		BY	DATE
PLAN	SURVEYED		
	PLOTTED		
NOTE BOOK	ALIGNMENT CHECKED		
	RT. OF WAY CHECKED		
١	CADD FILE NAME		

						72400100	72400200	72400500 RELOCATE	72400600	
EXISTING						REMOVE SIGN PANEL	REMOVE SIGN PANEL	SIGN PANEL	RELOCATE SIGN PANEL	RELOCATE
SIGN	PANEL DESCRIPTION	SIGN SUPPORT	STATION	OFFSET	ACTION	ASSEMBLY -	ASSEMBLY -	ASSEMBLY -	ASSEMBLY -	SIGN PANEL
NUMBER						TYPE A	TYPE B	TYPE A	TYPE B	BY OTHERS
NORTH AUROR	A ROAD (STA 98+00 TO 108+00)					(EACH)	(EACH)	(EACH)	(EACH)	(EACH)
E1-1	DO NOT DRIVE ON SHOULDER	SQUARE TUBE	08+20 85	22 20' PT	SIGN PANEL ASSEMBLY TO BE REMOVED	1	(LACII)	(LACII)	(LACII)	(LACII)
E1-1	CITY OF AURORA	SQUARE TOBE	90+20.03	32.29 NI	31GN PANEL ASSEMBLI TO BE REMOVED	1				
E1-2b	PURPLE HEART	DUAL SQUARE TUBE	100413 20	51 58' 17	SIGN PANEL ASSEMBLY TO BE RELOCATED				1	
E1-26	POPULATION	DUAL SQUARE TOBE	100/13.29	31.30 27	STON TANEL ASSEMBLY TO BE RELOCATED				1	
E1-2C	V20 UTILITY	PLASTIC TUBE	101+03 67	21 20' DT	SIGN ON PLASTIC TUBE TO BE RELOCATED BY OTHERS					1
E1-3	NO TRESPASSING	U-CHANNEL			SIGN PANEL ASSEMBLY TO BE RELOCATED BY OTHERS					1
	SPEED LIMIT 40	U-CHANNEL	101+37.71	04.42 LI	SIGN PANEL ASSEMBLE TO BE RELOCATED BE OTHERS					1
E1-5a	DIVIDED HIGHWAY	U - CHANNE L	102+17.65	31.22' LT	SIGN PANEL ASSEBLY TO BE REMOVED		1			
E1-5b	DO NOT DRIVE ON SHOULDER	SQUARE TUBE	102:10 56	22 151 07	SIGN PANEL ASSEMBLY TO BE REMOVED	1				
E1-6	TRUCKS	MOUNTED ON POWER POLE			SIGN PANEL ASSEMBLY TO BE REMOVED	1				
E1-7	SPEED LIMIT 40	MOUNTED ON POWER POLE	103+94.00	32.12 KI	SIGN PANEL TO BE RELOCATED	1				
E1-8a		SQUARE TUBE	105+10.29	23.41' RT	SIGN PANEL ASSEBLY TO BE REMOVED	1				
E1-8b	NO PARKING ANY TIME	II CHANNEI	105 / 41 02	60 131 17	CLON DANIEL ACCEMBLY TO BE RELOCATED BY OTHERS					1
E1-9	STORE SIGN	U-CHANNEL			SIGN PANEL ASSEMBLY TO BE RELOCATED BY OTHERS	1				1
E1-18	DO NOT DRIVE ON SHOULDER	SQUARE TUBE			SIGN PANEL ASSEMBLY TO BE REMOVED	1				1
E1-10	NAPERVILLE TOWNSHIP ROAD DISTRICT	TWO WOODEN POSTS	100+08.26	40.50° RI	SIGN PANEL ASSEMBLY TO BE RELOCATED BY OTHERS					1
E1-11a	NO PARKING	MOUNTED ON FENCE	100.22.22	66 071 07	CLON DANSE TO DE DELOCATED DV OTHERS					
E1-11b	STOP SIGN	MOUNTED ON FENCE	106+22.33	66.87 RI	SIGN PANEL TO BE RELOCATED BY OTHERS					1
E1-11c	SECURITY WARNING		105.00.57	24 251 17						
E1-12	GASLINE WARNING	U - CHANNE L			SIGN PANEL ASSEMBLY TO BE RELOCATED BY OTHERS					1
E1-13	GASLINE WARNING	U - CHANNE L			SIGN PANEL ASSEMBLY TO BE RELOCATED BY OTHERS					1
E1-14	AX 37 UTILITY	STEEL TUBE			SIGN ON STEEL TUBE TO BE RELOCATED BY OTHERS					1
E1-15	407 UTILITY	PLASTIC TUBE			SIGN ON PLASTIC TUBE TO BE RELOCATED BY OTHERS					1
E1-16	UTILITY	SQUARE TUBE	107+78.49	29.96' R1	SIGN ON SQUARE TUBE TO BE RELOCATED BY OTHERS					1
E1-17a	SIGNAL AHEAD	SQUARE TUBE	107+91.01	32.81' RT	SIGN PANEL ASSEMBLY TO BE REMOVED		1			
E1-17b	FRONTENAC ROAD									
	A ROAD (STA 108+00 TO 117+85)		1.00 57 00		T		T		T	
E2-1	RAILROAD CLEARANCE	MOUNTED ON BRIDGE	109+57.32	0.00	SIGN PANEL TO BE RELOCATED BY OTHERS					1
E2-2	RAILROAD CLEARANCE	MOUNTED ON BRIDGE	109+87.17	0.00	SIGN PANEL TO BE RELOCATED BY OTHERS					I
E2-4	TRUCKS	U - CHANNE L	_		SIGN PANEL ASSEMBLY TO BE RELOCATED	1				
E2-6	STRAIGHT LANE AHEAD, LEFT TURN	U - CHANNE L	_		SIGN PANEL ASSEMBLY TO BE REMOVED	1				
E2-7	DO NOT ENTER	U - CHANNE L	114+90.17	31.03' RI	SIGN PANEL ASSEMBLY TO BE REMOVED	1				
E2-8a	STOP SIGN	SQUARE TUBE	115+23.	51.02' RT	SIGN PANEL ASSEBLY TO BE REMOVED		1			
E2-8b	RIGHT TURN ONLY									
E2-9	DO NOT ENTER	U - CHANNE L	115+55.07	31.44' RT	SIGN PANEL ASSEMBLY TO BE REMOVED	1				
E2-10a	PARKING PROHIBITED									
E2-10b	WEIGHT RESTRICTION	SQUARE TUBE	115+57.06	32.35' RT	SIGN PANEL ASSEMBLY TO BE RELOCATED				1	
E2-10c	CITY OF NAPERVILLE									
E2-11	SPEED LIMIT 40	U - CHANNE L			SIGN PANEL ASSEMBLY TO BE REMOVED	1				
E2-12	STATE CHAMPS	SQUARE TUBE			SIGN PANEL ASSEMBLY TO BE RELOCATED			1		
E2-13	RIGHT TURN ONLY	2 U-CHANNEL	119+49.43	38.25' LT	SIGN PANEL ASSEMBLY TO BE REMOVED	1				
	A ROAD (STA 121+00 TO 128+00)						1			
E3-1	RIGHT LANE MUST TURN RIGHT	U - CHANNE L	121+55.24	34.49' LT	SIGN PANEL ASSEMBLY TO BE REMOVED	1				
				1	TOTAL:	12	3	1	2	12

EXISTING SIGNS AND REMOVALS/RELOCATIONS

PAVEMENT MARKING LEGEND

- 1 THERMOPLASTIC PAVEMENT MARKING LINE 4" (WHITE LINES 10' LINE 30' SPACE)
- 2 THERMOPLASTIC PAVEMENT MARKING LINE 4" (2 YELLOW LINES AT 11" C-C)
- (3) THERMOPLASTIC PAVEMENT MARKING LINE 24" (WHITE)
- THERMOPLASTIC PAVEMENT MARKING LINE 6" (WHITE)
- (5) THERMOPLASTIC PAVEMENT MARKING LINE 12" (YELLOW DIAGONALS AT 50' SPACING)
- (6) THERMOPLASTIC PAVEMENT MARKING LINE 6" (WHITE LINES 2' LINE 6' SPACE)
- 7) THERMOPLASTIC PAVEMENT MARKING LETTERS & SYMBOLS
- 8 RAISED REFLECTIVE PAVEMENT MARKER
- 9) THERMOPLASTIC PAVEMENT MARKING LINE 4" (YELLOW)

PAVEMENT MARKING NOTES:

- 1. ALL SIDEWALKS AND CURB RAMPS SHALL BE CONSTRUCTED AS SHOWN ON THE PLAN AND PROFILE SHEETS
- 2. ALL PAVEMENT MARKINGS SHALL BE IN CONFORMANCE WITH THE ILLINOIS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS. THERMOPLASTIC PAVEMENT MARKINGS ON HMA PAVEMENT (NORTH AURORA ROAD).
- 3. DIMENSIONS TO PAVEMENT MARKINGS SHALL BE TO THE CENTER OF A SINGLE LINE OR THE CENTER OF THE GAP FOR DOUBLE LINES UNLESS OTHERWISE INDICATED.
- 4. TURN LANES MARKINGS (ARROW AND "ONLY") TO BE INSTALLED AS SHOWN ON DISTRICT ONE TYPICAL PAVEMENT MARKINGS DETAILS.
- 5. MEDIAN DIAGONAL LINE PAVEMENT MARKINGS TO BE INSTALLED AS SHOWN ON DISTRICT ONE TYPICAL PAVEMENT MARKING DETAILS.
- 6. RAISED REFLECTIVE PAVEMENT MARKERS TO BE INSTALLED AS SHOWN ON DISTRICT ONE TYPICAL APPLICATIONS OF RAISED REFLECTIVE PAVEMENT MARKERS DETAIL.

SIGNING NOTES:

 ALL EXISTING PRIVATE SIGNS WITHIN THE STATE RIGHT OF WAY THAT INTERFERE WITH CONSTRUCTION OPERATIONS WILL BE REMOVED AND REPLACED BY OTHERS. THE CONTRACTOR SHALL NOTIFY THE OWNERS PRIOR TO START OF CONSTRUCTION.

PAVEMENT MARKING SYMBOL LEGEND

TURN ARROW (LEFT OR RIGHT ARROW)
15.6 SQ. FT.

ONLY LETTERS 20.8 SQ. FT.

SIGN LEGEND

- ₱ PROPOSED SIGN SUPPORT

 PROPOSED SIGN
- EXISTING SIGN SUPPORT

NEW SIGN PANEL (TYPE 1 OR 2)
W/ MUTCD SIGN CODE & SIGN NUMBER
SEE SIGN SCHEDULE ON SHEET NO. 102
(ALL SIGNS SHALL USE TYPE ZZ SHEETING)
P1-1

- E1-1 EXISTING SIGN TO REMAIN IN PLACE SEE SIGN SCHEDULE ON THIS SHEET
- E1-1
 EXISTING SIGN TO BE REMOVED
 NEW SIGN PANEL MATCHING EXISTING
 SIGN TO BE INSTALLED AT LOCATION
 SHOWN ON SIGN SCHEDULE ON SHEET NO. 102
- EXISTING SIGN TO BE REMOVED SEE SIGN SCHEDULE ON THIS SHEET

TRANSYSTEMS

USER NAME = brvanderwal	DESIGNED	-	MKW	REVISED -
	DRAWN	-	BMS	REVISED -
PLOT SCALE = 100.00000 '/ in.	CHECKED	-	BVW	REVISED -
PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

NORTH AURORA ROAD PENNSBURY LANE TO FRONTENAC ROAD											
EXISTING SIGN AND REMOVAL/RELOCATION SCHEDULE											
SCALE: NONE	SHEET	1 OF	4	SHEETS	STA.	TO STA.					

F.A.U. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
1509	06-00133-00-BF	₹	DuPAGE	426	101
			CONTRACT	NO. 6	51G79

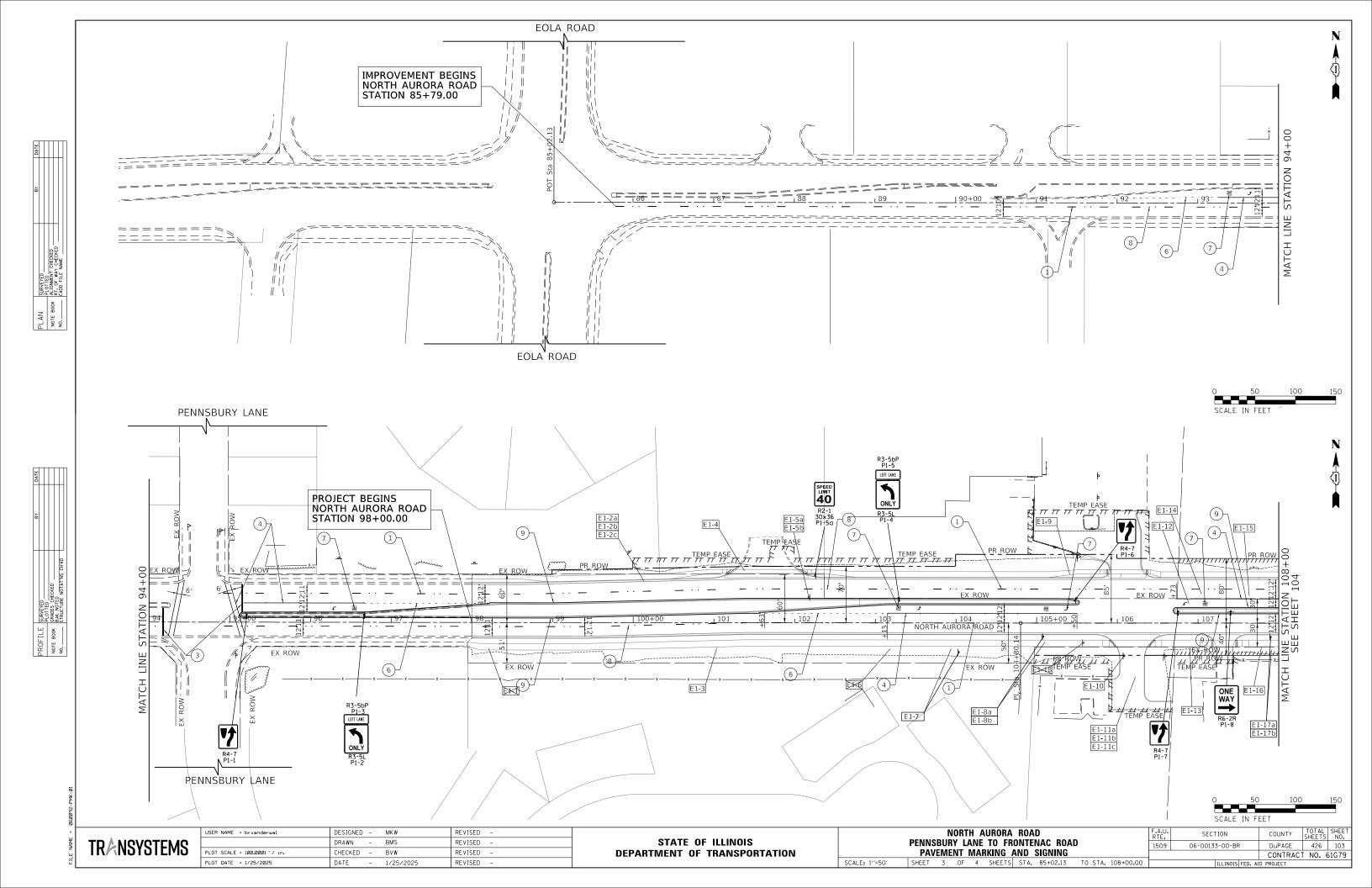
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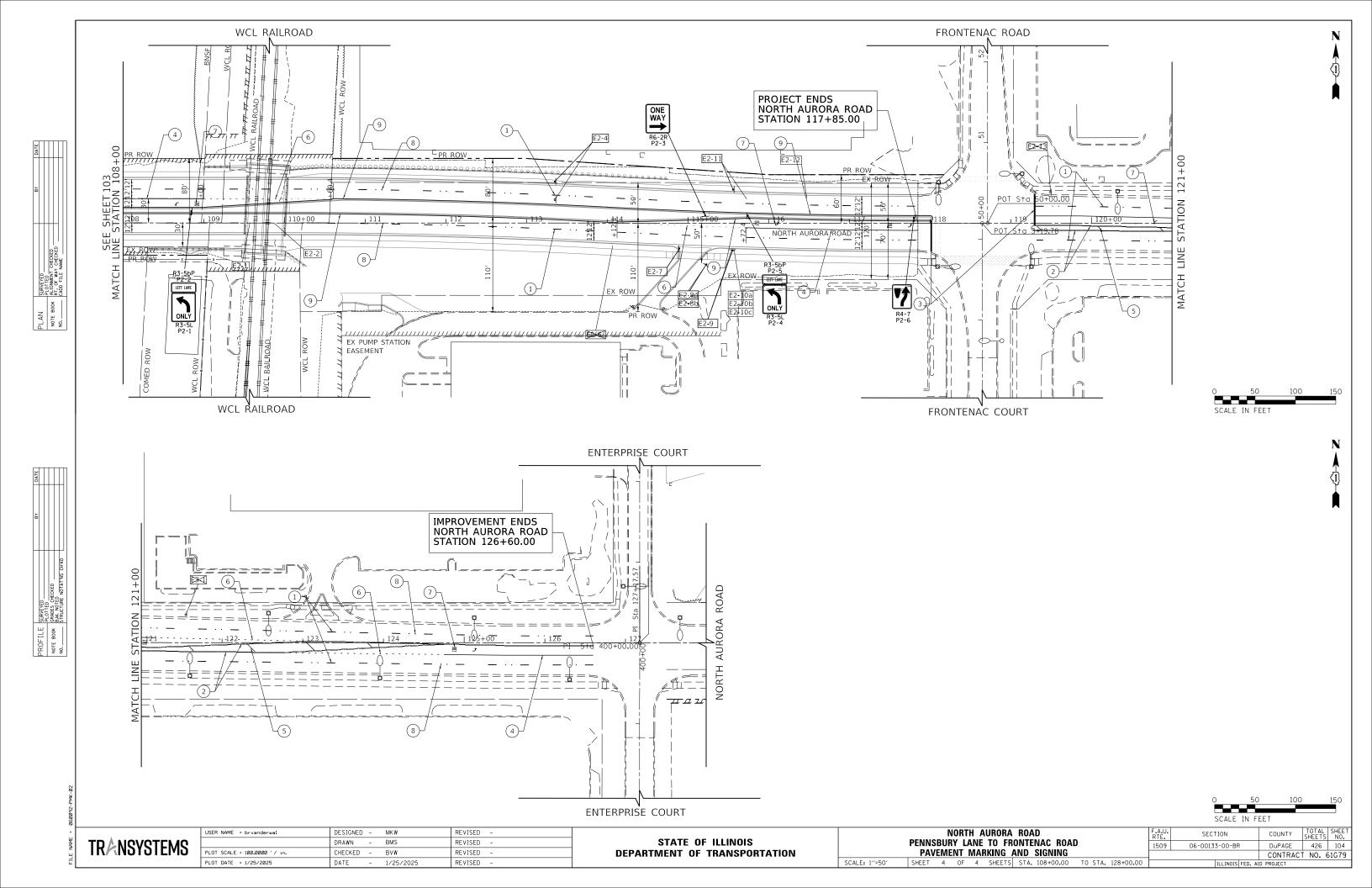
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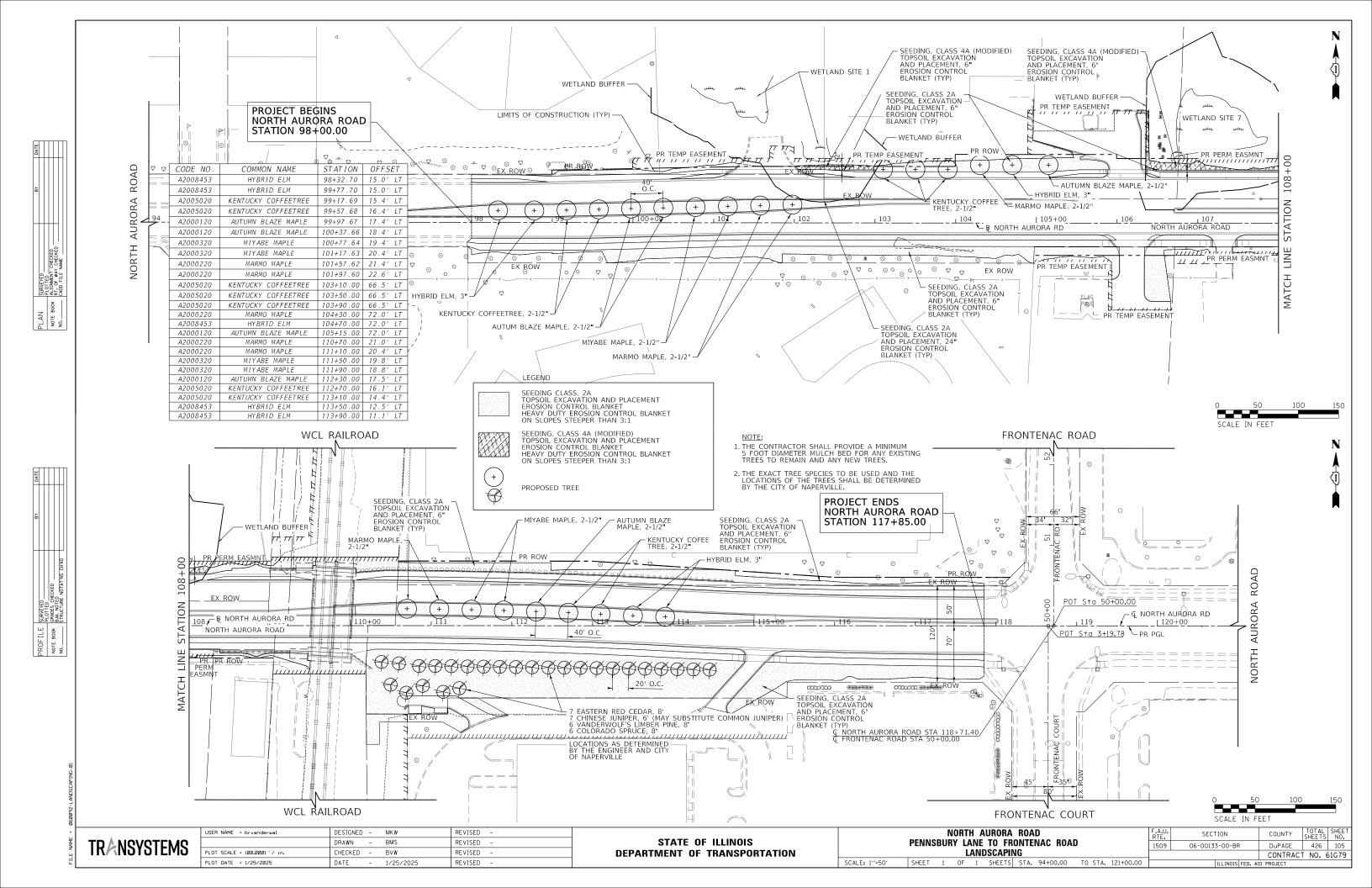
				PRO	OPOSED	SIGNS								
										72000100		72800100	72900100	72900200
PROPOSED SIGN NUMBER	SIGN DESIGNATION	PANEL DESCRIPTION	STATION	OFFSET	LT/RT	FACING TRAFFIC	COLOR	WIDTH	HEIGHT	SIGN PANEL - TYPE 1	NUMBER OF POSTS	TELESCOPING STEEL SIGN SUPPORT	METAL POST - TYPE A	METAL POST - TYPE B
				(FT)				(IN)	(IN)	(SQ FT)		(FT)	(FT)	(FT)
NORTH AU	RORA ROAD (ST	A 98+00 TO 108+00)		1 ()		1		(1.1.7	1 (1117)	(34 ///		(, , ,	()	(,,,
P1-1	R4-7	KEEP RIGHT	95+21	9.1	LT	EB	WHITE	24	30	5	1.0			13.5
P1-2	R3 - 5 L	LEFT TURN ONLY				WB	WHITE	30	36	7.5				
P1-3	R3 - 5bP	LEFT LANE	96+67	9.3	LT	WB	WHITE	30	12	2.5	2.0		29.0	
P1-5a	R2 - 1	SPEED LIMIT 40	102+25	58.8	LT	WB	WHITE	30	36	7.5	1.0	15.0		
P1-4	R3 - 5 L	LEFT TURN ONLY	400.00			EB	WHITE	30	36	7.5				
P1-5	R3-5bP	LEFT LANE	103+30	27.5	LT	EB	WHITE	30	12	2.5	2.0		29.0	
E1-7	W 1 1 - 1 O	TRUCKS	104+05	36.3	RT	EB	YELLOW	36	36	9	2.0		27.0	
E1-8a	R2 - 1	SPEED LIMIT 40	105:00	16.0		EB	WHITE	30	36	7.5		17.0		
E1-8b	R7 - 1	NO PARKING ANY TIME	105+08	16.8	RT	EB	WHITE	12	18	1.5	1.0	17.0		
P1-6	R4-7	KEEP RIGHT	105+47	27.5	LT	WB	WHITE	24	30	5	1.0			13.5
P1-7	R4-7	KEEP RIGHT	106+76	14.5	LT	EB	WHITE	24	30	5	1.0			13.5
P1-8	R6 - 2R	ONE WAY RIGHT	107+40	14.5	LT	NB	WHITE	24	30	5	1.0			13.5
E1-17a	W3-3	SIGNAL AHEAD	107:00	16.0	O.T.	EB	YELLOW	36	36	9	1.0	16.0		
E1-17b	W16-8	FRONTENAC ROAD	107+90	16.8	RT	EB	YELLOW	36	9	2.25	1.0	16.0		
NORTH AU	RORA ROAD (ST	A 108+00 TO 117+85)		•										
P2-1	R3 - 5 L	LEFT TURN ONLY	108+84	14.5	1.7	WB	WHITE	30	36	7.5	2.0		29.0	
P2-2	R3-5bP	LEFT LANE	100704	14.5	LI	WB	WHITE	30	12	2.5	2.0		29.0	
E2-4	W 1 1 - 10	TRUCKS	113+35	51.4	LT	WB	YELLOW	36	36	9	2.0		27.0	
E2-7	R5 - 1	DO NOT ENTER	114+90	31.6	RT	EB	WHITE	36	36	9	2.0		27.0	
E2-8a	R1 - 1	STOP SIGN	115+23	51.0	RT	NB	RED	30	30	6.25	1.0	18.0		
E2-8b	R3 - 5R	RIGHT TURN ONLY	113+23	31.0	I NI	NB	WHITE	30	36	7.5	1.0	10.0		
P2-3	R6 - 2R	ONE WAY RIGHT	115+21	9.3	LT	NB	WHITE	24	30	5	1.0			13.5
E2-9	R5 - 1	DO NOT ENTER	115+55	34.0	RT	SB	WHITE	36	36	9	2.0		27.0	
E2-11	R2 - 1	SPEED LIMIT 40	115+56	41.7	LT	WB	WHITE	30	36	7.5	2.0		27.0	
P2-4	R3 - 5L	LEFT TURN ONLY	115+74	9.3	LT	EB	WHITE	30	36	7.5	2.0		29.0	
P2-5	R3-5bP	LEFT LANE	113+/4	3.3	L'	EB	WHITE	30	12	2.5	2.0		29.0	
P2-6	R4-7	KEEP RIGHT	117+80	6.0	LT	WB	WHITE	24	30	5	1.0			13.5
			1			TOTAL:				155.0	28	66.0	251.0	81.0

USER NAME = brvanderwal	DESIGNED	-	MKW	REVISED -
	DRAWN	-	BMS	REVISED -
PLOT SCALE = 100.00000 '/ in.	CHECKED	-	BVW	REVISED -
PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -

	NORTH AURORA		F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	PENNSBURY LANE TO FROI	1509	06-00133-00-BR	DuPAGE	426	102	
	PROPOSED SIGN SC	•		CONTRAC	T NO. 6	51G79	
SCALE: NONE	SHEET 2 OF 4 SHEETS S	STA. TO STA.		ILLINOIS FED. AI	ID PROJECT		







SCHEDULE OF QUANTITIES

PAY ITEM NAME	UNIT	QTY TOTAL	NORTH AURORA RD & EOLA RD	NORTH AURORA RD & GENESEE RD	NORTH AURORA RD & PENSBURY LN	NORTH AURORA RD & FRONTENAC RD	MOT / DETOUR ROUTE	INTERCONNECT	
ELECTRIC UTILITY SERVICE CONNECTION	LSUM	1			0.50	0.50			1
UNDERGROUND CONDUIT, GALVANIZED STEEL, 2" DIA.	FOOT	233				233			1
UNDERGROUND CONDUIT, GALVANIZED STEEL, 4" DIA.	FOOT	4,140						4,140	7 *
HANDHOLE	EACH	4				1		3	
MAINTENANCE OF EXISTING TRAFFIC SIGNAL INSTALLATION	EACH	2	2						1
ELECTRIC CABLE IN CONDUIT, LEAD-IN, NO. 14 1 PAIR	FOOT	1,070				1,070			1
DRILL EXISTING HANDHOLE	EACH	2						2	
DRILL EXISTING HEAVY DUTY HANDHOLE	EACH	4				4			1
INDUCTIVE LOOP DETECTOR	EACH	3				3			1
DETECTOR LOOP, TYPE I	FOOT	216				216			1
TEMPORARY TRAFFIC SIGNAL INSTALLATION	EACH	2			1	1			
MODIFY EXISTING CONTROLLER CABINET	EACH	1				1			
REMOVE EXISTING TRAFFIC SIGNAL EQUIPMENT	EACH	1				1			
CONDUIT SPLICE	EACH	1				1			1
ACCESSIBLE PEDESTRIAN SIGNALS	EACH	16			8	8			
RE-OPTIMIZE TRAFFIC SIGNAL SYSTEM LEVEL 1	EACH	1				1			
TEMPORARY TRAFFIC SIGNAL TIMING	EACH	5			1	1	3		1

100% OF INTERCONNECT COST SHALL BE PAID BY CITY OF NAPERVILLE

TRAFFIC SIGNAL GENERAL NOTES:

- IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND CONDITIONS EXISTING IN THE FIELD PRIOR TO ORDERING MATERIALS AND BEGINNING CONSTRUCTION.
- THE EXACT LOCATION OF ALL UTILITES SHALL BE FIELD VERIFIED BY THE CONTRACTOR BEFORE ORDERING ANY MATERIALS AND STARTING ANY WORK. FOR LOCATIONS OF UTILITIES, LOCALLY OWNED EQUIPMENT, LEASED ENFORCEMENT CAMERA SYSTEM FACILITIES AND UNDERGROUND FACILITIES, CONTACT THE LOCAL COUNTIES, MUNICIPALITIES AND IDOT FOR LOCATES. THE CONTRACTOR SHALL CALL "JULIE" AT (800) 892-0123 OR 811, FOR FIELD LOCATIONS OF BURIED UTILITIES (48 HOURS NOTIFICATION REQUIRED).
- THE CONTRACTOR SHALL CHECK THE TRAFFIC SIGNAL EQUIPMENT LOCATIONS FOR OVERHEAD UTILITY CONFLICTS. THE CONTRACTOR SHALL COORDINATE ANY CONFLICTS WITH THE UTILITY COMPANIES AND THE RESIDENT ENGINEER BEFORE ORDERING MATERIALS.
- THE CONTRACTOR SHALL COORDINATE CONSTRUCTION ACTIVITIES WITH UTILITY COMPANIES, LOCAL GOVERNMENT AGENCIES AND IDOT.
- MATERIAL SUBMITTALS FOR THE NORTH AURORA ROAD PENNSBURY LANE SIGNAL SHALL BE REVIEWED BY THE CITY OF AURORA AND/OR AN AUTHORIZED REPRESENTATIVE PRIOR TO BEING SUBMITTED FOR LOCAL ROADS REVIEW.

