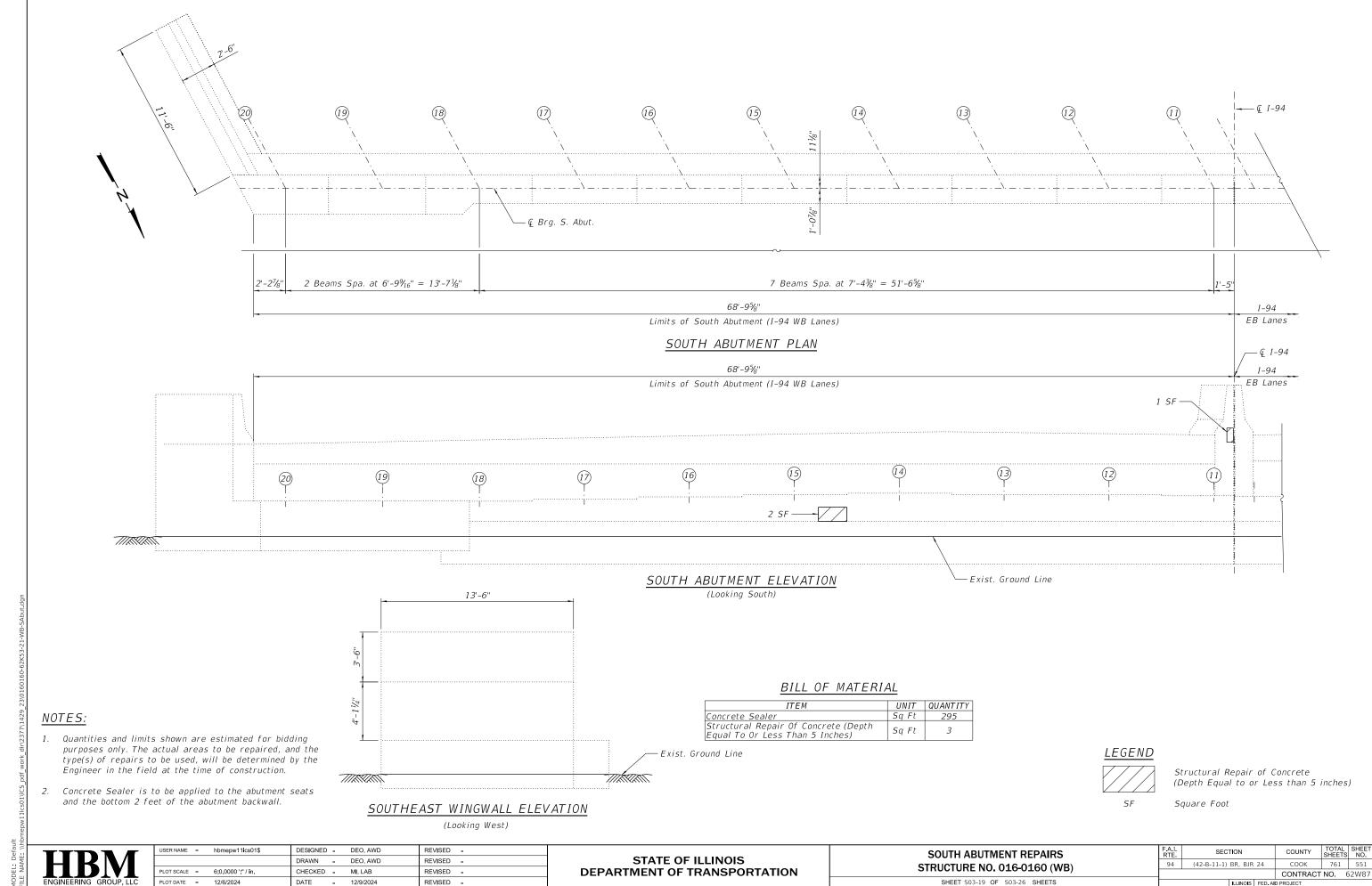
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

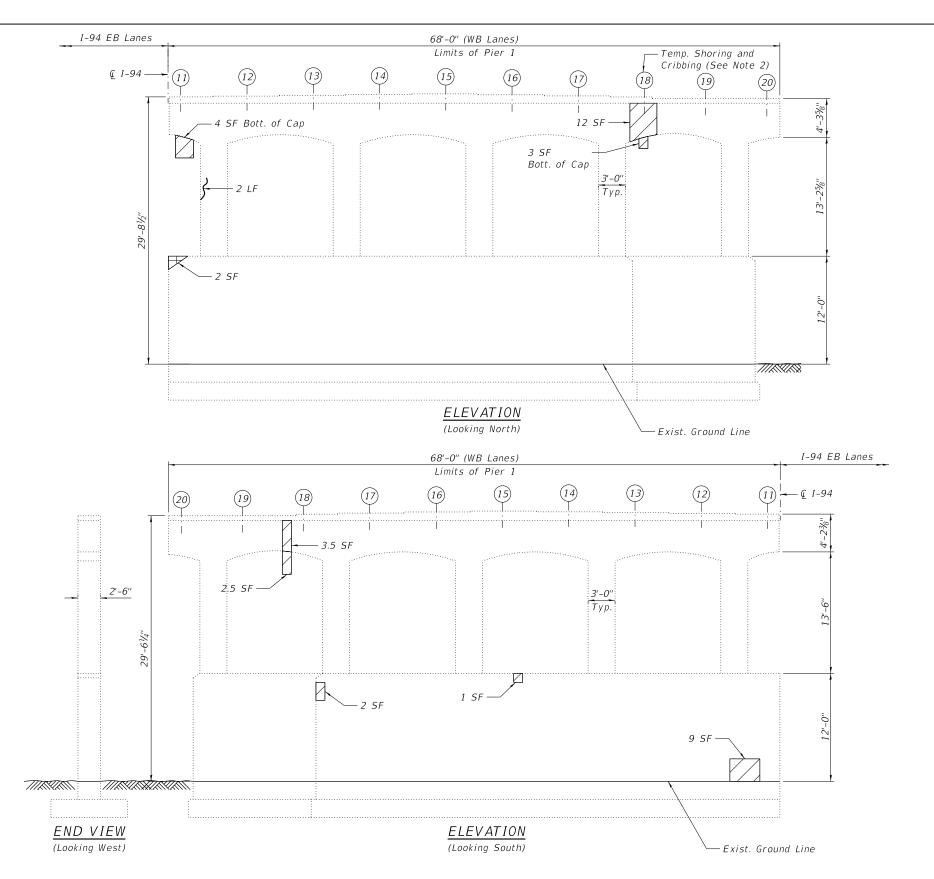
SHEET S03-18 OF S03-26 SHEETS

LEGEND

SF



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BILL OF MATERIAL

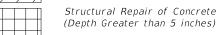
ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	2
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	37
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	2
Temporary Shoring And Cribbing	Each	1

SUMMARY OF REACTIONS			
Pier 1, Beam 18			
R DL	(k)	100.7	
R LL (k) 53.7			
R IM (k) 13.5			
R Total	(k)	167.9	

LEGEND



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)





Epoxy Crack Injection (Width > 0.06")

SF Square Foot

LF Linear Foot

<u>NOTES:</u>

- . Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

H	\mathbf{B}	M
ENGINEER	NG GR	OUP, LLC

	USER NAME	-	hbmepw11ics01\$	DESIGNED	-	DEO, AWD	REVISED	-
				DRAWN	-	DEO, AWD	REVISED	-
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

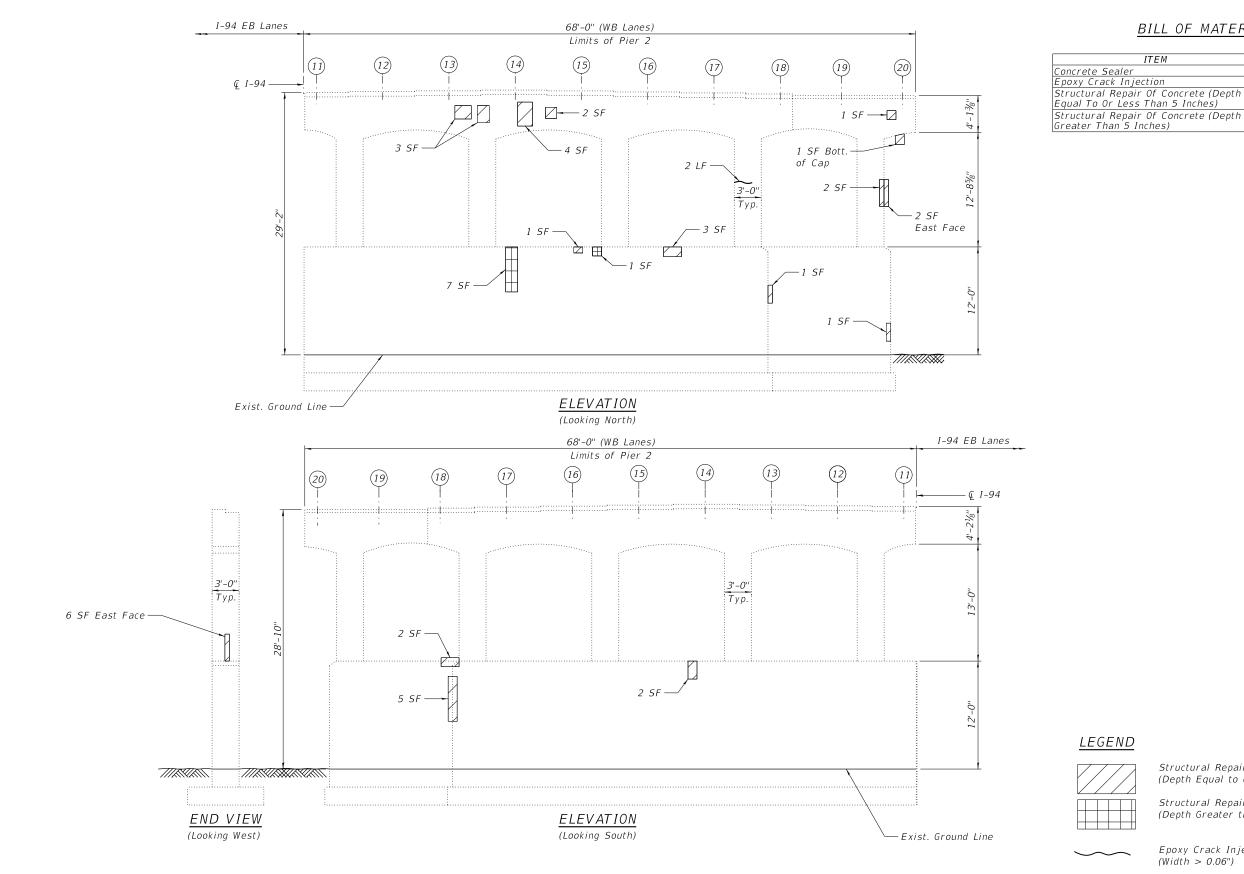
PIER 1 REPAIRS

STRUCTURE NO. 016-0160 (WB)

SHEET S03-20 OF S03-26 SHEETS

.I. E.	SECTION		COUNTY	TOTAL SHEETS	SHEE'	
4	(42-B-11-1) BR, BJR 24	1	COOK	761	552	
			CONTRACT	NO. 6	52W87	
ILLINOIS FED. AID PROJECT						

12/6/2024 8:49:06 PM



NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- 2. Concrete Sealer is to be applied to the exposed surfaces of the pier.

	٦
ENGINEERING GROUP, LLC	

USER NAME =	•	hbmepw11ics01\$	DESIGNED	-	DEO, AWD	REVISED -
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

PIER 2 REPAIRS STRUCTURE NO. 016-0160 (WB) SHEET S03-21 OF S03-26 SHEETS

F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1) BR, BJR	24	COOK	761	553
			CONTRACT	NO. 6	52W87
	ILLINOIS	EED AL	D PRO IECT		

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Structural Repair of Concrete (Depth Greater than 5 inches)

BILL OF MATERIAL

ITEM

UNIT QUANTITY
Sq Ft 3600

Foot

Sq Ft

Sq Ft

3600

39

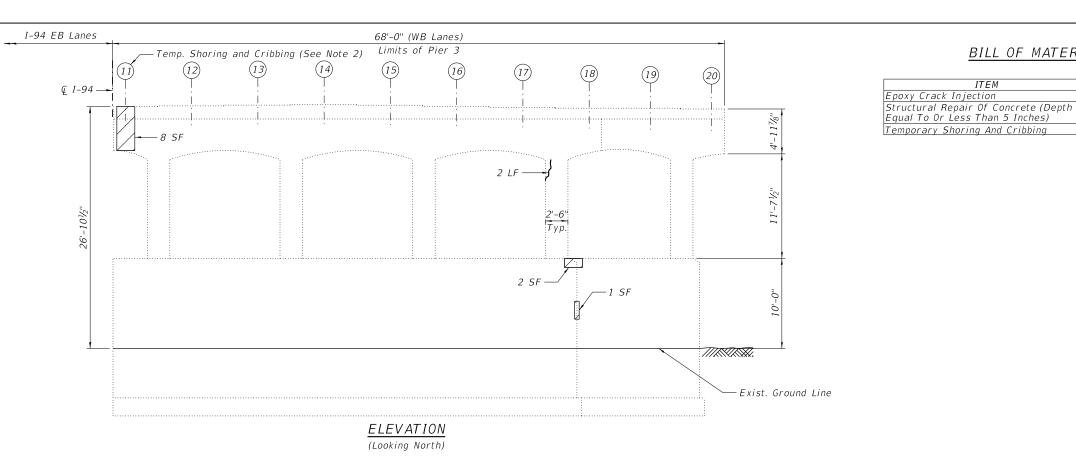
8

Epoxy Crack Injection (Width > 0.06")

SF Square Foot

LF Linear Foot

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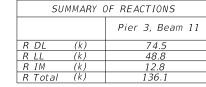
68'-0" (WB Lanes) Limits of Pier 3

1 SF ——□

2'-6'' Typ.

(18)

(19)



BILL OF MATERIAL

UNIT QUANTITY

13

Foot

Sq Ft

Each

ITEM

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Epoxy Crack Injection

(Width > 0.06")

Square Foot SF

Linear Foot LF

NOTES:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

END VIEW

(Looking West)

2. Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.

HBM
ENGINEERING GROUP, LLC

USER NAME	-	hbmepw11ics01\$	DESIGNED	-	DEO, AWD	REVISED -
			DRAWN	-	DEO, AWD	REVISED -
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ELEVATION

(Looking South)

PIER 3 REPAIRS								
STRUCTURE NO. 016-0160 (WB)								
SHEET 503-22	OF	S03-26	SHEETS					

-Exist. Ground Line

I-94 EB Lanes

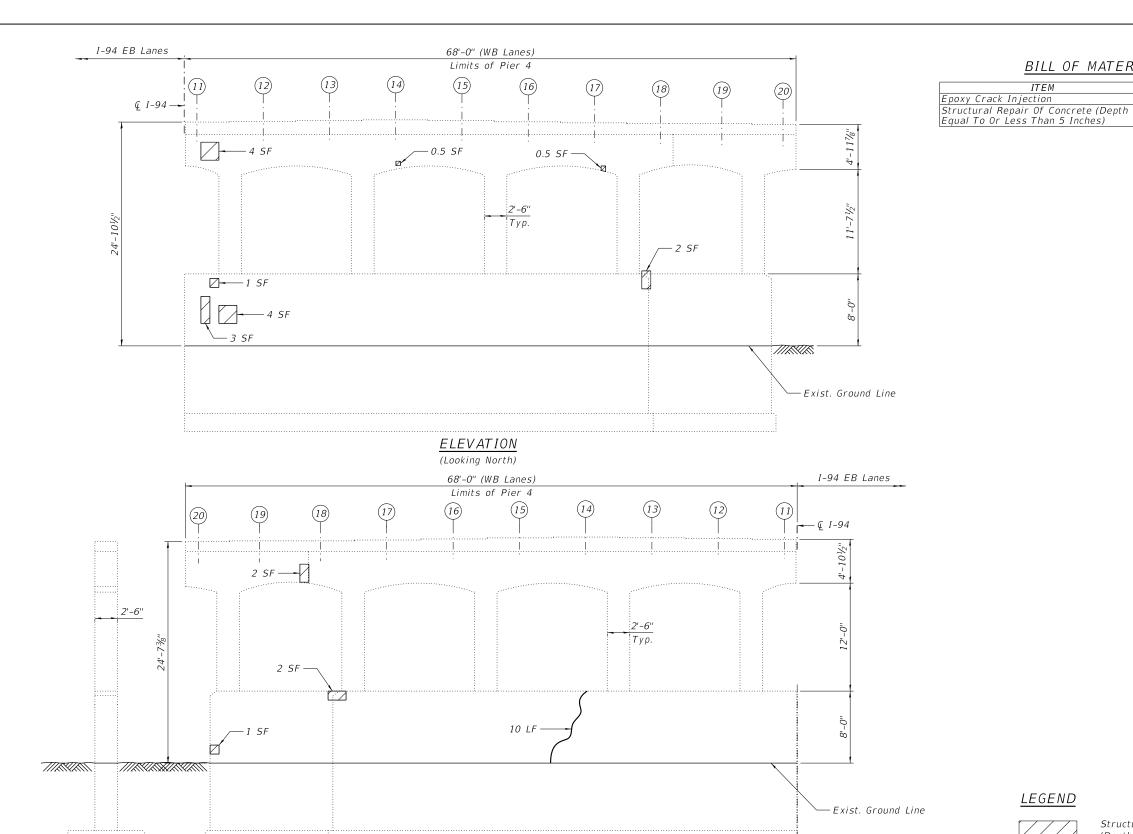
--- € I-94

(12)

1 SF —

F.A.I. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.		
94	(42-B-11-1) BR, BJR	24	COOK	761	554		
		CONTRACT	NO. 6	52W87			
ILLINOIS FED. AID PROJECT							

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LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Epoxy Crack Injection (Width > 0.06")

SF

Square Foot

LF Linear Foot

BILL OF MATERIAL

UNIT QUANTITY

20

Foot

Sq Ft

ITEM

NOTE:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

<u>END V</u>IEW

(Looking West)

USER NAME =	•	hbmepw11ics01\$	DESIGNED	-	DEO, AWD	REVISED	-
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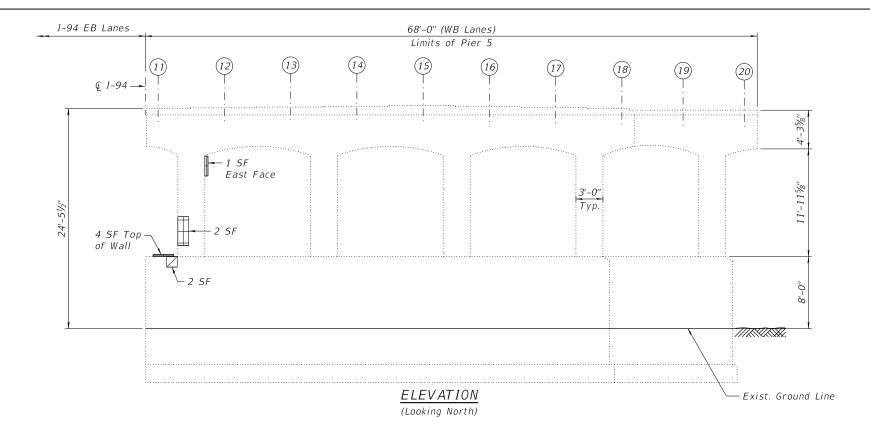
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

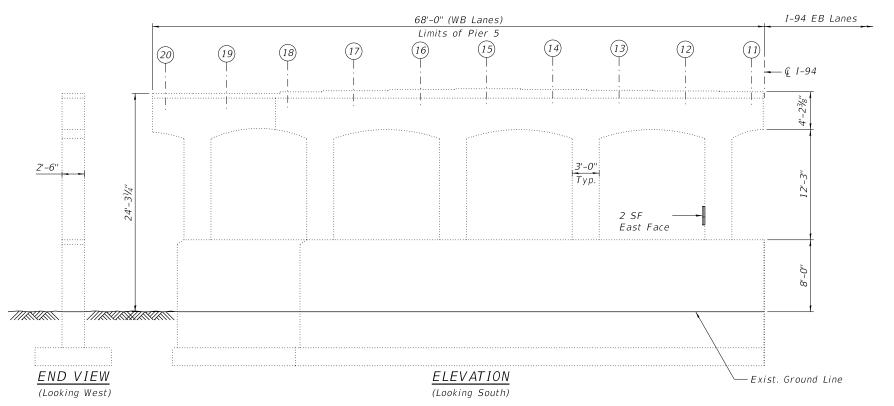
ELEVATION

(Looking South)

PIER 4 REPAIRS STRUCTURE NO. 016-0160 (WB) SHEET S03-23 OF S03-26 SHEETS

A.I. TE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.			
94	(42-B-11-1) BR, BJR	24	COOK	761	555			
		CONTRACT	NO. (52W87				
	ILLINOIS FED. AID PROJECT							





NOTE:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



(Depth Greater than 5 inches)

SF Square Foot

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F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
94	(42-B-11-1) BR, BJR 24	соок	761	556
		CONTRACT	NO. 6	52W87
	ILLINOIS FED.	AID PROJECT		

Structural Repair of Concrete

BILL OF MATERIAL

Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)

Structural Repair Of Concrete (Depth Greater Than 5 Inches)

UNIT QUANTITY

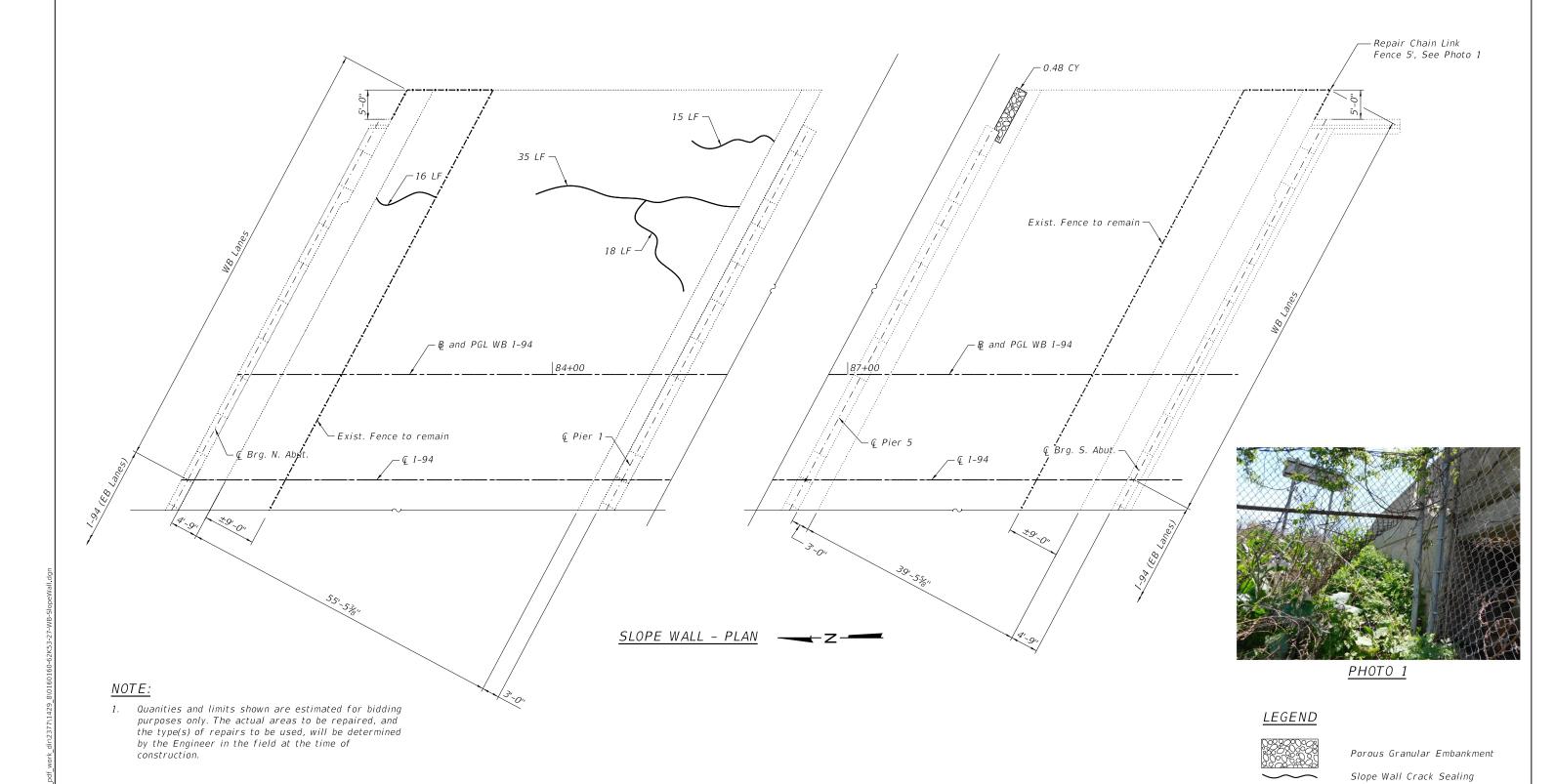
Sq Ft

2

9

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Porous Granular Embankment	Cu Yd	1
Chain Link Fence, 5'	Foot	5
Slope Wall Crack Sealing	Foot	84