TEMPORARY TRAFFIC SIGNAL GENERAL NOTES:

- ONLY CONTROLLERS SUPPLIED BY ONE OF THE DISTRICT APPROVED CLOSED LOOP EQUIPMENT MANUFACTURERS WILL BE APPROVED FOR USE AT TEMPORARY SIGNAL LOCATIONS. ALL CONTROLLERS USED FOR TEMPORARY TRAFFIC SIGNALS SHALL BE FULLY ACTUATED NEMA MICROPROCESSOR BASED WITH RS232 DATA ENTRY PORTS COMPATIBLE WITH EXISTING MONITORING SOFTWARE APPROVED BY IDOT DISTRICT 1, INSTALLED IN A NEMA TS1 OR TS2
- ALL TRAFFIC SIGNAL SECTIONS AND PEDESTRIAN SIGNAL SECTIONS SHALL BE 12". HEADS SHALL BE PLACED AS INDICATED ON THE TEMPORARY TRAFFIC SIGNAL PLAN OR AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHALL FURNISH ENOUGH CABLE SLACK TO RELOCATE HEADS TO ANY OTHER POSITION ON THE SPAN WIRE OR AT LOCATIONS ILLUSTRATED ON THE PLANS FOR CONSTRUCTION STAGING. THE TEMPORARY TRAFFIC SIGNAL SHALL REMAIN IN OPERATION DURING ALL SIGNAL HEAD RELOCATIONS. EACH TEMPORARY TRAFFIC SIGNAL HEAD SHALL HAVE ITS OWN CABLE FROM THE CONTROLLER CABINET TO THE SIGNAL HEAD.
- ANY TEMPORARY SIGNAL WITHIN AN EXISTING CLOSED LOOP TRAFFIC SIGNAL SYSTEM SHALL BE INTERCONNECTED TO THAT SYSTEM USING SIMILAR BRAND CONTROLLER EQUIPMENT.
- ALL CONTROL EQUIPMENT INCLUDING EMERGENCY PRE-EMPTION AND COMMUNICATION DEVICES FOR THE TEMPORARY TRAFFIC SIGNAL(S) SHALL BE FURNISHED BY THE CONTRACTOR, EXCEPT AS NOTED IN THE PLANS.
- ANY TEMPORARY TRAFFIC SECTIONS NOT IN USE DURING A STAGE OF CONSTRUCTION SHALL BE BAGGED AND DEACTIVATED.
- ALL VIDEO DETECTION ZONES ARE TO BE REDEFINED DURING EACH STAGE OF CONSTRUCTION AND ARE TO BE INCLUDED IN THE PAY ITEM OF TEMPORARY TRAFFIC SIGNAL INSTALLATION.
- IF TEMPORARY TRAFFIC SIGNALS ARE IN CONFLICT WITH THE PERMANENT TRAFFIC SIGNALS, THE MAST ARMS SHALL BE ROTATED OR REMOVED AND STORED OUTSIDE OF R.O.W. AND WILL BE THE CONTRACTOR'S RESPONSIBILITY TO KEEP EQUIPMENT SAFE. THIS IS TO BE INCLUDED IN
- IF ANY CONSTRUCTION STAGE CHANGE WILL IMPACT THE TRAFFIC SIGNAL OPERATIONS, THE CONTRACTOR SHALL LET THE ENGINEER KNOW THREE DAYS PRIOR TO THE DATE OF CHANGE.
- THE RADIO INTERCONNECT SHALL OPERATE PROPERLY AT ALL TIMES AND DURING ALL CONSTRUCTION STAGES. IF WIRELESS INTERCONNECT FAILS DURING TESTING OR OPERATIONS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING THE NECESSARY POLES, FIBER OPTIC CABLE AND OTHER INFRASTRUCTURE PROVIDING TEMPORARY FIBER INTERCONNECT AT NO COST TO THE CONTRACT.
- PEDESTRIAN SIGNAL HEADS AND APS PUSH-BUTTONS SHALL BE PROVIDED FOR ALL CROSSWALKS THAT ARE ACTIVE DURING THE CONSTRUCTION STAGE. APS PEDESTRIAN PUSH-BUTTONS MAY BE LOCATED ON BARREL MOUNTED POSTS OR TEMPORARY SIGNAL WOOD POLES, HOWEVER ALL ACTIVE PEDESTRIAN SIGNALS AND APS PUSH-BUTTONS SHALL MEET THE REQUIREMENTS OF DISTRICT ONE STANDARD TRAFFIC SIGNAL DESIGN DETAILS. THIS MAY BE ACCOMPLISHED BY RELOCATING THE BARREL MOUNTED POSTS, APS PUSH-BUTTON BRACKET EXTENSIONS, PROVIDING TEMPORARY SIDEWALK SURFACE, OR A COMBINATION OF THESE METHODS. REFER TO TEMPORARY TRAFFIC SIGNAL INSTALLATION PLANS FOR SUGGESTED LOCATIONS OF PEDESTRIAN SIGNALS AND APS PUSH-BUTTONS, WHICH MAY BE MODIFIED AS NEEDED OR AS DIRECTED BY THE ENGINEER. THE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT COST FOR TEMPORARY TRAFFIC SIGNAL INSTALLATION.
- 11. REFER TO TEMPORARY CABLE PLANS AND TEMPORARY PHASE DESIGNATION DIAGRAMS FOR ACTIVE PEDESTRIAN PHASES UNDER EACH CONSTRUCTION STAGE. INACTIVE PEDESTRIAN SIGNAL HEADS SHALL BE DE-ENERGIZED AND BAGGED. INACTIVE PUSH-BUTTONS SHALL BE DE-ENERGIZED AND COVERED OR REMOVED. THE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT COST FOR TEMPORARY TRAFFIC SIGNAL INSTALLATION.
- 12. THE TEMPORARY TRAFFIC SIGNAL CONTROLLER FOR THIS PROJECT SHALL BE "ECONOLITE" TO MATCH EXISTING EQUIPMENT.

TRANSYSTEMS

USER NAME = brvanderwal	DESIGNED	-	GR	REVISED -
	DRAWN	-	GR	REVISED -
PLOT SCALE = 40.0000 '/ in.	CHECKED	-	BVW	REVISED -
PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

	NORTH	AURORA	ROAD				FRONTENAC ROA)	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
			00111		FIC SIGI				1509	06-00133-00-BR	DuPAGE	426	106
			2CH	EDULE	UF UU	ANTITIES					CONTRAC	T NO.	61G79
S	CALE: NONE	SHEE	T 1 (OF 32	SHEETS	STA.	TO STA.		ILLINOIS FED. AID PROJECT				

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TRAFFIC SIGNAL LEGEND

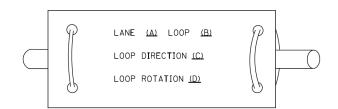
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MARINELLE COUNTY CONTROL MARINELLE COUNTY CO	Commentation order:	MARKED STATES CONTROLLED 100 1	MANAGEMENT CONTROLLER					(IIII III USALL)				
AND CONTROLLED STATE CONTROLL	### PRESIDENCE STATE OF THE PROPERTY STATE O	Commentation Comment	AND MARKETON COUNTY OF THE PROPERTY OF THE PRO	ITEM	EXISTING	<u>PROPOSED</u>	ITEM	EXISTING	<u>PROPOSED</u>	ITEM	EXISTING	PROPOSED
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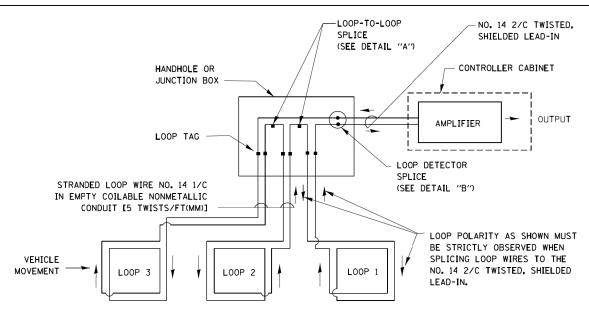
LOOP DETECTOR NOTES

- 1. EACH PAIR OF LOOP WIRES SHALL BE PLACED IN A SEPARATE EMPTY COILABLE NONMETALLIC CONDUIT FROM THE EDGE OF PAVEMENT TO THE HANDHOLE. SPACING BETWEEN THE HOLES DRILLED IN THE PAVEMENT SHALL NOT BE LESS THAN 6" (150 mm). EMPTY COILABLE NONMETALLIC CONDUIT SHALL BE INCLUDED IN THE COST OF THE LOOP WIRE.
- 2. THE NUMBER OF LOOP TURNS SHALL BE AS RECOMMENDED BY THE AMPLIFIER MANUFACTURER. ALL ADJACENT SIDES OF THE LOOPS SHALL BE INSTALLED IN SUCH A WAY THAT THE CURRENT FLOW IS IN THE SAME DIRECTION TO REINFORCE ITS MAGNETIC FIELDS FOR SMALL VEHICLE DETECTION.
- 3. EACH LOOP LEAD-IN SHALL BE IDENTIFIED AND PERMANENTLY TAGGED IN THE HANDHOLE. EACH LEAD-IN CABLE TAG SHALL INDICATE THE LOCATION OF THE LOOP, LOOP ROTATION (CLOCKWISE/COUNTERCLOCKWISE), LOOP LEAD-IN DIRECTION (IN OR OUT), LOOP CABLE NUMBER AND LOCATION IN CABINET, AND NUMBER OF TURNS IN THE DETECTOR LOOPS IN WATER PROOF INK AS INDICATED ON THE DISTRICT 1 STANDARD TRAFFIC SIGNAL DESIGN DETAIL. THE CONTRACTOR SHALL MARK LOOP LOCATIONS ON RECORD DRAWINGS AND PRESENT TO THE ENGINEER AFTER FINAL INSPECTION. LOOPS SHALL BE MARKED BY LANE AND LOOP NUMBER. SEE DETAIL BELOW.
- 4. ALL LOOP CABLE SHALL BE FASTENED WITH PLASTIC TIE WRAP TO THE HANDHOLE HOOKS.
- 5. IN ASPHALT PAVEMENT, LOOPS SHOULD BE PLACED IN THE BINDER AND DIVEHOLES MARKED AT THE CURB WITH A SAW-CUT. THE SAW-CUT SHALL BE CUT IN ACCORDANCE WITH LOCAL AND E.P.A. DUST CONTROL REQUIREMENTS. DETECTOR LOOP(S) SHALL NOT BE INSTALLED IN WET CONDITIONS AND THE SAW-CUTS MUST BE FREE OF DEBRIS AND RESIDUE SUCH AS DUST AND WATER WHICH IS TO BE ACHIEVED BY THE USE OF COMPRESSED AIR, WIRE BRUSHING AND HEAT DRYING ACCORDING TO SEALANT MANUFACTURER REQUIREMENTS. THE DETECTOR WIRE SHALL BE HELD IN PLACE BY THE USE OF FORM WEDGES. WEDGES SHALL BE SPACED NO MORE THAN 18" (450 mm) APART.
- 6. LOOP SPLICES SHALL BE SOLDERED USING A SOLDERING IRON. BLOW TORCHES OR OTHER DEVICES WHICH OXIDIZE COPPER CABLE SHALL NOT BE ALLOWED FOR SOLDERING OPERATIONS. SEE DETAIL BELOW RIGHT.
- 7. PREFORMED DETECTOR LOOPS SHALL BE USED, AS SHOWN ON THE PLANS, WHERE NEW CONCRETE PAVEMENT IS PROPOSED. THE INSTALLATION OF PREFORMED LOOPS SHALL BE IN ACCORDANCE WITH THE DISTRICT 1 SPECIFICATIONS OR AS DIRECTED BY THE ENGINEER.

LOOP LEAD-IN CABLE TAG

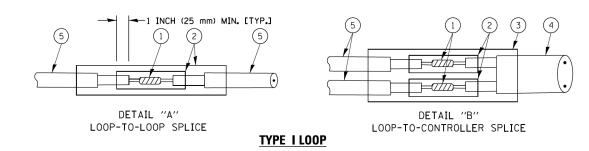


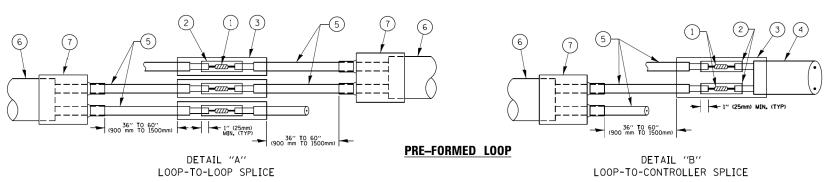
- A. LANE 1 IS THE LANE CLOSEST TO THE CENTERLINE OF THE ROADWAY
- B. LOOP *1 IS THE LOOP IN THE LANE CLOSEST TO THE INTERSECTION.
- C. LABEL LOOP CABLE "IN" OR LOOP CABLE "OUT".
- D. LABEL LOOP CABLE CLOCKWISE OR LOOP CABLE COUNTERCLOCKWISE.



DETECTOR LOOP WIRING SCHEMATIC

- LOOPS SHALL BE SPLICED IN SERIES.
- SAW-CUTS SHALL BE A MINIMUM WIDTH OF 5/16" (8 mm).
- SAW-CUT DEPTHS SHALL BE 3" (75 mm). IF IN CONCRETE,
 THE SAW-CUT DEPTH SHALL BE TO THE TOP OF THE REINFORCEMENT.
- LOOP CORNERS SHALL BE DRILLED WITH A 2" (50 mm) DIAMETER CORE.





LOOP DETECTOR SPLICE

- 1 WESTERN UNION SPLICE SOLDERED WITH ROSIN CORE FLUX. ALL EXPOSED SURFACES OF THE SOLDER SHALL BE SMOOTH. THE WESTERN UNION SPLICES SHALL BE STAGGERED.
- (2) WCSMW 30/100 HEAT SHRINK TUBE, MINIMUM LENGTH 3" (75 mm), UNDERWATER GRADE.
- (3) WCS 200/750 HEAT SHRINK TUBE, MINIMUM LENGHT 6" (150 mm), UNDERWATER GRADE.

SCALE: NONE

(4) NO. 14 2/C TWISTED, SHIELDED CABLE.

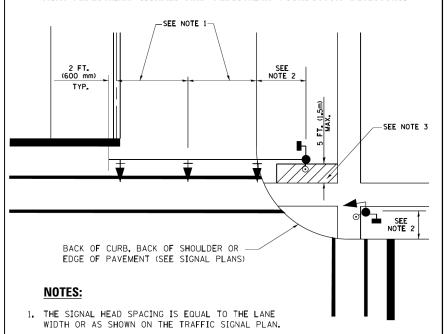
- 5 LOOP CONDUCTOR WITH FLEXIBLE PLASTIC TUBE.
- 6 PRE-FORMED LOOP
- 7 XL POLYOLEFIN 2 CONDUCTOR BREAKOUT SEALS, TYCO CBR-2 OR APPROVED EQUAL

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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

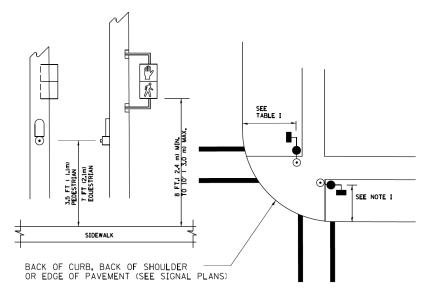
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SHEET 3	OF 32	SHEETS	STA.	TO STA.	FED. R	DAD DIST. NO. 1 ILLINOIS FED. A	ID PROJECT		

TRAFFIC SIGNAL MAST ARM AND SIGNAL POST MAST ARM MOUNTED SIGNALS IN EXISTING, PROPOSED OR FUTURE SIDEWALKBICYCLE PATH AREA. INTERSECTION SHOWN WITH PEDESTRIAN SIGNALS AND PEDESTRIAN PUSHBUTTON DETECTORS.



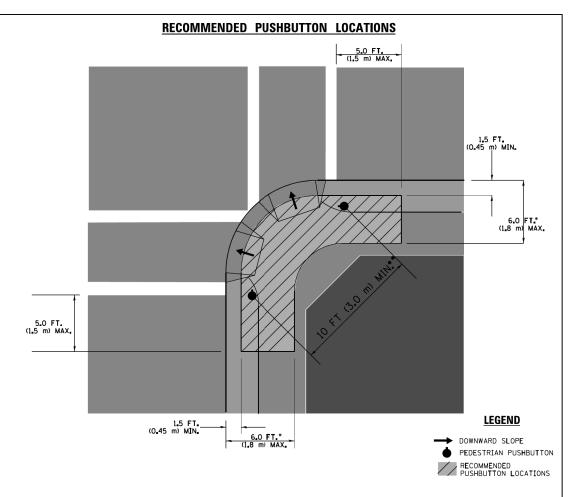
- 2. REFER TO THE TRAFFIC SIGNAL EQUIPMENT OFFSET TABLE.
- PROVIDE A LEVEL ALL-WEATHER SURFACE (CONCRETE SIDEWALK, ASPHALT BICYCLE PATH SURFACE OR MATCHING MATERIAL TO THE ADJACENT SURFACE) UP TO THE MAST ARM SHAFT OR THE SIGNAL POST.
- 4. THE FACE OF THE PEDESTRIAN PUSHBUTTON SHALL BE PARALLEL TO THE CROSSWALK TO BE USED.
- 5. THE LOCATIONS AND INSTALLATION OF PEDESTRIAN SIGNAL HEADS AND PEDESTRIAN PUSHBUTTONS SHALL MEET THE REQUIREMENTS OF THE MUTCD AND INFORMATION FOUND IN THE "AMERICANS WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES FOR BUILDINGS AND FACILITIES."

<u>PEDESTRIAN SIGNAL POST</u> <u>AND</u> PEDESTRIAN PUSH BUTTON POST



NOTES:

- 1. REFER TO THE TRAFFIC SIGNAL EQUIPMENT OFFSET TABLE.
- 2. PROVIDE A LEVEL ALL-WEATHER SURFACE (CONCRETE SIDEWALK, ASPHALT BICYCLE PATH SURFACE OR MATCHING MATERIAL TO THE ADJACENT SURFACE) UP TO THE PEDESTRIAN SIGNAL POST OR THE PEDESTRIAN PUSH BUTTON POST.
- 3. THE FACE OF THE PEDESTRIAN PUSHBUTTON SHALL BE PARALLEL TO THE CROSSWALK TO BE USED.
- 4. THE LOCATIONS AND INSTALLATION OF PEDESTRIAN SIGNAL HEADS AND PEDESTRIAN PUSHBUTTONS SHALL MEET THE REQUIREMENTS OF THE MUTCD AND INFORMATION FOUND IN THE "AMERICANS WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES FOR BUILDINGS AND FACILITIES."



- WHERE THERE ARE CONSTRAINTS THAT MAKE IT IMPRACTICAL TO PLACE THE PEDESTRIAN PUSHBUTTON BETWEEN 1.5 FT (0.45 m) AND 6 FT (1.8 m) FROM THE EDGE OF THE CURB, SHOULDER, OR PAVEMENT, IT SHOULD NOT BE FURTHER THAN 10 FT (3 m) FROM THE EDGE OF CURB, SHOULDER, OR PAVEMENT.
- •• WHERE THERE ARE CONSTRAINTS ON A PARTICULAR CORNER THAT MAKE IT IMPRACTICAL TO PROVIDE THE 10 FT (3 m) SEPERATION BETWEEN THE TWO PEDESTRIAN PUSHBUTTONS, THE PUSHBUTTONS MAY BE PLACED CLOSER TOGETHER OR ON THE SAME POLE,

NOTES:

- 1. PEDESTRIAN SIGNAL HEADS SHALL BE MOUNTED WITH THE BOTTOM OF THE SIGNAL HOUSING INCLUDING BRACKETS NOT LESS THAN 8 FT (2.4 m) OR MORE THAN 10 FT (3 m) ABOVE SIDEWALK LEVEL, AND SHALL BE POSITIONED AND ADJUSTED TO PROVIDE MAXIMUM VISIBILITY AT THE BEGINNING OF THE CONTROLLED CROSSWALK.
- 2. THE BOTTOM OF THE SIGNAL HOUSING (INCLUDING BRACKETS) OF A VEHICULAR SIGNAL FACE THAT IS NOT LOCATED OVER A HIGHWAY SHALL BE AT LEAST 8 FT (2.4 m) BUT NOT MORE THAN 19 FT (5.8 m) ABOVE THE SIDEWALK OR, IF THERE IS NO SIDEWALK, ABOVE THE PAVEMENT GRADE AT THE CENTER OF THE ROADWAY.
- 3. THE BOTTOM OF THE SIGNAL HOUSING AND ANY RELATED ATTACHMENTS TO A SIGNAL FACE LOCATED OVER ANY PORTION OF A HIGHWAY SHALL BE ACCORDING TO CURRENT STATE STANDARDS 877001, 877002, 877006, 877011 AND 877012 WITH A MINIMUM OF 16 FT (5.0 m) AND A MAXIMUM OF 18 FT. (5.5 m) FROM THE HIGHEST POINT OF PAVEMENT.
- 4. THE BOTTOM OF THE TEMPORARY SPAN WIRE MOUNTED SIGNAL HOUSING AND ANY RELATED ATTACHMENTS TO A SIGNAL FACE LOCATED OVER ANY PORTION OF A HIGHWAY SHALL BE ACCORDING TO CURRENT STATE STANDARD 880001 WITH A MINIMUM OF 17 FT (5.18 m) FROM THE HIGHEST POINT OF PAVEMENT.
- 5. THE TOP OF THE SIGNAL HOUSING OF A SIGNAL FACE LOCATED OVER ANY PORTION OF A HIGHWAY SHALL NOT BE MORE THAN 25.6 FT (7.8 m) ABOVE THE PAVEMENT.

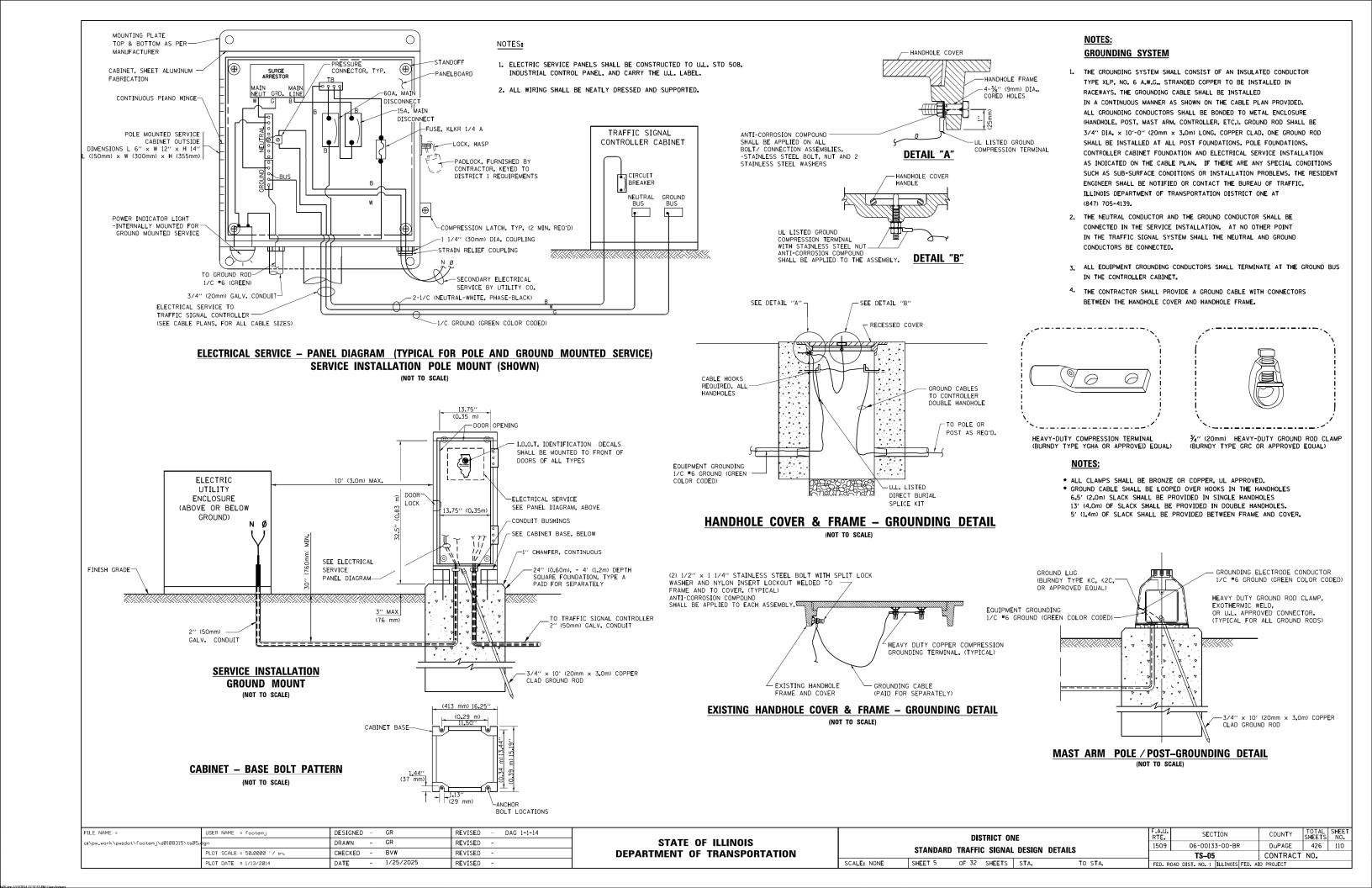
TRAFFIC SIGNAL EQUIPMENT OFFSET

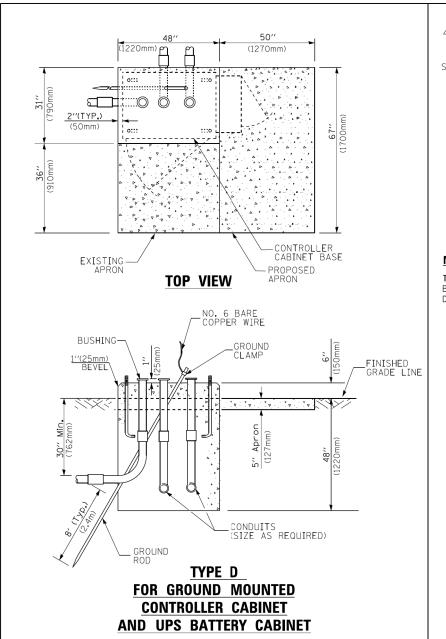
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TRAFFIC SIGNAL EQUIPMENT	COMBINATION CONCRETE CURB AND GUTTER (MINIMUM DISTANCE FROM BACK OF CURB TO CENTERLINE OF FOUNDATION)	SHOULDER/NON-CURBED AREA (MINIMUM DISTANCE FROM EDGE OF PAVEMENT TO CENTERLINE OF FOUNDATION)
TRAFFIC SIGNAL MAST ARM POLE	6 FT (1.8m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
TRAFFIC SIGNAL POST	4 FT (1.2m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
PEDESTRIAN SIGNAL POST	4 FT (1 _• 2m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
PEDESTRIAN PUSHBUTTON POST	4 FT (1•2m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
TEMPORARY WOOD POLE	6 FT (1.8m)	SHOULDER WIDTH + 2 FT (0.6m), MINIMUM 10 FT (3.0m)
CONTROLLER CABINET	6 FT (1.8m) MINIMUM DISTANCE SEE NOTE 2	SHOULDER WIDTH + 6 FT (1.8m), MINIMUM 16 FT (4.9m) SEE NOTE 3.
SERVICE INSTALLATION, GROUND MOUNT	6 FT (1.8m) MINIMUM DISTANCE SEE NOTE 2	SHOULDER WIDTH + 6 FT (1.8m), MINIMUM 16 FT (4.9m) SEE NOTE 3.

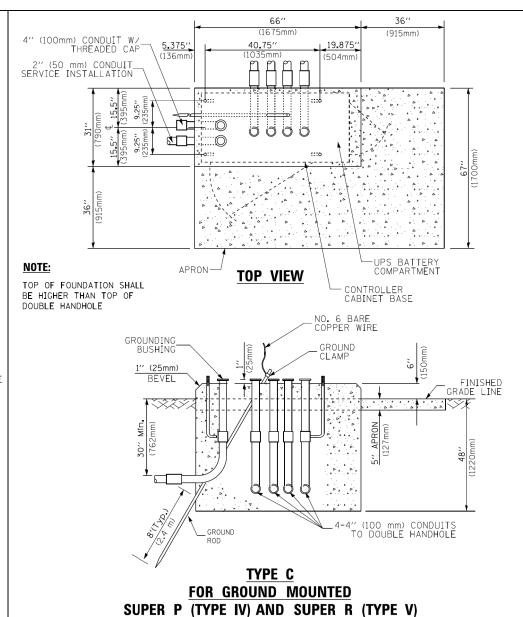
NOTES:

- 1. CONTACT THE "AREA TRAFFIC SIGNAL MAINTENANCE AND OPERATIONS ENGINEER" FOR ASSISTANCE IN LOCATING THE TRAFFIC SIGNAL EQUIPMENT WHEN THERE ARE CONFLICTS WITH DITCHES OR THE MINIMUM OFFSET DISTANCES CANNOT BE MET.
- 2. MINIMUM DISTANCE FROM THE BACK OF CURB TO THE ROADWAY SIDE OF THE FOUNDATION.
- 3. MINIMUM DISTANCE FROM THE EDGE OF PAVEMENT TOTHE ROADWAY SIDE OF THE FOUNDATION.
- 4. ANY CHANGES TO THE OFFSETS OF THE FOUNDATIONS, FROM THE MINIMUM DISTANCES LISTED IN THE "TRAFFIC SIGNAL EQUIPMENT OFFSET" CHART AND THE TRAFFIC SIGNAL INSTALLATION PLAN, COULD EFFECT THE PLACEMENT OF THE SIGNAL HEADS, PEDESTRIAN SIGNAL HEADS AND THE PEDESTRIAN PUSHBUTTONS. THE SIGNAL HEAD PLACEMENT ON THE MAST ARMS SHALL REMAIN AS PER THE TRAFFIC SIGNAL INSTALLATION PLAN AND THE "TRAFFIC SIGNAL MAST ARM AND SIGNAL POST" DETAIL ABOVE. THE PROPOSED MAST ARM LENGTHS MAY NEED TO BE REVISED TO MEET THE ABOVE REQUIREMENTS. THE PEDESTRIAN SIGNAL HEADS AND PEDESTRIAN PUSHBUTTONS MUST MEET THE REQUIREMENTS UNDER THE DETAILS ON THIS SHEET.

GR FILE NAME = DESIGNED -USER NAME = footem. REVISED DAG 1-1-14 SECTION COUNTY SHEETS DISTRICT ONE DRAWN GR REVISED STATE OF ILLINOIS :\pw_work\pwidot\footemj\d0108315\ts0 06-00133-00-BR DuPAGE 426 109 1509 STANDARD TRAFFIC SIGNAL DESIGN DETAILS BVW **DEPARTMENT OF TRANSPORTATION** PLOT SCALE = 50.0000 '/ in. CHECKED REVISED TS-05 CONTRACT NO. SHEET 4 OF 32 SHEETS STA. PLOT DATE = 1/13/2014 DATE 1/25/2025 REVISED SCALE: NONE TO STA. ID PROJECT







CONTROLLER CABINETS

DEPTH
4'-0" (1.2m)
4'-0" (1.2m)
4'-0" (1.2m)
4'-0'' (1 . 2m)

CE-GROUND MOUNT) 3.0 1.0 DEPTH OF FOUNDATION

Mast Arm Length	 Foundation Depth 	Foundation Diameter	Spiral Diameter	Quantity of Rebars	Size of Rebars
Less than 30′ (9.1 m)	10'-0" (3.0 m)	30" (750mm)	24" (600mm)	8	6(19)
Greater than or equal to	13'-6" (4∎1 m)	30" (750mm)	24" (600mm)	8	6(19)
30′ (9.1 m) and less †han 40′ (12.2 m)	11'-0" (3.4 m)	36" (900mm)	30" (750mm)	12	7(22)
Greater than or equal to 40' (12.2 m) and less than 50' (15.2 m)	13'-0" (4.0 m)	36" (900mm)	30" (750mm)	12	7(22)
Greater than or equal to 50' (15.2 m) and up to 55' (16.8 m)	15'-0" (4.6 m)	36'' (900mm)	30" (750mm)	12	7(22)
Greater than or equal to 56' (16.8 m) and less than 65' (19.8 m)	21'-0" (6.4 m)	42'' (1060mm)	36" (900mm)	16	8(25)
Greater than or equal to 65' (19.8 m) and up to 75' (22.9 m)	25'-0" (7.6 m)	42'' (1060mm)	36" (900mm)	16	8(25)

65" (SEE NOTE 4) (1651mm)

CABINET

SEE NOTE 5-

TRAFFIC SIGNAL-CONTROLLER CABINET

3/4" (19mm) TREATED PHYWOOD DECK

 $6^{\prime\prime}$ × $6^{\prime\prime}$ (152mm × 152mm TREATED WOOD POSTS

3. PLATFORM SIZE FOR CONTROLLER CABINET TYPE IV.

NOTES:

- 1. These foundation depths are for sites which have cohesive soils (clayey silt, sandy clay, etc.) along the length of the shaft, with an average Unconfined Compressive Strength (Ou) > 1.0 tsf (100 kpa). This strength shall be verified by boring data prior to construction or with testing by the Engineer during foundation drilling. The Bureau of Bridges & structures should be contacted for a revised design if other conditions are encountered.
- 2. Combination mast arm assemblies under 55 feet (16.8 m) shall use $36^{\prime\prime}$ (900 mm) diameter foundations.
- 3. Combination mast arm assemblies under 56 feet (16.8 m) through 75 feet (22.9 m) shall use 42" (1060 mm) diameter foundations
- 4. For mast arm assemblies with dual arms refer to state standard 878001..

BASED ON CONTROLLER CABINET TYPE IV WITH BASE DIMENSIONS OF 26" x 44" (660mm x 1118mm).
 ADJUST PLATFORM SIZE TO FIT CABINET BASE DIMENSIONS BEING SUPPLIED

4. PLATFORM SIZE FOR CONTROLLER CABINET TYPE IV AND UNINTERRUPTIBLE POWER SUPPLY CABINET.

2. BASED ON UNINTERRUPTIBLE POWER SUPPLY CABINET WITH BASE DIMENSIONS OF 16" x 25" (406mm x 635mm). ADJUST PLATFORM SIZE TO FIT CABINET BASE DIMENSIONS BEING SUPPLIED.

5. DRILLED HOLES THROUGH THE PLATFORM BASE TO MATCH THE CONTROLLER CABINET BOLT TEMPLATE. FASTEN THE CONTROLLER CABINET TO THE PLATFORM WITH CARRIAGE BOLTS, WASHERS AND NUTS.

TEMPORARY SIGNAL CONTROLLER

WOOD SUPPORT PLATFORM

6. FASTEN ALL SUPPORT WOOD FRAMING TO THE WOOD POSTS WITH 2 LAG SCREWS FOR EACH CONNECTION.

DEPTH OF MAST ARM FOUNDATIONS, TYPE E

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		PLOT SCALE = 50.0000 ' / in.	CHECKED -	BVW	REVISED -	DEPARTMENT OF TRANSPORTATION		STANDA	ARD TRAFFIC SIG	NAL DESIG	N DETAILS	1303	TS-05	CONTRACT		
		PLOT DATE = 1/13/2014	DATE -	1/25/2025	REVISED -		SCALE: NONE	SHEET 6	OF 32 SHEET	S STA.	TO STA.	FED. ROAI		ID PROJECT		\rightarrow

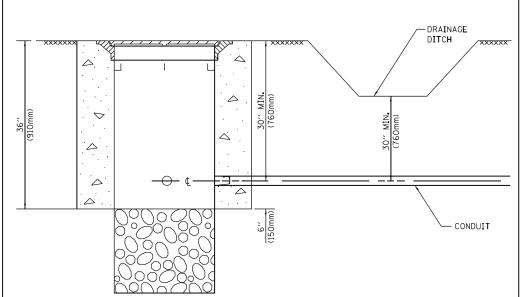
CABLE SLACK LENGTH 6.5 DOUBLE HANDHOLE 13.0 SIGNAL POST 0.6 2.0 MAST ARM 0.6 0.5 2.0 CONTROLLER CABINET FIBER OPTIC AT CABINET 1.5 13.0 4.0 ELECTRIC SERVICE AT (CABINET OR SERVICE LOCATION) 0.5 GROUND CABLE (SIGNAL POST, MAST ARM, CABINET) 0.5 GROUND CABLE (BETWEEN FRAME AND COVER) 1.6

VERTICAL CABLE LENGTH	FEET	METER
MAST ARM POLE (MAST ARM MOUNTED SIGNAL HEAD)		
(L = MAST ARM LENGTH - DISTANCE TO SIGNAL HEAD FROM END OF ARM)	20.0+L	6.0+L
BRACKET MOUNTED (MAST ARM POLE OR SIGNAL POLE)	13.0	4.0
PEDESTRIAN PUSH BUTTON	6.0	2.0
SERVICE INSTALLATION POLE MOUNT TO SERVICE DROP	13.5	4.1
SERVICE INSTALLATION POLE MOUNT TO GROUND	13.5	4.1
SERVICE INSTALLATION GROUND MOUNT	6.0	2.0
FOUNDATION (SIGNAL POST, MAST ARM POLE, CONTROLLER CABINET, SERVICE-GROUND MOUNT)	3.0	1.0
The state of the s	220	

VERTICAL CABLE LENGTH

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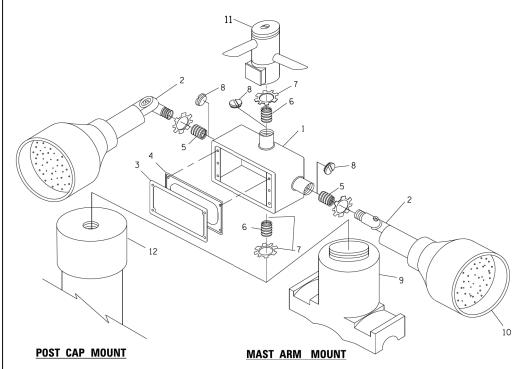
CABLE SLACK



NOTES:

- 1. CONDUIT DEPTH SHALL BE A MINIMUM OF 30" (760mm) BELOW THE BOTTOM OF THE DRAINAGE DITCH OR ANY SLOPING GROUND
- 2. THE MINIMUM CONDUIT DEPTH APPLIES TO ALL CONDUIT PLACED UNDER ROADWAY PAVEMENT, MULTI-USE PATHS, SIDEWALKS AND SOIL SURFACES.
- 3. THE MINIMUM CONDUIT DEPTH APPLIES TO ALL HANDHOLES, HEAVY DUTY HANDHOLES AND DOUBLE HANDHOLES.

HANDHOLE WITH MINIMUM CONDUIT DEPTH



EMERGENCY VEHICLE DETECTOR WITH CONFIRMATION BEACON MOUNTING DETAIL

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	PLOT DATE = 1/13/2014	DATE	-	1/25/2025	REVISED	_		

(1675mm) (915mm) 40.75" 19.875" (136mm) (1035mm) (504mm) -CONTROLLER CABINET BASE PROPOSED -APRON **TOP VIEW** (NOT TO SCALE) NO. 6 BARE COPPER WIRE _ NO. 3 DOWEL 18" (450mm) LONG (8 REQ.) BUSHING -_GROUND CLAMP / ANCHOR BOLTS BEVEL -EXISTING CONDUITS XISTING GROUND ROD MODIFY EXISTING TYPE "D" FOUNDATION TO TYPE "C" FOUNDATION (NOT TO SCALE)

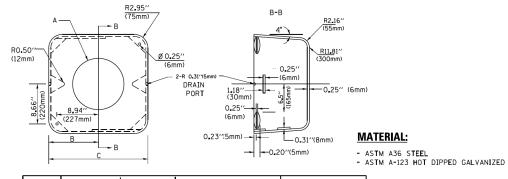
ITEM NO. IDENTIFICATION 1 OUTLET BOX- GALV. 21 CU.IN. (0.000344 CU-M) 2 LAMP HOLDER AND COVER 3 OUTLET BOX COVER 4 RUBBER COVER GASKET 5 REDUCING BUSHING 6 ¾4"(19 mm) CLOSE NIPPLE 7 ¾"(19 mm) LOCKNUT 8 ¾4"(19 mm) HOLE PLUG 9 SADDLE BRACKET - GALV. 10 6 WATT PAR 38 LED FLOOD LAMP 11 DETECTOR UNIT 12 POST CAP [18 FT. (5.4 m) POST MIN.]

NOTES:

- ALL ELECTRICAL ITEMS, EXCEPT ITEMS *2 AND *11 SHALL BE ALUMINUM OR GALVANIZED
- 2. ITEM #1- OZ/GEDNEY FSX-1-50 OR EQUIVALENT
 ITEM #2- MULBERRY CON-O-SHADE LAMP SHIELD OR EQUIVALENT
 ITEM #9- "BAND-IT" SADDLE BRACKET OR EQUIVALENT
- 3. WHEN POST MOUNTING IS SPECIFIED, ITEM *9 SHALL NOT BE REQUIRED. THE DETECTION UNIT SHALL BE MOUNTED DIRECTLY ON TOP OF THE CAP BY DRILLING AND TAPPING A 3/4 "(19 mm) HOLE WITH PIPE THREADS. THE POST CAP SHALL EITHER BE SCREWED TO THE TOP OF THE POST OR A MINIMUM OF 3 TIGHTENING SCREWS SHALL BE REQUIRED ON EACH CAP.

STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

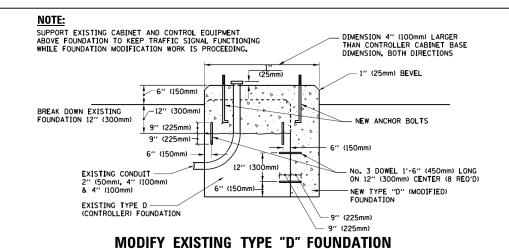


Α	В	С	HEIGHT	WEIGHT
VARIES	9 . 5′′(241mm)	19''(483mm)	7" (178mm) - 12" (300mm)	53 lbs (24kg)
VARIES	10.75"(273mm)	21.5"(546mm)	7" (178mm) - 12" (300mm)	68 lbs (31 kg)
VARIES	13.0"(330mm)	26''(660mm)	7" (178mm) - 12" (300mm)	81 lbs (37 kg)
VARIES	18.5''(470mm)	37''(940mm)	7" (178mm) - 12" (300mm)	126 lbs (57 kg)

SHROUD

NOTES:

- DIMENSION "A" IS EQUAL TO THE DIAMETER OF THE MAST ARM POLE AT THE TOP OF THE SHROUD.
 THE SHROUD SHALL BE TIGHT TO THE MAST ARM POLE.
- 2. THE SUPPLIER SHALL VERIFIED THE ABOVE DIMENSIONS BASED ON MAST ARM REQUIREMENTS.
- 3. THE HEIGHT OF THE SHROUD SHALL COVER THE ANCHOR BOLTS, NUTS AND MAST ARM POLE BASE.

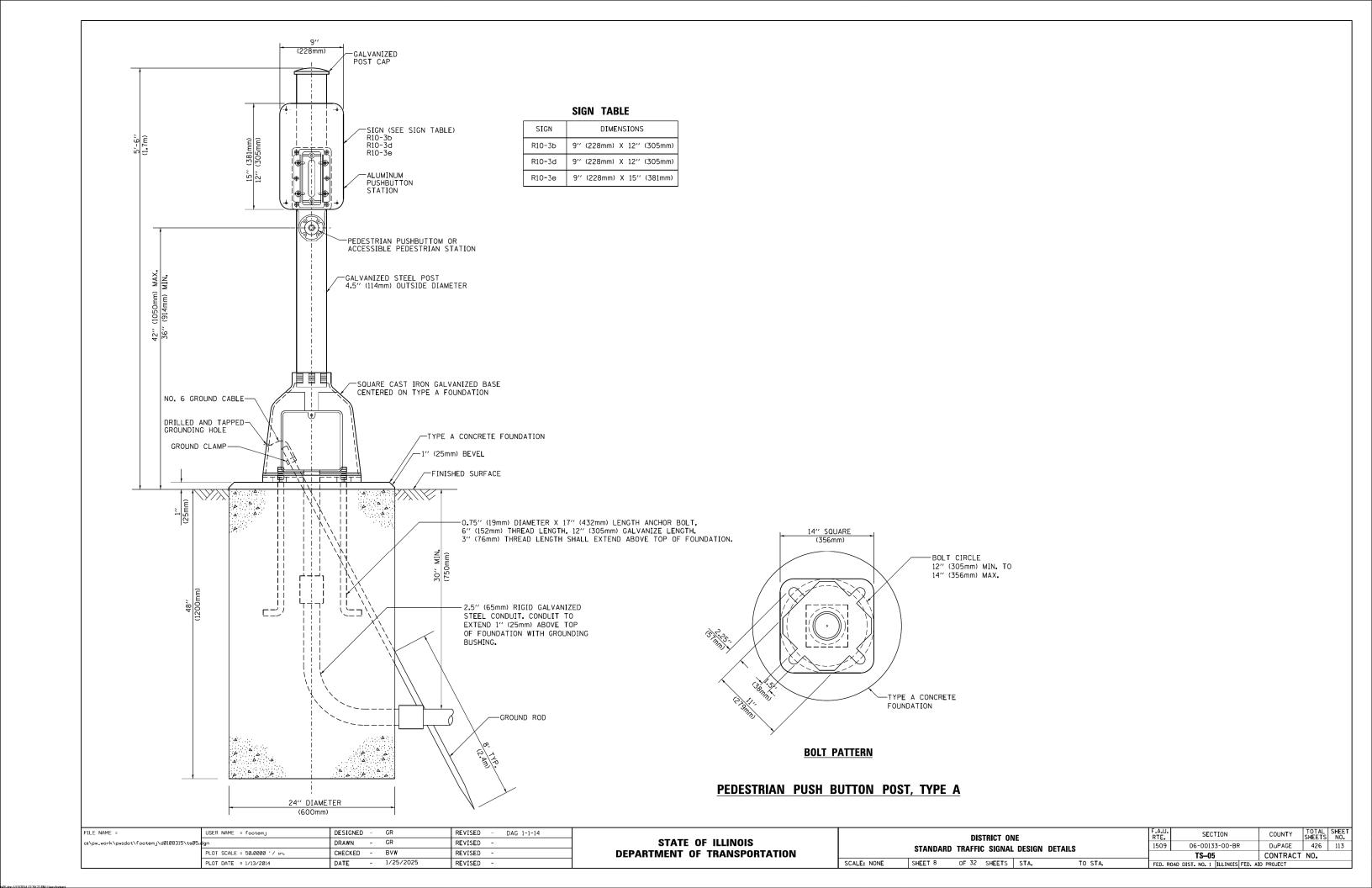


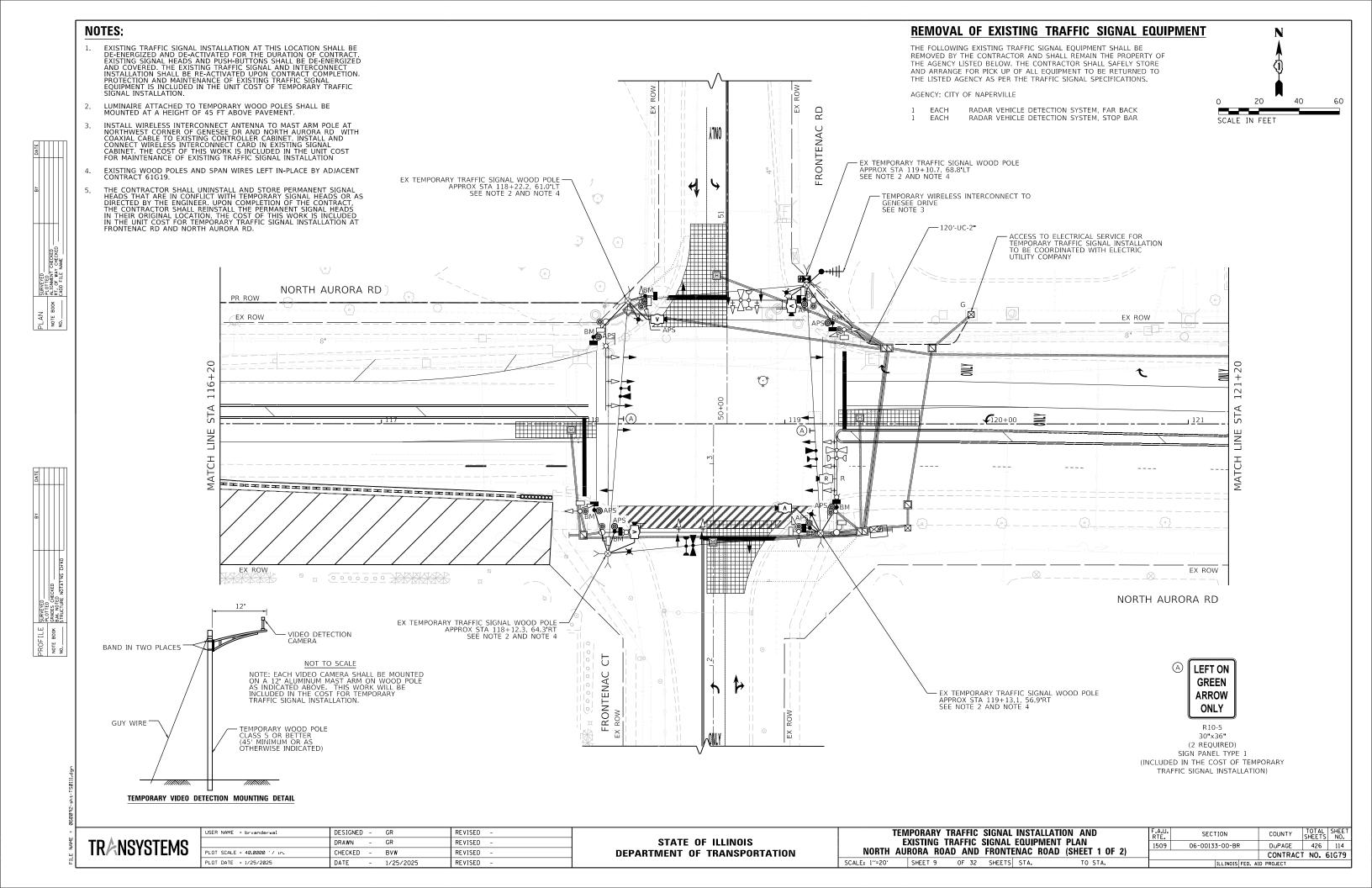
GALVANIZED STEEL HOOKS 21 1/2 MIN, (545mm) CONDUIT BUSHING EXISTING CONDUIT TO BE REMOVED EXISTING CONDUIT TO REMAIN EXISTING CONDUIT TO REMAIN EXISTING CONDUIT TO REMAIN EXISTING CONDUIT TO REMAIN PLAN

NOTES:

- 1. HANDHOLE CONSTRUCTED PER STATE STANDARD 814001.
- 2. REMOVAL OF THE EXISTING CONDUIT FROM THE HANDHOLE AND THE INSTALLATION OF THE CONDUIT BUSHINGS SHALL BE INCLUDED WITH THE COST OF THE HANDHOLE.

HANDHOLE TO INTERCEPT EXISTING CONDUIT





 PLAN
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NORTH AURORA RD

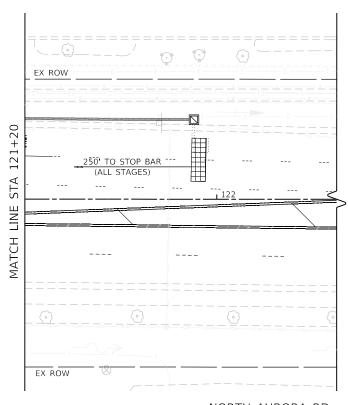
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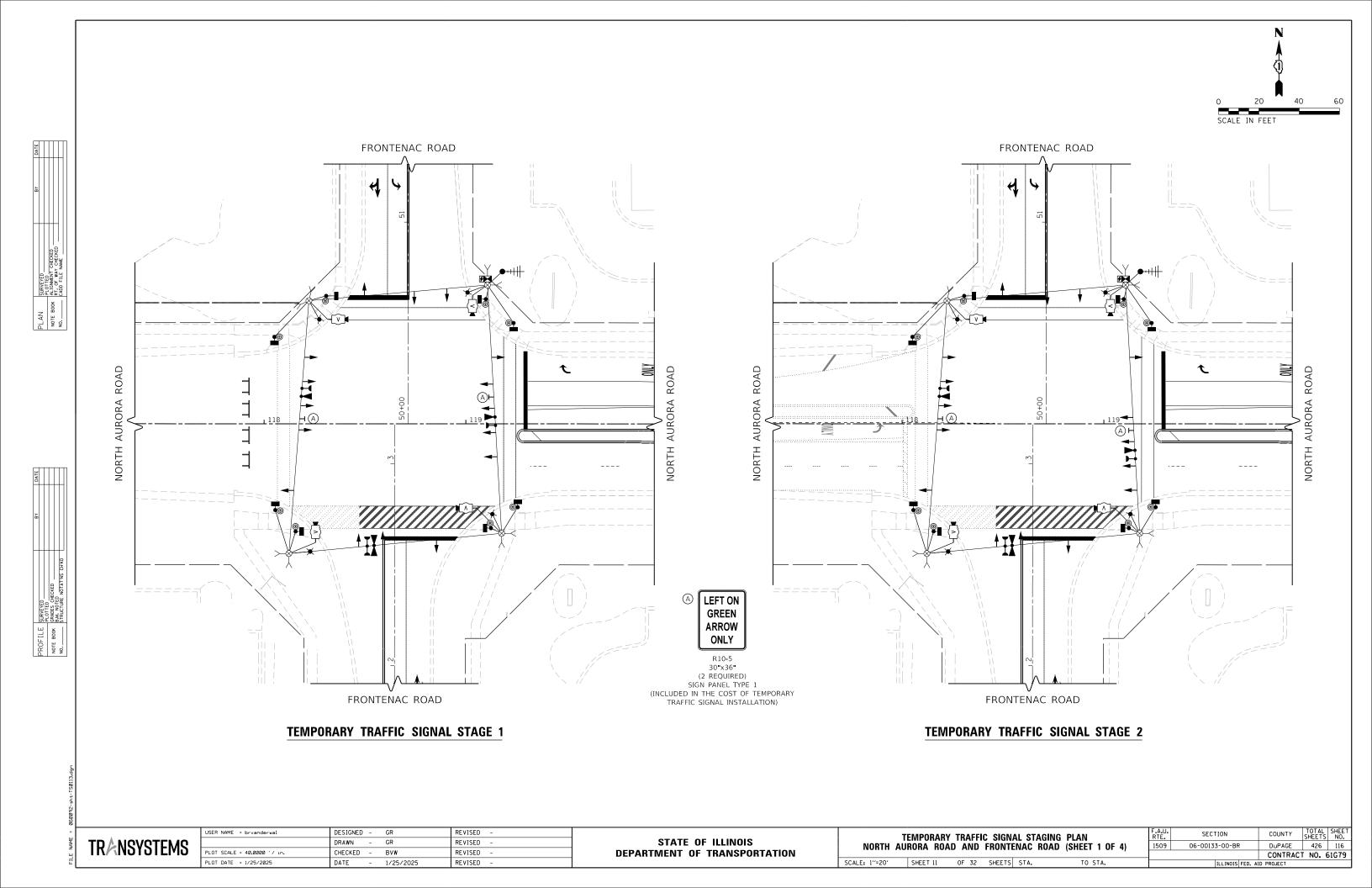
NORTH AURORA RD