HBM	
ENGINEERING GROUP, LLC	

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SLOPE WALL REPAIRS
STRUCTURE NO. 016-0160 (WB)

SHEET 503-25 OF 503-26 SHEETS

A.I.	SECT	ΓΙΟΝ		COUNTY	TOTAL SHEETS	SHEET NO.		
94	(42-B-11-1)	BR, BJR	24	COOK	761	557		
			CONTRACT	NO. 6	52W87			
	ILLINOIS FED AID DOO ISST							

Linear Foot

Cubic Yard

LF

CY

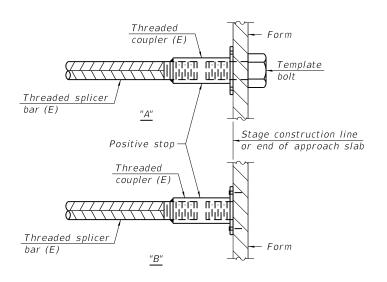
STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

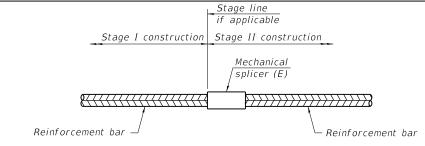
Location	Bar Size	No. assemblies	Minimum lap length	
N. Abutment Jt.	#5	9	3'-0"	
N. Abutillelli Jt.	#6	6	3'-7"	
Pier 2 Jt.	#5	18	3'-0"	
S. Abutment Jt.	#5	9	3'-0"	
J. ADULINEIN JI.	#6	6	3'-7"	



INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required	

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Par splicer assemblies shall be energy scarted assembling to the

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

5-15-2023

HBM ENGINEERING GROUP LIC

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			DRAWN	-	DEO, AWD	REVISED	-
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PLOT DATE	=	12/6/2024	DATE	-	12/9/2024	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

BAR SPLICER ASSEMBLY & MECHANICAL SPLICER DETAIL STRUCTURE NO. 016-0160 (WB)

SHEET 503-26 OF 503-26 SHEETS

F.A.I. SECTION COUNTY TOTAL SHEETS NO.

94 (42-B-11-1) BR, BJR 24 COOK 761 558

CONTRACT NO. 62W87

ODEL: Defaul LE NAME: \\hl

DESIGN SPECIFICATIONS Existing Structure: S.N. 016-0160 (EB) was originally built in 1949 and was reconstructed in 1993. Structural steel repairs were performed in 2023. The bridge is a six-span structure (six rolled beam spans) carrying four lanes of EB I-94 over three railroads (IHB, Conrail, and Baltimore and Ohio Chicago Terminal). The structure has a back-to-back 2002 AASHTO Standard Specifications abutment length of 416'-7\%" and an out-to-out deck width of 61'-6\%". The superstructure consists of a 7\%"-thick reinforced concrete deck supported on noncomposite for Highway Bridges (17th Edition) W36x160 and W36x210 beams at 6'-6" spacing. The substructure consists of reinforced concrete abutments and multi-column piers on concrete piles. RECONSTRUCTION 1992 Traffic is to be maintained utilizing staged construction. 1983 AASHTO Standard No salvage. Specifications for Highway Bridges with 1984 thru 1988 Interims 416'-7⅓" Bk. to Bk. Abutments 1983 Guide Specifications 2'-87/8" 280'-103/4" Limits of Protective Shield 2'-91/4" for Seismic Design of Highway Bridges N. Approach S. Approach with 1985 & 1988 Interim 56'-03/4" 74'-2" 56'-7³/₄'' 74'-9" 74'-9" Specifications Span 2 Span 4 Span 5 Span 1 Span 3 Span 6 - © Pier 4 −¢ Pier 1 −¢ Pier 3 @ Pier 5 — © Pier 2 & Brg. N. Abut. & Brg. S. Abut. -Reconstruct € Brg. ₽ Brg. Reconstruct Expansion Joint Exist W36 Ream-Reconstruct Expansion Joint Expansion Joint E ∭ E Track No. 3 Exist. Conc. Piles Perform Structural (typ. at North and Track No. 2 Exist. Fence Repair of Concrete (typ. Exist. Fence South Abutments) to remain at North and South to remain Exist. Ground Line Abutments) NOTES: Track No. Perform Slope Wall Repair 1. All stations are to the I-94 (typ. at both abutments) EB PGL and taken from Track No. 1 existing plans. Perform Structural Repair ELEVATION Exist. Conc. Piles Perform Structural Repair of Concrete at Pier 1 2. No future wearing surface (typ. at Piers) of Concrete and Epoxy Crack (Looking East) Injection at Piers 2-5 416'-75%" Bk. to Bk. Abutments N. Approach S. Approach 56'-73/4" 56'-03/4" 74'-9" 74'-9" 74'-2" 74'-9" Span 2 Span 4 Span 5 Span 1 Span 3 Span 6 2'-87/8'' 2'-91/4" Preformed 13'-6" Drainage Scupper Joint Seal, 21/2" to be adjusted and 12'-0" Dra<u>inage Scupper</u> Bk. of S. Abut. extended to be adjusted and 28°02'00" Sta 187+42.06 extended , Structure -Q I-94 -Bk. of N. Abut. Sta 183+25.43 Location Pier 2 C Bra. S. Abut Pier 1 Pier 4 Brg. N. Abut Sta 187+39.31; Sta. 184+02.37 Sta. 184+77.11 j Sta. 183+28.20 Sta. 185+33.76 Sta. 186+08.51 Increas 185+00 187+00 - ¢ Pier 5[™] Structure ₽ and PGL _ Sta. 185+33.7 Sta. 186+83.26 ĒB I-94 Reconstruct Stage Expansion Joint Reconstruct Const. Line Expansion Joint Perform Bridge Deck Grooving Overhead (Longitudinal) Utility Lines LOCATION SKETCH Slope Wall Track No. 3 -S. Slope Wall Perform Partial Depth 15'-03/4'' 2 Spaces @ |13'-5" 3 Spaces @ 3 Spaces at and Full Depth Deck Overhead Utility Lines 15'-0"=45'-0" 15'-0"=45'-0" 15'-0"=30'-0" Slab Repairs Track No. 2 13'-47/8" Track No. GENERAL PLAN AND ELEVATION 6" Ø Floor Drain Spacing 2 Spa. @ Apply 2" Stone-Matrix Asphalt (SMA) Track No. 1 15'-0"=30'-0' Overlay, typ. both approaches. For SMA Perform ¾" Bridge Deck EB I-94 OVER B. & O.C.T. R.R. & I.H.B. R.R. items, see Roadway Plans. Scarification, Apply 3" Bridge F.A.I. ROUTE 94 Deck Latex Concrete Overlay, PLAN Perform 1/4" Diamond Grinding MOUSSA SECTION 2019-180-RS&T ISSA and apply Protective Coat Dr. Moussa A. Issa, S.E. IL. Lic. No. 081-005738 081-005738 COOK COUNTY CHICAGO, ILLINOIS Expires 11-30-2026 STATION 185+33.75 12/06/2024 __ For Sheets S04-01 thru S04-26. S.N. 016-0160 (EB) DESIGNED - CJS, LR REVISED -COUNTY **STATE OF ILLINOIS** DRAWN - CJS, LR REVISED -(42-B-11-1) BR, BJR 24 COOK 761 559 STRUCTURE NO. 016-0160 (EB) 40:0.0000 ':" / in REVISED -**DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 62W87 SHEET S04-01 OF S04-26 SHEETS PLOT DATE = 12/6/2024 DATE 12/9/2024 REVISED -

GENERAL NOTES:

- 1. Reinforcement bars designated (E) shall be epoxy coated.
- 2. Plan dimensions and details relative to the existing structure have been taken from existing plans and are subject to nominal construction variations. The Contractor shall field-verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not cause for additional compensation for a change in the scope of the work; however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 3. The Contractor may request copies of existing construction plans that are currently on file with the Illinois Department of Transportation (IDOT). The request shall be in writing with the understanding that any reproduction cost will be the Contractors expense and at no additional cost to the Department.
- 4. All exposed concrete edges shall have a ¾" x 45" chamfer except where shown otherwise.
- 5. Protective coat shall be applied to the top of reconstructed transverse joint areas, top of new latex concrete overlay, and top and inside faces of parapets.
- 6. Joint openings shall be adjusted according to Article 520.04 of the standard specifications when the deck is poured at an ambient temperature other than 50°F.
- 7. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose detrimental foreign material shall be removed from the surfaces in contact with concrete (SSPC-SP3 standards). Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be paid for according to Article 109.04 of the Standard Specifications. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding $lam{1}{4}$ inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 8. The Contractor shall take all necessary precautions for the protection of passing vehicles and pedestrians from falling objects and/or materials until completion of the work.
- 9. It shall be the Contractors responsibility to locate and protect any utilities or facilities on, within or under the bridge deck including but not limited to under deck lighting, traffic signals or signs attached to the structure. Any damage to existing utilities caused by the Contractor in the performance of the work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 10. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to existing elements to remain cause by the Contractor in the performance of the work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.
- 11. Cleaning and field painting of structural steel shall be done under a separate painting
- 12. For SMA overlay on Approach Slab, see Civil Sheets.
- 13. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.
- 14. Adjacent I-94 WB bridge is not shown throughout the plans for clarity.
- 15. The Contractor is responsible to protect the existing conduit and junction box embedded in the parapet during removal and construction. Any damage to the existing conduit and junction box shall be repaired by the Contractor at no additional cost to the Department.
- 16. Concrete Sealer shall be applied to the designated areas of the abutments and piers.
- 17. Prior to the application of the Concrete Sealer, the Contractor shall clean all existing debris from the abutment seats. The method of debris removal shall not damage the existing concrete and shall be approved by the Engineer. See special provision for Debris Removal.
- 18. The Engineer shall show actual locations and size of deck repairs on As-built Plans.

INDEX OF SHEETS

504-01	General Plan and Elevation
<i>S04-02</i>	General Notes, Index of Sheets & TBOM
504-03	Stage Construction (Sheet 1 of 2)
504-04	Stage Construction (Sheet 2 of 2)
504-05	Temporary Concrete Barrier
504-06	Deck Repair Plan (Sheet 1 of 2)
504-07	Deck Repair Plan (Sheet 2 of 2)
<i>504-08</i>	Drainage Scupper Adjustment Details
504-09	N. Abut. Joint Removal & Replacement (Sht. 1 of 3
S04-10	N. Abut. Joint Removal & Replacement (Sht. 2 of 3
S04-11	N. Abut. Joint Removal & Replacement (Sht. 3 of 3
504-12	Pier 2 Joint Removal & Replacement (Sht. 1 of 2)
S04-13	Pier 2 Joint Removal & Replacement (Sht. 2 of 2)
504-14	S. Abut. Joint Removal & Replacement (Sht. 1 of 3
S04-15	S. Abut. Joint Removal & Replacement (Sht. 2 of 3
504-16	S. Abut. Joint Removal & Replacement (Sht. 3 of 3
S04-17	Preformed Joint Strip Seal
504-18	North Abutment Repairs
504-19	South Abutment Repairs
504-20	Pier 1 Repairs
504-21	Pier 2 Repairs
504-22	Pier 3 Repairs
504-23	Pier 4 Repairs
504-24	Pier 5 Repairs
<i>S04-25</i>	Slope Wall Repairs

S04-26 Bar Splicer Assembly & Mechanical Splicer Detail

TOTAL BILL OF MATERIAL

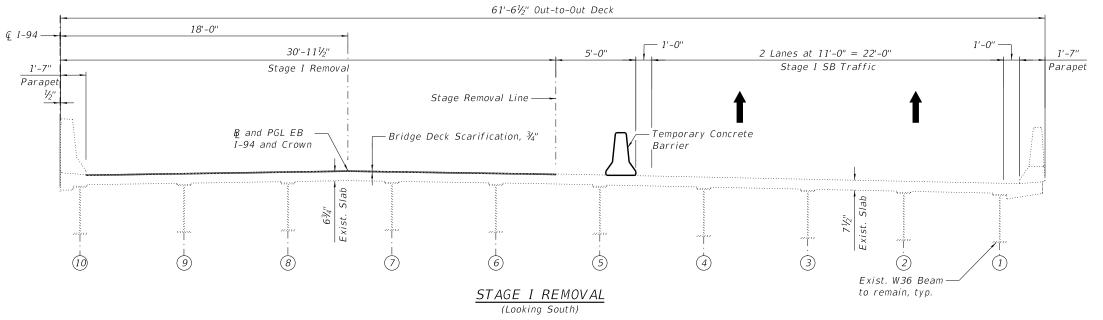
ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	Cu Yd	-	1	1
Concrete Removal	Cu Yd	32.6	-	32.6
Protective Shield	Sq Yd	1,921	-	1,921
Concrete Superstructure	Cu Yd	36.9	-	36.9
Protective Coat	Sq Yd	3,120	-	3,120
Reinforcement Bars, Epoxy Coated	Pound	5,050	-	5,050
Bar Splicers	Each	48	-	48
Preformed Joint Seal 2 1/2"	Foot	414	-	414
Preformed Joint Strip Seal	Foot	210	-	210
Concrete Sealer	Sq Ft	-	4,241	4,241
Epoxy Crack Injection	Foot	ı	36	36
Bolt Replacement	Each	I	2	2
Bridge Deck Grooving (Longitudinal)	Sq Yd	2,217	ı	2,217
Slope Wall Crack Sealing	Foot	ı	50	50
Deck Drain Extensions	Each	16	-	16
Drainage Scuppers To Be Adjusted	Each	2	-	2
Bridge Deck Latex Concrete Overlay, 3 Inches	Sq Yd	2,617	-	2,617
Bridge Deck Scarification 3/4"	Sq Yd	2,617	ı	2,617
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	I	268	268
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	-	3	3
Deck Slab Repair (Full Depth, Type I)	Sq Yd	0.1	-	0.1
Diamond Grinding (Bridge Section)	Sq Yd	2,511	-	2,511
Temporary Shoring And Cribbing	Each	-	10	10

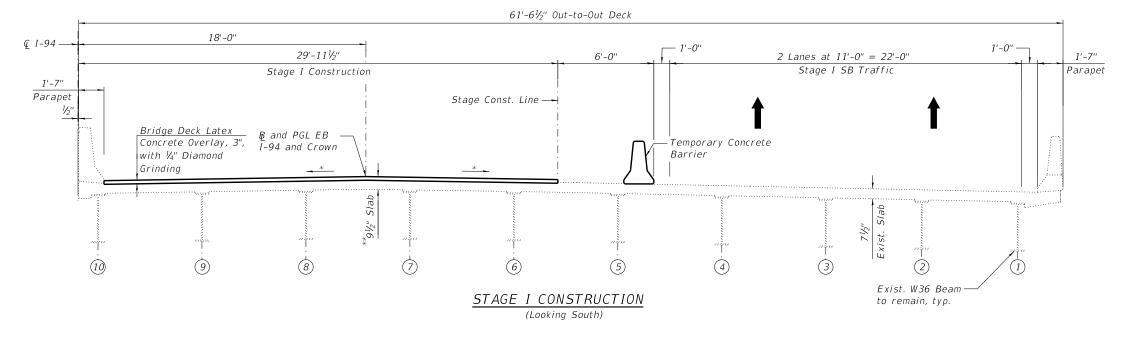
PROPOSED SCOPE OF WORK

- 1. Provide Protective shield within limits indicated on the plans.
- Perform Deck Slab Repairs and adjust and extend existing scuppers and floor drains as required
- 3. Perform ¾" Bridge Deck Scarification.
- Reconstruct Expansion Joints at the North and South abutments and Pier 2 and install new preformed joint strip seals.
- 5. Apply 3" Bridge Deck Latex Concrete Overlay on Bridge Deck.
- Perform $\frac{1}{4}$ " Diamond Grinding to top of bridge deck and abutment hatch block.
- 7. Perform Bridge Deck Grooving (Longitudinal) on traffic lanes.
- Apply protective coat to the top of reconstructed transverse joint areas, top of new latex concrete overlay and top and inside faces of parapets.
- Perform structural repair of concrete to all spalled and delaminated areas, and perform low pressure epoxy injection to all open cracks (1/8"-wide and wider), for the abutments and piers as noted on the plans.
- 10. Perform Slope Wall repairs.

USER NAME =	hbmepw11ics01\$	DESIGNED	-	DEO, AWD	REVISED	-
		DRAWN	-	DEO, AWD	REVISED	•
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STAGE I REMOVAL

- 1. Install temporary concrete barrier as shown to locate traffic on the west side of the existing structure.
- 2. Perform ¾" bridge deck scarification.
- 3. Remove portions of bridge deck/approach slab adjacent to expansion joints at the North and South Abutments, and remove portions of bridge deck slab adjacent to expansion joint at Pier 2.

STAGE I CONSTRUCTION

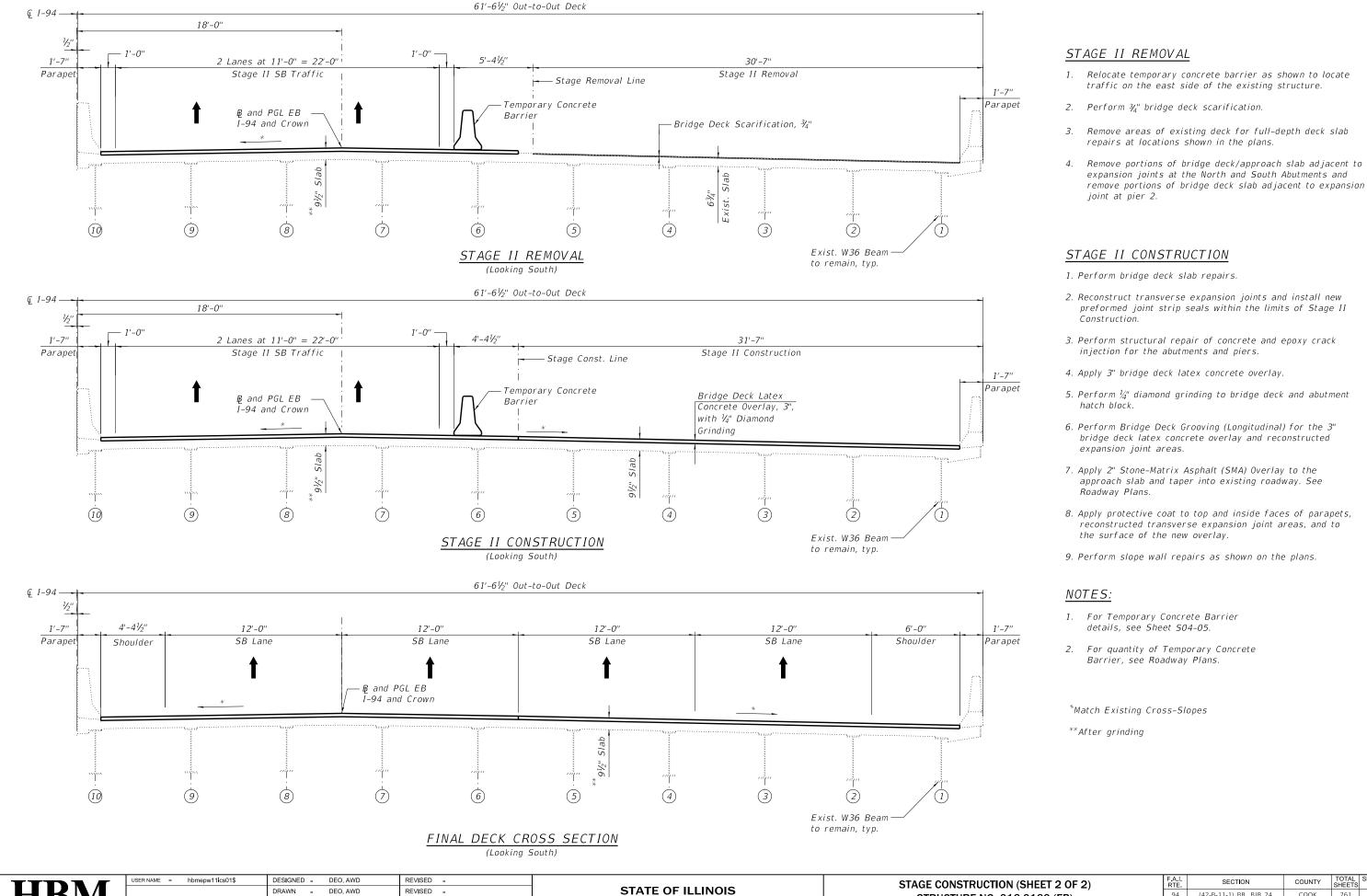
- 1. Perform bridge deck slab repairs.
- 2. Reconstruct transverse expansion joints and install new preformed joint strip seals within the limits of Stage I Construction.
- 3. Adjust existing drainage scuppers per the details shown in the plans.
- 4. Perform structural repair of concrete and epoxy crack injection for the abutments and piers.
- 5. Apply 3" bridge deck latex concrete overlay.
- 6. Perform ¼" diamond grinding to bridge deck and abutment hatch block.
- 7. Perform Bridge Deck Grooving (Longitudinal) for the 3" bridge deck latex concrete overlay and reconstructed expansion joint areas.
- 8. Apply 2" Stone–Matrix Asphalt (SMA) Overlay to the approach slab and taper into existing roadway. See Roadway Plans.
- 9. Apply protective coat to top and inside faces of parapets, reconstructed transverse expansion joint areas and to the surface of the new overlay.
- 10. Perform slope wall repairs as shown on the plans.

NOTES:

- 1. For Temporary Concrete Barrier details, see Sheet SO4-O5.
- 2. For quantity of Temporary Concrete Barrier, see Roadway Plans.

*Match Existing Cross-Slopes

** After grinding



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- 12/9/2024

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STAGE CONSTRUCT
STAGE CONSTRUC

AGE CONSTRUCTION (SHEET 2 OF 2)

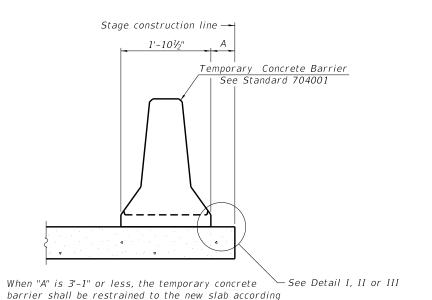
STRUCTURE NO. 016-0160 (EB)

SHEET 504-04 OF 504-26 SHEETS

F.A.I. SECTION COUNTY TOTAL SHEETS NO.