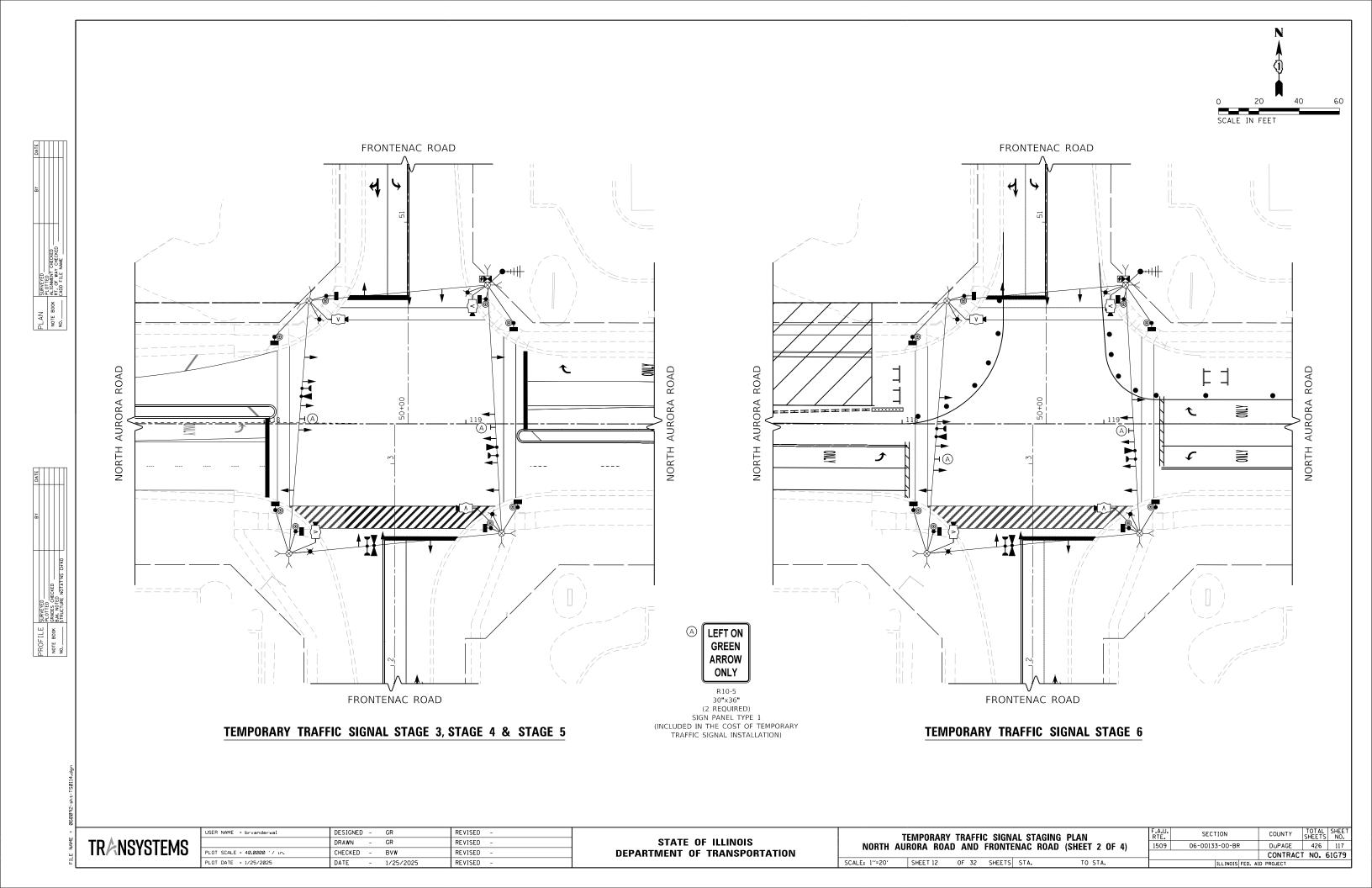
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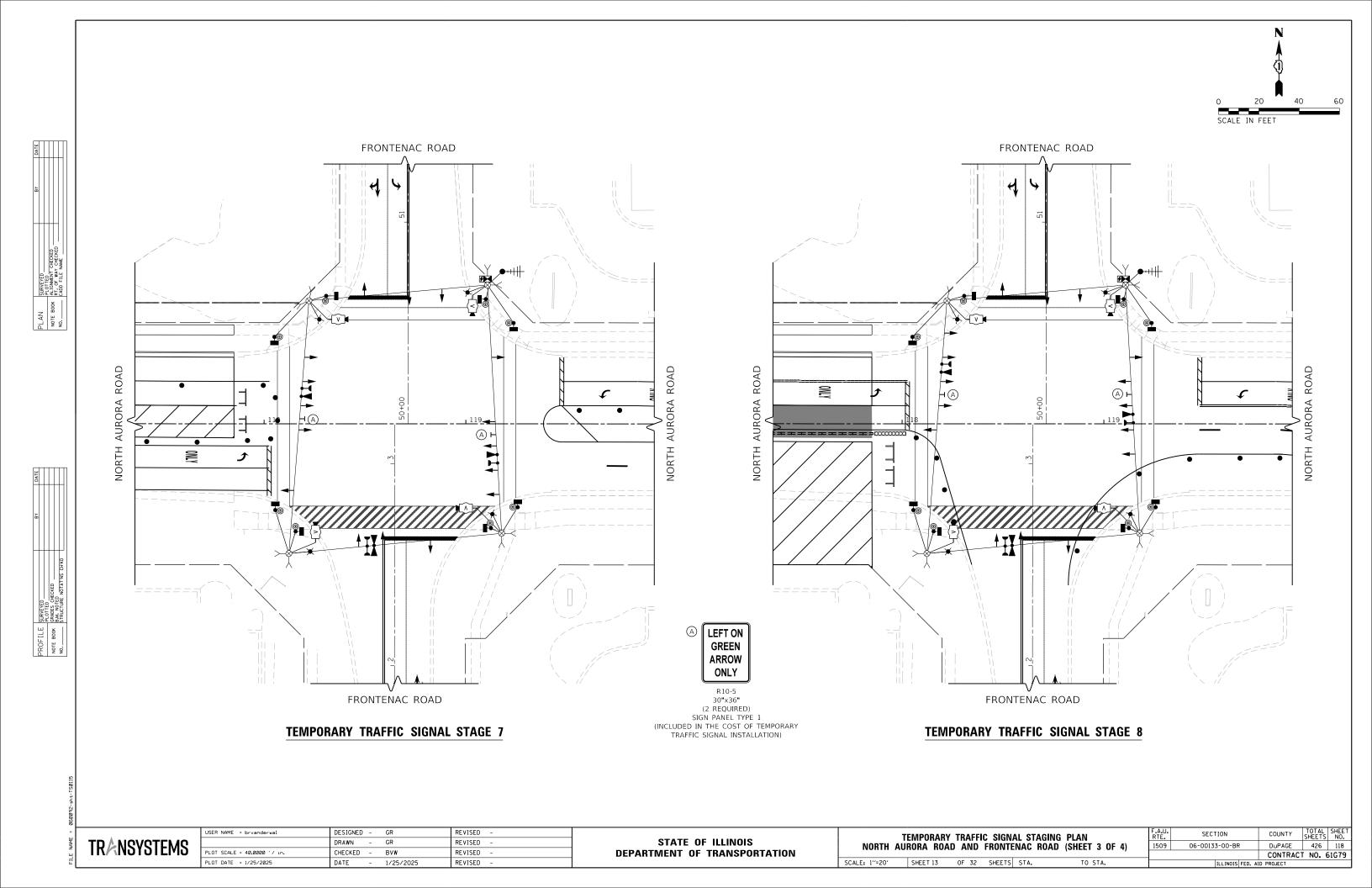
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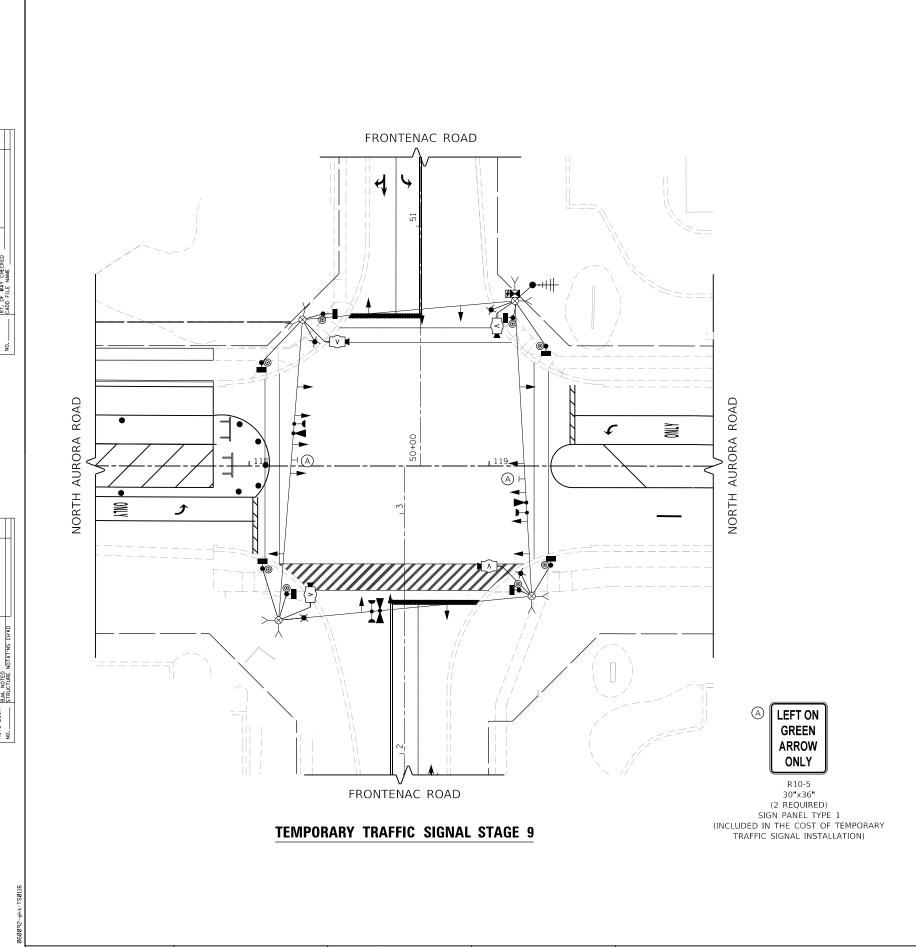
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ALE: 1"=20"		SHEET	10	OF	32	SHEETS	STA.		TO STA.	

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TRANSYSTEMS

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

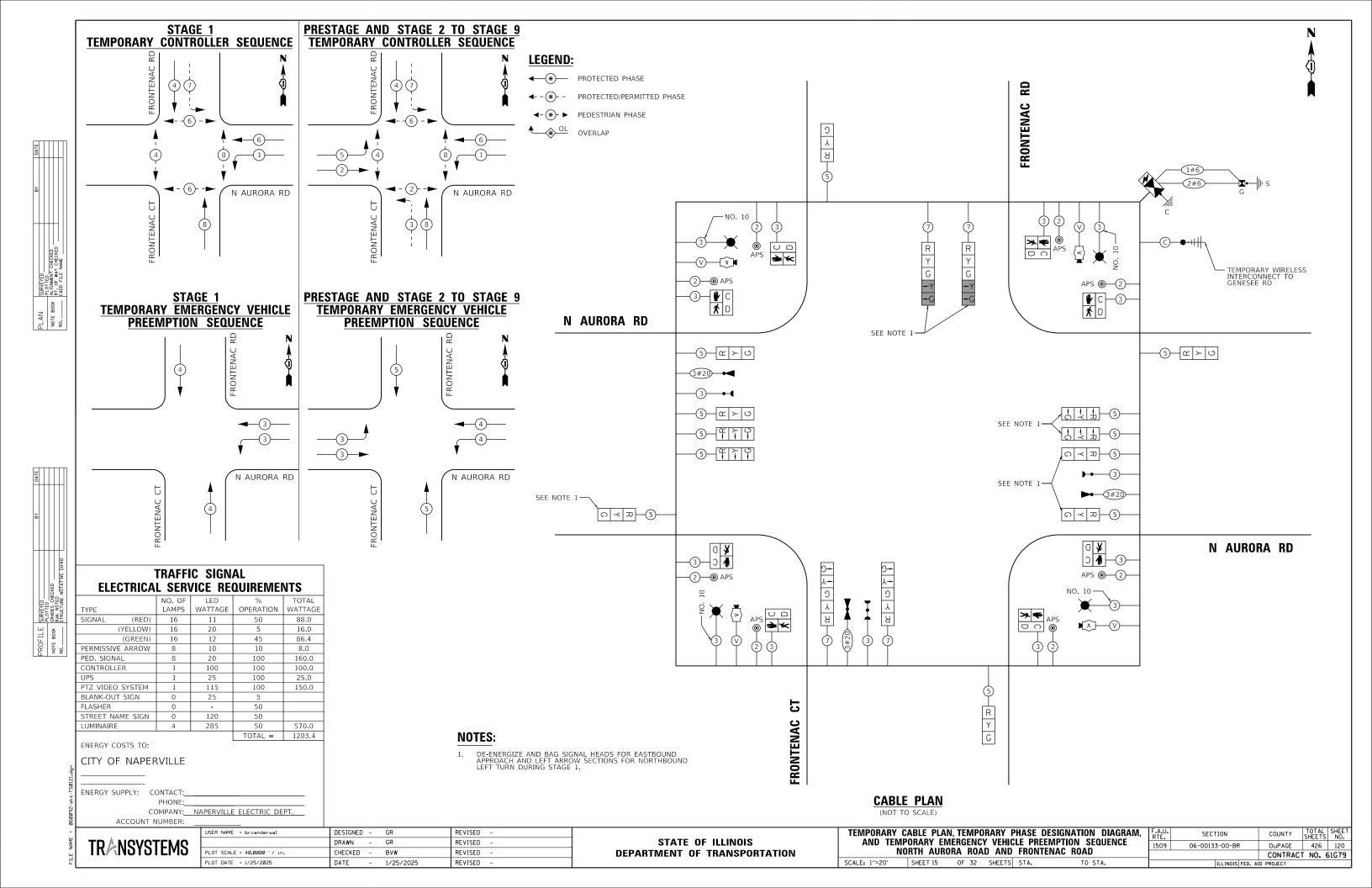
TEMPORARY TRAFFIC SIGNAL STAGING PLAN NORTH AURORA ROAD AND FRONTENAC ROAD (SHEET 4 OF 4)

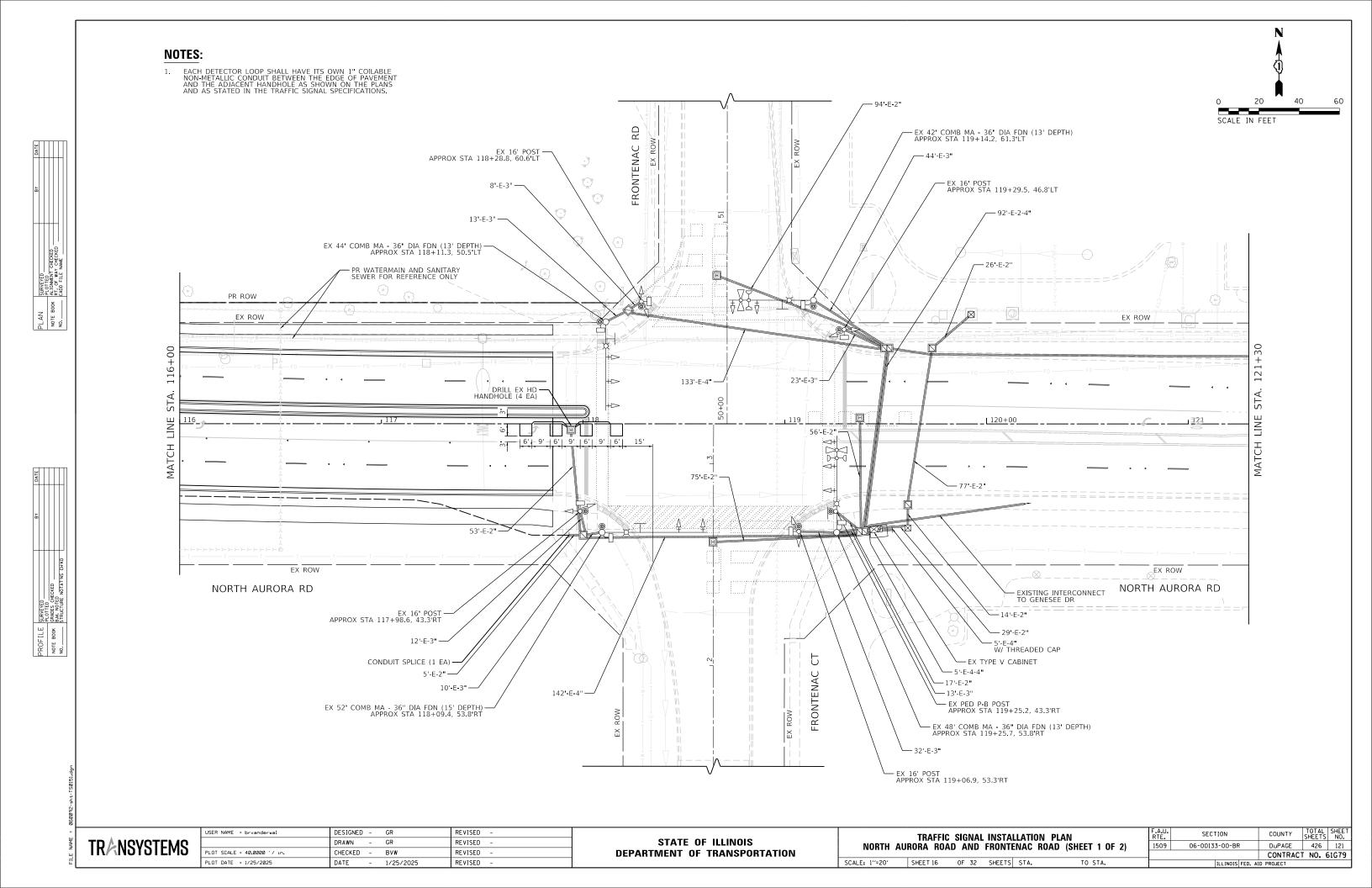
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F.A.U. RTE. SECTION COUNTY SHEETS NO. 1509 06-00133-00-BR DUPAGE 426 119

CONTRACT NO. 61G79

| ILLINOIS | FED. AID | PROJECT |

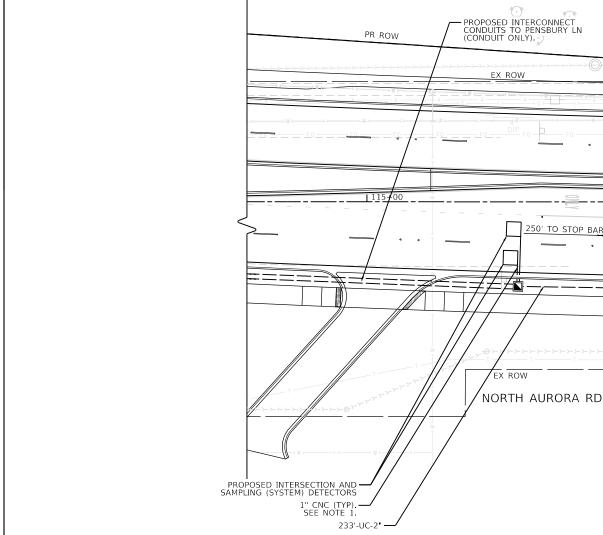


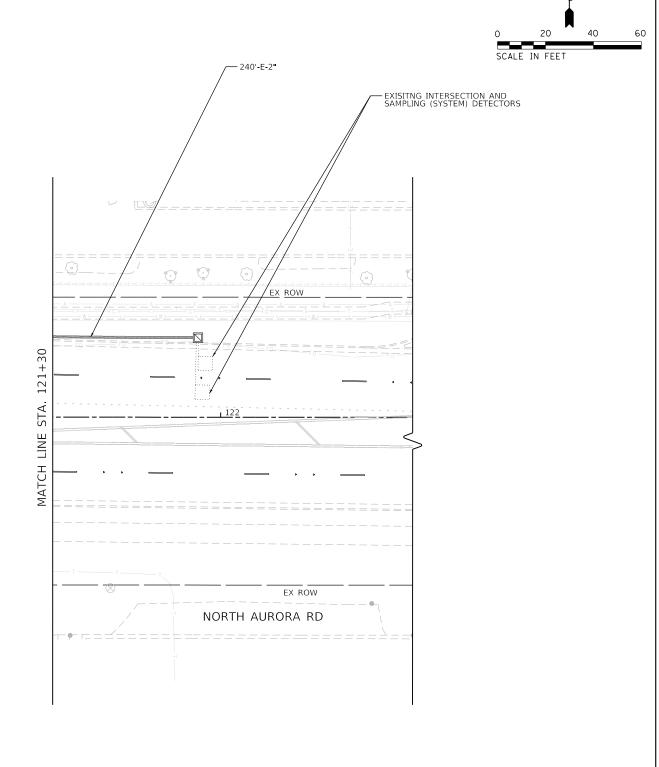


NOTES:

EACH DETECTOR LOOP SHALL HAVE ITS OWN 1" COILABLE NON-METALLIC CONDUIT BETWEEN THE EDGE OF PAVEMENT AND THE ADJACENT HANDHOLE AS SHOWN ON THE PLANS AND AS STATED IN THE TRAFFIC SIGNAL SPECIFICATIONS.





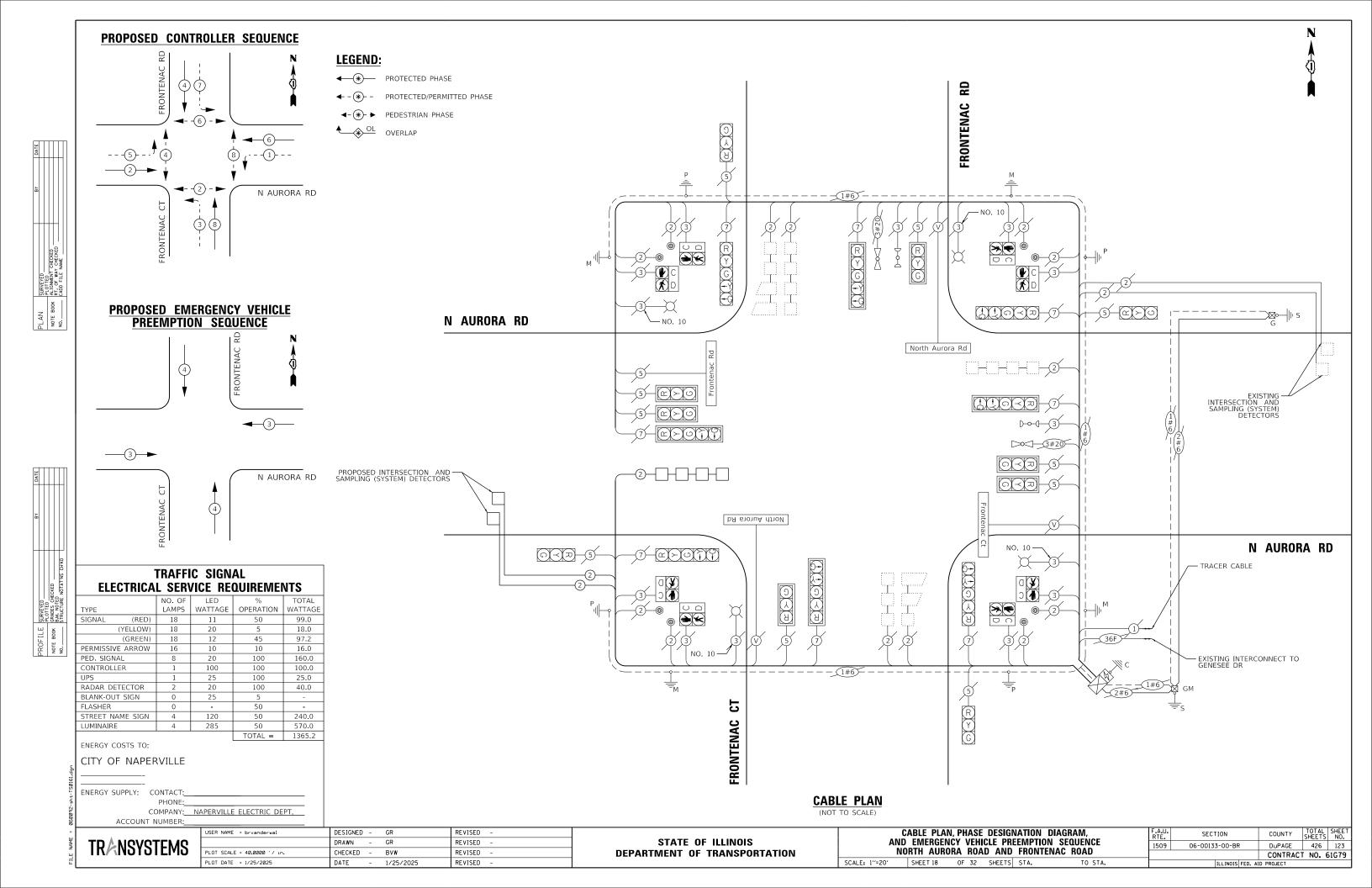


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TRAFFIC SIGNAL INSTALLATION PLAN								F.
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				CONTR	4C1	NO. 6	51G7
		ILLINOIS	FED. Al	D PROJECT			



SCHEDULE OF QUANTITIES

PAY ITEM NAME	UNIT	QUANTITY
ELECTRIC UTILITY SERVICE CONNECTION	LSUM	0.50
UNDERGROUND CONDUIT, GALVANIZED STEEL, 2" DIA.	FOOT	233
HANDHOLE	EACH	1
ELECTRIC CABLE IN CONDUIT, LEAD-IN, NO. 14 1 PAIR	FOOT	1,070
DRILL EXISTING HEAVY DUTY HANDHOLE	EACH	4
NDUCTIVE LOOP DETECTOR	EACH	3
DETECTOR LOOP, TYPE I	FOOT	216
TEMPORARY TRAFFIC SIGNAL INSTALLATION	EACH	1
MODIFY EXISTING CONTROLLER CABINET	EACH	1
REMOVE EXISTING TRAFFIC SIGNAL EQUIPMENT	EACH	1
CONDUIT SPLICE	EACH	1
RE-OPTIMIZE TRAFFIC SIGNAL SYSTEM LEVEL 1	EACH	1
TEMPORARY TRAFFIC SIGNAL TIMING	EACH	1

		В	DATE
PLAN	SURVEYED		
	PLOTTED		
NOTE BOOK	ALIGNMENT CHECKED		
	RT. OF WAY CHECKED		
0	CADD FILE NAME		

FILE SURVEYED PLOTTED PROOF GRADES CHECKED
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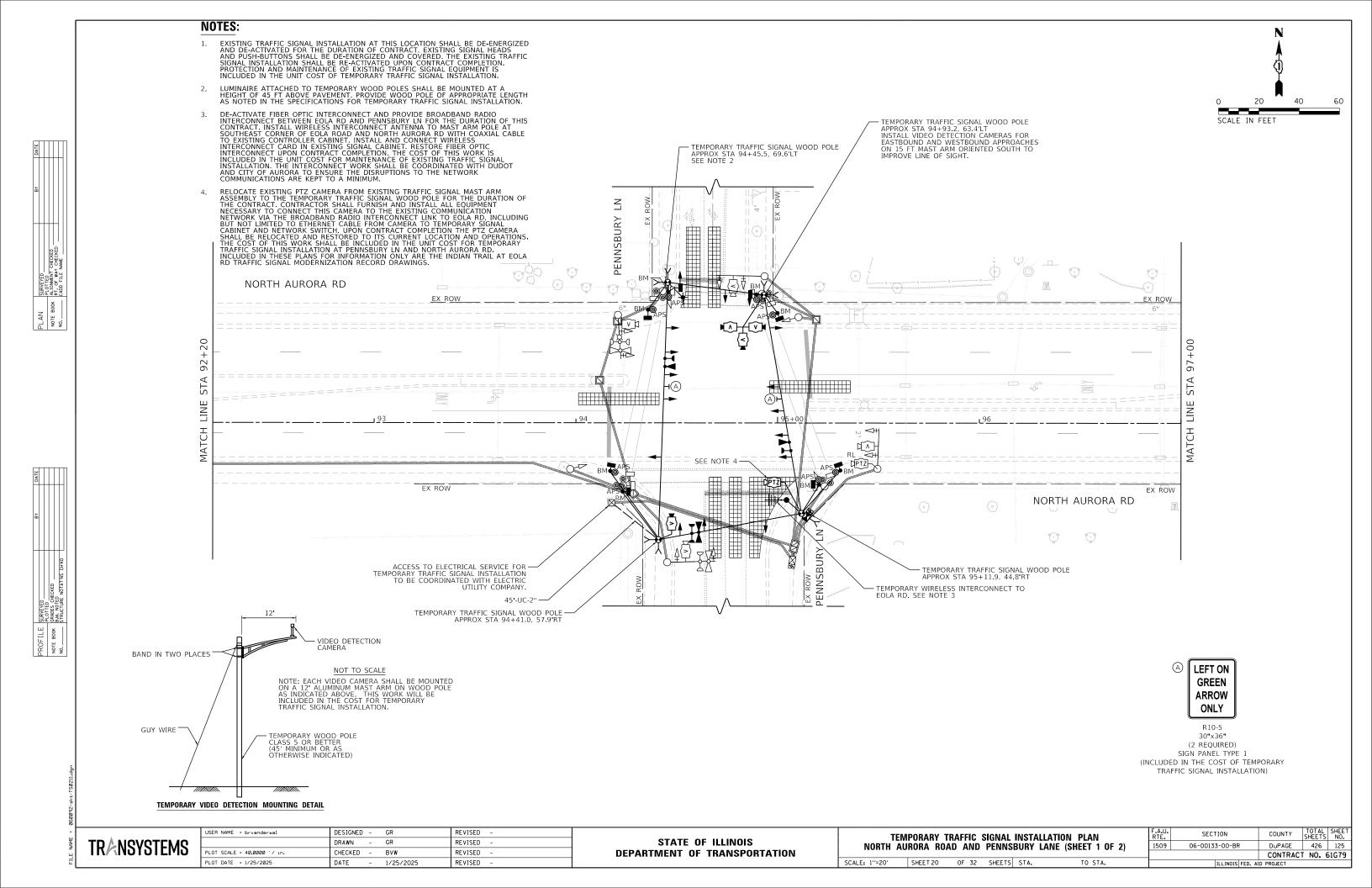
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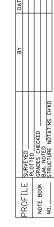
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	DRAWN	-	GR	REVISED -
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PLOT DATE = 1/25/2025	DATE	_	1/25/2025	REVISED -

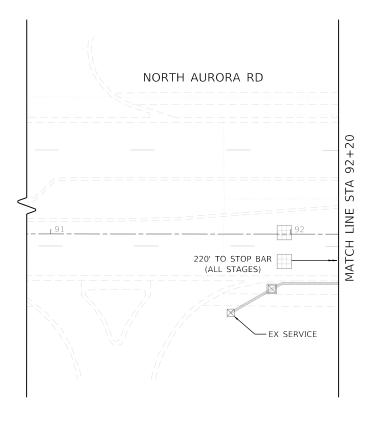
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

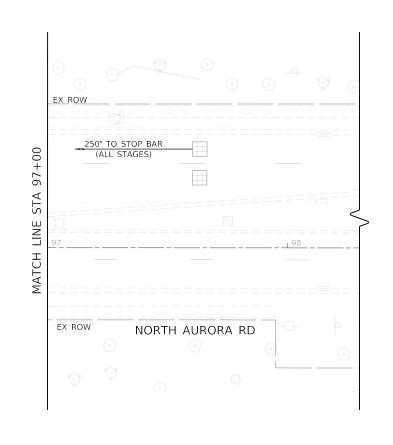
SCHEDULE OF QUANTITIES NORTH AURORA ROAD AND FRONTENAC ROAD						
SCALE: 1"=20"	SHEET 19	OF 32	SHEETS	STA.	TO STA.	