94 (42-B-11-1) BR, BJR 24 COOK 761 562

CONTRACT NO. 62W87



├- Stage removal line - Stage removal line 1'-101/2" 1'-101/2" Temporary Concrete Barrier See Standard 704001 min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint *When hot-mix asphalt wearing surface is present, embedment is required when "A" is greater than 3'-1". shall be 3" plus the wearing surface depth.

NEW SLAB OR NEW DECK BEAM

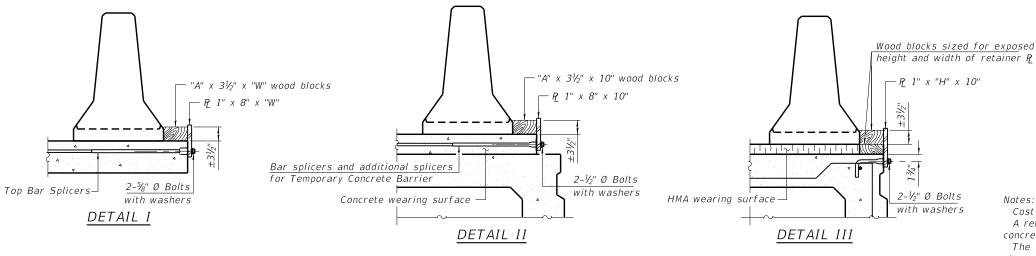
to Detail I, II or III. No restraint is required

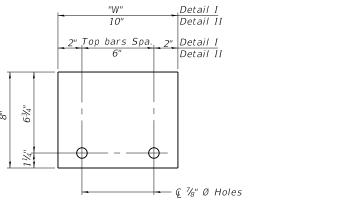
when "A" is greater than 3'-1".

EXISTING SLAB

EXISTING DECK BEAM

SECTIONS THRU SLAB OR DECK BEAM

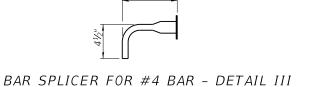




STEEL RETAINER P 1" x 8" x "W"	
(Detail I and II)	

—⊊ ¾" Ø Holes

STEEL RETAINER P 1" x "H" x 10"



RESTRAINING PIN

-7∕₁₆" Ø hole

Cost of retainer assembly is included with Temporary Concrete Barrier. A retainer assembly shall be located at the approximate Q of each temporary concrete barrier.

1x8 UNC

1" Ø pin

US Std. $1\frac{1}{16}$ " I.D. \times $2\frac{1}{2}$ " O.D. x approx. 8 gauge thick washer

The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.

When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.

- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

ENGINEEDING GROUP LLC	

RAILING CRITERIA

440

5-15-2023

NCHRP 350 Test Level

Railing Weight (plf)

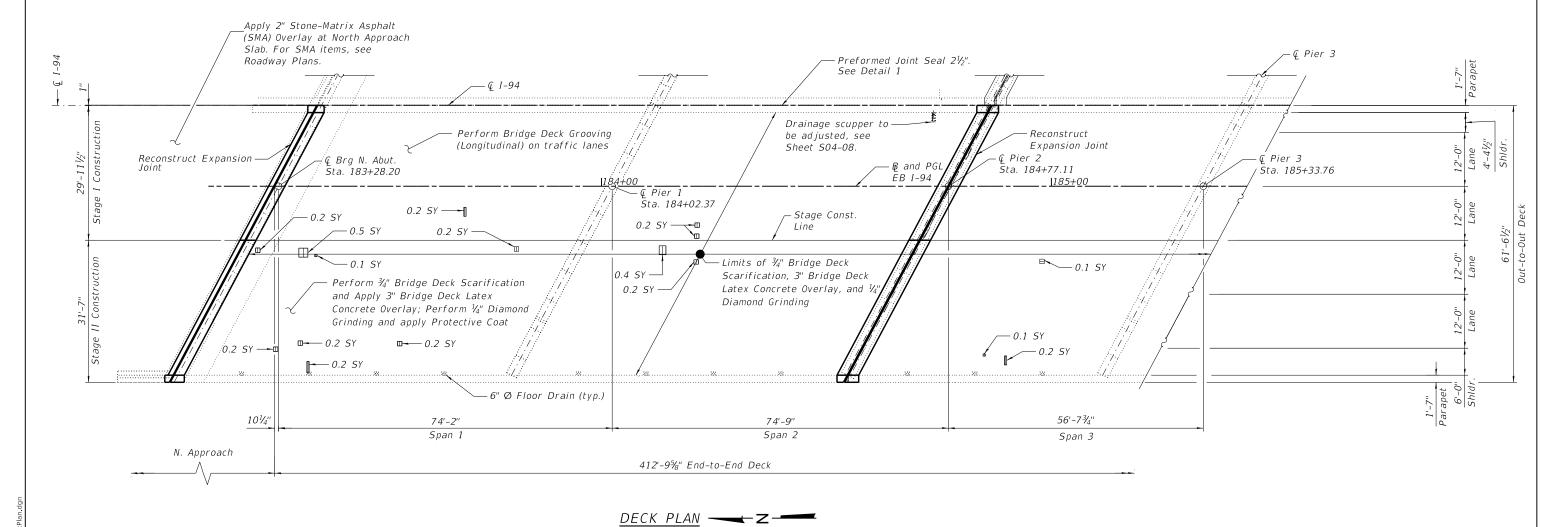
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION COUNTY **TEMPORARY CONCRETE BARRIER** (42-B-11-1) BR. BJR 24 COOK 761 563 STRUCTURE NO. 016-0160 (EB) CONTRACT NO. 62W87 SHEET S04-05 OF S04-26 SHEETS

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R-27



*Areas of Deck Slab Repair (Partial) are provided for information only and shall be included in the cost of Bridge Deck Latex Concrete Overlay, 3"

LEGEND

SY.

NOTE:

1. For Notes, Bill of Material and Detail 1, see Sheet S04-07.

HBM ENGINEERING GROUP, LLC

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

DECK REPAIR PLAN (SHEET 1 OF 2)
STRUCTURE NO. 016-0160 (EB)

SHEET 504-06 OF 504-26 SHEETS

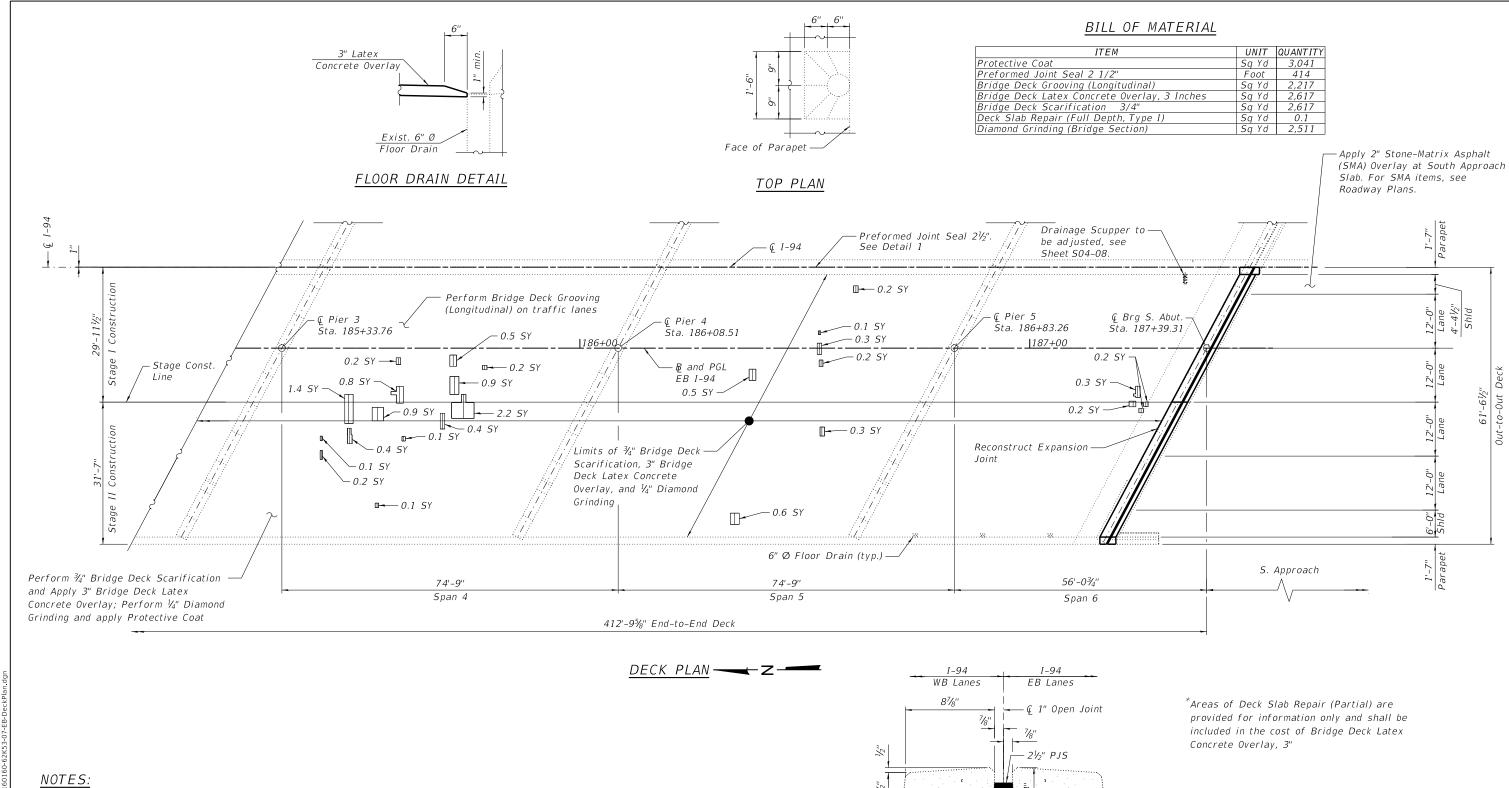
Square Yard

Deck Slab Repair (Partial)*

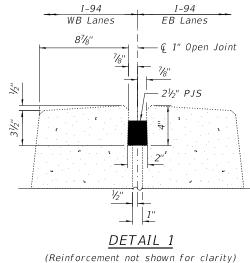
Deck Slab Repair (Full Depth, Type I)

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- 1. Areas of deck repair shown are estimated. The Engineer shall show actual locations of deck repairs at the time of construction.
- 2. For bridge deck final cross section, see Sheet S04-04.
- 3. For North Abutment, Pier 2, and South Abutment transverse joint removal and reconstruction, see Sheets S04-09 thru S04-16.
- 4. Perform V_4 " Diamond Grinding to top of bridge deck and abutment hatch block.
- 5. Any reinforcement bars that are damaged during concrete removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.



LEGEND

Deck Slab Repair (Partial)*

SY Square Yard



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DECK REPAIR PLAN (SHEET 2 OF 2) STRUCTURE NO. 016-0160 (EB) TYPICAL SCUPPER PLAN

2'-1"

Exist. Frame 2'-5/8''

1'-11"

1'-105/8"

Exist. Frame Opening

1'-10¾'' Exist. Grate and Proposed Adjusting Ring A

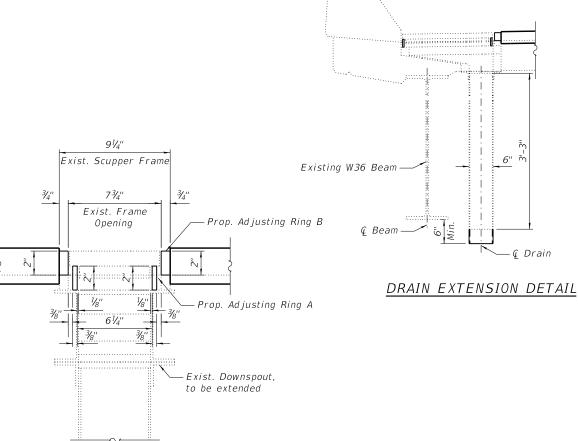
Exist. 11/4" grate -

Prop. Adjusting -Ring A

−Bridge Deck Scarification, ¾" -Bridge Deck Latex Concrete Overlay, 3", before ½" diamond

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Deck Drain Extensions	Each	16
Drainage Scuppers To Be Adjusted	Each	2



NOTES:

- 1. The Contractor shall field-verify Existing Dimensions and Details of the Existing Scuppers and downspouts and make necessary approved adjustments prior to construction or ordering of material for Adjusting Drainage Scuppers and extending downspouts.
- 2. All Cast Iron Parts shall be Grey Iron conforming to the requirements of AASHTO M 105, Class 35B.
- Cast Iron Parts shall be unfinished.

SECTION B-B

- Bolts, washers and nuts shall conform to the requirements of ASTM A307 and be galvanized according to the requirements of AASHTO M232.
- 5. The Contractor shall take appropriate measures to ensure that Protective Coat is not applied to the scuppers.
- Adjusting Rings shall be from Neenah or approved equal. Structural steel weldments or equal sections and of the same configuration may be submitted in place of Cast Iron. Fillet or full penetration welds may be used for weldments. Details shall be submitted to the Engineer for approval.
- 7. Provide 1/8" Fillet Weld around perimeter of new Adjusting Rings to secure to existing Scupper. Electrode shall be compatible with cast iron if existing scupper elements are cast iron construction.
- Cost of all labor and materials necessary to clean all existing floor drains and scuppers, fabricate and install adjusting scupper rings, remove and reinstall grates is included in the cost of Drainage Scuppers to be Adjusted.

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L	ENGINEERING GROUP, LLC

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Exist. Beam, to remain

grinding

- Prop. Adjusting Ring B

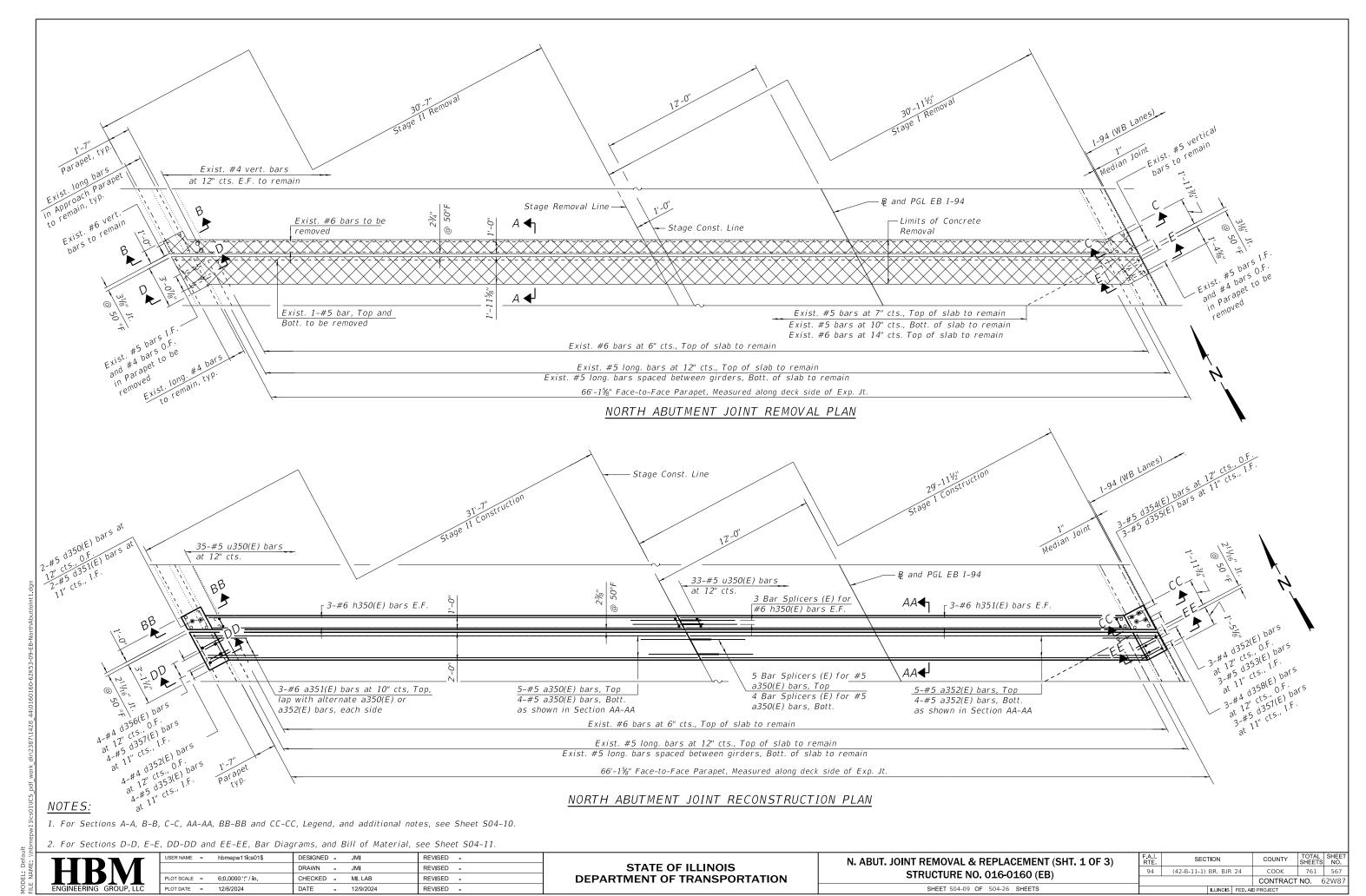
Exist. Scupper

* Existing slope

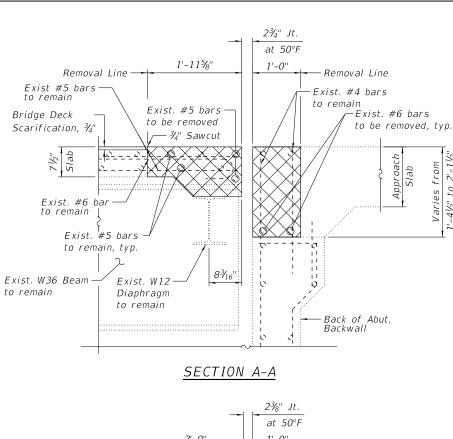
to remain

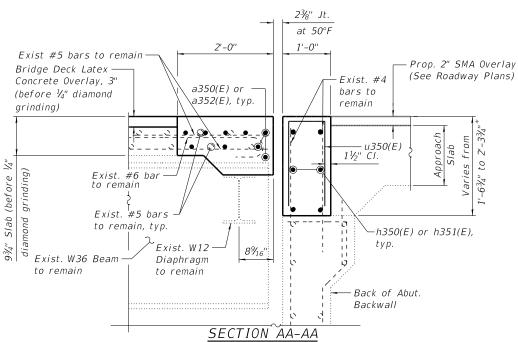
Exist. Downspout, to be extended

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- 1. For preformed joint strip seal details, see Sheet S04-17.
- 2. For bar splicer assembly details, see Sheet S04-26.
- 3. Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 4. Epoxy grout d350(E), d351(E), d354(E) and d355(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.
- 5. Existing reinforcement bars extending into the removal area shall be cleaned, straightened, and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system at the contractor's expense.
- 6. Any reinforcement bars that are damaged during Concrete Removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to "Concrete Removal".
- 7. Dimensions are based on a Roller Rail Strip Seal joint. If the Contractor elects to use the Welded Rail Strip Seal Joint, deck dimensions may require adjustments to satisfy the details on Sheet S04-17.

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Exist. #6 bars to remain Exist. #4 bars to remain, typ.

SECTION B-B (Reinforcement in the pour strip

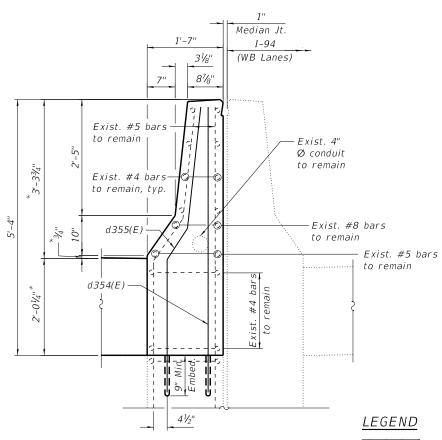
not shown for clarity)

1'-7" 87/8" 31/8" Exist. #6 bars to remain Exist. #4 bars to remain, typ. - d351(E) d350(E) SECTION BB-BB (Reinforcement in the pour strip

not shown for clarity)

STATE OF ILLINOIS

not shown for clarity)



SECTION C-C

(Reinforcement in the pour strip

1" Median Jt.