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE1		
1509	06-00133-00-BF	DuPAGE	426	124		
		CONTRACT	NO. 6	31G79		
ILLINOIS FED. AID PROJECT						







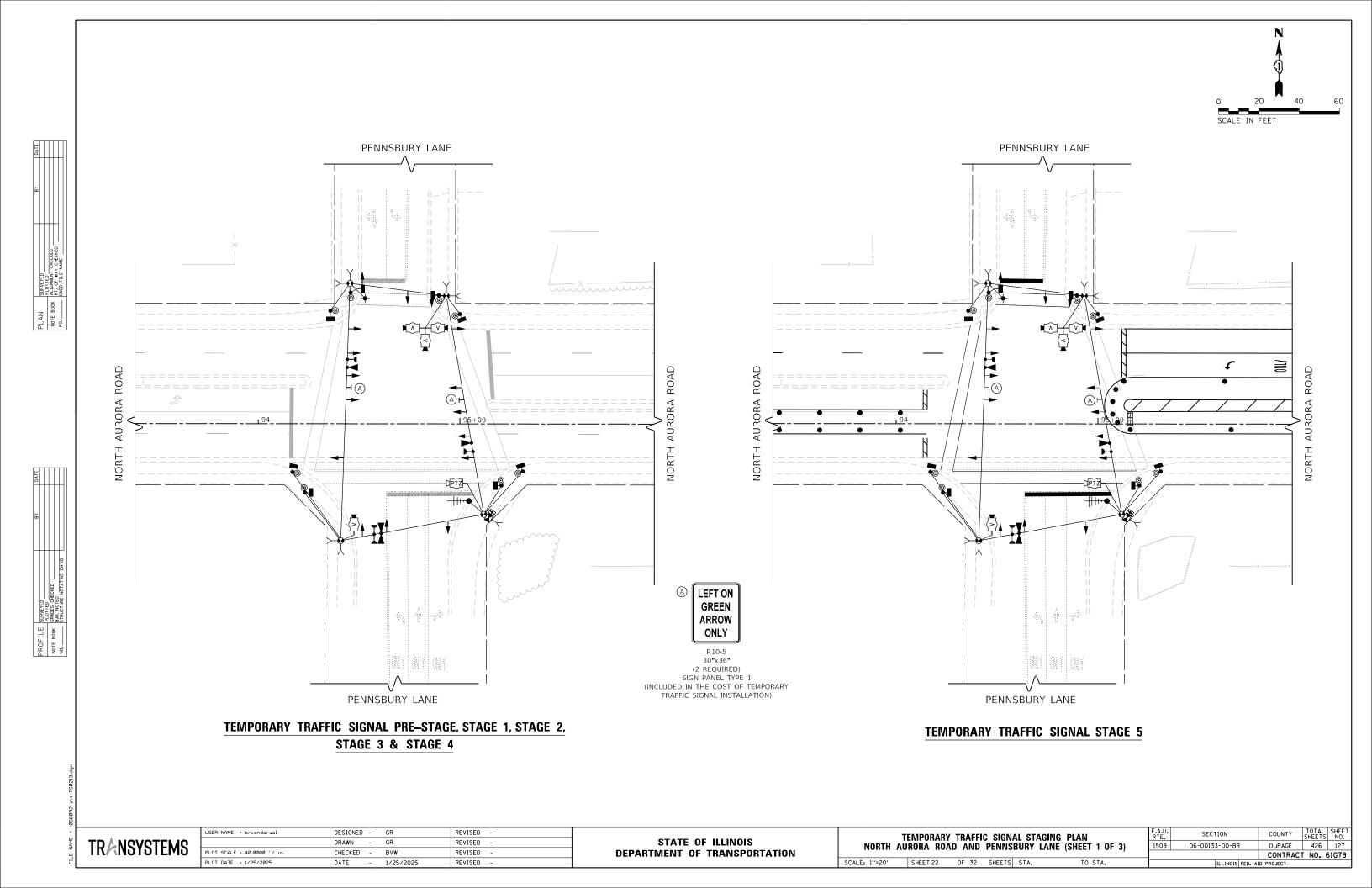


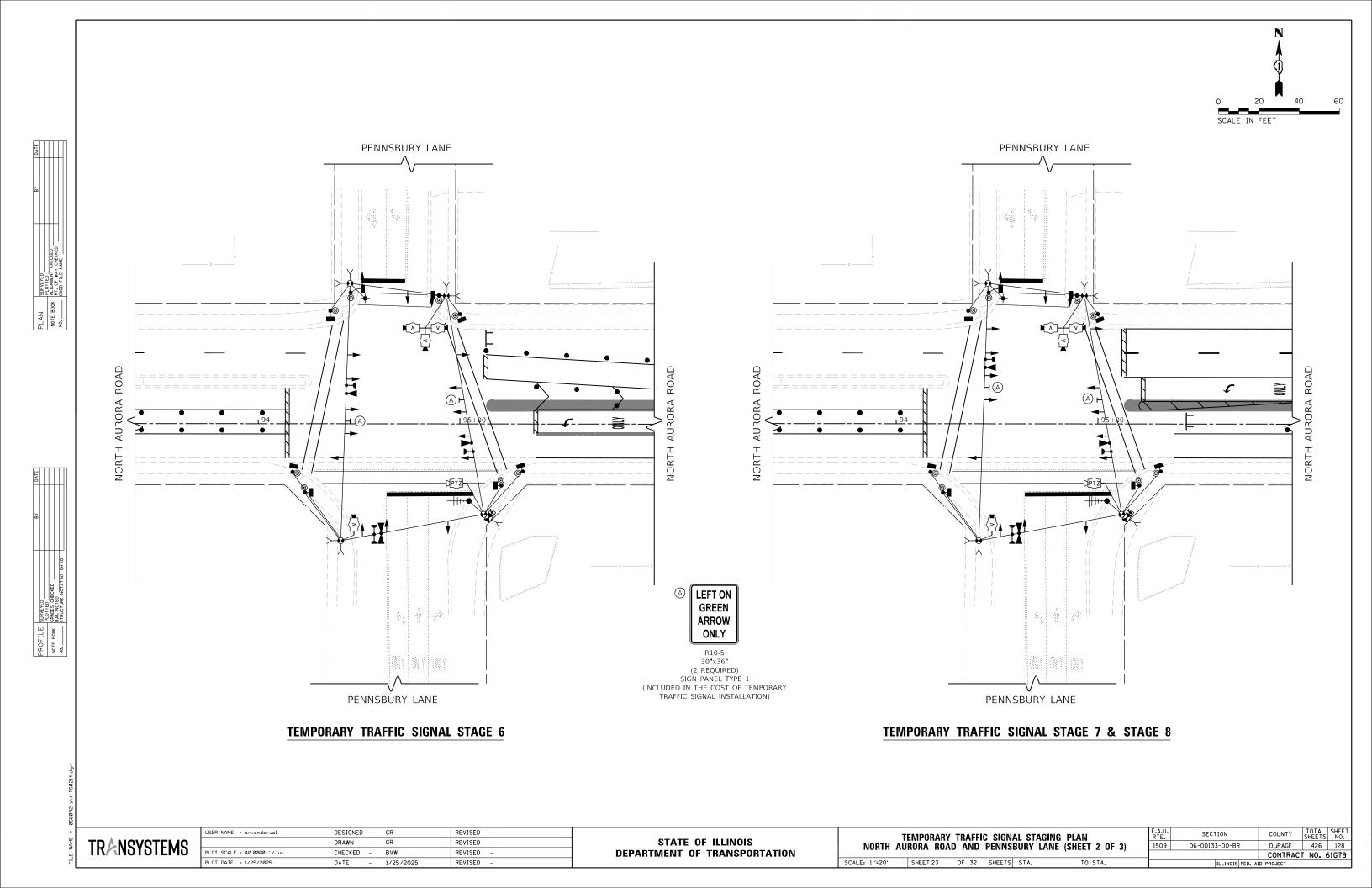
USER NAME = bryanderwal	DESIGNED	_	GR	REVISED -
	DRAWN	_	GR	
	DRAWN	_	GR	REVISED -
PLOT SCALE = 40.0000 '/ in.	CHECKED	-	BVW	REVISED -
PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

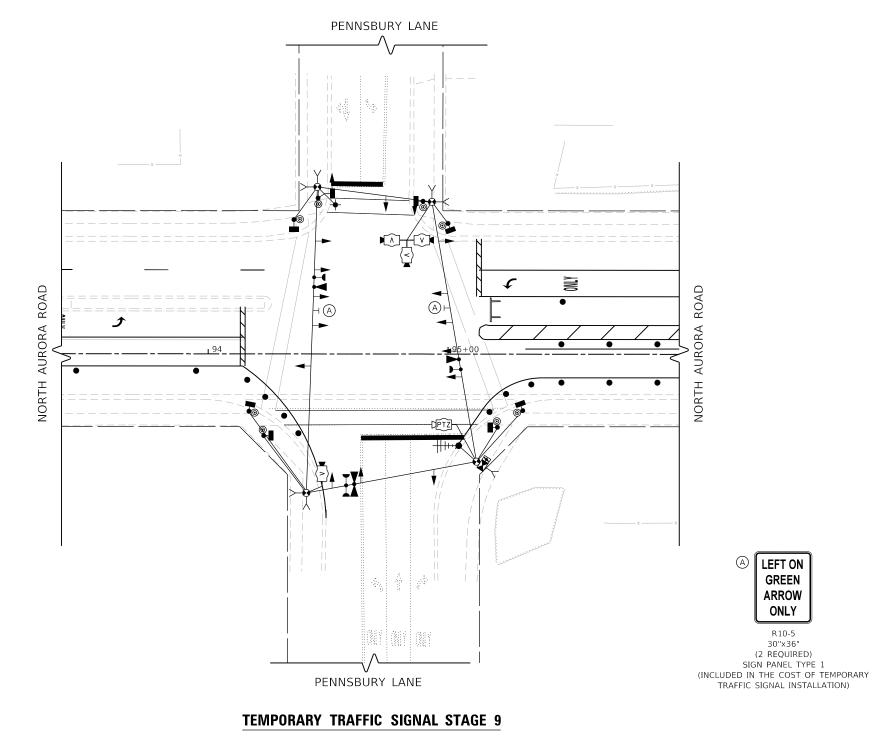
				ALLATION PLAN LANE (SHEET 2 OF 2)
SCALE: 1"=20"	SHEET 21	0F 32	SHEETS STA.	TO STA.

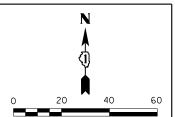
F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEE NO.
1509	06-00133-00-BR	DuPAGE	426	126
		CONTRACT	NO. (51G79
	ILLINOIS FED.	AID PROJECT		











STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

TEMPORARY TRAFFIC SIGNAL STAGING PLAN AURORA ROAD AND PENNSBURY LANE (SHEET 3 OF 3)

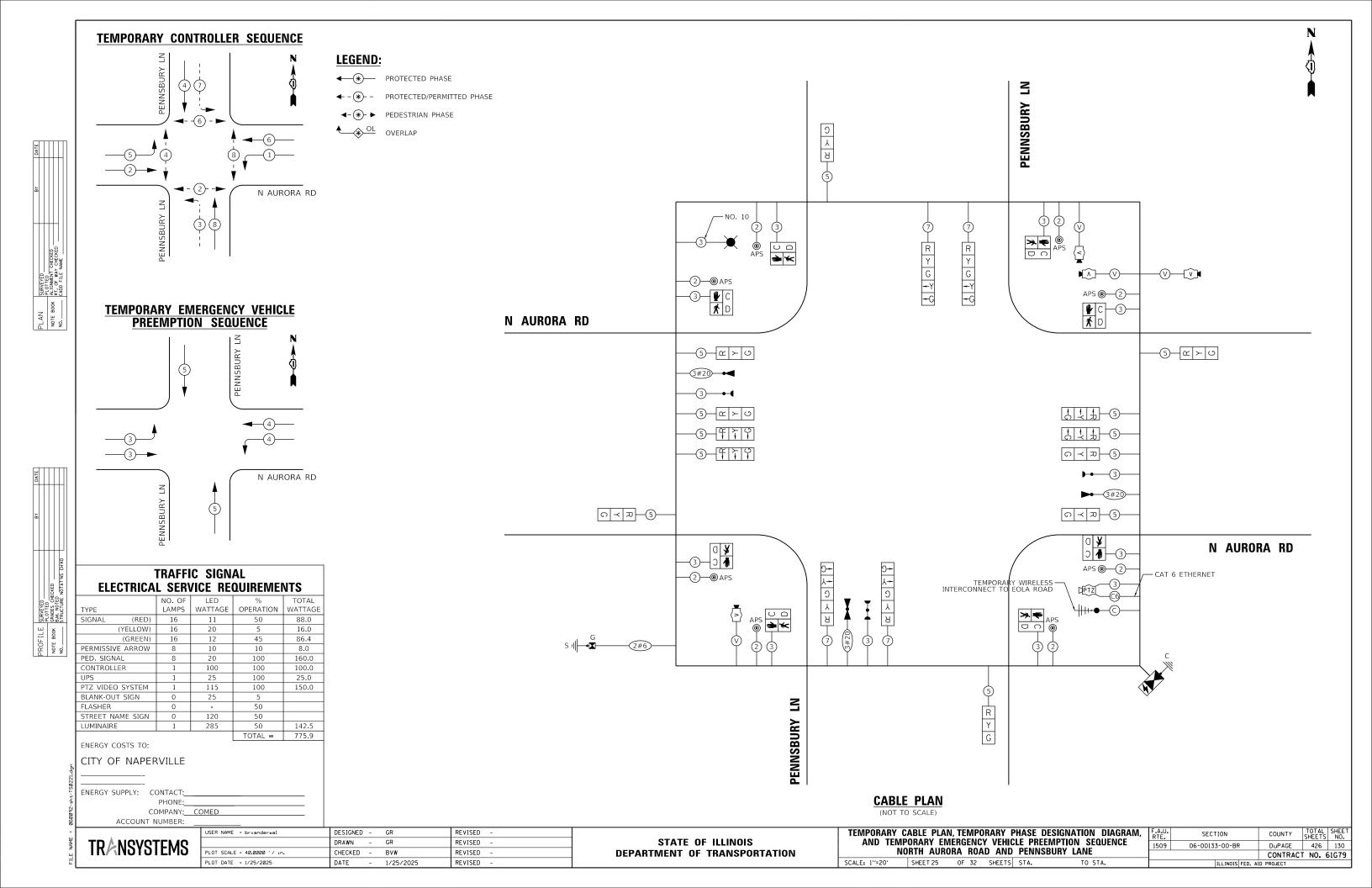
SCALE: 1"=20" SHEET 24 OF 32 SHEETS STA. TO STA.

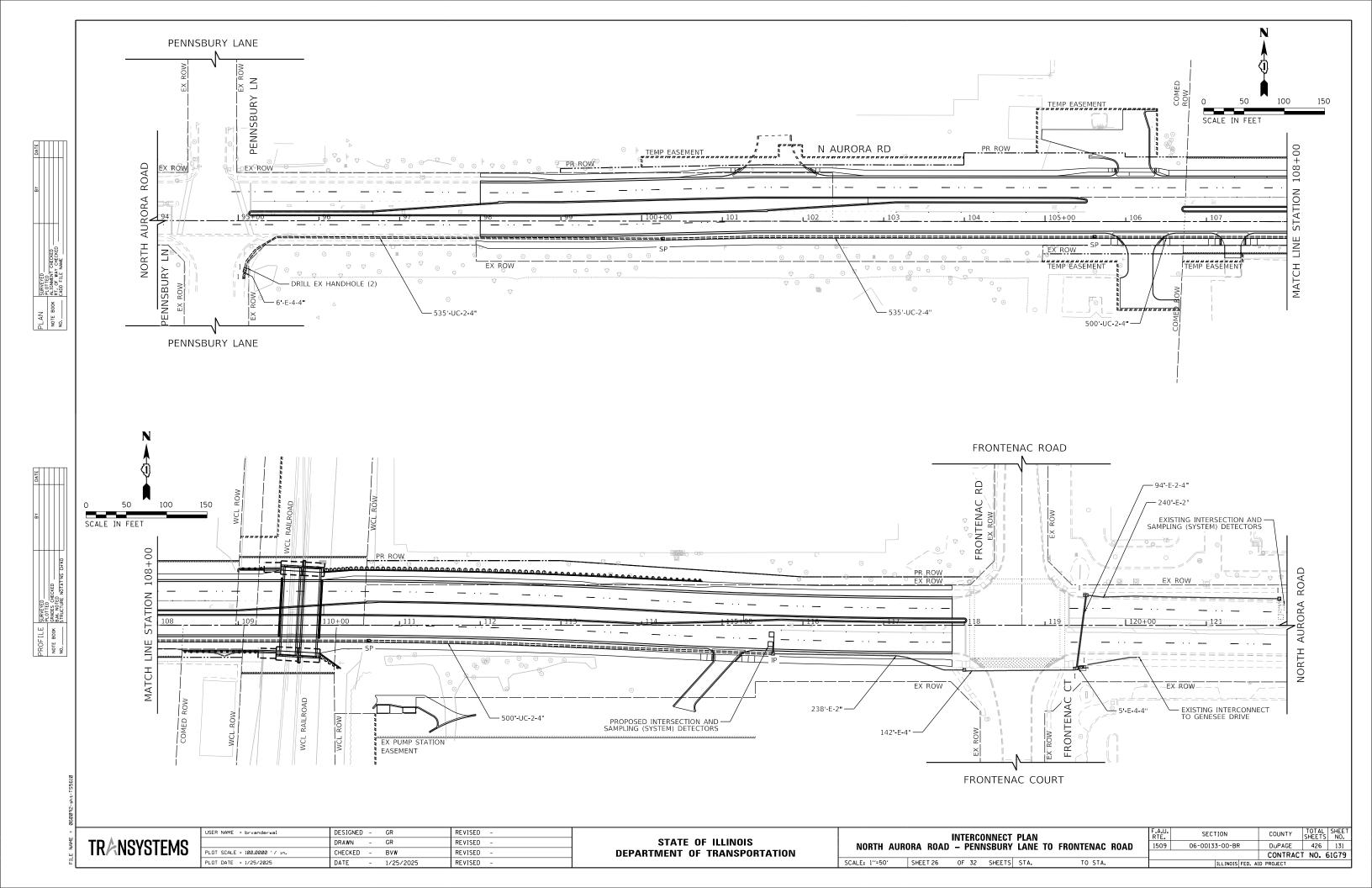
F.A.U. RTE. SECTION COUNTY SHEETS NO.

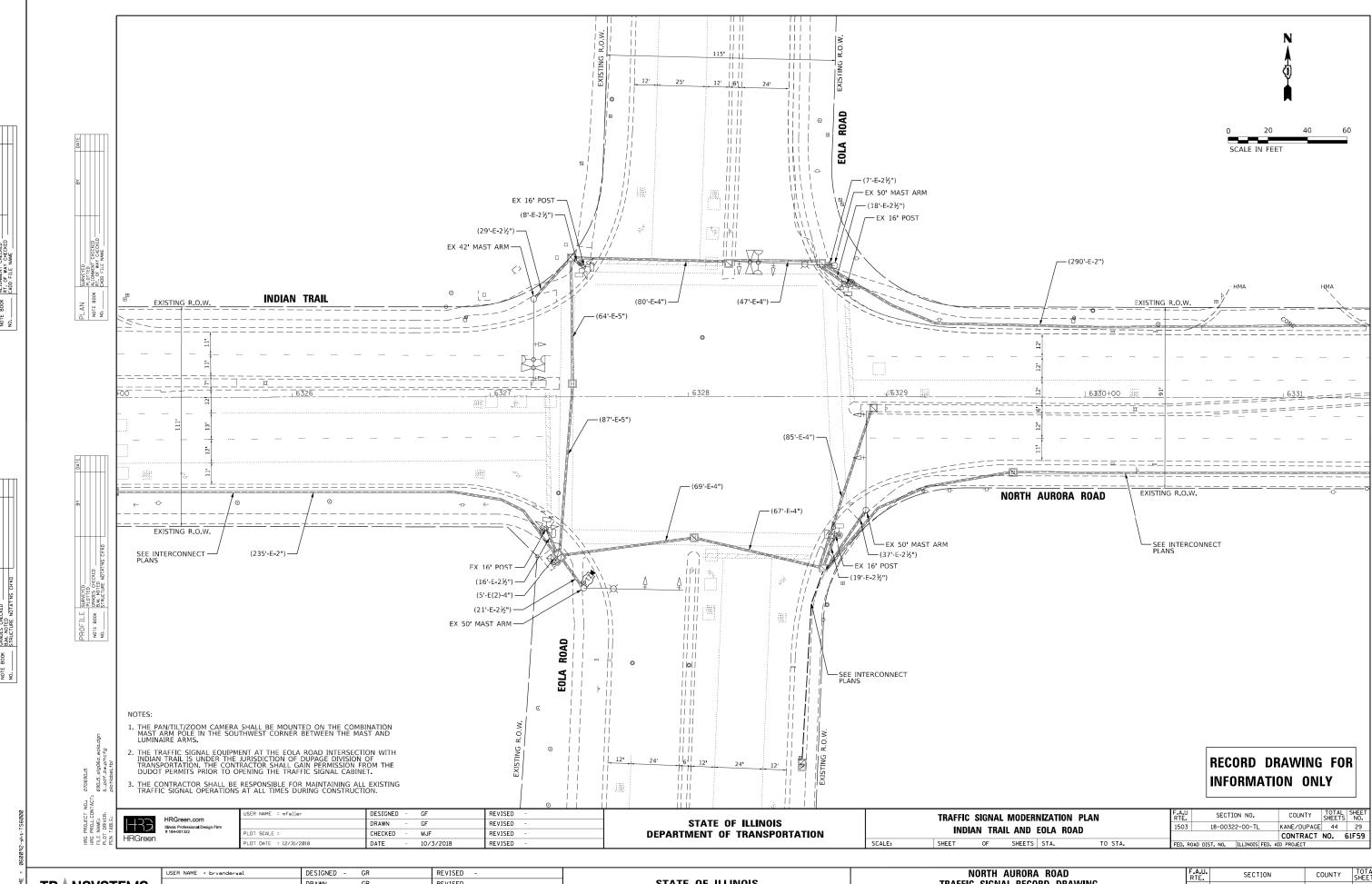
1509 06-00133-00-BR DUPAGE 426 129

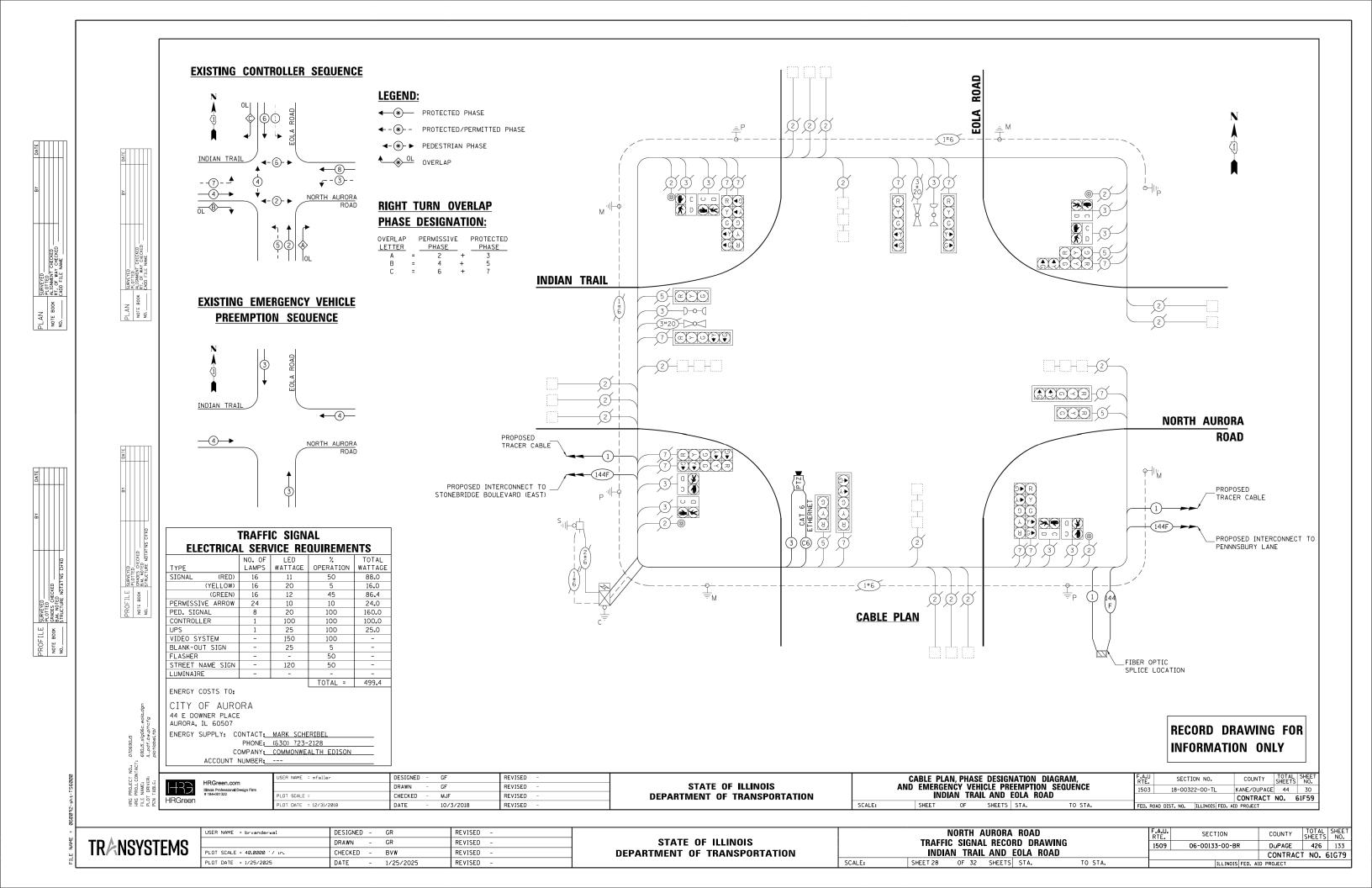
CONTRACT NO. 61679

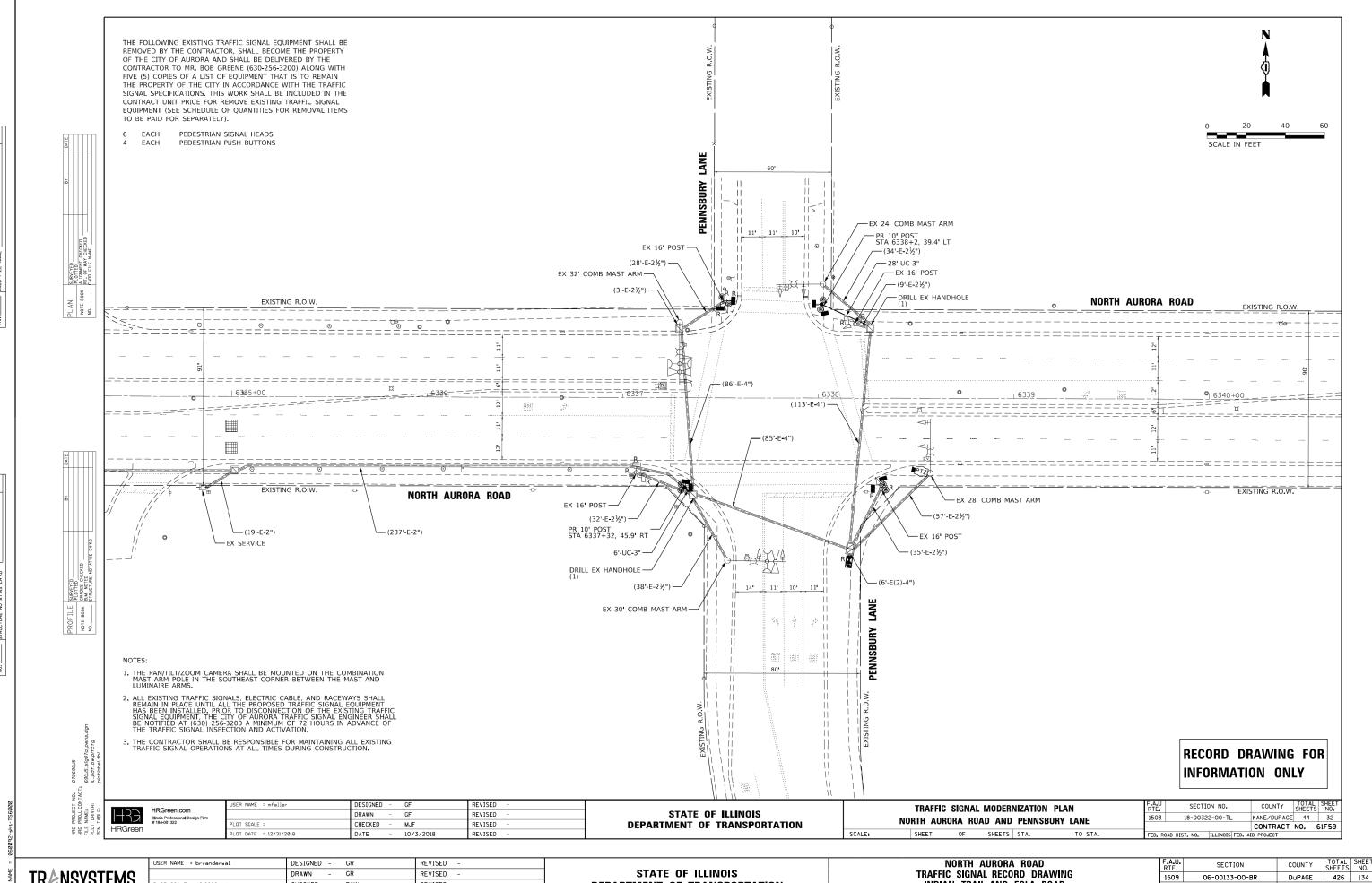
| ILLINOIS FED. AID PROJECT



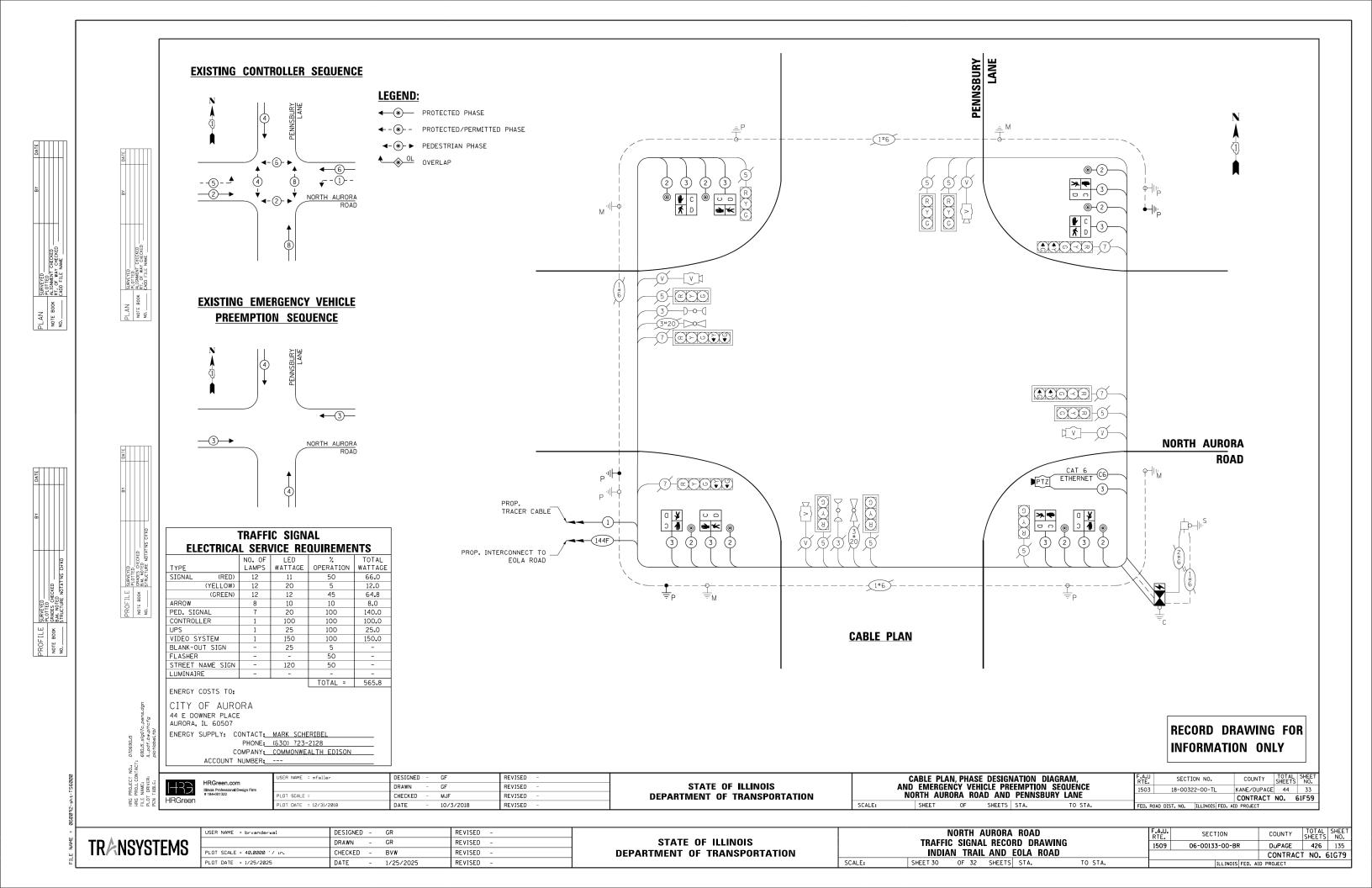


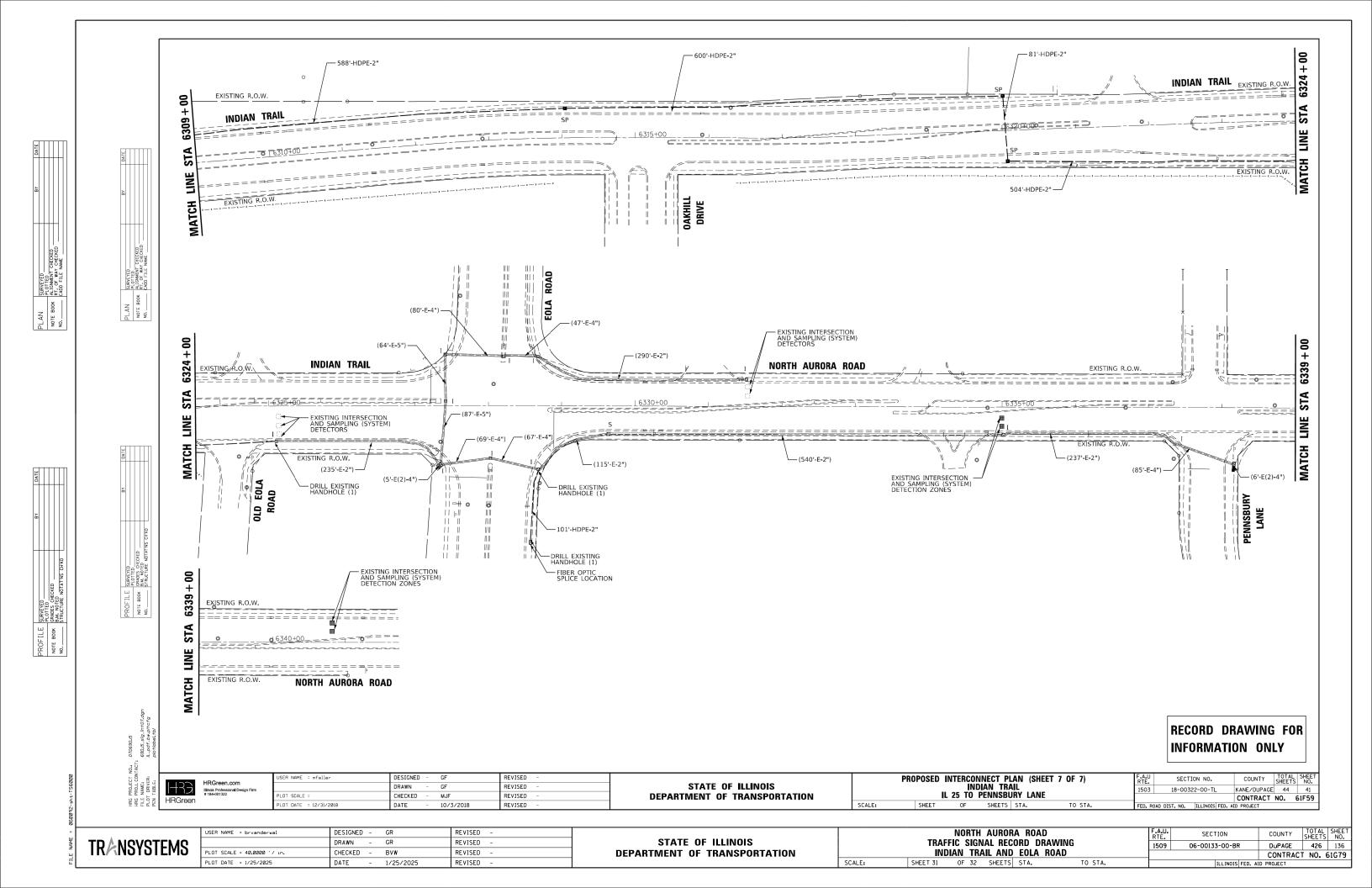


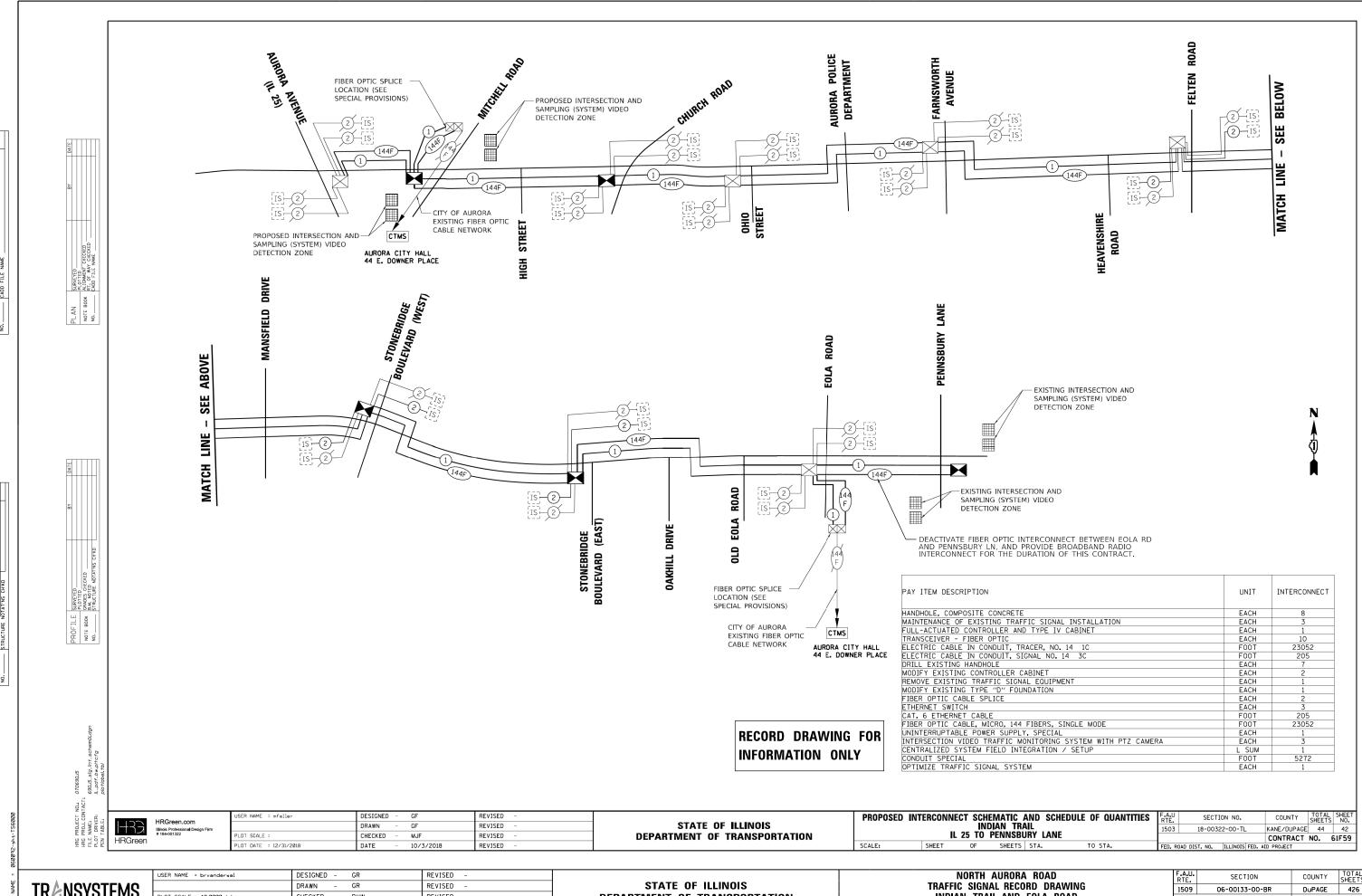




1509 06-00133-00-BR CHECKED -BVW REVISED **DEPARTMENT OF TRANSPORTATION** INDIAN TRAIL AND EOLA ROAD CONTRACT NO. 61G79 SHEET 29 OF 32 SHEETS STA. REVISED 1/25/2025







TRANSYSTEMS

LOT SCALE = 40.0000 '/ in. CHECKED BVW REVISED PLOT DATE = 1/25/2025 DATE REVISED 1/25/2025

DEPARTMENT OF TRANSPORTATION

INDIAN TRAIL AND EOLA ROAD SHEET 32 OF 32 SHEETS STA. TO STA.

SCALE:

DuPAGE 426 137 CONTRACT NO. 61G79

SYMBOL LIST

	EXISTING COMBINATION LIGHTING - TRAFFIC SIGNAL POLE
O—E	EXISTING LIGHT POLE TO REMAIN
	EXISTING AURORA LIGHTING UNIT TO BE REMOVED AND REINSTALLED AT DIFFERENT LOCATION
R -0-	EXISTING AURORA LIGHTING UNIT REINSTALLED ON CONCRETE FOUNDATION WITH PROPOSED LUMINAIRE, LED HORIZONTAL MOUNT, OUTPUT DESIGNATION H

PROPOSED NAPERVILLE LIGHTING UNIT, ON HELIX FOUNDATION, ALUMINUM, 40' M.H., 12' M.A. AND LUMINAIRE, LED, HORIZONTAL MOUNT, OUTPUT DESIGNATION G

PROPOSED AURORA LIGHTING UNIT, ON CONCRETE FOUNDATION,
40' M.H., TWIN 12' M.A. AND LUMINAIRES, LED
HORIZONTAL MOUNT, OUTPUT DESIGNATION H

PROPOSED AURORA LIGHTING UNIT, ON CONCRETE FOUNDATION, 30' M.H., TWIN 12' M.A. AND LUMINAIRES, LED HORIZONTAL MOUNT, OUTPUT DESIGNATION H

PROPOSED AURORA LIGHTING UNIT, ON CONCRETE FOUNDATION, 30' M.H., 12' M.A. AND LUMINAIRE, LED HORIZONTAL MOUNT, OUTPUT DESIGNATION H

PROPOSED UNDERPASS LUMINAIRE, LED, OUTPUT DESIGNATION C, WALL MOUNTED

PROPOSED LIGHTING CONTROLLER, BASE MOUNTED, 240 VOLT, 100 AMP

PROPOSED LIGHTING JUNCTION BOX. TYPE AND SIZE AS NOTED

TO REMAIN IN OPERATION DURING CONSTRUCTION

EXISTING UTILITY POLE

EXISTING UTILITY POLE AND LIGHTING LUMINAIRE

≟ GROUND ROD

J

 \bigcirc

(P)

ETR

UTILITY PEDESTAL (NED INSTALL)

EXISTING LIGHTING CONTROLLER

▲ UTILITY TRANSFORMER POLE MOUNTED

UNIT DUCT. NO. AND SIZE OF WIRES
AS NOTED ON PLANS

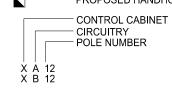
— — — UNIT DUCT (EMPTY)

- - - WIRING IN CONDUIT ATTACHED

- - - ∃ RGC UNDERGROUND CONDUIT

EXISTING HANDHOLE

PROPOSED HANDHOLE



ABBREVIATIONS

AMPS DIA DIAMETER CONDUIT EOP EDGE OF PAVEMENT GND GROUND HPS HIGH PRESSURE SODIUM IDOT ILLINOIS DEPARTMENT OF TRASPORTATION LIGHT EMITTING DIODE LT LEFT MC MEDIUM CUTOFF NED NAPERVILE ELECTRIC DEPARTMENT PUSHED PHOTO CONTROL PVC POLYVINYL CHLORIDE RT RIGHT RIGID GALVANIZED STEEL CONDUIT STA STATION UNO UNLESS NOTED OTHERWISE **VOLTS**

BILL OF MATERIALS

WATTS

ELECTRIC SERVICE INSTALLATION	EACH	1
ELECTRIC UTILITY SERVICE CONNECTION	L SUM	1
UNDERGROUND CONDUIT, GALVANIZED STEEL, 2 1/2" DIA.	FOOT	140
UNDERGROUND CONDUIT, GALVANIZED STEEL, 4" DIA.	FOOT	265
CONDUIT ATTACHED TO STRUCTURE, 3/4" DIA., PVC COATED GALVANIZED STEEL	FOOT	80
CONDUIT ATTACHED TO STRUCTURE, 3" DIA., PVC COATED GALVANIZED STEEL	FOOT	40
JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 6" X 6" X 4"	EACH	6
JUNCTION BOX, STAINLESS STEEL, ATTACHED TO STRUCTURE, 12" X 12" X 8"	EACH	í.
HANDHOLE	EACH	(1)
UNIT DUCT, 600V, 3-1C NO.6, 1/C NO.6 GROUND, (XLP-TYPE USE), 1 1/4" DIA. POLYETHYLENE	FOOT	2,170
UNIT DUCT, 600V, 4-1C NO.4, 1/C NO.6 GROUND, (XLP-TYPE USE), 1 1/2" DIA. POLYETHYLENE	FOOT	1,050
ELECTRIC CABLE IN CONDUIT, 600V (XLP-TYPE USE) 3-1/C NO. 2	FOOT	250
AERIAL CABLE, 2-1/C NO. 6 WITH MESSENGER WIRE	FOOT	1,947
LUMINAIRE, LED, ROADWAY, OUTPUT DESIGNATION G	EACH	(
LUMINAIRE, LED, ROADWAY, OUTPUT DESIGNATION H	EACH	18
LUMINAIRE, LED, UNDERPASS, WALLMOUNT, OUTPUT DESIGNATION C	EACH	(
LIGHTING CONTROLLER, BASE MOUNTED, 240VOLT, 100AMP	EACH	2
LIGHT POLE, ALUMINUM, 40 FT. M.H., 12 FT. MAST ARM	EACH	(
LIGHT POLE, WOOD, 50 FOOT, CLASS 4, WITH 12FT MAST ARM	EACH	10
LIGHT POLE, WEATHERING STEEL, 30 FT. M.H., 12 FT. MAST ARM	EACH	2
LIGHT POLE, WEATHERING STEEL, 30 FT. M.H., 12 FT. MAST ARM-TWIN	EACH	11
LIGHT POLE, WEATHERING STEEL, 40 FT. M.H., 12 FT. MAST ARM - TWIN	EACH	
LIGHT POLE FOUNDATION, 24" DIAMETER	FOOT	76
BREAKAWAY DEVICE, TRANSFORMER BASE, 11.5 INCH BOLT CIRCLE	EACH	10
REMOVAL OF POLE FOUNDATION	EACH	
RELOCATE EXISTING LIGHTING UNIT	EACH	***
DRILL EXISTING HANDHOLE	EACH	7
HELIX FOUNDATION AND BREAKAWAY DEVICE	EACH	(
MAINTENANCE OF LIGHTING SYSTEM	CAL MO	28
WOOD POLE, 35 FT, CLASS 4	EACH	- 1
LIGHT POLE FOUNDATION, 24" DIAMETER, OFFSET	FOOT	1:
LUMINAIRE SAFETY CABLE ASSEMBLY	EACH	25

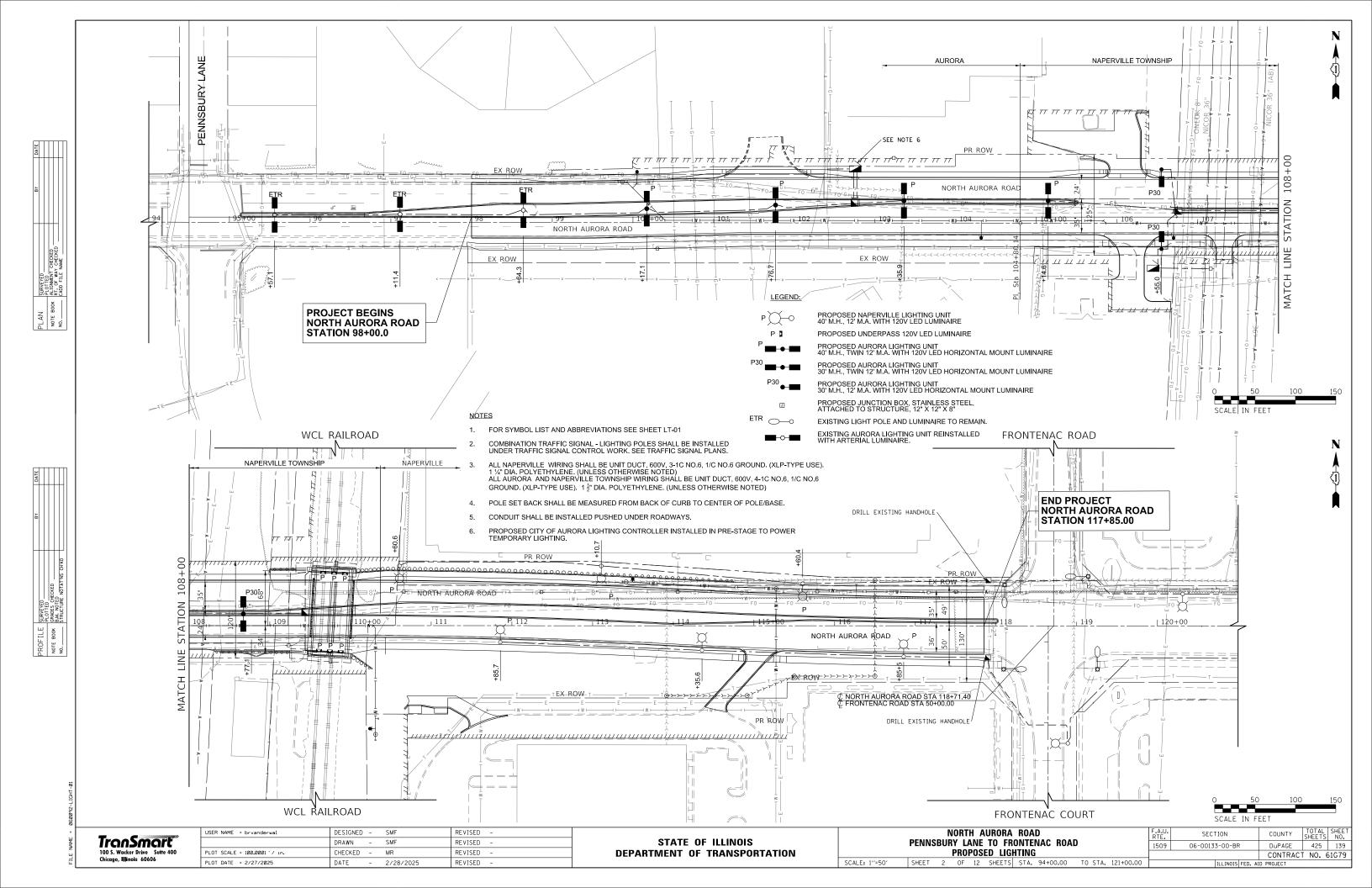
GENERAL NOTES

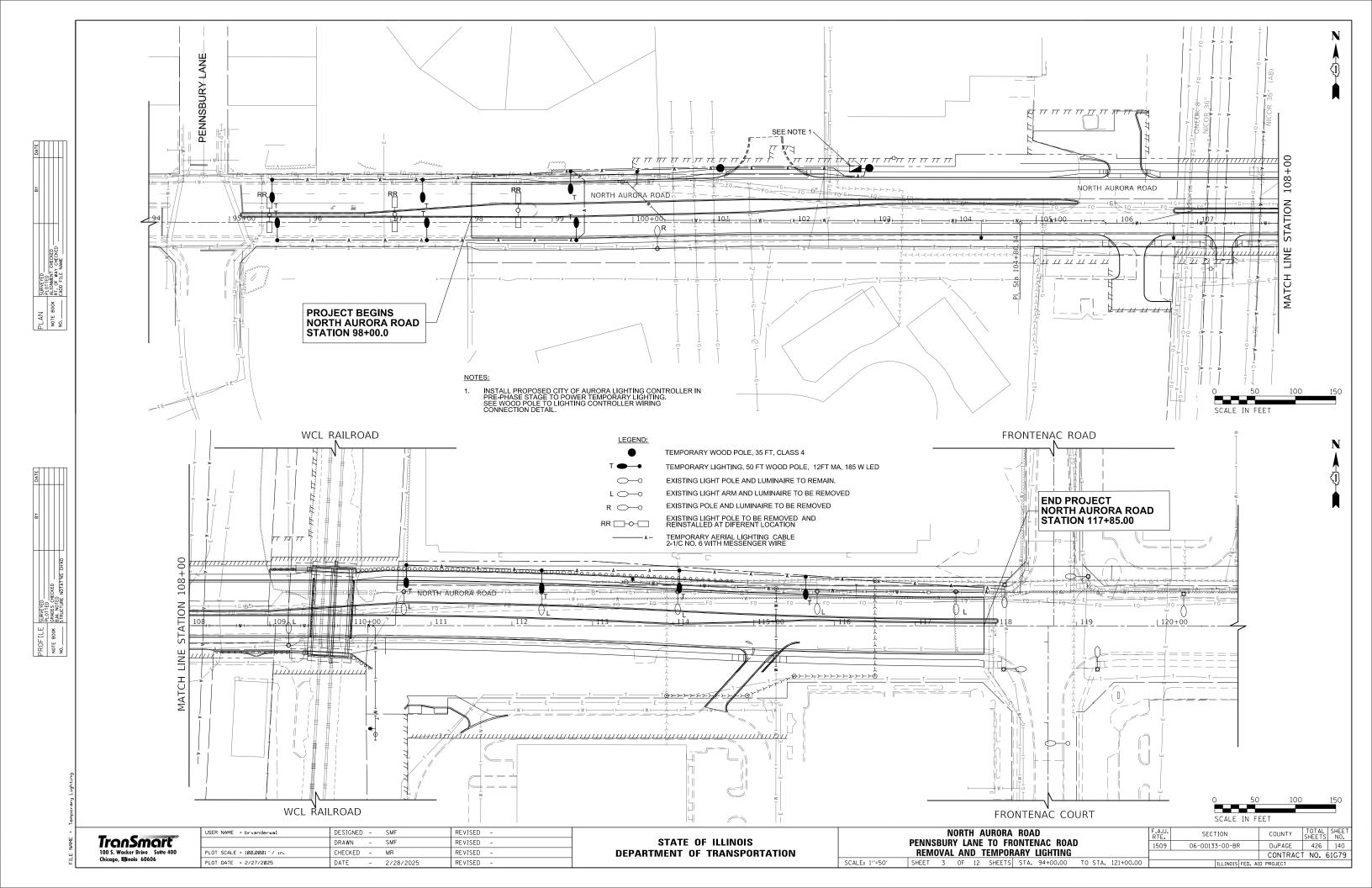
- SPLICING OF CONDUCTORS SHALL BE IN POLE BASES OR WEATHER TIGHT
 JUNCTION BOXES ONLY. SPLICES BELOW GRADE WILL NOT BE PERMITTED.
- LIGHTING CIRCUITS SHALL BE WIRED IN ACCORDANCE WITH THE PLANS. DEVIATIONS WILL NOT BE PERMITTED WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- 3. THE CONTRACTOR SHALL REQUEST A FORMAL MAINTENANCE TRANSFER FOR NAPERVILLE MAINTAINED LIGHTING BEFORE ANY WORK BEGINS. THE CONTRACTOR SHALL CONTACT THE CITY OF NAPERVILLE, DEPARTMENT OF PUBLIC WORKS AT 630-420-6187 AND/OR AURORA UTILITIES PRIOR TO ANY WORK.
- THE CONTRACTOR SHALL TAKE ALL NECESSARY MEASURES TO RESTORE ANY SPECIALIZED LADSCAPING (DECORATIVE ROCKS, PLANTS, ETC.).
- ALL WORK SHALL CONFORM TO THE LATEST IDOT DISTRICT ONE STANDARDS, SPECIAL PROVISIONS, SUPPLEMENTAL SPECIFICATIONS, THE NATIONAL ELECTRICAL CODE, AND THE NATIONAL ELECTRICAL SAFETY CODE.
- 6. ALL ELECTRICAL EQUIPMENT SHALL BE UL LISTED AND LABELED.
- 7. ALL CONDUITS SHALL BE SEALED.
- 8. ALL CIRCUIT WIRES SHALL BE LABELED WITH CIRCUIT IDENTIFICATION.
- 9. CIRCUITS SHALL BE TESTED PER SPECIFICATIONS.
- 10. THE LOCATIONS OF ALL PROPOSED EQUIPMENT ARE ILLUSTRATED DIAGRAMMATICALLY. THE ACTUAL LOCATION IN THE FIELD SHALL MEET THE APPROVAL OF THE ENGINEER.
- 11. ALL MEASUREMENTS ARE APPROXIMATE. THE CONTRACTOR SHALL VERIFY MEASUREMENTS IN THE FIELD.
- 12. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE EXISTING INSTALLATIONS AND DATA PRIOR TO BIDDING.
- 13. THE CONTRACTOR MUST MAINTAIN SAFE EQUIPMENT AND WORKING
 CLEARENCES FROM THE EXISTING COMED/NED OVERHEAD ELECTRIC LINES.
 THE CONTRACTOR SHALL PLAN HIS WORK CONSIDERING ELECTRIC LINES ARE
 TO BE IN SERVICE AND ENERGIZED THROUGHOUT THE CONSTRUCTION PERIOD.
- 14. THE CONTRACTOR SHALL REMOVE CABLES FROM ALL EXISTING UNIT DUCTS. EXISTING UNIT DUCTS SHALL BE ABANDONED IN PLACE.
- 15. THE COST OF SPLICING TEMPORARY CONDUCTORS TO EXISTING CONDUCTORS
 IN EXISTING POLES SHALL BE INCLUDED IN THE COST OF THE PAY ITEM FOR UNIT DUCT.
- 16. GROUNDING CONDUCTORS SHALL BE CONTINUOUS.
- 17. ALL NEW UNIT DUCTS AND CONDUITS SHALL BE PLACED A MINIMUM OF 30" BENEATH THE GROUND SURFACE (FINAL GRADE).
- 18. ALL EMPTY HDPE CONDUIT SHALL INCLUDE A TONEABLE/TRACEABLE PULL STRING FOR FUTURE LOCATING.
- NAPERVILLE LUMINAIRES SHALL BE THEIR STANDARD LEOTEK MODEL NO: GC2-120F-MV-NW-2-GY-700 PCR7-SC-WL. AURORA LUMINAIRES SHALL BE LUMEC MODEL NO: RVM-190W112LED4K-G2-LE2.
- 20. INSTALLATION OF REMOTELY MOUNTED PHOTOCELL SHALL BE INCLUDED IN THE COST OF THE PAY ITEM FOR THE LIGHTING CONTROLLER.
- 21. EXISTING ROADWAY LIGHTING SHALL REMAIN OPERATIONAL UNTIL THE TIME CONSTRUCTION REQUIRES THE REMOVAL OF POLES OR LUMINAIRES.

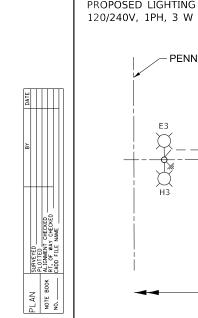
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100 S. Wacke		
Chicago, Illino	ois 606	06

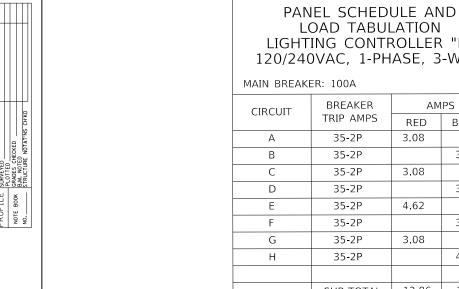
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	DRAWN	-	SMF	REVISED -
PLOT SCALE = 100.0000 ' / in.	CHECKED	-	MR	REVISED -
PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -

	ELECTRICAL SYMBOLS, ABBREVIATIONS						COUNTY	TOTAL SHEETS	SHEET NO.
	AND	1509	06-00133-00-BR	DuPAGE	425	138			
AND GENERAL NOTES							CONTRAC	T NO. 6	51G79
SCALE: AS SHOWN	SHEET 1 OF	12 SHEETS	STA.	TO STA.		ILL INDIS FED. A	ID PROJECT		

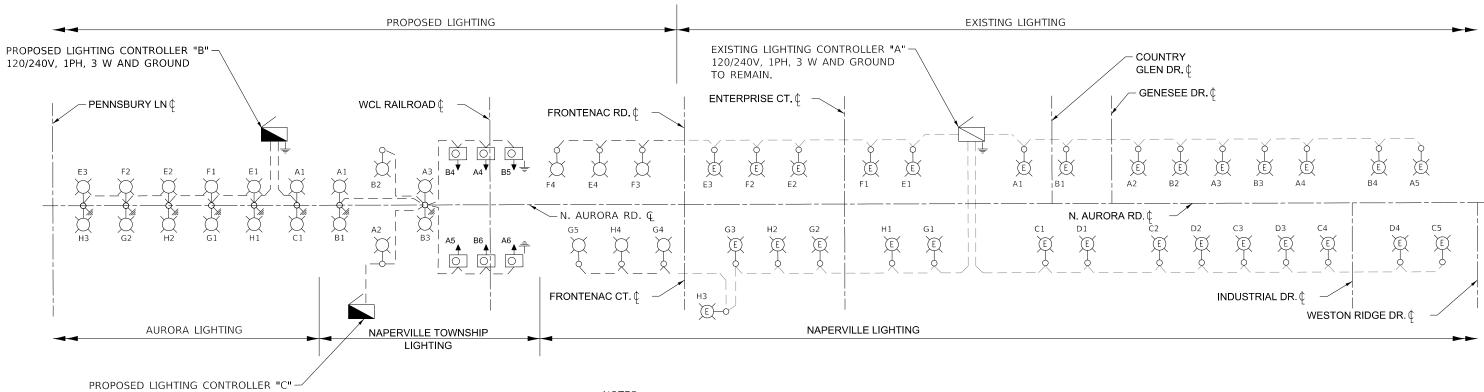








120/240V, 1PH, 3 W AND GROUND



NOTES:

- 1. FOR SYMBOL LIST AND GENERAL ELECTRICAL NOTES. SEE SHEET E-01.
- 2. INTERSECTION LUMINAIRES POWERED BY TRAFFIC SIGNAL CONTROLLERS ARE NOT SHOWN.