(WB Lanes)

-Exist. #8 bars to remain

Exist. 4"

Ø conduit

to remain

Exist. #5 bars

to remain

3½"

87/8"

Exist. #5 bars

Exist. #4 bars

to remain, typ.

to remain

SECTION CC-CC

(Reinforcement in the pour strip not shown for clarity)

Concrete Removal

Each Face

I.F. Inside Face

0.F. Outside Face

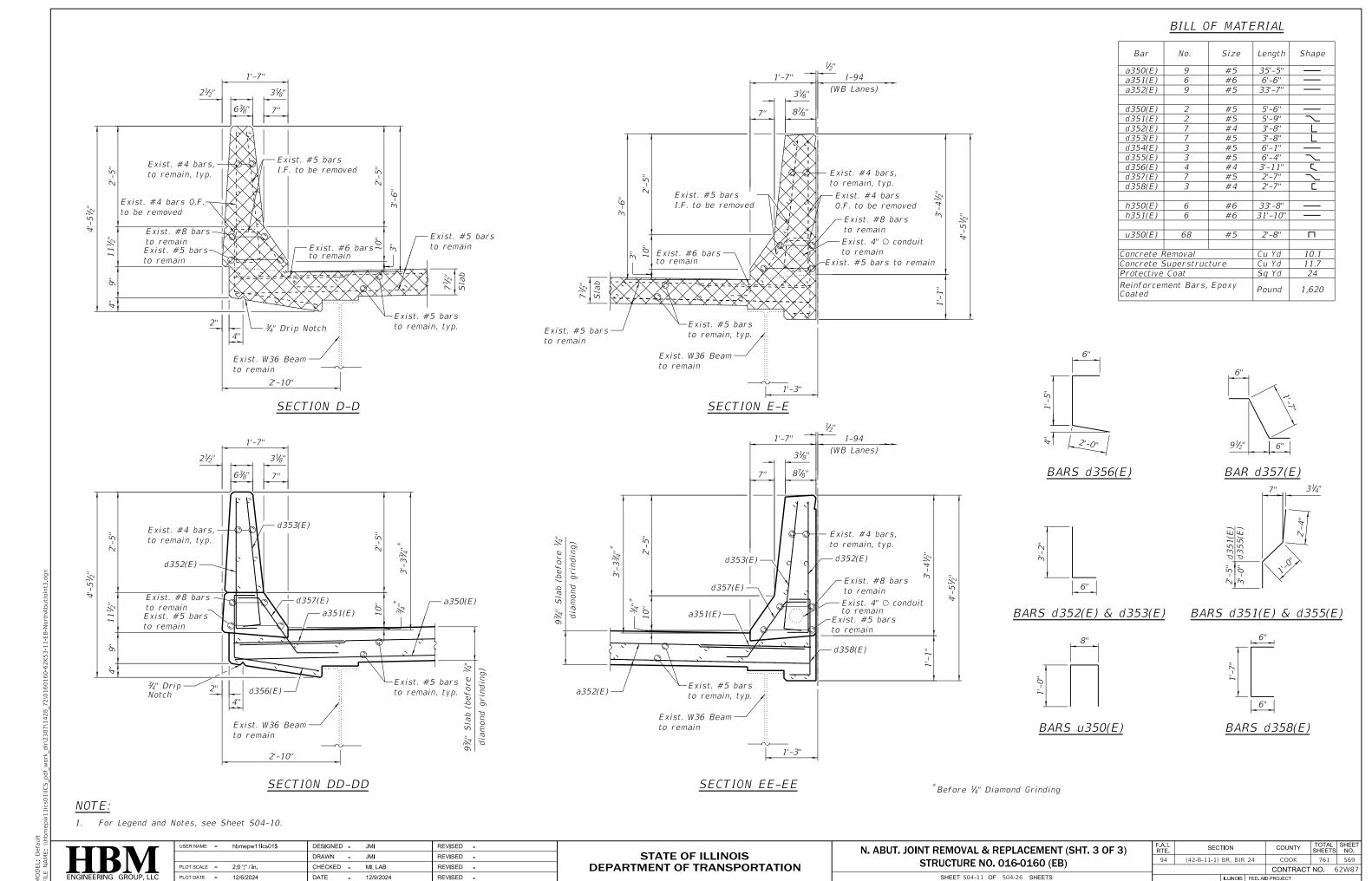
DEPARTMENT OF TRANSPORTATION

N. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 2 OF 3) STRUCTURE NO. 016-0160 (EB) SHEET S04-10 OF S04-26 SHEETS

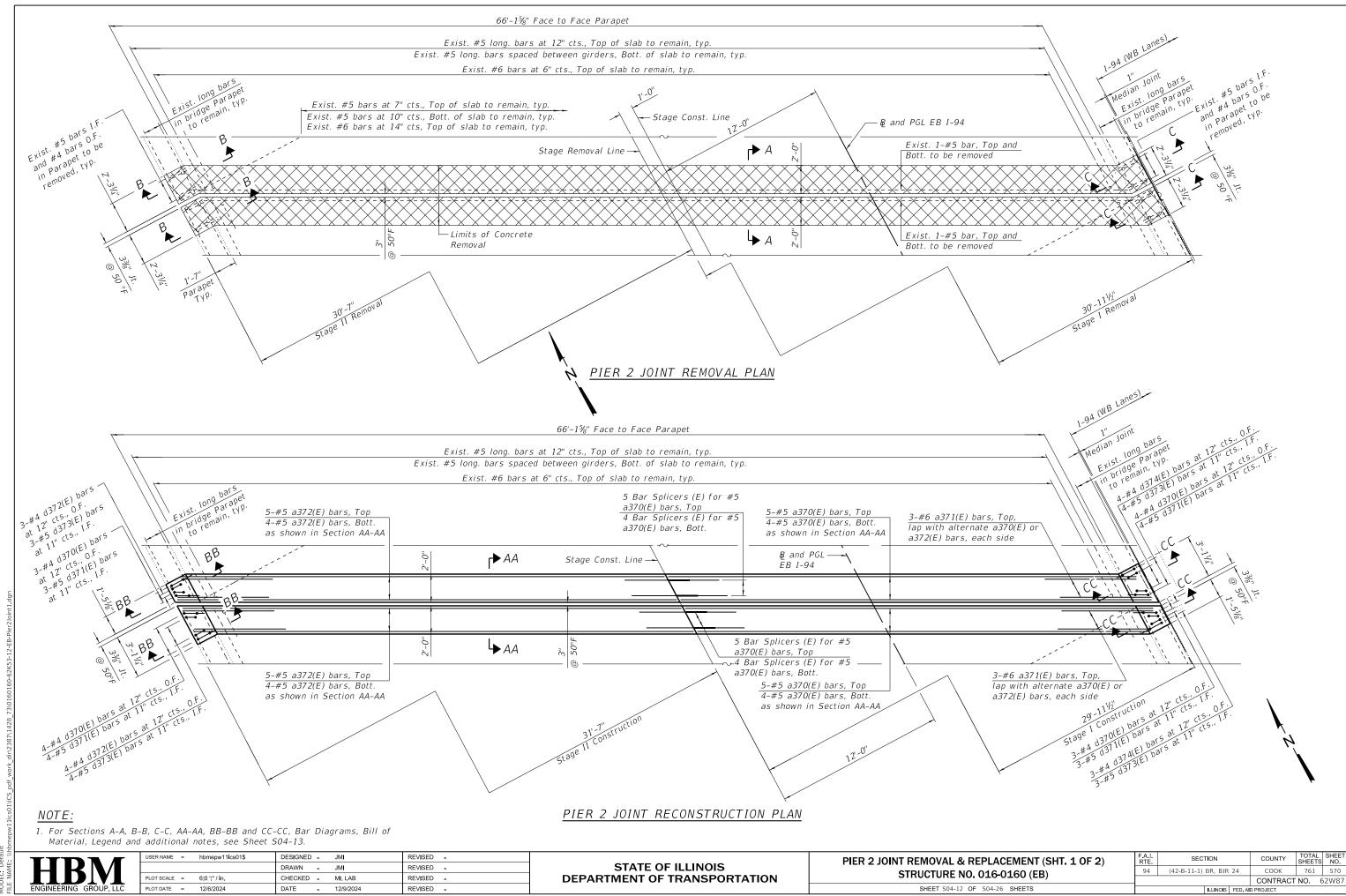
*Before ¼" Diamond Grinding

SECTION COUNTY 94 (42-B-11-1) BR, BJR 24 COOK 761 568 CONTRACT NO. 62W87

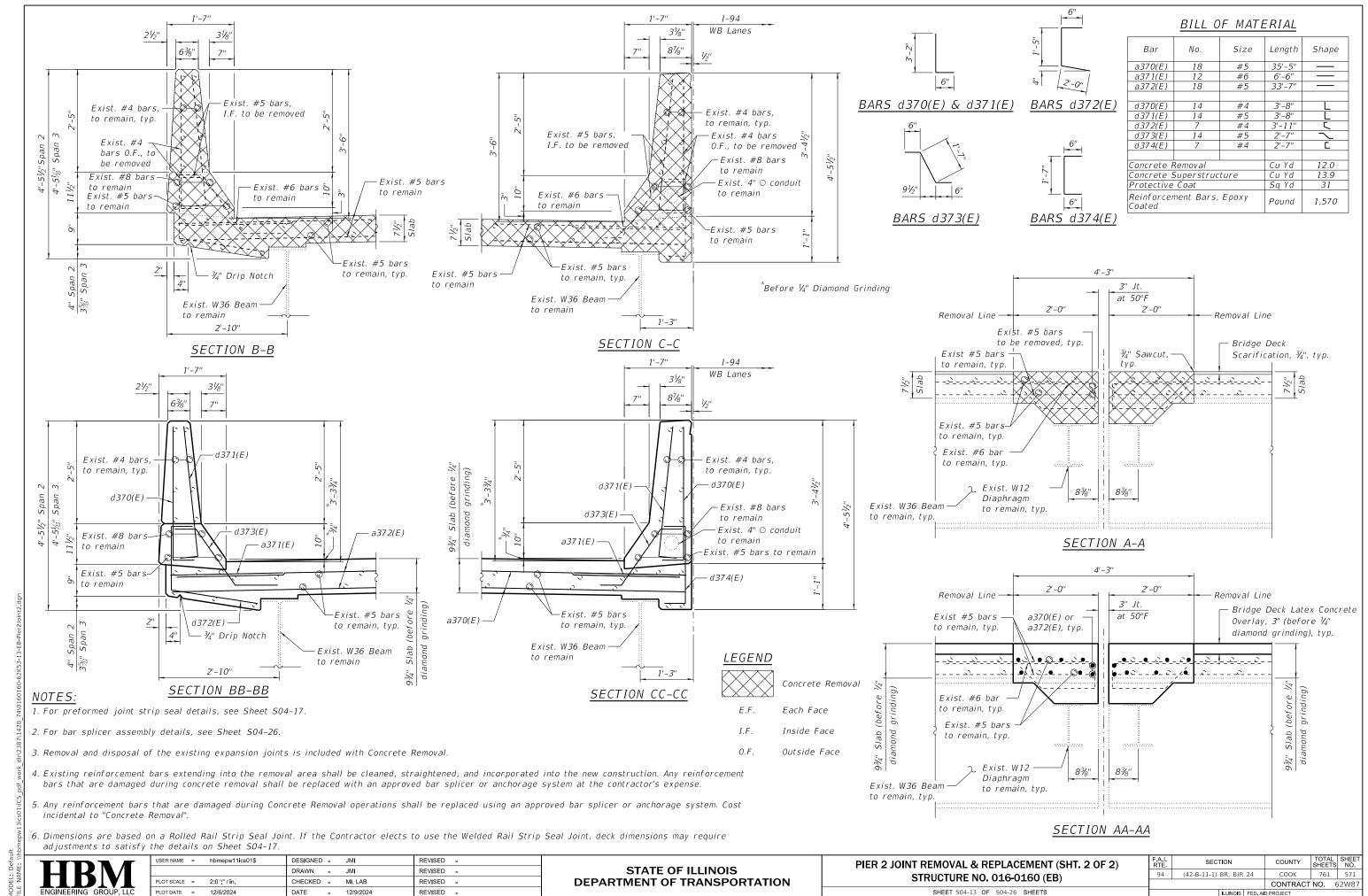
E.F.



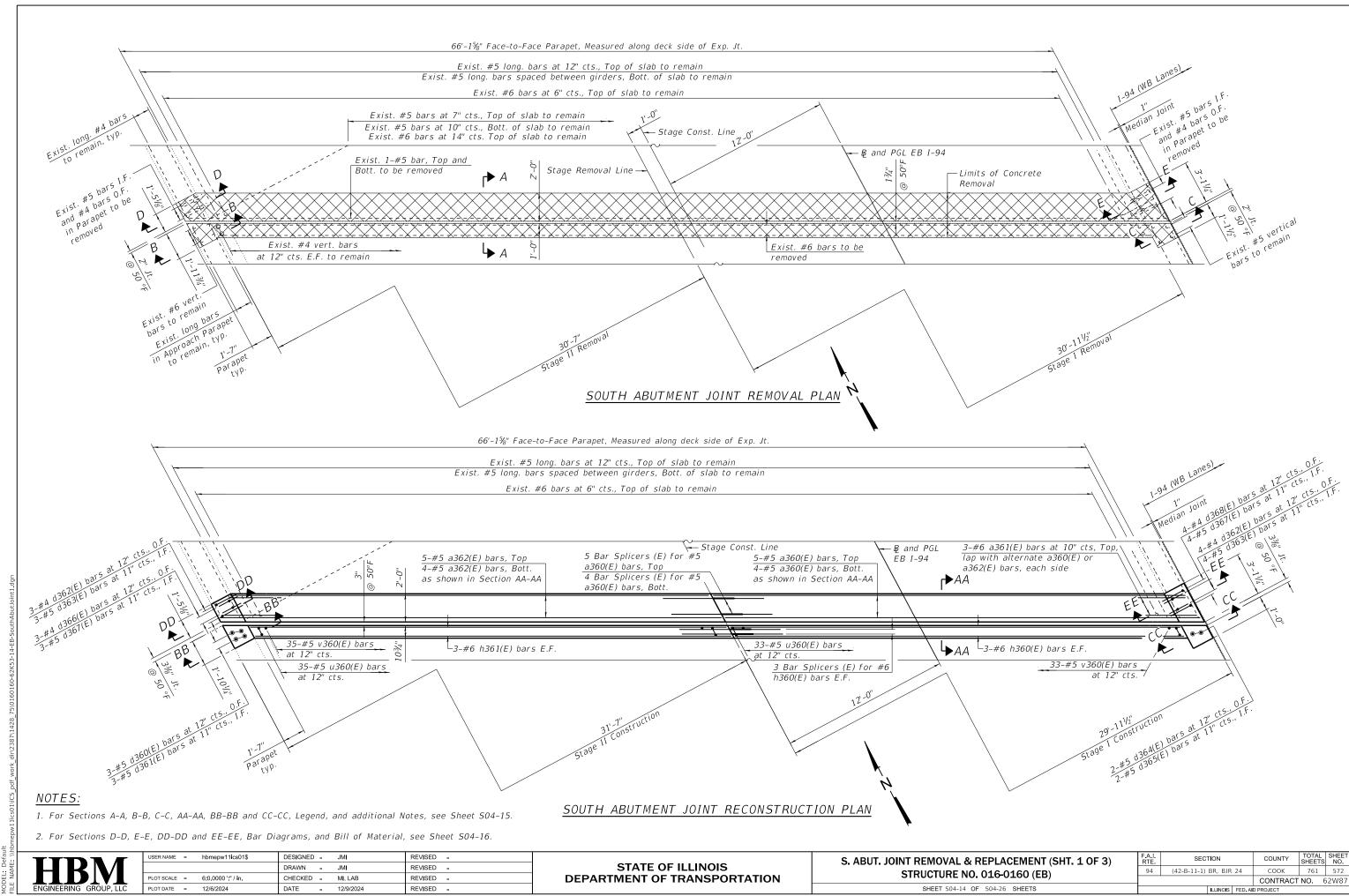
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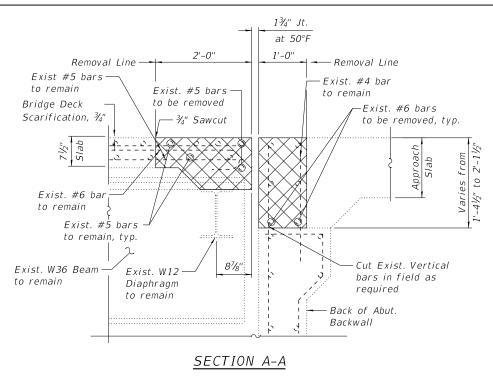
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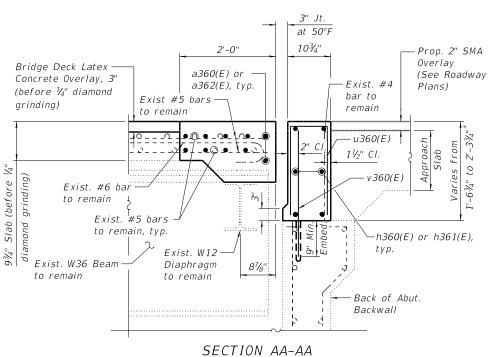


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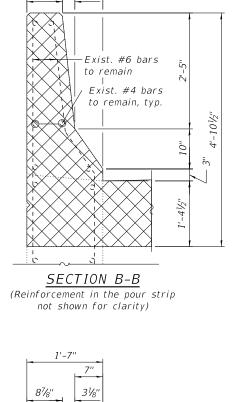
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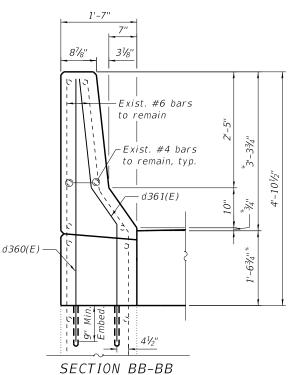






- 1. For preformed joint strip seal details, see Sheet SO4-17.
- 2. For bar splicer assembly details, see Sheet S04-26.
- 3. Removal and disposal of the existing expansion joints is included with Concrete Removal.
- 4. Epoxy grout d360(E), d361(E), d365(E) and v360(E) bars according to Article 584 of the Standard Specifications. Drill to miss existing reinforcement. Cost included with Concrete Superstructure.
- 5. Existing reinforcement bars extending into the removal area shall be cleaned, straightened, and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal shall be replaced with an approved bar splicer or anchorage system at the contractor's expense.
- 6. Any reinforcement bars that are damaged during Concrete Removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to "Concrete Removal".
- 7. Dimensions are based on a Roller Rail Strip Seal joint. If the Contractor elects to use the Welded Rail Strip Seal Joint, deck dimensions may require adjustments to satisfy the details on Sheet SO4-17.





(Reinforcement in the pour strip

*Before 1/4" Diamond Grinding

SECTION CC-CC (Reinforcement in the pour strip not shown for clarity)

SECTION C-C (Reinforcement in the pour strip not shown for clarity) 1" Median Jt. I-94 31/8" (WB Lanes) 8⁷/8'' Exist. #5 bars to remain Exist. #4 bars to remain, typ. Exist. #8 bars d365(E, to remain Exist. #5 bars to remain Exist. 4" Ø conduit

Median Jt. I-94

3½"

87/8"

Exist. #5 bars to remain

Exist. #4 bars-

to remain, typ.

(WB Lanes)

-Exist. #8 bars

- Exist. #5 bars

to remain

to remain

Exist. 4" Ø conduit

to remain

LEGEND

Concrete Removal Each Face

I.F. Inside Face

0.F. Outside Face

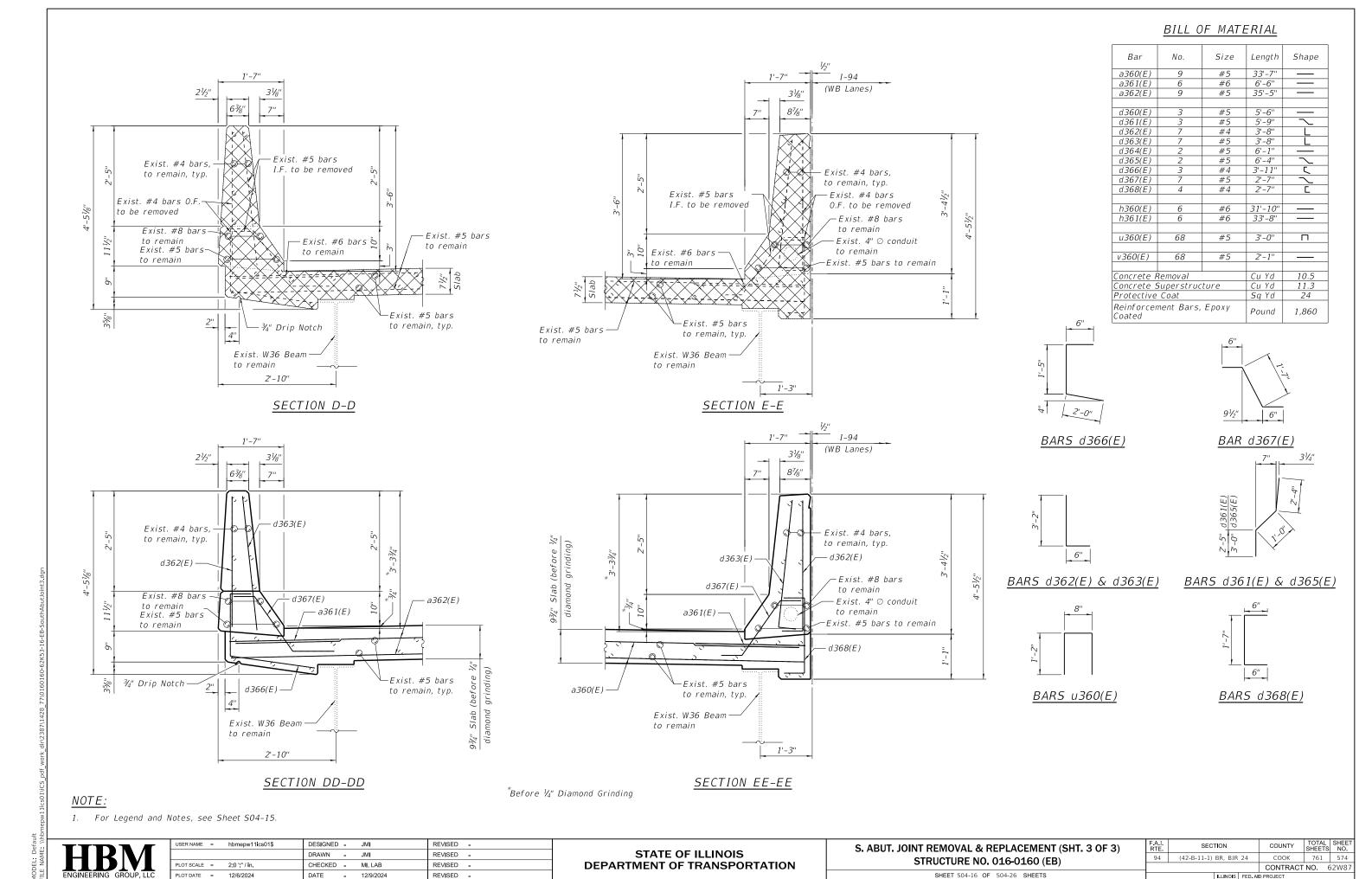
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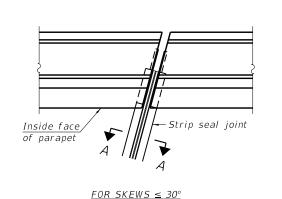
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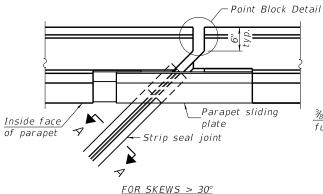
d364(E

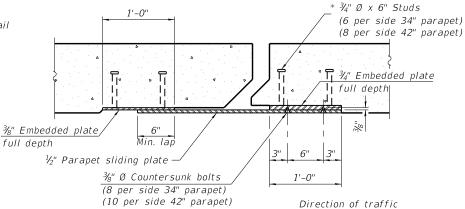
SECTION COUNTY (42-B-11-1) BR, BJR 24 COOK 761 573 CONTRACT NO. 62W87



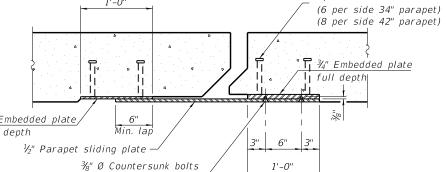
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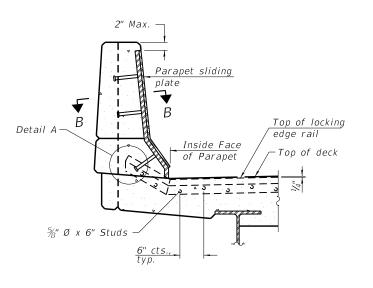




SECTION B-B



PLAN AT PARAPET

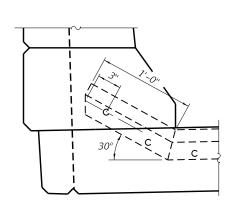




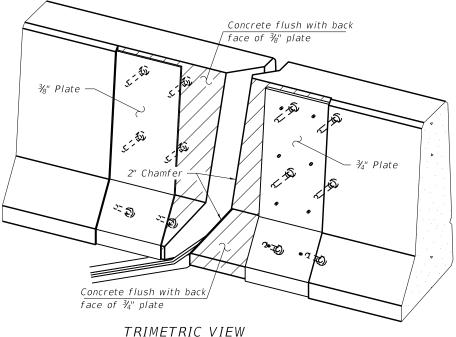
(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)

Locking edge rail -

Top of concrete



DETAIL A



TRIMETRIC VIEW (Showing embedded plates only)

Locking edge railat 50° F Top of concrete * $\frac{5}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs) $\frac{3}{6}$ " ϕ threaded rods in $\frac{7}{16}$ " ϕ holes at ± 4 '-0" cts. at 50° F for holding the proper joint opening based on the temperature during the deck pour. Place to

SHOWING ROLLED RAIL JOINT

at 50° F

Joint	Α	В	С
North Abut.	11/2"	23/8"	3"
Pier 2	21/8"	3"	35/8"
South Abut.	11/5"	23/6"	3"

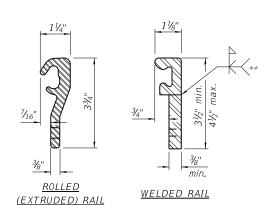
SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.

miss studs. All rods shall be burned, or sawed

off flush with the plates after concrete is set.

*** Before 1/4" Diamond Grinding



Notes:

rated movement of 4 inches.

shall be followed.

rail splice detail.

similar as noted.