PANEL SCHEDULE AND

LOAD TABULATION

LIGHTING CONTROLLER "C"

3. LUMINAIRE CIRCUIT RUNS ARE TO BE ALTERNATING.

LIGHTING CONTROLLER SINGLE LINE DIAGRAM NOT TO SCALE

LOAD TABULATION LIGHTING CONTROLLER "B" 120/240VAC, 1-PHASE, 3-WIRE AMPS RED BLACK 3.08 3.08 3.08 4.62 3.08 3.08 4.62 13.86 13.86 SUB-TOTAL TOTAL 27.7

120/240VAC, 1-PHASE, 3-WIRE MAIN BREAKER: 100A BREAKER AMPS CIRCUIT TRIP AMPS RED BLACK 35-2P Α В 35-2P 5.85 35-2P 35-2P D Ε 35-2P 35-2P 35-2P G Н 35-2P 5.85 5.85 SUB-TOTAL TOTAL 11.7

SCALE: NTS

LIGHTING CONTROLLER "A" 120/240VAC, 1-PHASE, 3-WIRE MAIN BREAKER: 100A

PANEL SCHEDULE AND

LOAD TABULATION

CIRCUIT	BREAKER	AMPS			
CINCOIT	TRIP AMPS	RED	BLACK		
А	35-2P	10.88			
В	35-2P		8.70		
С	35-2P	10.88			
D	35-2P		8.70		
E	35-2P	8.70			
F	35-2P		8.70		
G	35-2P	10.88			
Н	35-2P		8.70		
	SUB-TOTAL	41.34	34.8		
	TOTAL	76.14			

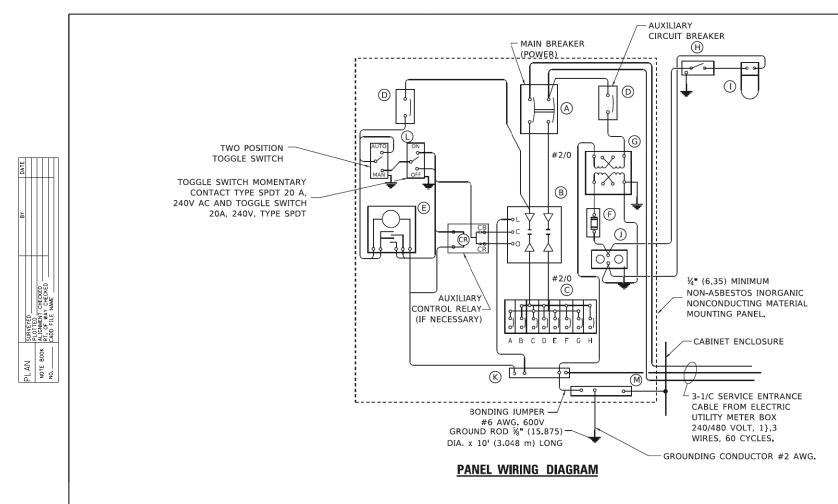
T
TranSmart"
100 S. Wacker Drive Suite 400
Chicago, Illinois 60606

USER NAME = brvanderwal	DESIGNED	-	SMF	REVISED -	Π
	DRAWN	-	SMF	REVISED -	
PLOT SCALE = 100.00000 '/ in.	CHECKED	-	MR	REVISED -	
PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -	

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

NORTH AURORA ROAD	F.A.U. RTE.			
PENSBURY LANE TO FRONTENAC R	1509	06-		
SINGLE LINE DIAGRAM				
CHEET A OF 12 CHEETS STA NIZA	TO STA	N/A		

SECTION DuPAGE 425 141 -00133-00-BR CONTRACT NO. 61G79



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1/25/2025

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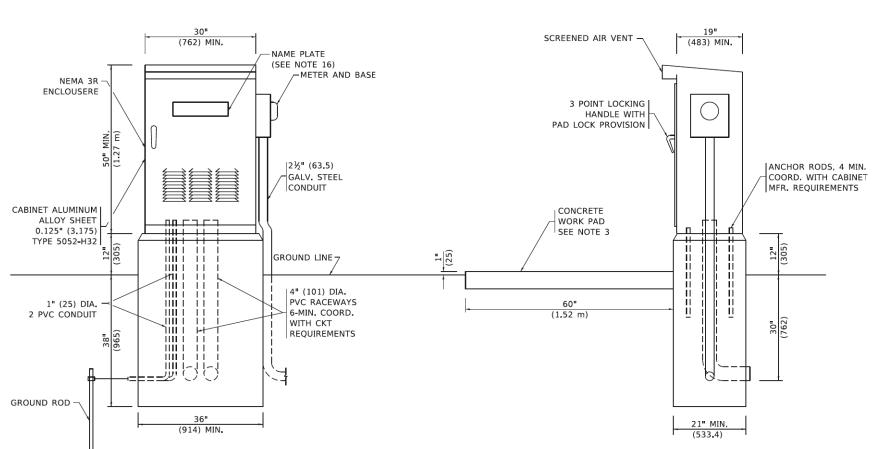
USER NAME = bryanderwal

PLOT DATE = 1/25/2025

PLOT SCALE = 40.0000 '/ in.

TranSmart

100 S. Wacker Drive Suite 400 Chicago, Illinois 60606



REVISED

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REVISED

STATE OF ILLINOIS

DEPARTMENT OF TRANSPORTATION

PANEL EQUIPMENT

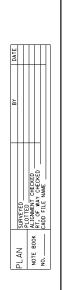
		BILL OF MATERIAL
ITEM	QUANTITY	DESCRIPTION
Α	1	MAIN CIRCUIT BREAKER, 2 POLE, 600 VOLT 100 AMP. FRAME, 100 AMP. NON-INTERCHANGEABLE TRIP INTERRUPTING RATING NEMA-22000 AMP. AT 480 VOLT.
В	1	REMOTE CONTROL SWITCH, ELECTRICALLY OPERATED, MECHANICALLY HELD, 2 POLE, SINGLE THROW, 100 AMP., 600 VOLTS CONTROL CIRCUIT 240 VOLT.
С	8	CIRCUIT BREAKERS, 1 POLE, 100AMP. FRAME, 50 AMP. NON-INTERCHANGEABLE TRIP INTERRUPTING RATING NEMA-10,000 AMP. AT 240 V.
D	2	CONTROL CIRCUIT-CIRCUIT BREAKER. 1 POLE, 240 V., 100 AMP. FRAME, 15 AMP. NON-INTERCHANGEABLE TRIP INTERRUPTING RATING NEMA-5000 AMP. AT 240 V.
Е	1	ASTRONOMIC MICROPROCESSOR-BASED 2-CHANNEL CONTROLLER (TIME SWITCH).
F	1	20 AMP, 120 VOLT FUSE.
G	1	1.5 KVA, SINGLE PHASE, ENCAPSULATED TRANSFORMER 240 X 480 / 120 X 240 VOLT, 60 Hz.
Н	1	SPST 20A SWITCH ON DOOR, TO TURN LIGHT ON WHEN DOOR IS OPEN,
I	1	INCANDESCENT LIGHTING FIXTURE ENCLOSED AND GASKETED WITH 60 WATT, LED EQUIVALENT, 120 V. LAMP.
J	1	20 AMP, 120 VOLT, DUPLEX RECEPTACLE, GFCI.
K	1	COPPER GROUND BUS 1#4" (6.35) X 1" (25.4) X 12" (304.8 mm) LONG MOUNTED ON PANEL WITH LUGS AND 4 SPARE LUGS
٦	1	TOGGLE SWITCHES MOUNTED IN 4" (101.6) X 4" (101.6 mm) BOX.
М	1	COPPER GROUND BUS 1#4" (6.35) X 1" (25.4) X 12" (304.8 mm) LONG MOUNTED ON PANEL WITH LUGS AND SPARE LUGS.

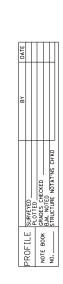
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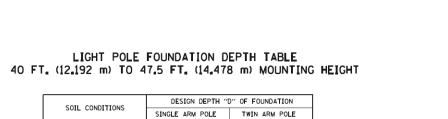
- 1. ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.
- FOUNDATION SIZE SHALL BE COORDINATED WITH CABINET SIZE AND MFR.
- 3. IN FRONT OF CONTROL CABINET DOOR, REMOVE VEGETATION AND 2" (50.8 mm) TOP SOIL, LEVEL THE AREA AND ON TOP, PLACE LENGTH WISE PARALLEL TO CONTROL CABINET, A CONCRETE PAD 36" (914.4 mm) x 60" (18.288 m) x 4" (101 mm) MIN. SIZE. THE COST OF LABOR AND MATERIALS ARE INCLUDED IN THE COST OF THE CONTROLLER.
- DOOR SHALL BE CONSTRUCTED FROM SAME TYPE OF MATERIAL AND THICKNESS AS CABINET.
- DOOR SHALL BE EQUIPPED WITH THREE POINT LATCHING MECHANISM WITH NYLON ROLLERS AT TOP THE BOTTOM.
- DOOR HINGE SHALL BE A HEAVY GAUGE CONTINUOUS HINGE WITH A 1#4" (6.35 mm) DIA. STAINLESS STEEL HINGE PIN.
- 7. ALL EXTERNAL HARDWARE SHALL BE STAINLESS STEEL.
- CONTROL WIRING TO BE #12 AWG, 600V, TYPE "SIS" GRAY SWITCH BOARD WIRE, STRANDED COPPER.
- METER BOX SHALL BE MOUNTED ON THE SIDE OF CONTROL CABINET, NEAR TO THE SERVICE POLE.

- 10. CABINETS SHALL BE PRIMED AND PAINTED AS SPECIFIED.
- 11. THE HEADS OF CONNECTORS SCREWS SHALL BE PAINTED WHITE FOR NEUTRAL BAR CONNECTION AND GREEN FOR GROUND BAR CONNECTORS.
- 12. ALL WIRING WITHIN THE CABINET SHALL BE COLOR CODED AS INDICATED. $R = RED \qquad BL = BLUEW = WHITE \\ B = BLACK \qquad Y = YELLOW \qquad G = GREEN$
- PROVIDE SEALING GROMMETS FOR ALL OPEN WIRING EXTENDED FROM DEVICES IN BOXES OR CABINETS WITHIN THE CONTROL CABINET.
- 14. ALL WIRING SHALL BE NEATLY DRESSED AND SUPPORTED.
- 15. THE CONTROLLER SHALL BE CONSTRUCTED TO U.L. STD. 508 AND BEAR THE U.L. LABEL "ENCLOSED INDUSTRIAL CONTROL PANEL".
- 16. 12" (304.8) X 16" (406.4 mm) STAINLESS STEEL EXTERIOR NAMEPLATE SHALL BE ENGRAVED TO "NAPERVILLE ROADWAY LIGHTING CONTROLS" UNLESS OTHERWISE SPECIFIED.

NORTH AURORA ROAD - PENNSBURY LANE TO FRONTENAC ROAD						F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
LIGHTING CONTROLLER DETAIL								06-00133-00-BR	DuPAGE	426	142
									CONTRAC	T NO.	61G79
SCALE: NONE	SHEET 5	OF	12	SHEETS	STA.	TO STA.		ILLINOIS FED. AI	D PROJECT		







(4,57 m)

(3,23 m)

(2.44 m)

10'-0"

(3.05 m)

(2.74 m)

(2.74 m)

(3,96 m)

(2.09 m)

(2.13 m)

8'-3"

(2.52 m)

(2.36 m)

TOP OF ANCHOR ROD

60" (1500)

FOUNDATION EXTENSION DETAIL

USER NAME = bryanderwal

PLOT DATE = 1/25/2025

TranSmart

4" (100) MAX.

DESIGNED -

SMF

DRAWN

DATE

CHECKED

SOFT CLAY Qu = 0,375 TON/SQ. FT.

Qu = 0.75 TON/SQ,FT

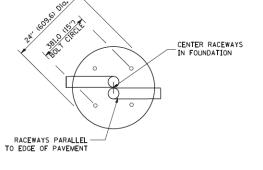
Qu = 1.50 TON/SQ. FT.

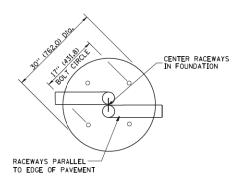
LOOSE SAND Ø = 34°

MEDIUM SAND

Ø = 37.5°

DENSE SAND



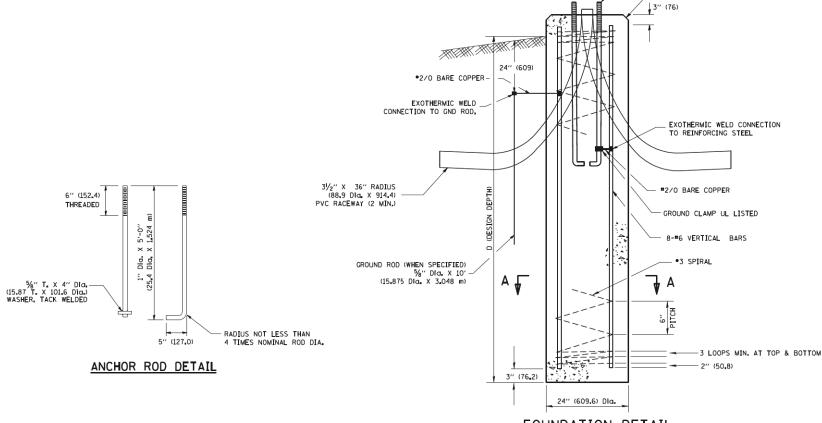


TOP VIEW TOP VIEW

ANCHOR ROD 4-1" Dig. X 5'-0"

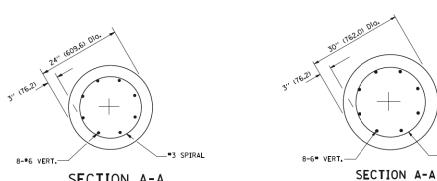
3/4" (19) CHAMFER

(4-25.4 Dia, X 1.524 m)



GROUND LINE





SECTION A-A

REVISED

REVISED

REVISED

REVISED

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SECTION NORTH AURORA ROAD - PENNSBURY LANE TO FRONTENAC ROAD LIGHT POLE FOUNDATION DETAIL 1509 06-00133-00-BR SHEET 6 OF 12 SHEETS STA.

NOTES

- ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN.
- THE ANCHOR RODS AND RACEWAYS SHALL BE PROPERLY SECURED IN PLACE BEFORE THE CONCRETE IN PLACED.
- THE FOUNDATION SHALL NOT PROTRUDE MORE THAN 100MM (4 IN.) ABOVE THE FINISHED GRADE WITHIN A 60 IN. (1.5 m) CHORD ACROSS THE FOUNDATION, WITH ANCHOR RODS INCLUDED, IN ACCORDANCE WITH AASHTO GUIDELINES, IF THE FOUNDATION HEIGHT, INCLUDING ANCHOR RODS, EXTENDS BEYOND THESE SPECIFIED LIMITS, THE FOUNDATION SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE. SEE FOUNDATION EXTENSION DETAIL.
- 4. THE HOLE FOR THE FOUNDATION SHALL BE MADE BY DRILLING WITH AN AUGER, OF THE SAME DIAMETER AS THE FOUNDATION. IF SOIL CONDITIONS REQUIRE THE USE OF A LINER TO FORM THE HOLE, THE LINER SHALL BE WITHDRAWN AS THE CONCRETE IS DEPOSITED.
- THE TOP OF THE FOUNDATION SHALL BE CONSTRUCTED LEVEL. A LINER OR FORM SHALL BE USED TO PRODUCE A UNIFORM SMOOTH SIDE TO THE TOP OF THE FOUNDATION. FOUNDATION TOP SHALL BE CHAMFERED 3/4-IN. (20 mm).
- THE CONCRETE SHALL BE CLASS SI. CONCRETE SHALL CURE ACCORDING TO ARTICLE 1020.13 BEFORE LIGHT POLES ARE INSTALLED.
- THE ANCHOR ROD SHALL BE A HOOK ROD TYPE, COLD BENDING OF THE ANCHOR ROD WILL NOT BE ALLOWED. THE RADIUS OF THE HOOK BEND SHALL NOT BE LESS THAN 4 TIMES THE NOMINAL DIAMETER OF THE ANCHOR ROD. A TACK WELDED ANCHOR ROD MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER.
- 8. THE ANCHOR RODS SHALL BE ACCORDING TO ASTM F1554 GRADE 725 (GRADE 105), NUTS SHALL BE HEXAGON NUTS ACCORDING TO ASTM A 194 2H OR ASTM A 563 DH, AND WASHERS SHALL BE
- 9. ANCHOR RODS, NUTS AND WASHERS SHALL BE COMPLETELY GALVANIZED BY EITHER THE HOT-DIPPED PROCESS CONFORMING WITH AASHTO M 232, THE MECHANICAL PLATING METHOD CONFORMING TO AASHTO M 298, CLASS 50 WITH A MAXIMUM COATING THICKNESS OF 150 UM(6 MILS) OR THE ELECTROLYTIC PROCESS ACCORDING TO ASTM F 1136.
- 10. THE ANCHOR RODS SHALL BE THREADED A MINIMUM OF 6 INCHES (150 mm) WITH A MINIMUM OF 3 INCHES (75 mm) OF THREADED ANCHOR ROD EMBEDDED IN THE FOUNDATION.
- ANCHOR RODS SHALL PROJECT $2\frac{\pi}{4}$ " (69.9 mm) ABOVE THE TOP OF THE FOUNDATION. IF BREAKAWAY COUPLINGS ARE SPECIFIED, THE CONTRACTOR SHALL CAREFULLY COORDINATE THE ANCHOR ROD PROJECTION WITH THE INSTALLATION REQUIREMENTS OF THE BREAKAWAY COUPLINGS.
- 12. THE CONTRACTOR SHALL USE A *3 SPIRAL AT 6" (152.4 mm) PITCH OR MAY SUBSTITUTE *3 TIES AT 12" (304.8 mm) O.C. WITH THE APPROVAL OF THE ENGINEER.

COUNTY

DuPAGE 426 143

CONTRACT NO. 61G79

- 13. THE CABLE TRENCHES AND FOUNDATION SHALL BE BACK FILLED AND COMPACTED AS SPECIFIED
- BEFORE THE LIGHT POLE IS ERECTED. 14. THE RACEWAYS SHALL PROJECT 1" (25.4 mm) ABOVE THE TOP OF THE FOUNDATION.

BY							
	SURVEYED	- PLOTTED	GRADES CHECKED	B.M. NOTED	STRUCTURE NOTATINS CHIKD		
	TROF ILE		NOTE ROOK		NO.		

FOUNDATION DESIGN TABLE

DESIGN DEPTH OF FOU		OF FOUNDATION	REINFORCEMENT IN FOUNDATION				
TYPE OF SOIL	SINGLE ARM	TWIN ARM	SINGLE	ARM	TWIN	ARM	
	D	D	VERT BARS	SPIRAL	VERT BARS	SPIRAL	
SOFT CLAY	13 '- 0"	15 '- 0"	8-#6X12 ' -6"	#3X122"	8-#6X14 ' -3 "	#3X141'	
	(3.962 m)	(4 . 572 m)	(3.810 m)	(37.186 m)	(4.343 m)	(42.977 m)	
MEDIUM CLAY	9'-6"	10 '- 9"	8-#6X9'-0 "	#3X90'	8-#6X10'-0"	#3X100'	
	(2.896 m)	(3.277 m)	(2.743 m)	(27.432 m)	(3.048 m)	(30.480 m)	
STIFF CLAY	7'-0"	8 '- 0"	8-#6X6'-6"	#3X66'	8-#6X7'-6 "	#3X76 '	
	(2.134 m)	(2.438 m)	(1.981 m)	(20.112 m)	(2.286 m)	(23.165 m)	
LOOSE SAND	9'-0"	10 '- 0"	8-#6X8'-6"	#3X85'	8-#6X9 ' -6 ''	#3X94 '	
	(2.743 m)	(3.048 m)	(2.591 m)	(25 . 908 m)	(2.896 m)	(28.651 m)	
MEDIUM SAND	8'-3"	9 '- 0"	8-#6X8'-0"	#3X78'	8-#6X8'-6"	#3X85'	
	(2.515 m)	(2.743 m)	(2.438 m)	(23.774 m)	(2.591 m)	(25.908 m)	
DENSE SAND	7'-9"	9 '- 0"	8-#6X7'-6"	#3X73'	8-#6X8 "-6"	#3X85'	
	(2.362 m)	(2.743 m)	(2.286 m)	(22.250 m)	(2.591 m)	(25.908 m)	
ROCK OR SOLIDIFIED SLAG	5'-0" (1.524 m)	5'-0" (1.524 m)	NONE	NONE	NONE	NONE	

OFFSET SCHEDULE

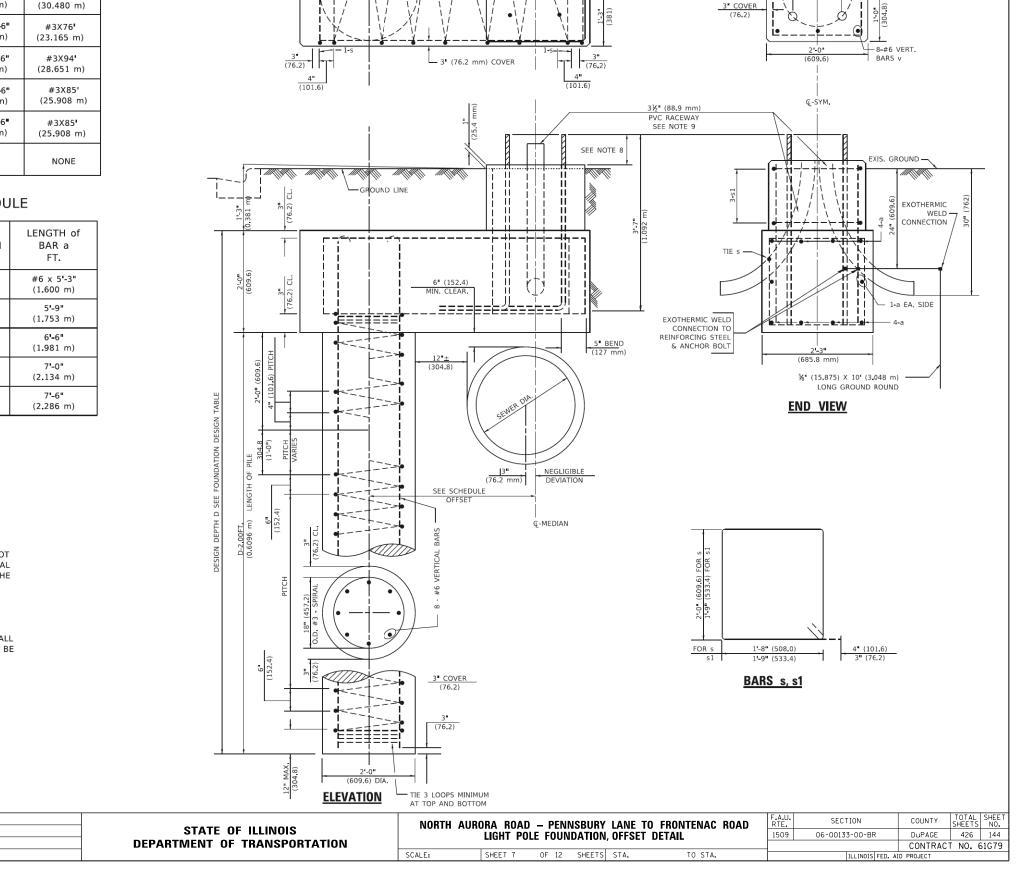
BILL OF MATERIAL

MARK	NO.	SIZE	LENGTH	SHAPE
a	10	6	SEE BELOW	_
s	12	4	8'-0" (2.438 m)	
s ₁	3	3	7 '- 6" (2.286 m)	
v ₁	8	6	2 '- 9" (0.838 m)	
v ₂			·	

SEWER	PILE OFFSET	LENGTH of
DIAM. d	from Q-MED'N	BAR a
IN.	FT.	FT.
UP TO 24"	3 '-3"	#6 x 5'-3"
(609.6 mm)	(0.991 m)	(1.600 m)
27" (685.8 m)TO	3'-9"	5'-9"
36" (914.4 mm)	(1.143 m)	(1.753 m)
42" (1066.8 mm) TO	4 '-6"	6 '- 6"
48" (1219.2 mm)	(1.372 m)	(1.981 m)
54" (1371.6 mm) TO	5'-0"	7'-0"
60" (1524.0 mm)	(1.524 m)	(2.134 m)
66" (1676.4 mm) TO	5'-6"	7'-6"
72" (1828.8 mm)	(1.676 m)	(2.286 m)

NOTES

- 1, ALL DIMENSIONS ARE IN INCHES (MILLIMETERS) UNLESS OTHERWISE SHOWN,
- 2. THE ENGINEER SHALL DETERMINE THE CLASS OF SOIL DURING EXCAVATION AND SELECT THE DESIGN DEPTH OF FOUNDATION FROM THE DESIGN TABLE.
- 3. EXCAVATION OF THE POLE FOUNDATION SHALL BE MADE WITH AN AUGER, 24" (609.6 mm) OR 30" (762.0 mm) IN DIAMETER.
- 4. THE ANCHOR ROD SHALL BE A HOOK ROD TYPE. COLD BENDING OF THE ANCHOR ROD WILL NOT BE ALLOWED. THE RADIUS OF THE HOOK BEND SHALL NOT BE LESS THAN 4 TIMES THE NOMINAL DIAMETER OF THE ANCHOR ROD. A TACK WELDED ANCHOR ROD MAY BE SUBSTITUTED WITH THE
- 5. THE ANCHOR BOLTS AND RACEWAYS SHALL BE PROPERLY SECURED IN PLACE BEFORE THE CONCRETE IS PLACED IN THE FORM.
- 6. THE ANCHOR RODS SHALL BE ACCORDING TO ASTM F1554 GRADE 725 (GRADE 105). NUTS SHALL BE HEXAGON NUTS ACCORDING TO ASTM A 194 2H OR ASTM A 563 DH, AND WASHERS SHALL BE ACCORDING TO ASTM F 436.
- 7. THE CONTRACTOR SHALL COORDINATE EXTENSION OF ANCHOR BOLTS ABOVE TOP OF FOUNDATION WITH THE BREAKAWAY DEVICE MANUFACTURER'S REQUIREMENTS. IF LIGHT POLE IS MOUNTED WITHOUT BREAKAWAY DEVICE, ANCHOR BOLTS SHALL PROJECT 23#4" (69.9 mm) ABOVE TOP OF THE FOUNDATION. THE CONTRACTOR SHALL CONFIRM ANCHOR BOLT EXTENTION WITH ENGINEER.
- 8. RACEWAYS SHALL PROJECT 1" (25.4 mm) ABOVE THE TOP OF THE FOUNDATION.
- 9. THE CABLE TRENCH SHALL BE BACKFILLED AND FIRMLY COMPACTED BEFORE THE LIGHT IS ERECTED.



PLAN-CAP BEAM

TOP VIEW

(152.4)

TOP VIEW

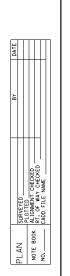
(134.9)

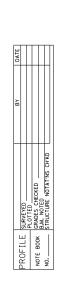
4-1" (25.4) ANCHOR BOLTS WITH HEX-NUTS AND WASHERS. ASTM 687 STEEL

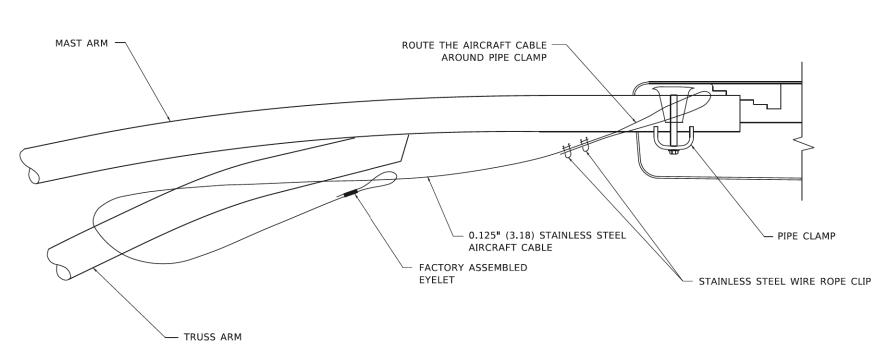
BOLT CIRCLE

55/16"

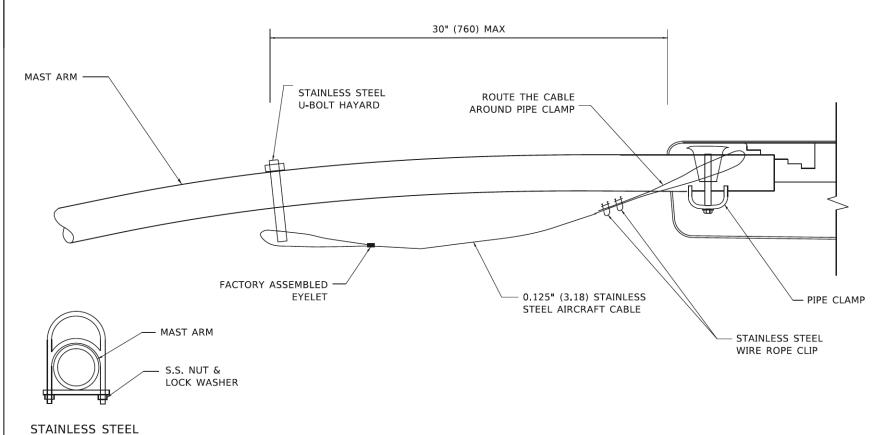




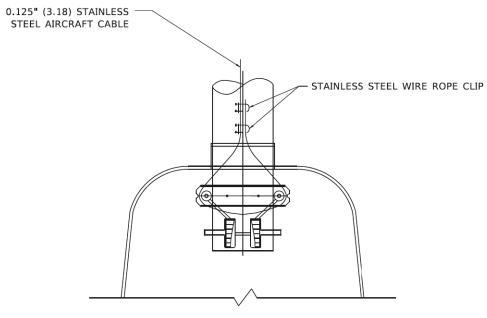




SIDE VIEW (TRUSS ARM) <u>N.T.S.</u>



SIDE VIEW (SINGLE MEMBER OR DAVIT ARM) <u>N.T.S.</u>



BOTTOM VIEW <u>N.T.S.</u>

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETERS (INCHES) UNLESS OTHERWISE SHOWN.
- 2. CONTRACTOR SHALL ADJUST THE WIRE CLIP TO ELIMINATE ANY SLACK FROM THE WIRE ROPE.
- 3. THE 0.125" (3.18) STAINLESS STEEL AIRCRAFT CABLE SHALL REMAIN VISIBLE FROM THE GROUND LEVEL.
- 4. THE BREAKING STRENGTH OF THE CABLE SHALL BE 1700 LBS. MIN.