The strip seal shall be made continuous and shall have

a minimum thickness of 1/4". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from

manufacturer to manufacturer provided they fit the application

however, will not be allowed. Locking edge rails may exceed the

 $4\frac{1}{2}$ " maximum depth provided the anchorage system is revised

All steel components shall be galvanized after fabrication

curb or parapet shall be welded as shown in the locking edge

The top surface of sidewalk sliding plates shall have a

Cost of parapet sliding plates, sidewalk sliding plates,

embedded plates, anchorage studs, and expansion anchors

39" constant slope barrier shown, 44" constant slope barrier

The concrete opening below the strip seal will vary based

parapet lengths shown elsewhere in the plans are dimensioned

to the concrete opening, not the joint opening, and are based

on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments

may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these

cases the pavement connector length shall be adjusted, not the

on the locking edge rail chosen by the Contractor. Deck and

according to Article 520.03 of the Standard Specifications. The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the

The manufacturer's recommended installation methods

according to the manufacturer's recommendation.

raised pattern according to ASTM A786.

included with Preformed Joint Strip Seal.

length of the bridge approach slab.

and meet the minimum anchorage shown. Flanged edge rails,

LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.

flush Omit weld at seal opening

LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	210



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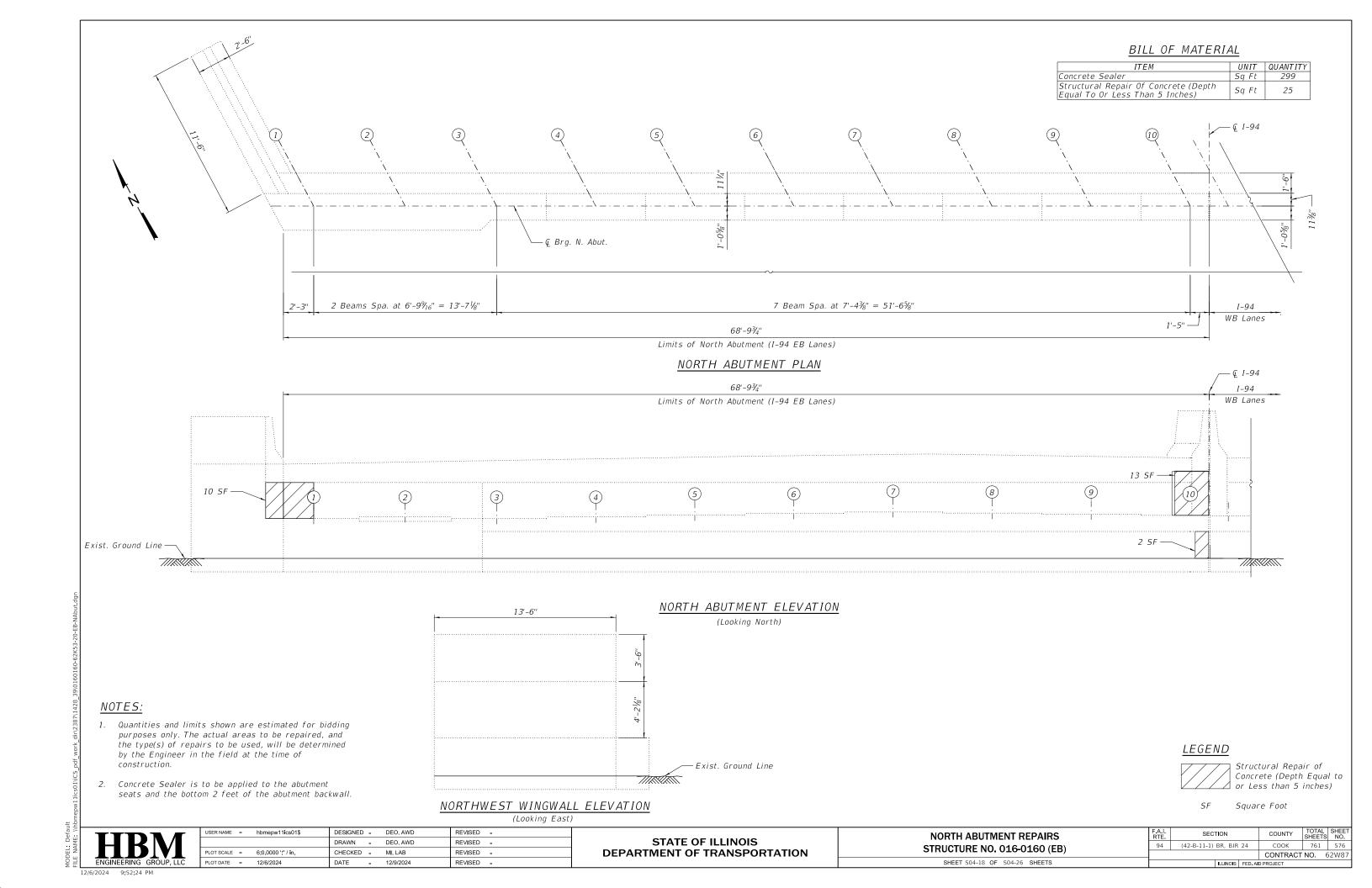
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

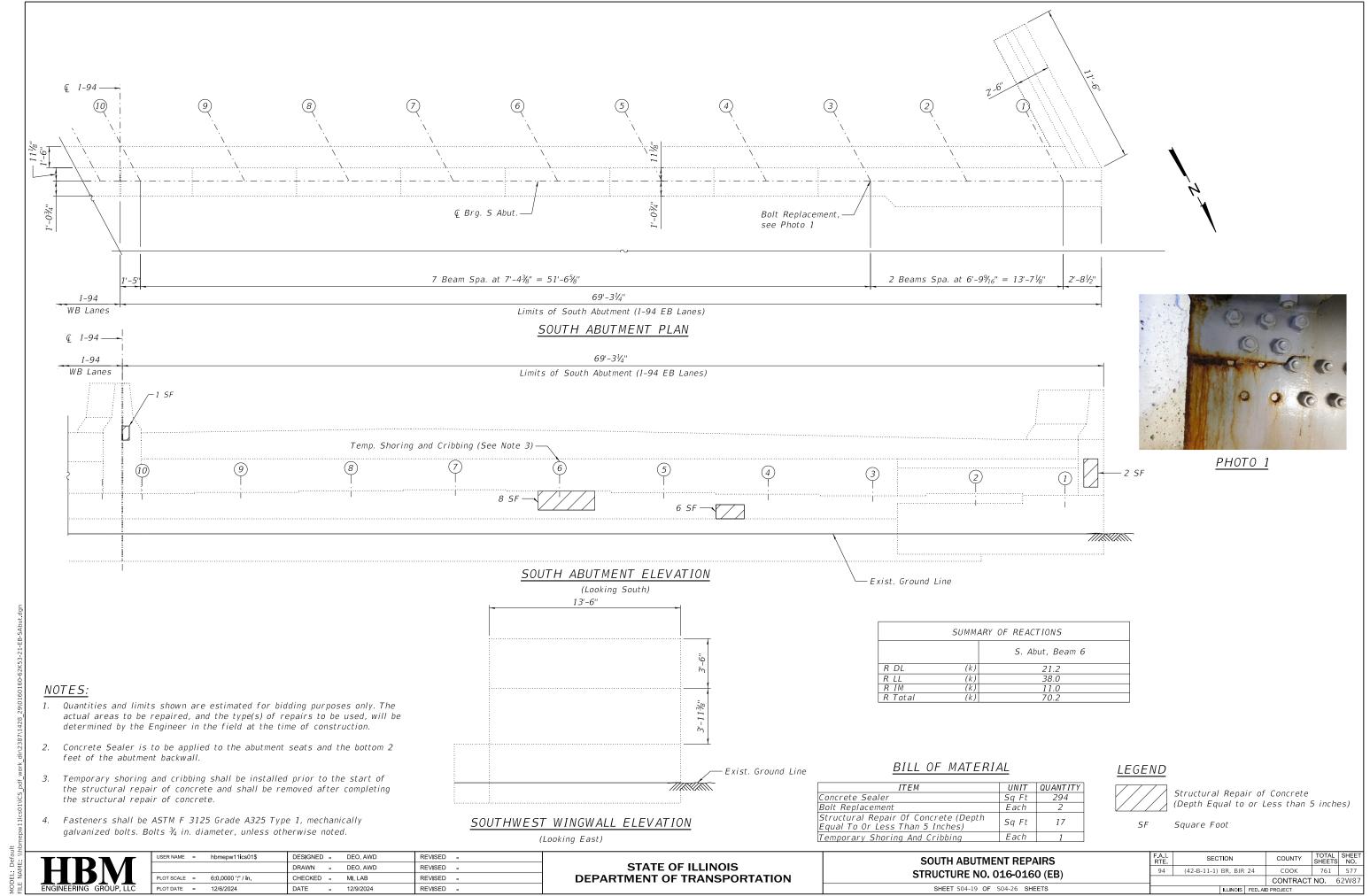
SHOWING WELDED RAIL JOINT

PREFORMED JOINT STRIP SEAL STRUCTURE NO. 016-0160 (EB) SHEET S04-17 OF S04-26 SHEETS

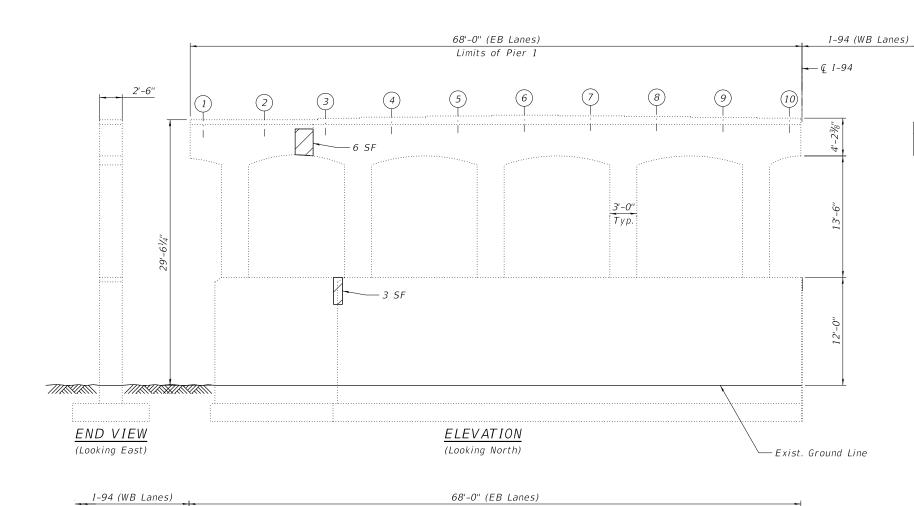
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CONTRACT NO. 62W87							

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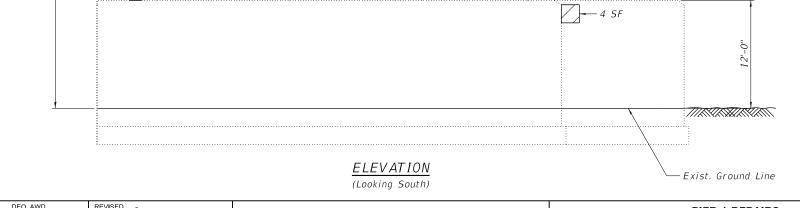
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ITEM	UNIT	QUANTITY
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	45

<u>NOTE:</u>

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.



Limits of Pier 1

LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

SF

Square Foot

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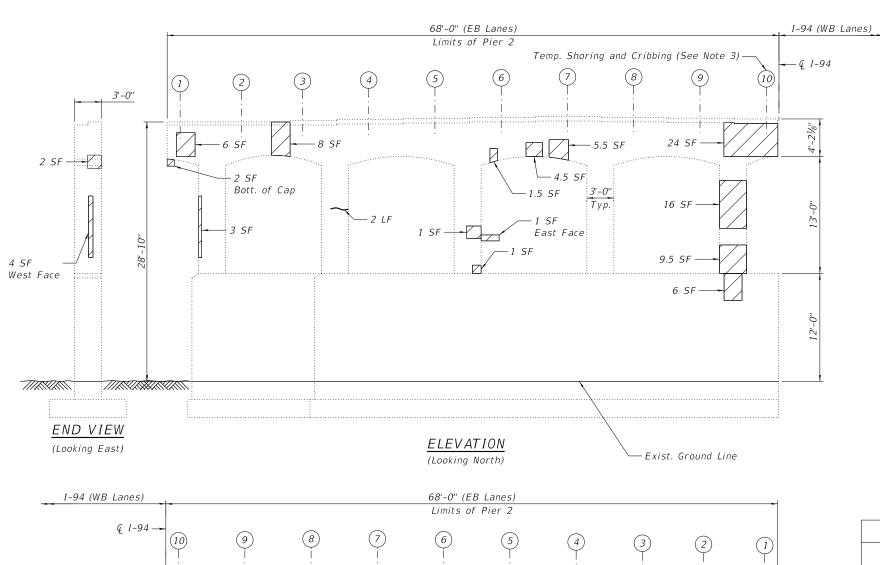
€ I-94 -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

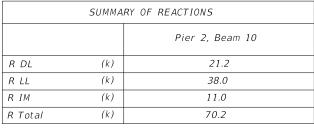
PIER 1 REPAIRS STRUCTURE NO. 016-0160 (EB) SHEET S04-20 OF S04-26 SHEETS

--- 16 SF West Face

> SECTION (42-B-11-1) BR, BJR 24 COOK 761 578 CONTRACT NO. 62W87

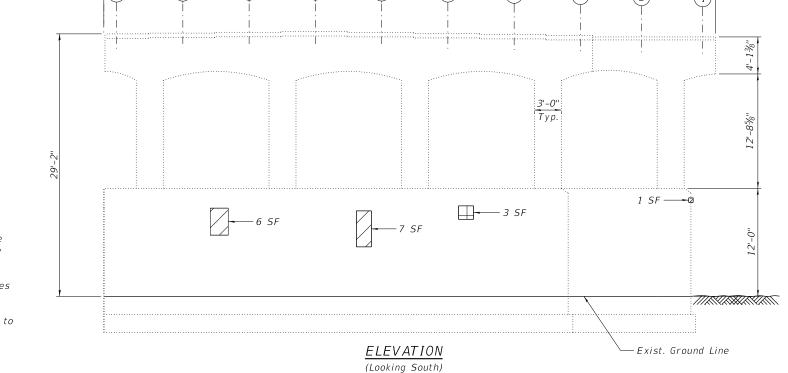


ITEM	UNIT	QUANTITY
Concrete Sealer	Sq Ft	3,648
Epoxy Crack Injection	Foot	2
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	109
Structural Repair Of Concrete (Depth Greater Than 5 Inches)	Sq Ft	3
Temporary Shoring And Cribbing	Each	1



NOTES:

- 1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.
- Concrete Sealer is to be applied to the exposed surfaces of the pier.
- Temporary shoring and cribbing shall be installed prior to the start of the structural repair of concrete and shall be removed after completing the structural repair of concrete.



LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Structural Repair of Concrete (Depth Greater than 5 inches)

Epoxy Crack Injection (Width > 0.06")

SF Square Foot

Linear Feet

HBM
ENGINEERING GROUP, LLC

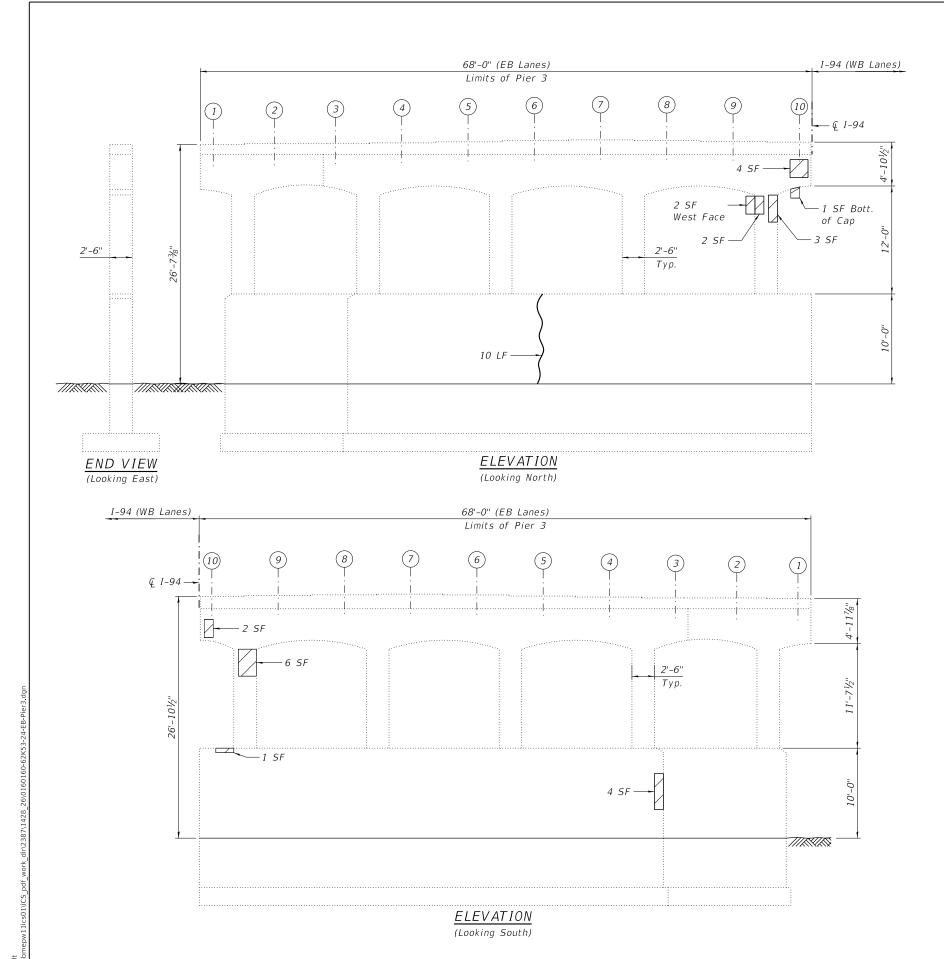
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

PIER 2 REPAIRS STRUCTURE NO. 016-0160 (EB) SHEET S04-21 OF S04-26 SHEETS

SECTION (42-B-11-1) BR, BJR 24 COOK 761 579 CONTRACT NO. 62W87

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ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	10
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	25

NOTE:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.





Structural Repair of Concrete (Depth Equal to or Less than 5 inches)



Epoxy Crack Injection (Width > 0.06")

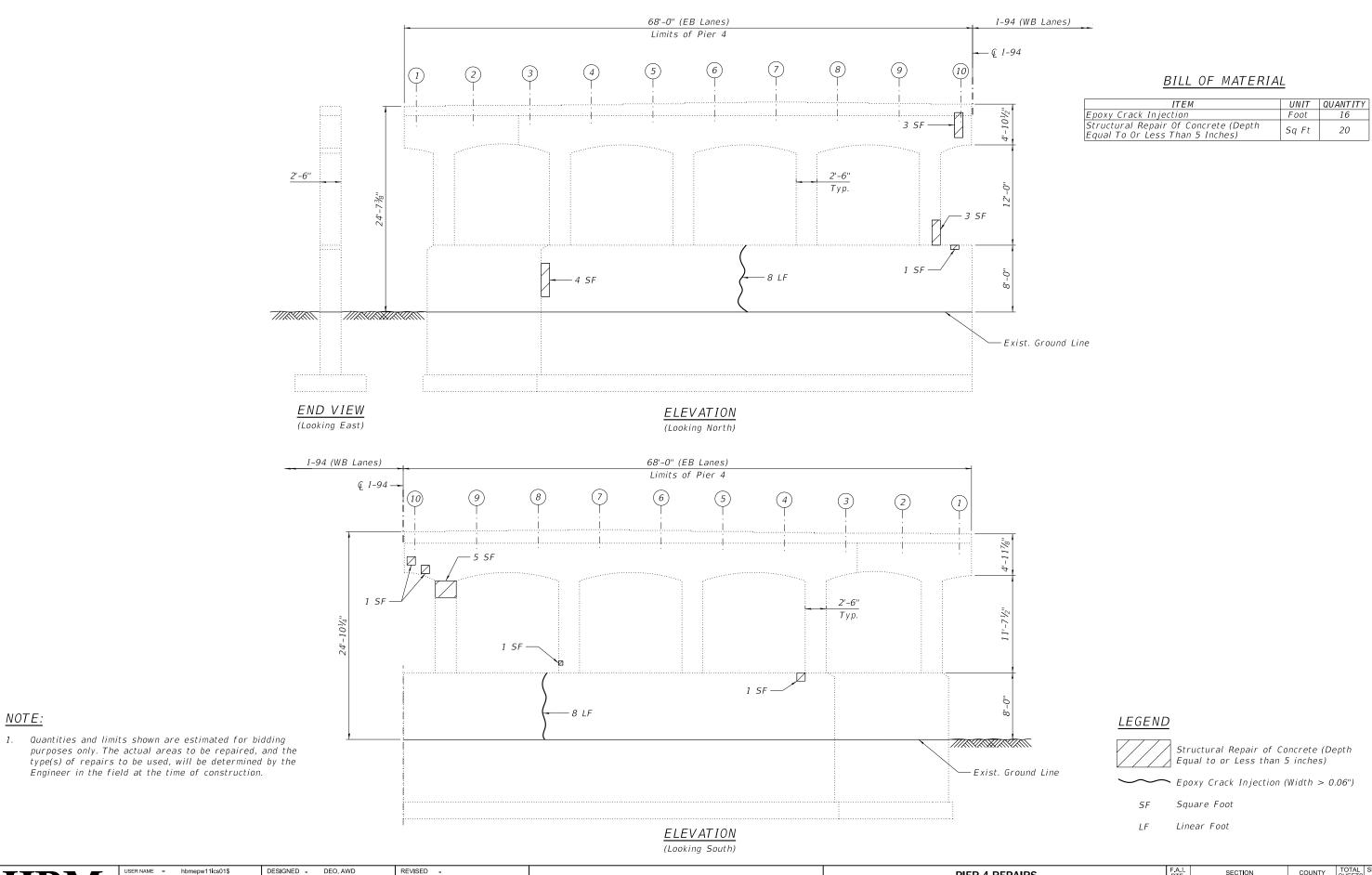
Square Foot

Linear Foot

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F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEE'
94	(42-B-11-1) BR, BJR 24	соок	761	580
		CONTRACT	NO. 6	52W87
	ILLINOIS FED. A	ID PROJECT		



NOTE:

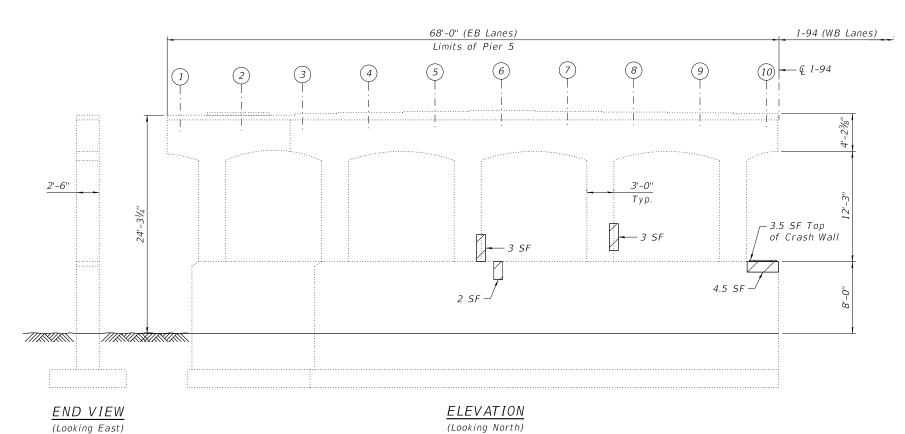
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

PIER 4 REPAIRS STRUCTURE NO. 016-0160 (EB) SHEET S04-23 OF S04-26 SHEETS

SECTION (42-B-11-1) BR, BJR 24 COOK 761 581 CONTRACT NO. 62W87



68'-0" (EB Lanes) Limits of Pier 5 € I-94 — (8) 6 3'-0" Тур. 2 SF East Face 8 LF -ELEVATION

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Epoxy Crack Injection	Foot	8
Structural Repair Of Concrete (Depth Equal To Or Less Than 5 Inches)	Sq Ft	27

NOTE:

1. Quantities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.

LEGEND



Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Epoxy Crack Injection (Width > 0.06")

SF

Square Foot

LF Linear Foot

HBM
ENGINEERING GROUP, LLC

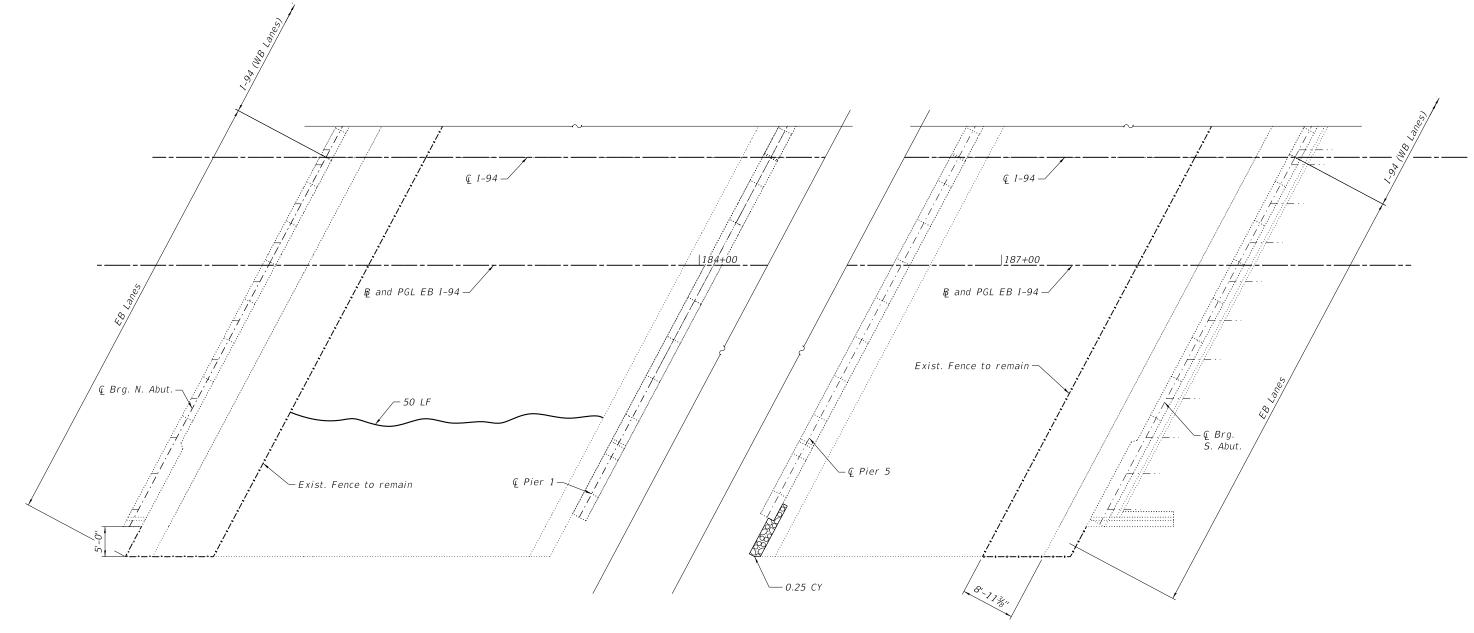
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(Looking South)

F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1) BR, BJR 24	соок	761	582
		CONTRACT	NO.	62W8
	ILLINOIS FED. A	ID PROJECT		

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ITEM	UNIT	QUANTITY
Porous Granular Embankment	Cu Yd	1
Slope Wall Crack Sealing	Foot	50



SLOPE WALL - PLAN

NOTE:

1. Quanities and limits shown are estimated for bidding purposes only. The actual areas to be repaired, and the type(s) of repairs to be used, will be determined by the Engineer in the field at the time of construction.



Porous Granular Embankment

Slope Wall Crack Sealing

LF Linear Foot

CYCubic Yard

TEGIDATE		12/0/2027	DAIL	-	12/3/2027	INLVIOLD	-
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SLOPE WALL REPAIRS			
STRUCTURE NO. 016-0160 (EB)			
OUEET COA 35 OF COA 36 OUEETO			

A.I. ΓΕ.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
94	(42-B-11-1) BR, BJR 24	соок	761	583
		CONTRACT	NO.	52W87
		•		

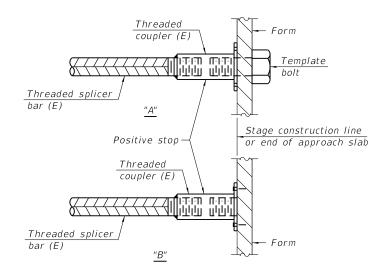
STANDARD BAR SPLICER ASSEMBLY PLAN

Only bar splicer assemblies as presented on the approved QPL list may be used.

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

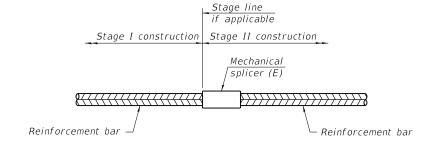
* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

Location	Bar Size	No. assemblies required	Minimum Iap length
N. Abutment Jt.	#5	9	3'-0"
N. Abutillelit Jt.	#6	6	3'-7"
Pier 2 Jt.	#5	18	3'-0"
S. Abutment Jt.	#5	9	3'-0"
3. Abutillelli Jt.	#6	6	3'-7"



INSTALLATION AND SETTING METHODS

"A": Set bar splicer assembly by means of a template bolt.
"B": Set bar splicer assembly by nailing to wood forms or cementing to steel forms.
(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications.

See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

5-15-2023



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LOADING HS 20-44 The bridge was constructed in 1948 under Section 0707.1-HB. It was widened in 1990 under Section 42 (VB 10, 12) and 0707.1 HB (B-Y-I-86). In 1992, the bridge was reconstructed under Section 42 No future wearing surface is allowed. (VB 10, 12, HB 14, B-11) and 0707.1 HB (R-3)90. This structure carries eight 12' lanes (four eastbound & four westbound) of I-94 over Dolton Avenue. The outer shoulder is 6' wide, and the inner shoulder is 4'-4½". The bridge has three spans (49-9½", 63'-10½", 49'-9½") and a skew of 12°-03'-45". The abutments are pile-supported stub abutments. The bridge has reinforced concrete DESIGN SPECIFICATION column piers on pile foundations with an extension at each end. 2002 AASHTO Standard Specifications for Highway Bridges, 17th Edition Traffic is to be maintained utilizing stage construction. 168'-11%" Bk. to Bk. Abutments EXISTING DESIGN STRESSES No Salvage. N. Approach S. Approach 49'-91/8" 49'-91/8" (1992 RECONSTRUCTION) 63'-101/8" Span 1 Span 2 Span 3 $f'c = 3500 \ psi \ (Superstructure)$ 2'-4" 2'-4" fy = 60,000 psi (Reinforcement 77'-2⁷/8" Superstructure) Reconstruct Exp. Jt. Limits of Protective Shield fs = 24,000 psi (Structural)5'-5" 1'-3" Reconstruct Exp. Jt. Sidewalk Sidewalk — ⊊ Pier 1 © Pier 2-© Brg. S. Abut. — PROPOSED DESIGN STRESSES f'c = 4,000 psi (Superstructure)fy = 60,000 psi (Reinforcement)G Brg. N. Abut. Exist. W30 Beams – Exist. Fence to remain — Ç Dolton Ave. Perform Structural Repair Exist of Concrete and Epoxy Exist. Fence Perform Structural Repair Slope Crack Injection at to remain of Concrete at North Abut. Wall South Abut Perform Structural Repair of Concrete and Epoxy Crack Exist. Ground line Injection at Pier 1 Concrete Piles Concrete Piles -Perform Slope Wall Repairs (typ. at Abutments) (typ. at Piers) 12'-0" 12'-0" 12'-0" 12'-0" JUNSHAN LIU (typ. at both Slope Walls) Lane - 4'-0" 081-008224 4'-0'' — 4'-0" ELEVATION -Perform Structural Repair of Concrete and Epoxy Crack (Looking East) Injection at Pier 2 OF ILLIN -Perform ⅔" Concrete Bridge 6" Ø Floor Drain 21'-5" 15'-0" Deck Scarification and Apply 60'-0" Spacing typ. ¾" Bridge Deck Thin Polymer Exp: 11/30/2026 Overlay Date: 12/05/2024 17'-0⁷/₈'' 17'-07/8" V.I.F. V.I.F. - Reconstruct <u>12°3'45"</u> Skew, typ. Exp. Jt. -Exist. fenc to remain - Perform Slope Wall Repairs (typ. at both Slope Walls) S. Slope Wall -Range 14E, 3rd P.M. Indiana Harboi – Exist. fence to remain Reconstruct Exp. Jt. ·Bk. of N. Abut - € Dolton Ave - N. Slope Wall Sta. 477+96.13 î Pier 1 ← Structure -Bk. of S.Abut & Brg. S.Abut & Brg. N. Abut Stoney Island Sta. 478+48.24 Sta. 478+80.19 Sta: 479+64.25 Sta. 479+61.90 Sta. 477+98.48 Location <u>|479+00</u> LOCATION SKETCH Stations B and P.G.L. @ Pier 2 ₩B I-94 Sta. 479+12.14 Increase Drainage Scupper (Special), typ. € I-94 Preformed Joint Seal, 21/2" GENERAL PLAN AND ELEVATION WB I-94 OVER DOLTON AVE F.A.I. ROUTE 94 49'-91/8" 63'-107/8" 49'-91/8" 2'-4" 2'-4" SECTION 2019 180-RS & T Span 1 Span 2 Span 3 S. Approach COOK COUNTY N. Approach 168'-11/8" Bk. to Bk. Abutments STATION: 478+80.19 S.N. 016-0161 1. All stations are to I-94 WB P.G.L. and taken from existing plans. PLANDESIGNED - SUR REVISED -COUNTY **STATE OF ILLINOIS** DRAWN - ME REVISED -Accurate 94 (42-B-11-1) BR, BJR 24 COOK 761 585 STRUCTURE NO. 016-0161 (WB) 20:0 ':" / in. CHECKED -REVISED -**DEPARTMENT OF TRANSPORTATION** CONTRACT NO. 62W8 SHEET S05-01 OF S05-27 SHEETS PLOT DATE = 12/9/2024 REVISED -DATE - 12/6/2024 12/9/2024 2:52:14 PM

GENERAL NOTES

- 1. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- 2. During repair operations the contractor shall locate and protect any utilities or facilities including but not limited to the fiber optic and/or electrical conduits, conduits under the bridge deck, under lighting, traffic signals or signs attached to the structure. This work is to be performed to the satisfaction of the engineer and will not be paid for separately, but shall be included with the contract. It will be the contractor's responsibility to restore and replace any damage utilities or facilities to the satisfaction of the engineer and the department.
- 3. All exposed concrete edges shall have a $\frac{3}{4}$ " x 45" chamfer except where shown otherwise.
- 4. Protective Coat shall be applied to the top and inside face of parapets.
- 5. Repairs shown are based upon inspection carried out at the time of plan preparation are for bidding purposes only. Actual area to be repaired and the type(s) of repairs to be used shall be determined by the engineer in the field at the time of construction.
- 6. The contractor shall take the necessary precautions for the protection of passing vehicles, bicycles, and pedestrians from falling objects and/or materials until completion of work.
- 7. Where underpass lighting is present on the structure, the Contractor shall adjust the Protective Shielding to ride above the existing lighting fixtures in order to maintain the existing level of lighting on the roadway underneath. Details shall be approved by the Engineer before installation.
- Any adjustment done to the Protective Shield System must not change the load carrying capacity (or containment specifications) as indicated in the Standard Specifications, Cost of adjusting shielding is included in the cost of Protective Shield.
- 9. Concrete Sealer shall be applied to the designated areas of the abutments.
- 10. Prior to the application of the Concrete Sealer, the Contractor shall clean all existing debris from the abutment seats. The method of debris removal shall not damage the existing concrete and shall be approved by the Engineer. See Special provision for Debris Removal.
- 11. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose detrimental foreign material shall be removed from the surfaces in contact with concrete (SSPCSP3 standards). Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be paid for according to Article 109.04 of the Standard Specifications. As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer. Any cracks that cannot be removed by grinding ¼ inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.
- 12. Cost of removal and re-installation of all members necessary to complete the work as detailed on the plans and as specified in the Special Provisions shall be included with Beam Straightening.
- 13. Existing reinforcement extended into the removal area shall be cleaned, straightened and incorporated into the new construction cost is included with concrete removal. Any reinforcement bars that are damaged during concrete removal operations shall be replaced using an approved bar splicer or anchorage system at the Contractor's expense.
- Cleaning and field painting of structural steel shall be done under a separate painting contract.
- 15. The Contractor is responsible to remove, support and reinstall all existing electrical conduits interfering with the work. See special provision "Protection and Maintenance of Existing Underpass Luminaires".
- 16. Reinforcement bars designated (E) shall be epoxy coated.

- 17. The Contractor is responsible to protect the existing conduit and junction box embedded in the parapet during removal and construction. Any damage to the existing conduit and junction box shall be repaired by the Contractor at no additional cost to the Department.
- 18. No field welding is permitted except as specified in the contract documents.
- 19. The Engineer shall show actual locations and size of deck repairs on As-built Plans.
- 20. Bars indicated thus, 3x2-#5, indicates 3 lines of #5 bars with 2 lengths of bar per line.
- 21. Joint openings shall be adjusted according to Article 520.04 of the Standard Specification when the deck is poured at an ambient temperature other than 50° F.
- 22. The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project.

INDEX OF SHEETS

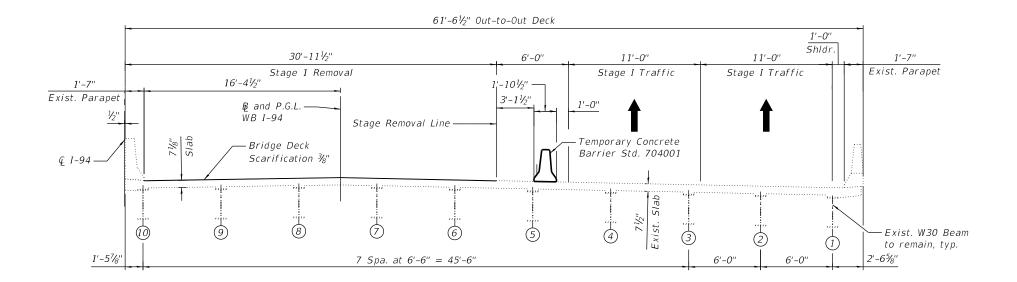
505-01	General Plan and Elevation
<i>S05-02</i>	General Notes, Index of Sheets & TBOM
S05-03	Stage Construction (Sheet 1 of 2)
S05-04	Stage Construction (Sheet 2 of 2)
<i>S05-05</i>	Temporary Concrete Barrier
505-06	Deck Repair Plan
S05-07	S. Abut. Joint Removal & Replacement (Sht. 1 of 3)
S05-08	S. Abut. Joint Removal & Replacement (Sht. 2 of 3)
<i>S05-09</i>	S. Abut. Joint Removal & Replacement (Sht. 3 of 3)
S05-10	N. Abut. Joint Removal & Replacement (Sht. 1 of 3)
S05-11	N. Abut. Joint Removal & Replacement (Sht. 2 of 3)
S05-12	N. Abut. Joint Removal & Replacement (Sht. 3 of 3)
S05-13	Preformed Joint Strip Seal
S05-14	Framing Plan
S05-15	Beam Straightening Details
S05-16	South Abutment Repairs
S05-17	North Abutment Repairs
S05-18	Pier 1 Repairs
505-19	Pier 2 Repairs
<i>S05-20</i>	Slope Wall Repairs
S05-21	Bar Splicer Assembly & Mechanical Splicer Details
<i>S05-22</i>	Existing Plans (Sheet 1 of 6)
S05-23	Existing Plans (Sheet 2 of 6)
<i>S05-24</i>	Existing Plans (Sheet 3 of 6)
S05-25	Existing Plans (Sheet 4 of 6)
<i>S05-26</i>	Existing Plans (Sheet 5 of 6)
<i>S05-27</i>	Existing Plans (Sheet 6 of 6)

SCOPE OF WORK

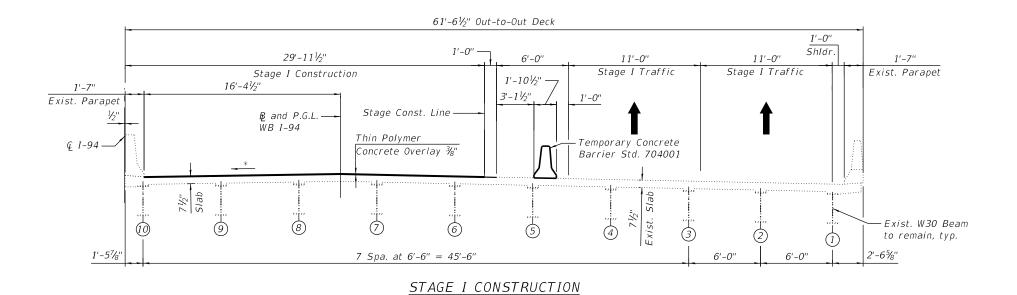
- 1. Provide Protective Shield within limits indicated on the plans.
- 2. Scarify 3/8" from the bridge deck slab.
- Remove and Reconstruct Expansion joints at North and South abutments and install new Preformed Joint Strip Seals.
- 4. Apply ¾" Thin Polymer Overlay on Bridge Deck.
- 5. Refer to Roadway plans for Approach Pavement Rehabilitation.
- Apply Protective Coat to the top of reconstructed transverse joint areas and top and inside faces of parapets.
- Perform structural concrete repairs to abutments and piers, as noted on plans.
- 8. Perform structural steel repairs to beams, as noted on plans.
- 9. Perform Slope Wall repairs.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment	Cu Yd	-	10	10
Concrete Removal	Cu Yd	18.2	-	18.2
Slope Wall Removal	Sq Yd	-	29	29
Protective Shield	Sq Yd	529	-	529
Concrete Superstructure	Cu Yd	18.2	-	18.2
Protective Coat	Sq Yd	203	-	203
Reinforcement Bars, Epoxy Coated	Pound	3,150	-	3,150
Bar Splicers	Each	32	-	32
Slope Wall 4 Inch	Sq Yd	-	29	29
Preformed Joint Seal, 2½"	Foot	167	-	167
Preformed Joint Strip Seal	Foot	120	-	120
Concrete Sealer	Sq Ft	-	942	942
Epoxy Crack Injection	Foot	-	44	44
Slope Wall Crack Sealing	Foot	-	139	139
Beam Straightening	L Sum	0.33	-	0.33
Bridge Deck Scarification ¾"	Sq Yd	1,042	-	1,042
Bridge Deck Thin Polymer Overlay %"	Sq Yd	1,042	-	1,042
Structural Repair of Concrete	Ca Ft		79	79
(Depth Equal to or less than 5")	Sq Ft	_	/9	/9
Structural Repair of Concrete	Ca E+		9	9
(Depth Greater Than 5")	Sq Ft	_	9	9



STAGE I REMOVAL (Looking North)



STAGE I CONSTRUCTION

STAGE I REMOVAL

1. Install temporary concrete barrier as shown to locate

traffic on the west side of the existing structure.

3. Remove portions of bridge concrete deck/approach slab

adjacent to expansion joints at the North and South

2. Perform ⅔" bridge deck scarification.

- 1. Perform bridge deck slab repairs.
- 2. Reconstruct transverse expansion joints and install new preformed joint strip steals within the limits of Stage I Construction.
- 3. Perform Structural repair of concrete and epoxy crack injection for the abutments and piers.
- 4. Apply ¾" Bridge Deck Thin polymer
- 5. Refer to Roadway plans for Approach Pavement Rehabilitation.
- 6. Apply protective coat to top and inside faces of West parapet, and reconstructed transverse expansion joints.
- 7. Perform Slope Wall repairs as shown on the plans.
- 8. Replace existing longitudinal preformed joint seal between NB West parapet and SB East parapet.

*Match existing cross slopes

Accurate

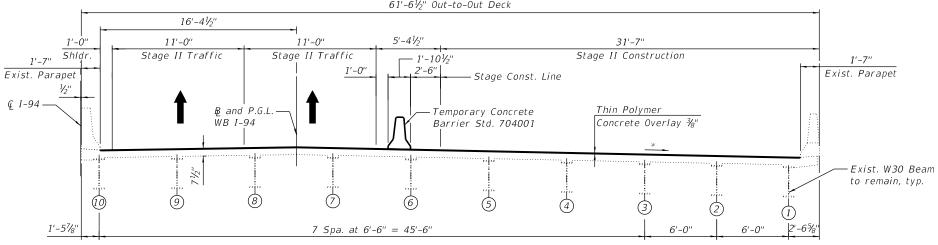
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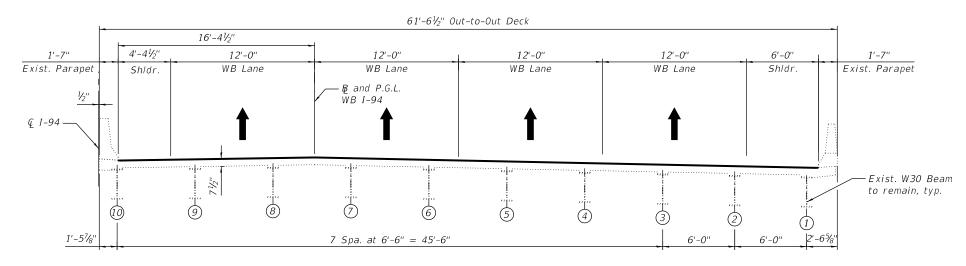
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** STAGE CONSTRUCTION (SHEET 1 OF 2) STRUCTURE NO. 016-0161 (WB) SHEET S05-03 OF S05-27 SHEETS

94 (42-B-11-1) BR, BJR 24 COOK 761 587 CONTRACT NO. 62W87

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STAGE II REMOVAL (Looking North)





FINAL CROSS SECTION

*Match existing cross slopes

STAGE II REMOVAL

South Abutments.

STAGE II CONSTRUCTION

Stage II Construction.

expansion joint areas.

Reconstruct expansion joints and install new

preformed joint strip seal within the limits of

2. Perform structural repair of concrete and epoxy crack injection for the abutments and piers.

4. Apply protective coat to top and inside faces of East parapet, and reconstructed abutment

3. Apply 3%" bridge deck thin polymer overlay.

structure.

1. Install temporary concrete barrier as shown to

2. Perform ¾" bridge deck scarification.

locate traffic on the East side of the existing

3. Remove portions of bridge concrete deck/approach slab adjacent to expansion joints at the North and

Accurate

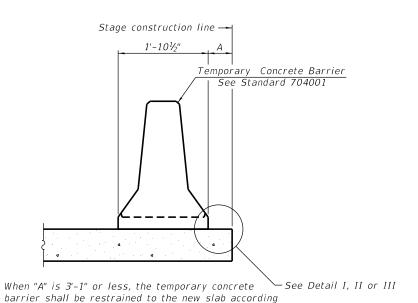
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION STAGE CONSTRUCTION (SHEET 2 OF 2) STRUCTURE NO. 016-0161 (WB) SHEET S05-04 OF S05-27 SHEETS

94 (42-B-11-1) BR, BJR 24 COOK 761 588 CONTRACT NO. 62W87

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61'-6¹/₂" Out-to-Out Deck Exist. Parapet STAGE II CONSTRUCTION (Looking North)



when "A" is greater than 3'-1".

to Detail I, II or III. No restraint is required

Stage removal line ← Stage removal line 1'-101/5" 1'-101/5" Temporary Concrete Barrier See Standard 704001 min. min. Drill 3-11/4" Ø Holes in existing slab for 1" Ø restraining pins. Traffic side only. Cost of restraining pins are included with Temporary Concrete Barrier. No restraint

* When hot-mix asphalt wearing surface is present, embedment shall be 3" plus the wearing surface depth.

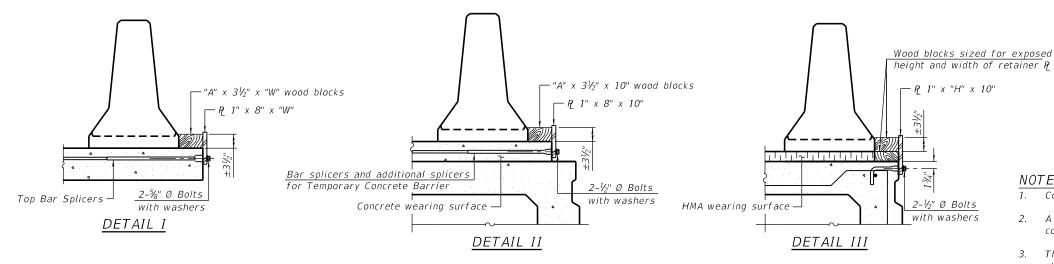
EXISTING DECK BEAM

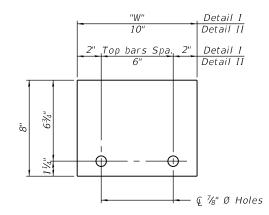
NEW SLAB OR NEW DECK BEAM

SECTIONS THRU SLAB OR DECK BEAM

is required when "A" is greater than 3'-1".

EXISTING SLAB





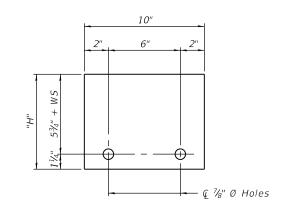
STEEL RETAINER P 1" x 8" x "W"

(Detail I and II)

RAILING CRITERIA

NCHRP 350 Test Level Railing Weight (plf)

R - 2705-15-2023



STEEL RETAINER P 1" x "H" x 10" (Detail III)

Cost of retainer assembly is included with Temporary Concrete Barrier.

1x8 UNC

RESTRAINING PIN

US Std. 11/16" I.D. x 21/2" O.D. x approx. 8 gauge thick washer

2. A retainer assembly shall be located at the approximate © of each temporary concrete barrier.

BAR SPLICER FOR #4 BAR - DETAIL III

- 3. The retainer plate shall not be removed until the concrete on the adjacent stage is ready to be poured. For Detail III applications the retainer plate shall not be removed until just prior to placing the adjacent beam.
- 4. When the 'A' dimension is less than $1\frac{1}{2}$ ", the wood block shall be omitted and the barrier shall be placed in direct contact with the steel retainer plate. For deck beam applications the minimum required 'A' distance is 6" to accommodate the shear key clamping device.
- Detail I Installation for a new bridge deck or bridge slab.
- Detail II Installation for a new deck beam with an initial concrete wearing surface. Additional bar splicers shall be provided at 6'-0" centers and paired with the bar splicers of the concrete wearing surface reinforcement to accommodate the installation of the retainer assemblies. The cost of the additional bar splicers is included with the concrete wearing surface.
- Detail III Installation for a new deck beam with no initial wearing surface or with an initial hot-mix asphalt (HMA) wearing surface present. The deck beam directly beneath the temporary concrete barrier shall be fabricated with bar splicer inserts in the side of the beam, as detailed, to accommodate the installation of the retainer assemblies. A pair of bar splicers, 6" apart, shall be placed at 6'-0" centers along the length of the beam. The cost of the bar splicers is included with the deck beam.

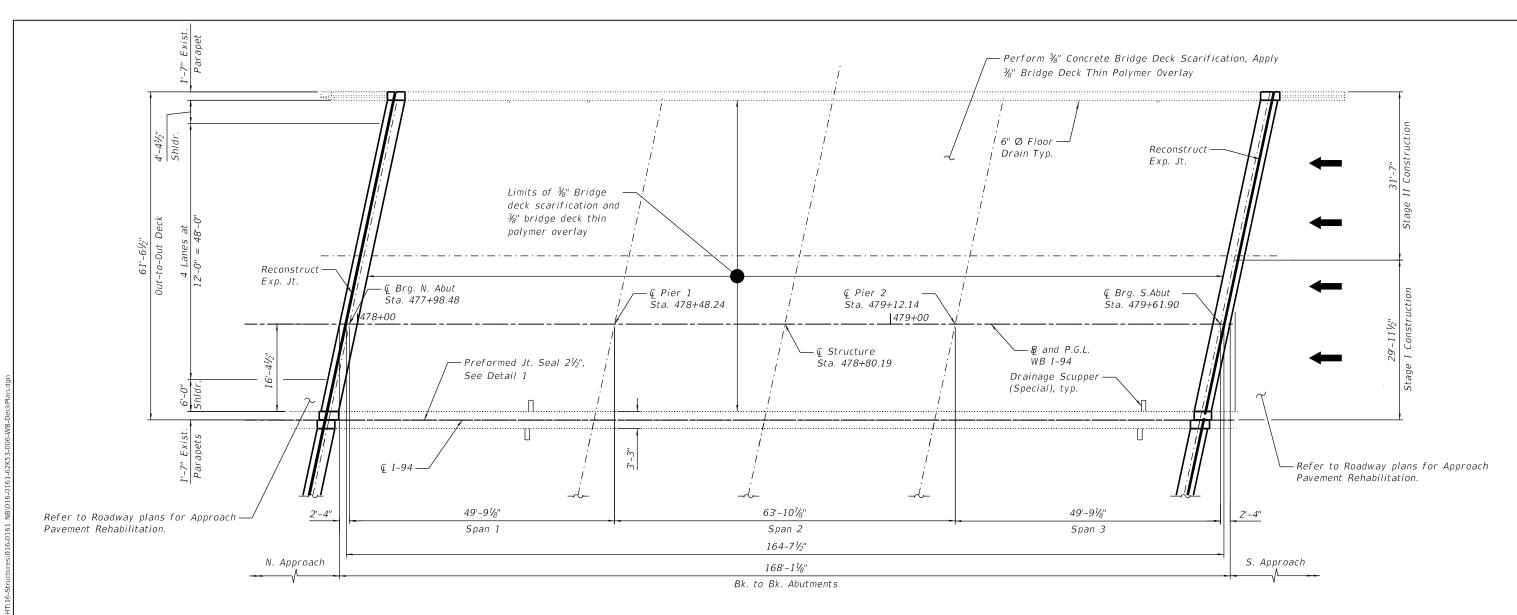
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** TEMPORARY CONCRETE BARRIER STRUCTURE NO. 016-0161 (WB) SHEET S05-05 OF S05-27 SHEETS

94 (42-B-11-1) BR, BJR 24 COOK 761 589 CONTRACT NO. 62W87

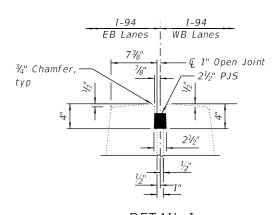
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DECK PLAN

NOTES:

- No deck slab repairs required, unless otherwise directed by Engineer.
- 2. For bridge deck final cross section, See Sheet S05-04.
- 3. For North and South Joint Removal and Replacement, See Sheets S05-07 through S05-12.
- 4. Protective Coat shall be applied to top and inside face of parapets, and the joint areas.
- 5. Any reinforcement bars that are damaged during concrete removal operations shall be replaced using an approved bar splicer or anchorage system. Cost incidental to Concrete Removal.
- 6. The Contractor shall exercise extreme caution during Concrete Removal to avoid damaging the steel beams and diaphragms to remain. Any damage to the existing steel beams and/or diaphragms to remain caused by the Contractor in the performance of his/her work shall be repaired by the Contractor, to the satisfaction of the Engineer, at no cost to the Department.



<u>DETAIL 1</u> (Reinforcement not shown for clarity)

BILL OF MATERIAL

UNIT	QUANTITY
Sq Yd	149
Foot	167
Sq Yd	1042
Sq Yd	1042
	Sq Yd Foot Sq Yd

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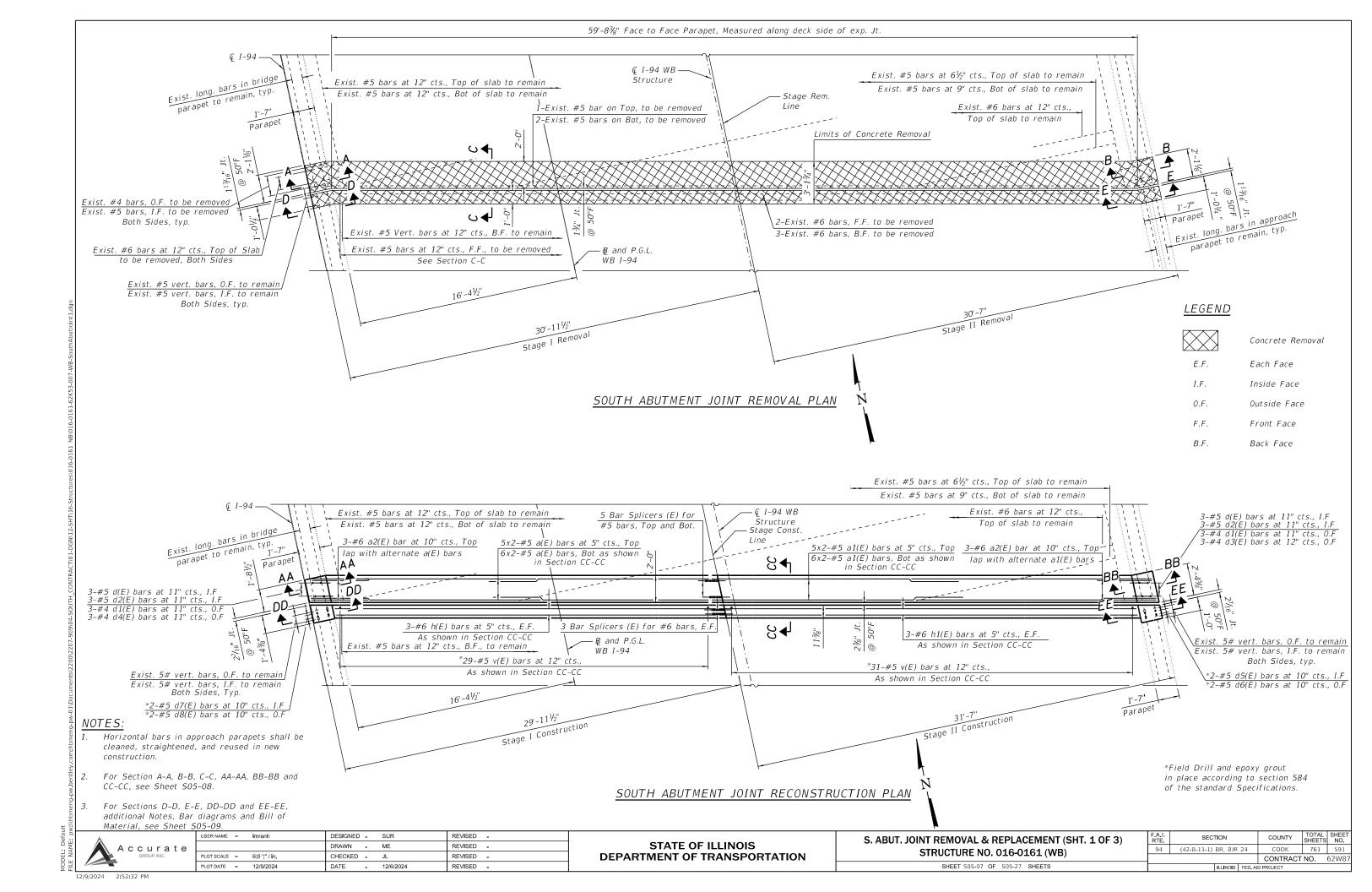
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DEPARTMENT OF TRANSPORTATION

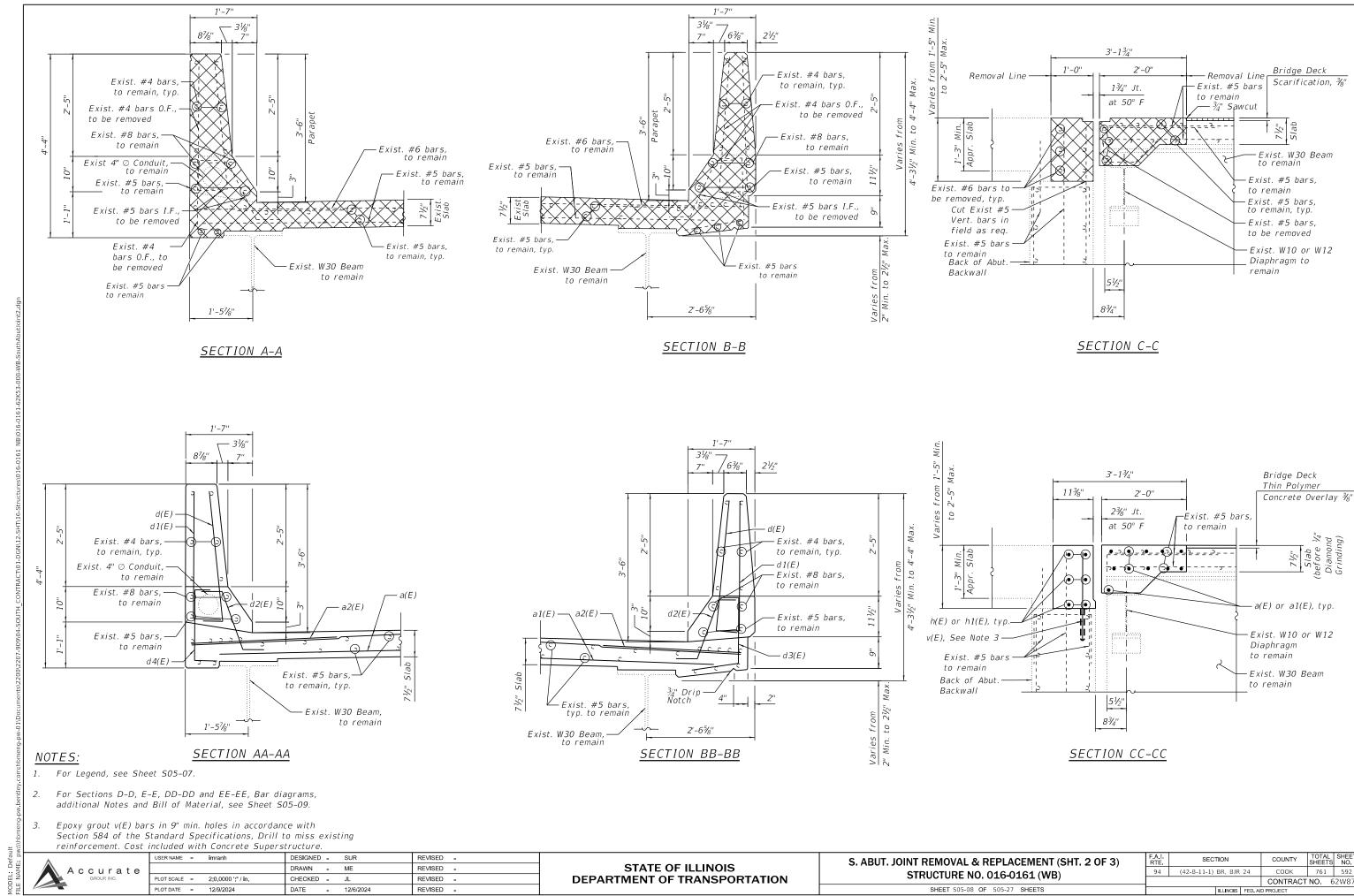
DECK REPAIR PLAN
STRUCTURE NO. 016-0161 (WB)

SHEET S05-06 OF S05-27 SHEETS

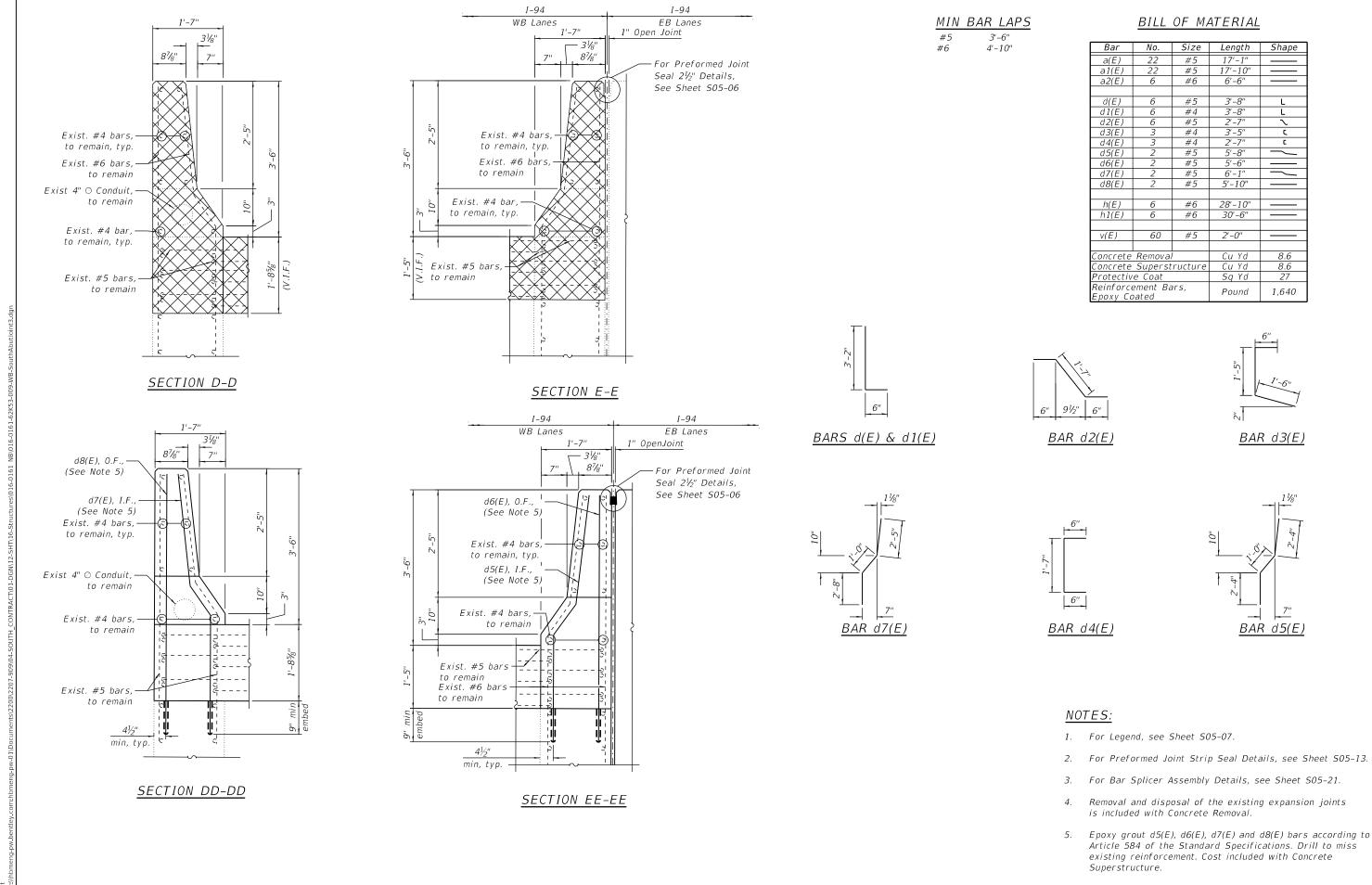
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DEPARTMENT OF TRANSPORTATION

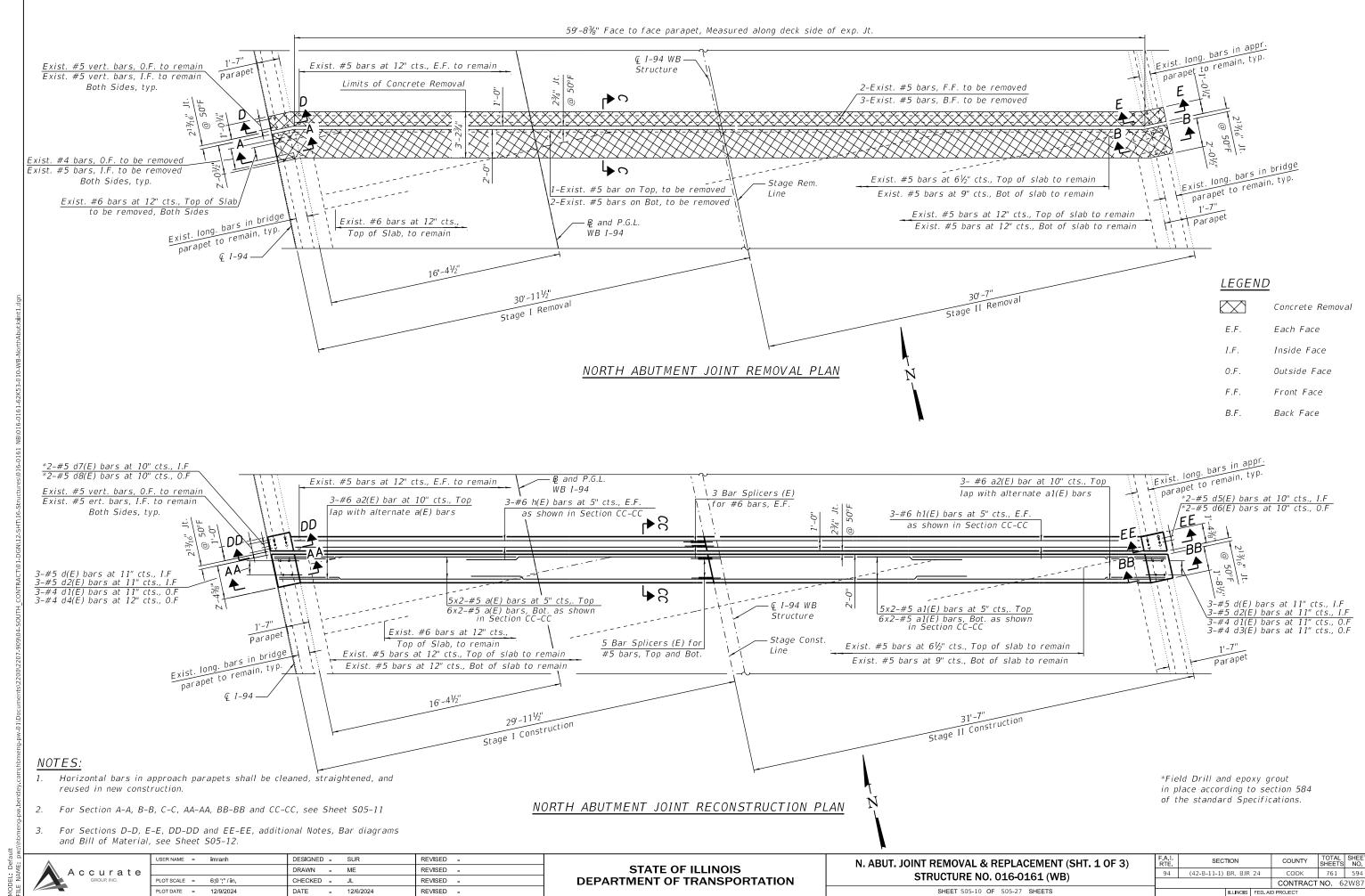
S. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 3 OF 3)
STRUCTURE NO. 016-0161 (WB)

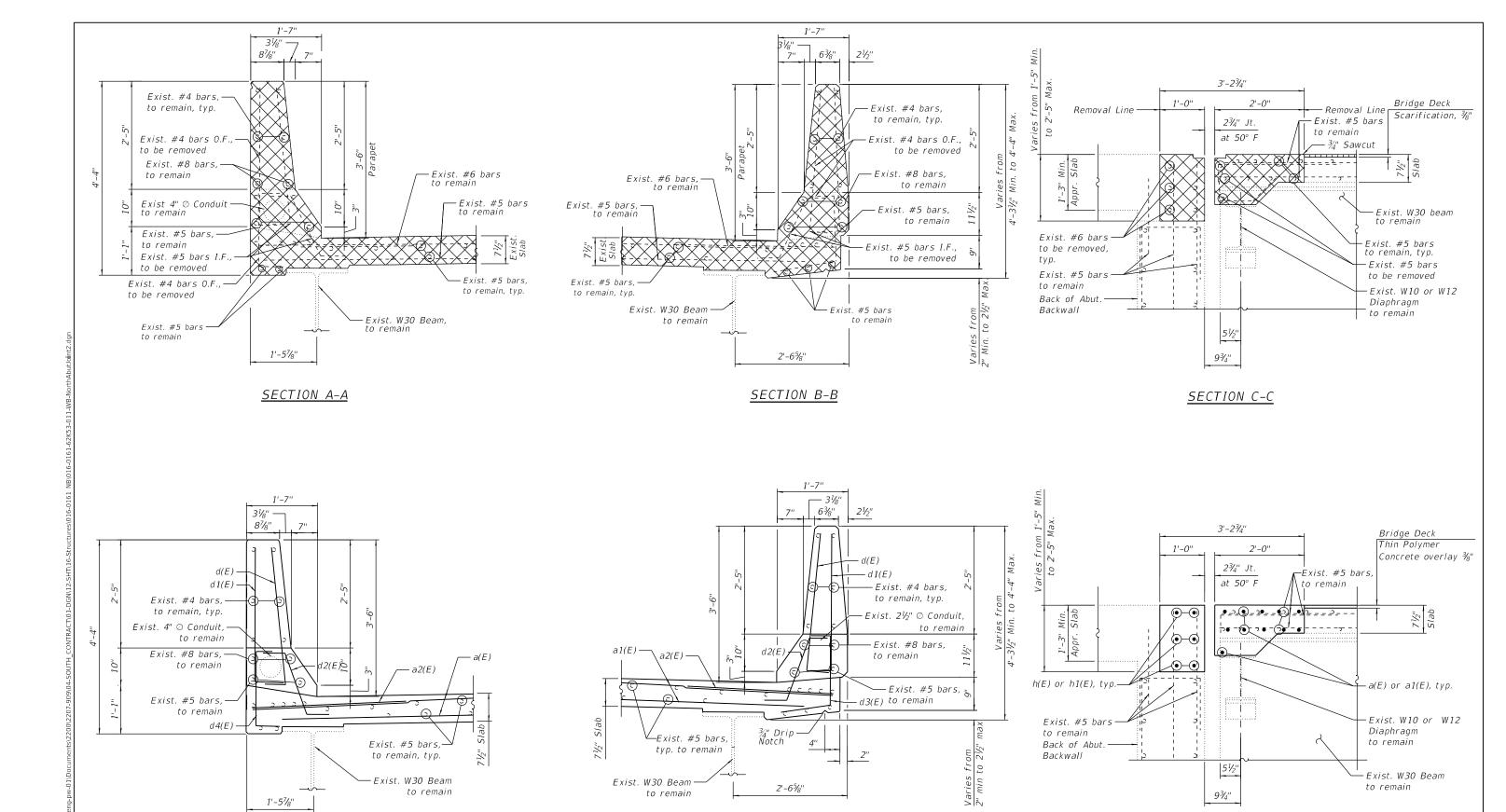
SHEET S05-09 OF S05-27 SHEETS

 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 94
 (42-B-11-1) BR, BJR 24
 COOK
 761
 593

 CONTRACT NO. 62W87





NOTES:

- 1. For Legend, see Sheet S05-10.
- 2. For Sections D-D, E-E, DD-DD and EE-EE, Bar diagrams, additional Notes and Bill of Material, see Sheet S05-12.

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SECTION AA-AA

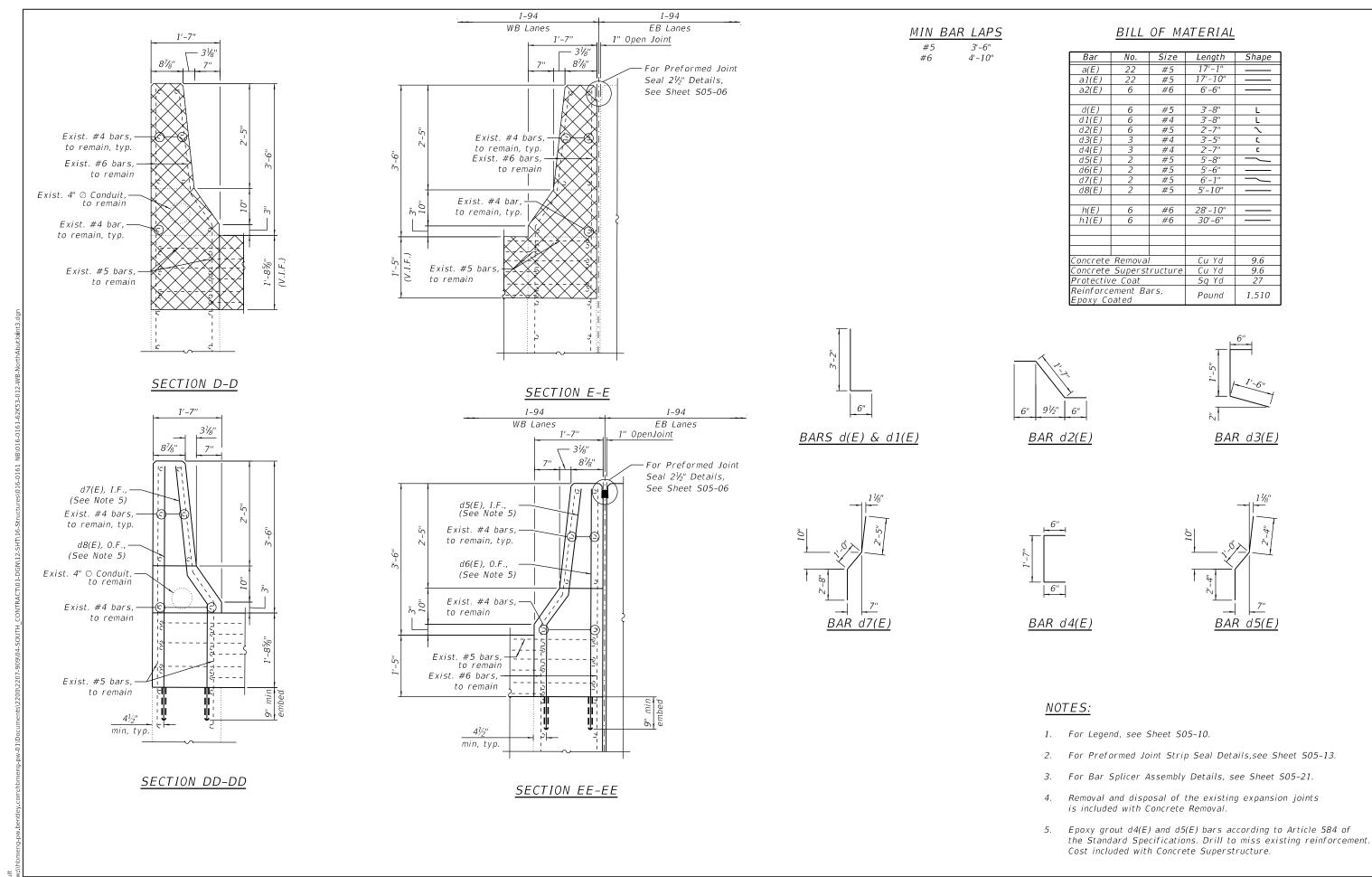
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

SECTION BB-BB

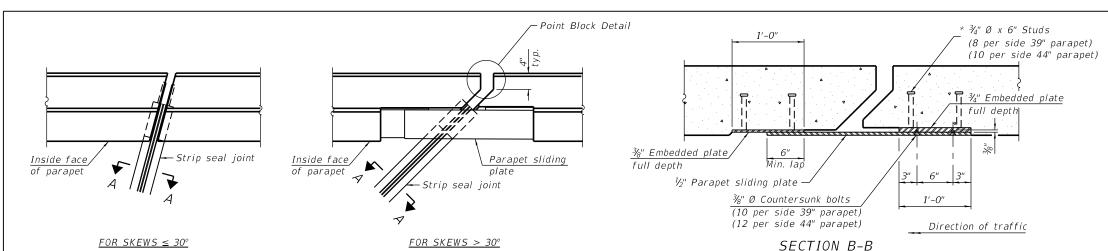
N. ABUT. JOINT REMOVAL & REPLACEMENT (SHT. 2 OF 3)
STRUCTURE NO. 016-0161 (WB)

SHEET S05-11 OF S05-27 SHEETS

SECTION CC-CC



A I RTE	SECT	ION		COUNTY	TOTAL SHEETS	SHEET NO.
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				CONTRACT	NO. 6	2W87
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PLAN AT PARAPET

Top of locking

-Top of deck

Concrete flush with back face of %" plate ¾" Plate 上一部 D. Concrete flush with back face of 3/4" plate

3¾" at 50° F (N. Abut.)

Notes:

The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

Cost of parapet sliding plates, embedded plates, and anchorage studs included with Preformed Joint Strip Seal.

39" constant slope barrier shown, 44" constant slope barrier similar as noted.

The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.

ELEVATION AT PARAPET

Parapet sliding

Inside Face

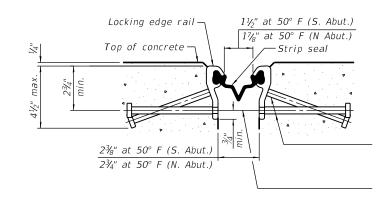
plate

2" Max.

Detail A

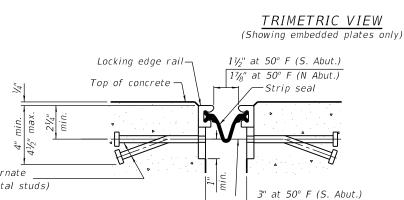
%" Ø x 6" Studs

(Skews > 30° shown. Skews ≤ 30° similar except as shown in plan view.)



SHOWING ROLLED RAIL JOINT

DETAIL A

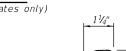


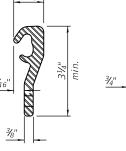
* $\frac{1}{8}$ " Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs)

 $\frac{3}{6}$ " ϕ threaded rods in $\frac{7}{16}$ " ϕ holes at ± 4 '-0" cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



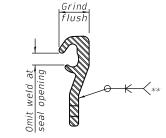


<u>ROLLED</u> (EXTRUDED) RAIL

LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.

WELDED RAIL



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	120



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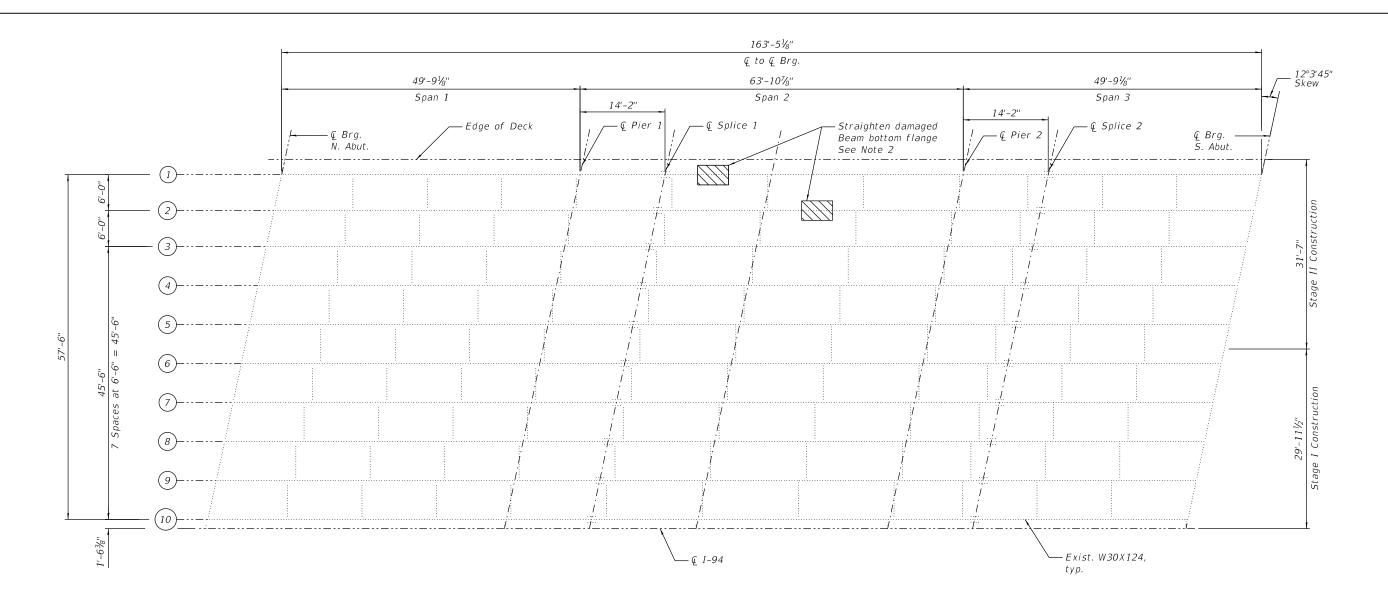
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

PREFORMED JOINT STRIP SEAL STRUCTURE NO. 016-0161 (WB) SHEET S05-13 OF S05-27 SHEETS

F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
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			CONTRACT	NO . 6	2W87
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SHOWING WELDED RAIL JOINT



FRAMING PLAN

BILL OF MATERIAL

ITEM	UNIT	QUANTITY
Beam Straightening	L Sum	0.33

<u>NOTES:</u>

- 1. All work is to be performed utilizing stage construction, See sheets S05-03 and S05-04 for details.
- 2. Gouges caused by the impact should be ground to eliminate sharp or sudden irregularities in the beam surface. Grinding should be done in such a way as to provide a smooth transition with a maximum slope of 3:1 between the damaged and undamaged surfaces. Cost included in Beam straightening.

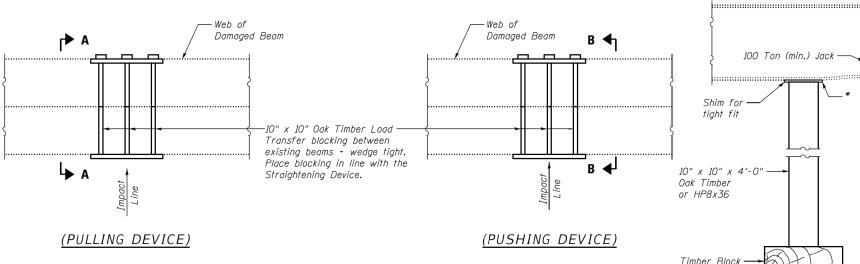




Straighten Damaged Beam Flange

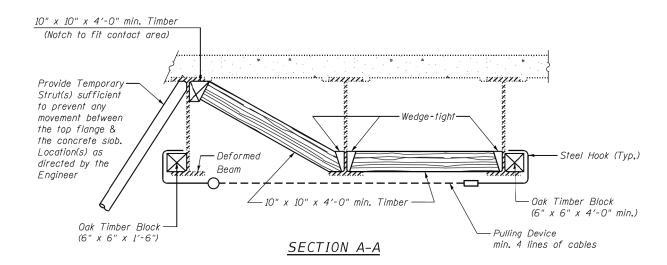
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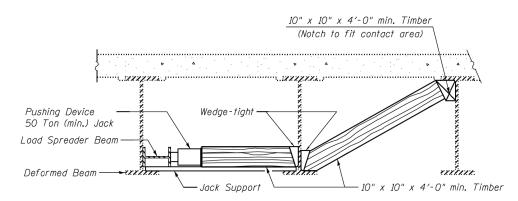
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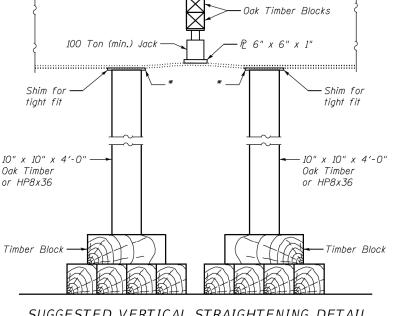
PARTIAL PLAN SUGGESTED BEAM STRAIGHTENING METHODS

NOTE: Straightening force shall be maintained on all load transfer blocking during beam straightening.





SECTION B-B



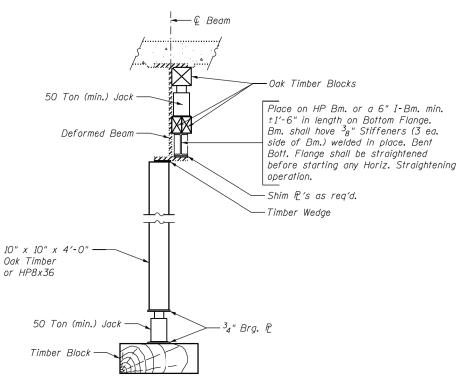
SUGGESTED VERTICAL STRAIGHTENING DETAIL

(To correct localized vertical flange deformations.)

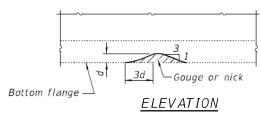
* Edge of plate shall line up with edge of deformation.

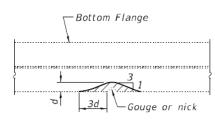
NOTE:

Braces and jack assembly shall be placed on same side of web. Bent bottom flange shall be straightened before starting any horizontal straightening



VERTICAL STRAIGHTENING DETAIL





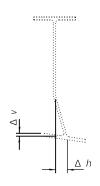
PLAN

GRINDING DETAILS

Grind existing nicks, gouges and shallow cracks in the damaged beams as detailed. Grinding shall be done parallel to the longitudinal axis of member. Ground surfaces shall be inspected for cracks using dye penetrant or magnetic particle testing prior to initiating any beam straightening operations. Any cracks that cannot be removed by grinding approximately $\frac{1}{4}$ " deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. Ground surfaces shall be spot cleaned and painted with an aluminum epoxy mastic primer followed by a finish coat to match the color of the existing beam. Cost of grinding, testing and spot painting is included with Beam Straightening.

Δ h: <u>Measure in Field</u>

Δ v: <u>Measure in Field</u>



EXISTING DEFORMATION TO BE STRAIGHTENED

(Looking North) (Approximate max. deflections) Deflected length of beam to be straightened is approximately 4'-0".

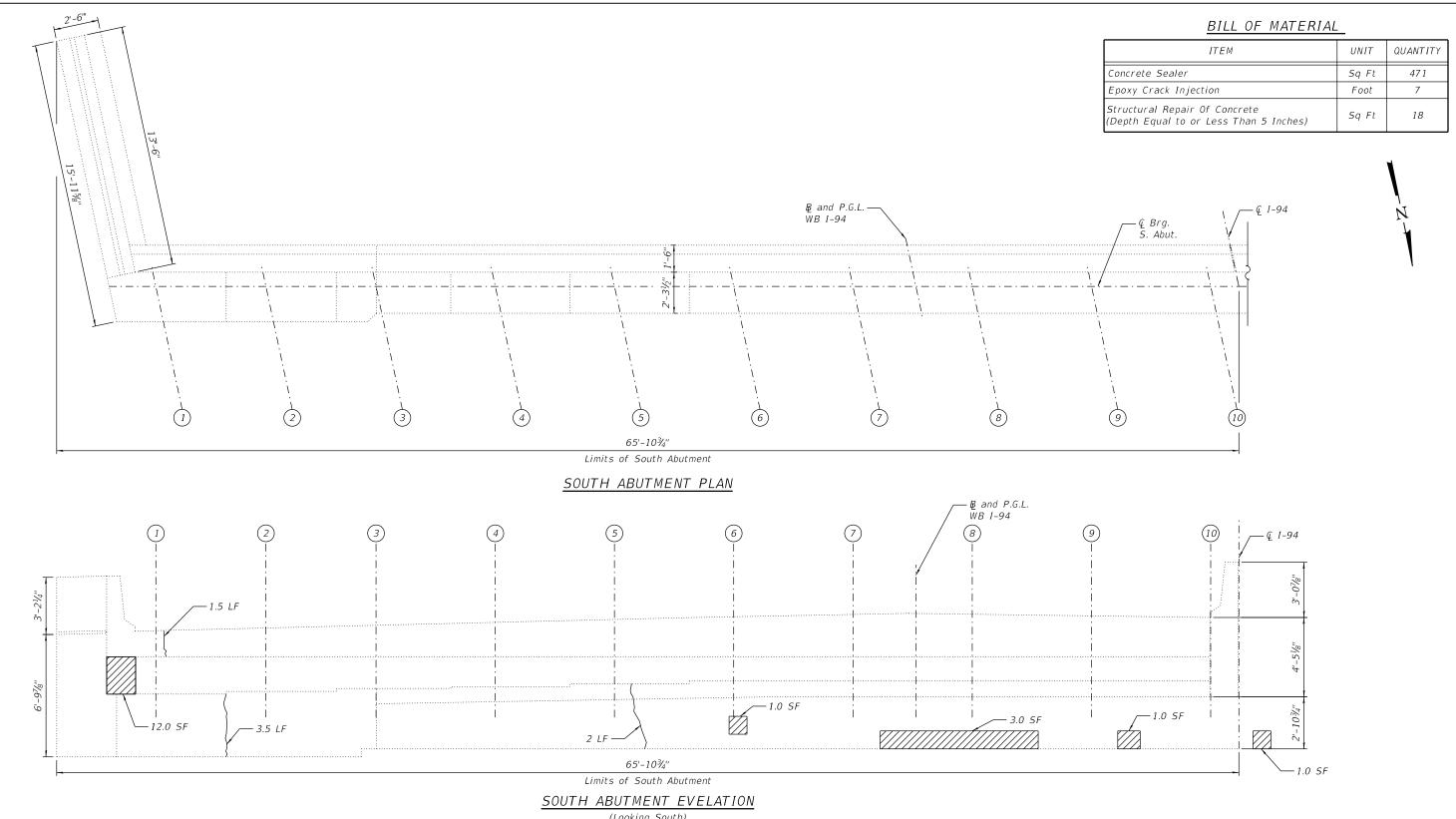
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

BEAM STRAIGHTENING DETAILS STRUCTURE NO. 016-0161 (WB) SHEET S05-15 OF S05-27 SHEETS

F.A.I. RTE	SECT	TION		COUNTY	TOTAL SHEETS	SHEE NO.
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			CONTRACT	NO. 62	W87	
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(Looking South)

NOTES:

- 1. Quantities and limit shown are estimated for bidding purpose only. The actual areas to be repaired and the type(s) of repairs to be used will be determined by the engineer in the field at the time of construction.
- For slope wall repairs, see sheet S05-20.
- Concrete Sealer is to be applied to the Abutment seats and the bottom 2 feet of the Abutment Backwall.

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LEGEND

Structural Repair of Concrete (Depth Equal to or Less than 5 inches)

Epoxy Crack Injection

LF

Linear Foot Square Foot

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SOUTH ABUTMENT REPAIRS STRUCTURE NO. 016-0161 (WB)

SHEET S05-16 OF S05-27 SHEETS

94 (42-B-11-1) BR, BJR 24 COOK 761 600 CONTRACT NO. 62W87

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