TranSmart"	
100 S. Wacker Drive Suite 400 Chicago, Illinois 60606	

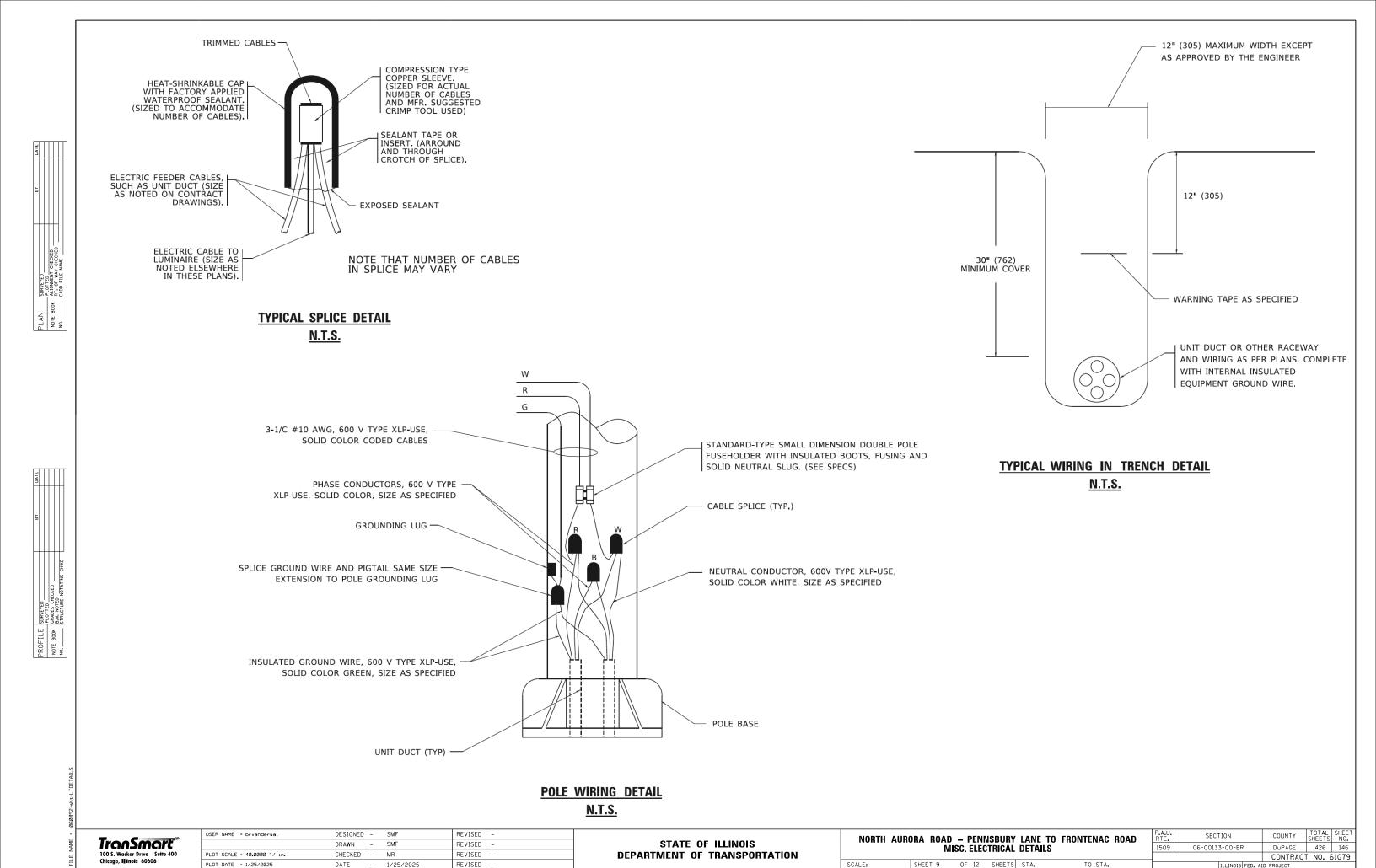
U-BOLT HAYARD

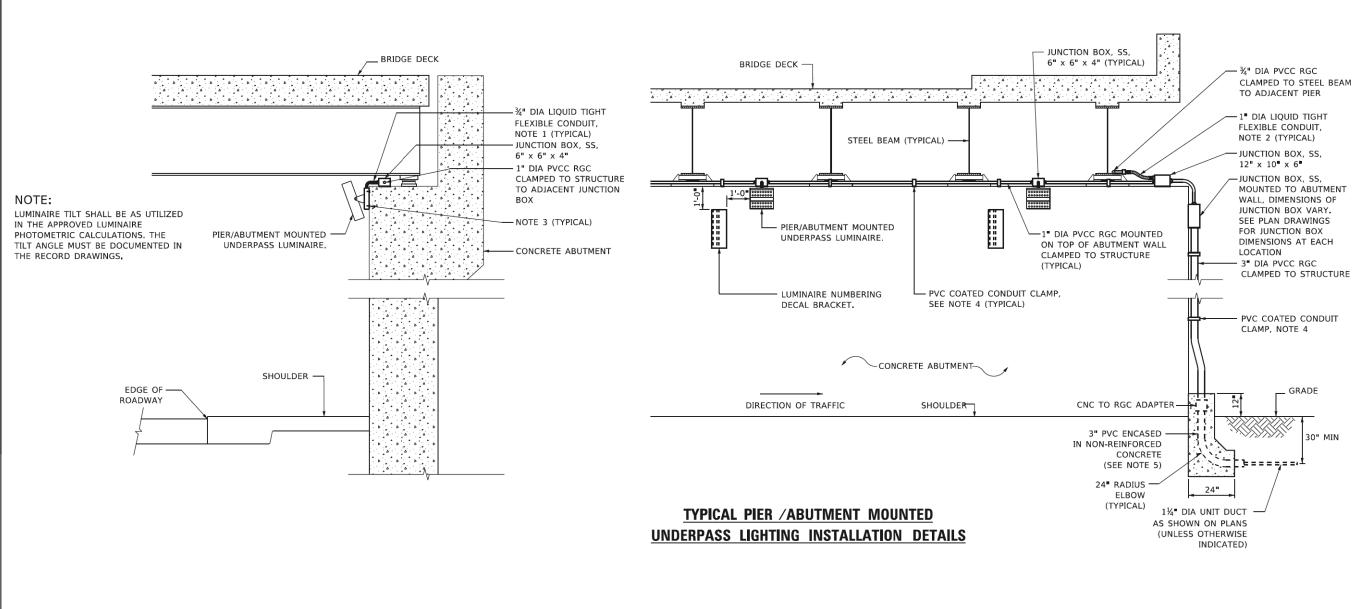
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PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -

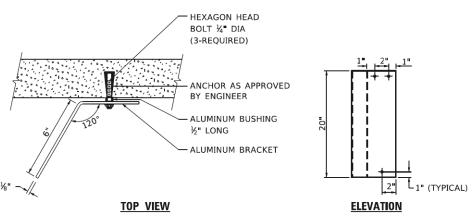
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SCALE:

NORTH AURO	ORA ROAD	– PENI	NSBURY	LANE T	O FRONTENAC ROAD	F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	LUMINA	AIRE SA	FTEY CA	BLE DE	TAILS	1509	06-00133-00-BR	DuPAGE	426	145
								CONTRAC	T NO.	61G79
CALE:	SHEET 8	OF 12	SHEETS	STA.	TO STA.		ILLINOIS FED.	VID PROJECT		







PVC COATED CONDUIT BEAM CLAMP

NOT TO SCALE



PVC COATED
CONDUIT CLAMP

NOT TO SCALE

NOTES:

- 1. LIQUID TIGHT FLEXIBLE METAL
 CONDUIT, MAXIMUM LENGTH 6'-0", TYPICAL
 FOR EACH INSTANCE AS SHOWN. PROVIDE PVC
 COATED RIGID GALVANIZED STEEL CONDUIT AS
 REQUIRED NOT TO EXCEED 6'-0" OF FLEXIBLE
 LIQUID TIGHT METAL CONDUIT. LIQUID TIGHT
 FLEXIBLE METAL CONDUIT WILL BE INCLUDED
 IN THE COST OF THE CONDUIT ATTACHED TO
 STRUCTURE, OF THE CORRESPONDING DIA.,
 GALVANIZED STEEL, PVC COATED PAY ITEM
 EXCEPT THAT THE COST OF THE "DIA.
 RIGID STEEL CONDUIT AND "DIA. FLEXIBLE
 CONDUIT SHALL BE INCLUDED IN THE LUMINAIRE
 INSTALLATION.
- SEE UNDERPASS LIGHTING PLANS FOR INSTALLATION LOCATION OF UNDERPASS LIGHTING LUMINAIRES.
- 3. UNDERPASS LUMINAIRE MOUNTED TO FACE OF PIER OR ABUTMENT WALL WITH ½" ALUMINUM SPACERS. MOUNTING HEIGHT OF 1" BELOW THE TOP OF PIER OR ABUTMENT WALL TYPICAL FOR ALL PIER/ABUTMENT MOUNTED UNDERPASS LUMINAIRES UNLESS OTHERWISE NOTED.

- EXPANSION ANCHOR, POWDER ACTUATED FASTENERS WILL NOT BE ALLOWED. EXPANSION ANCHOR MUST BE SIZED IN ACCORDANCE WITH MANUFACTURERS REQUIREMENTS.
- 5. SECURE THE CONDUIT WITH PVC COATED CONDUIT CLAMPS OR CONDUIT BEAM CLAMPS AS SHOWN AT 5'-0" INTERVALS FOR LATERALS AND WITHIN 2'-0" MAXIMUM FROM ANY JUNCTION BOX, FLEXIBLE CONDUIT, OR CHANGE IN DIRECTION. ALL PVC COATED CONDUIT CLAMPS OR BEAM CLAMPS SHALL BE INCLUDED WITH THE COST OF THE "CONDUIT ATTACHED TO STRUCTURE, OF THE CORRESPONDING DIA,, GALVANIZED STEEL, PVC COATED" PAY ITEM.
- THE CONCRETE ENCASED CONDUIT TRANSITION SHALL BE INCLUDED IN THE COST OF THE GALVANIZED RIGID STEEL CONDUIT PAY ITEMS.
- ALL CONDUIT ATTACHED TO STRUCTURE SHALL BE PVC COATED RIGID STEEL CONDUIT (PVCC RGC) TYPICAL.

LUMINAIRE NUMBERING DECAL BRACKET

NOT TO SCALE

Transmart^{**}
100 S. Wacker Drive Sulte 400
Chicago, Illinois 60606

USER NAME = brvanderwal	DESIGNED	-	SMF	REVISED -	
	DRAWN	-	SMF	REVISED -	
PLOT SCALE = 40.0000 '/ in.	CHECKED	-	MR	REVISED -	
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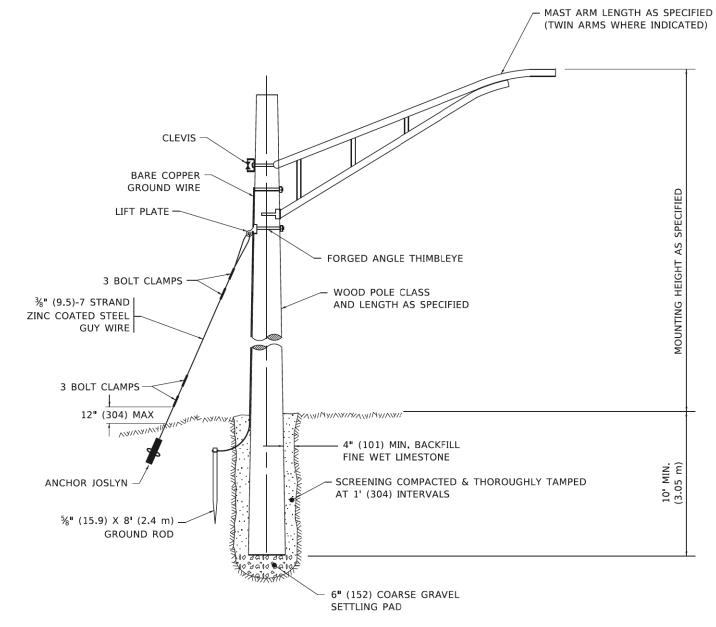
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

						O FRONTENAC ROAD AIRE INSTALL DETAILS
S	CALE:	SHEET 10	OF 12	SHEETS	STA.	TO STA.

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
1509	06-00133-00-BR	DuPAGE	426	147
		CONTRACT	NO. 6	31G79
	ILLINOIS FED. AI	ID PROJECT		







TEMPORARY LIGHT POLE ATTACHMENT DETAIL

WOOD POLE -

GROUND CLAMP -

AWG BARE COPPER

WATERPROOF INSULATION

PIERCING TAP CONNECTOR

GROUND WIRE

MESSENGER TIED TO INSULATOR -WITH FACTORY FORMED CABLE TIE

NOTE:

1. ALL DIMENSIONS IN INCHES (MILLIMETERS) UNLESS OTHERWISE INDICATED.

TEMPORARY LIGHT POLE DETAIL

2. MAST ARM SHALL BE RATED FOR THE SPECIFIED MOUNTING HEIGHT.

т.	ranSmart"
100	S. Wacker Drive Suite 400
Chie	ago, Illinois 60606

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PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -	

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

NORTH AURORA ROAD — PENNSBURY LANE TO FRONTI TEMPORARY LIGHT POLE DETAILS					
	SCALE:	SHEET 11	OF 12	SHEETS STA.	TO STA.

F.A.U. RTE.	SECTION	COUNTY	SHEETS	NO.	
1509	06-00133-00-BR	DuPAGE	426	148	
			CONTRACT	NO. 6	51G79
	ILLINOIS FE	D. AI	D PROJECT		

HEAVY DUTY INSULATED

- BARE COPPER GROUND WIRE EVERY THIRD POLE

PULLEY CLEVIS

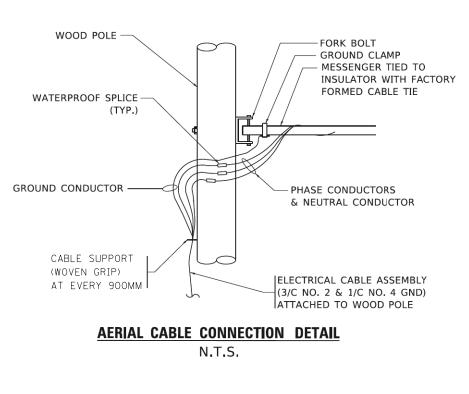
TO LUMINAIRE

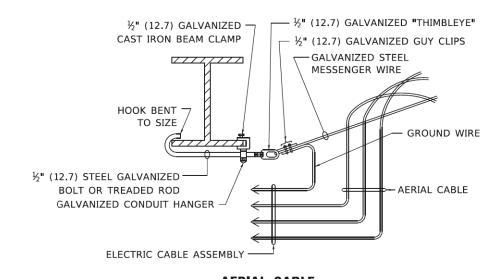
─ NEUTRAL CONDUCTOR

- PHASE CONDUCTOR

WATERPROOF FUSEHOLDER AND SOLID NEUTRAL SLUG

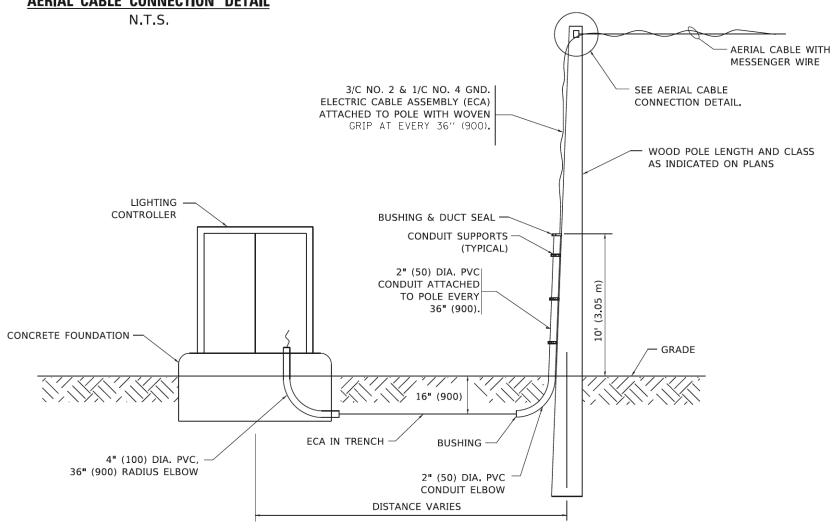
WATERPROOF FUSEHOLDER & FUSE





AERIAL CABLE ATTACHED TO STRUCTURE

NOT TO SCALE



NOTES:

- 1. ALL DIMENSIONS IN INCHES (MILLIMETERS) UNLESS OTHERWISE INDICATED.
- SEE PROPOSED LIGHTING PLAN FOR CONDUIT, CABLE AND ROUTING.
- 3. THE CONTRACTOR SHALL PROVIDE INTERMEDIATE SUPPORTS TO MAINTAIN MINIMUM CLEARANCES. REFER TO AERIAL AERIAL CABLE ATTACHED TO STRUCTURE DETAIL.
- 4. COST OF SPLICES AND MOUNTING HARDWARE SHALL BE INCLUDED IN THE UNIT PRICE FOR AERIAL CABLE.

WOOD POLE TO LIGHTING CONTROLLER WIRING CONNECTION DETAIL

N.T.S.

Transmart**

100 S. Wacker Drive Suite 400
Chicago, Illinois 60606

USER NAME = brvanderwal	DESIGNED	-	SMF	REVISED -
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PLOT DATE = 1/25/2025	DATE	-	1/25/2025	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

					FRONTENAC ROAD N DETAILS
SCALE:	SHEET 12	OF 12	SHEETS	STA.	TO STA.

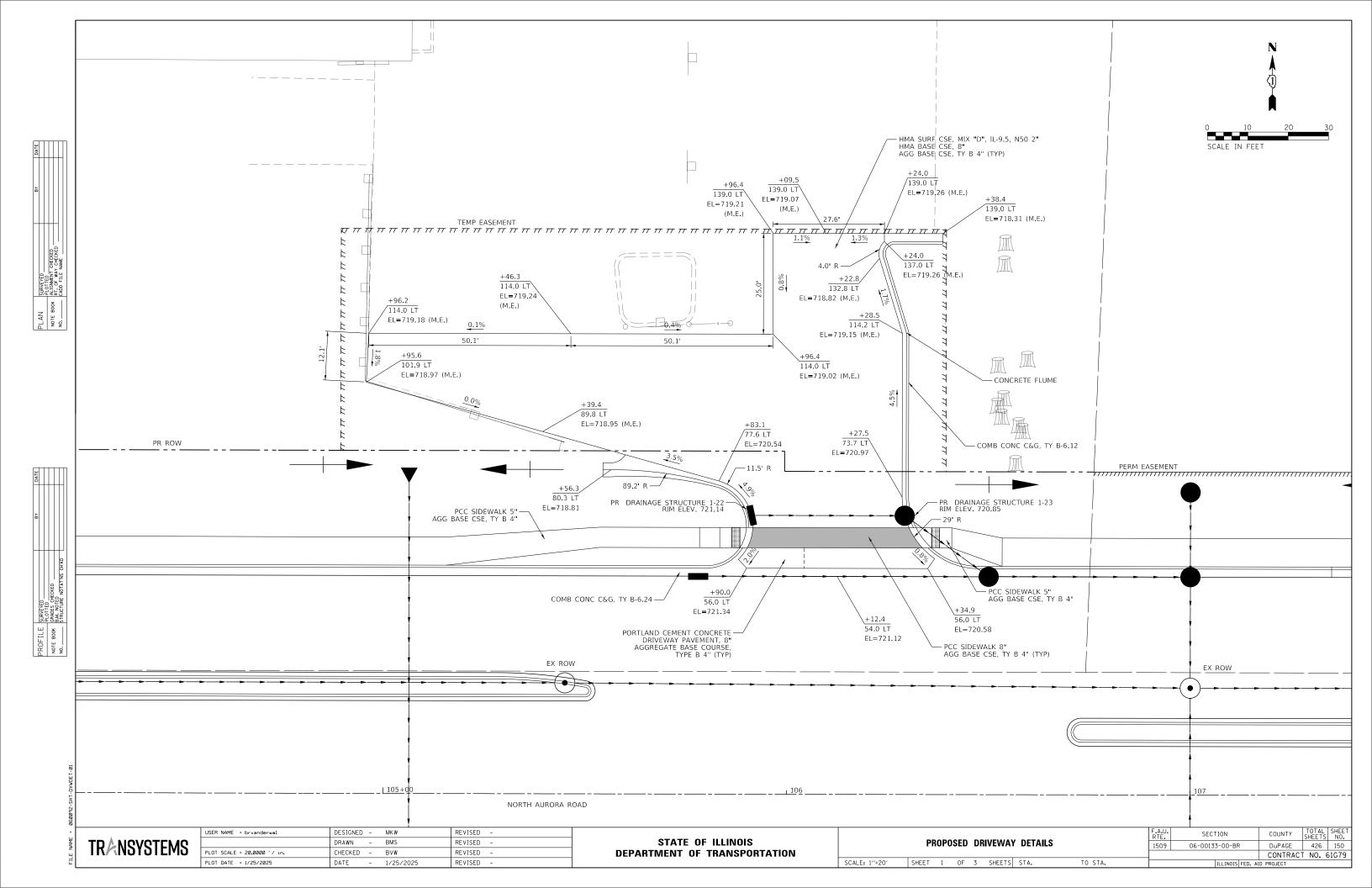
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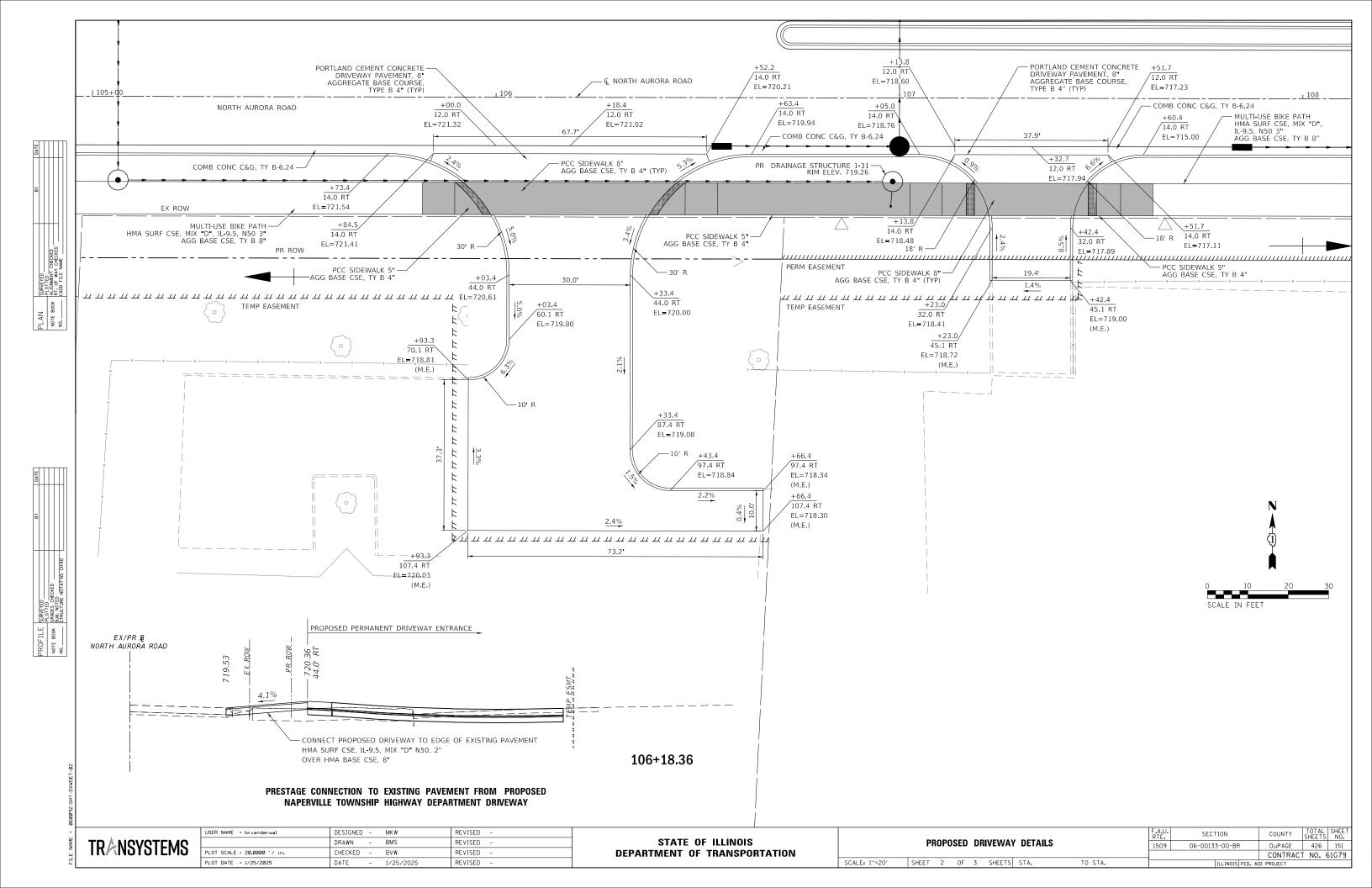
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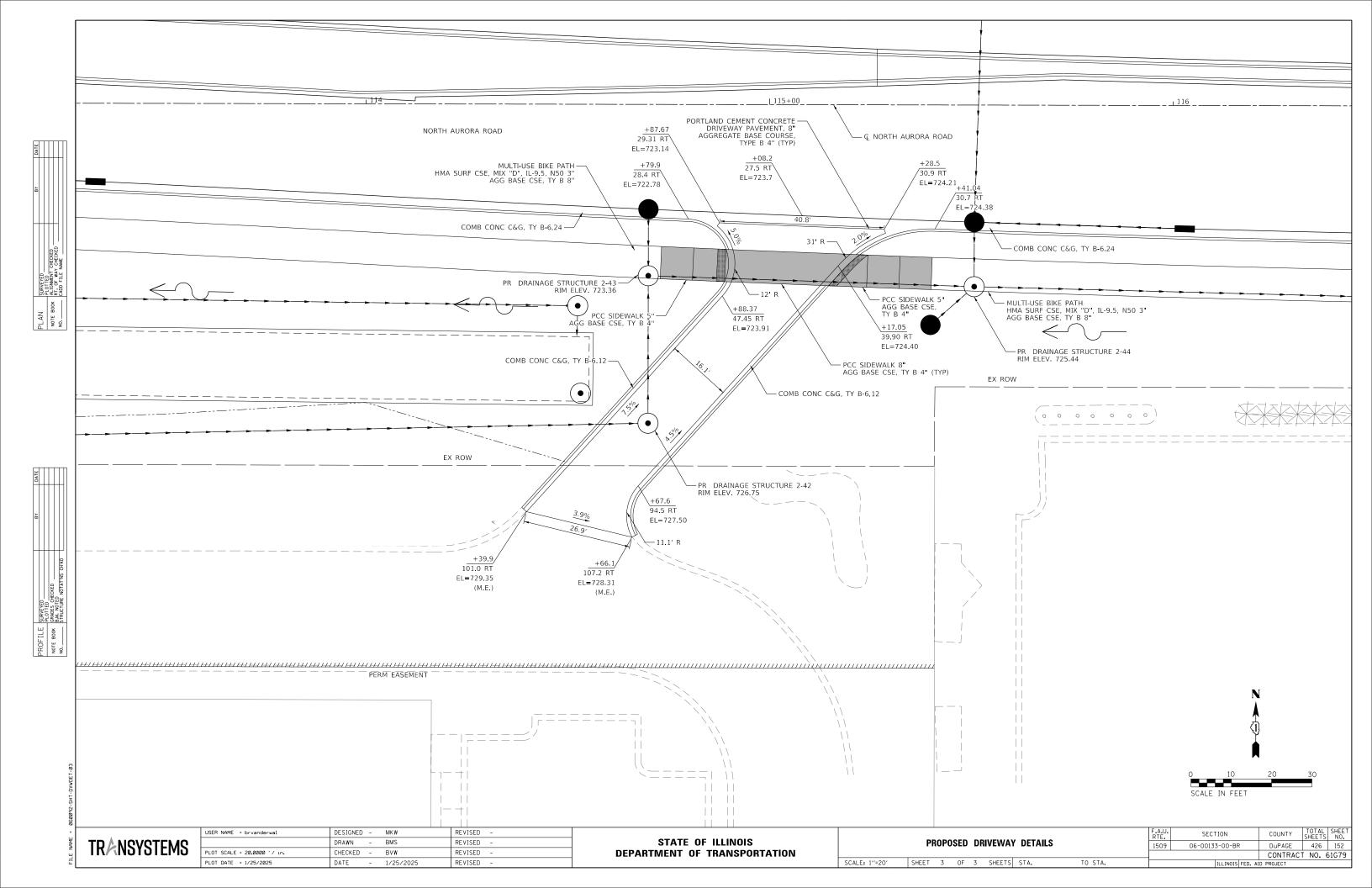
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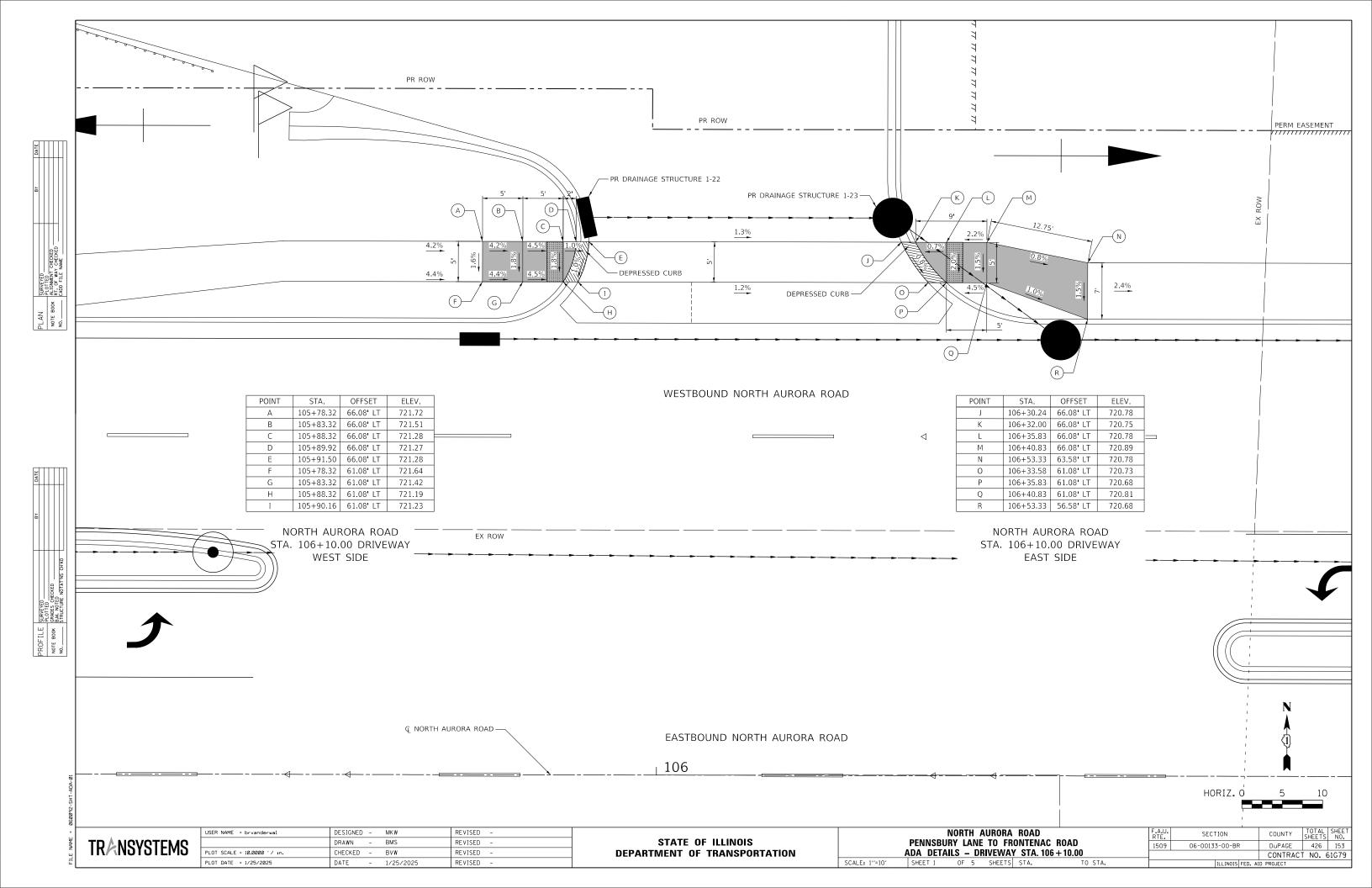
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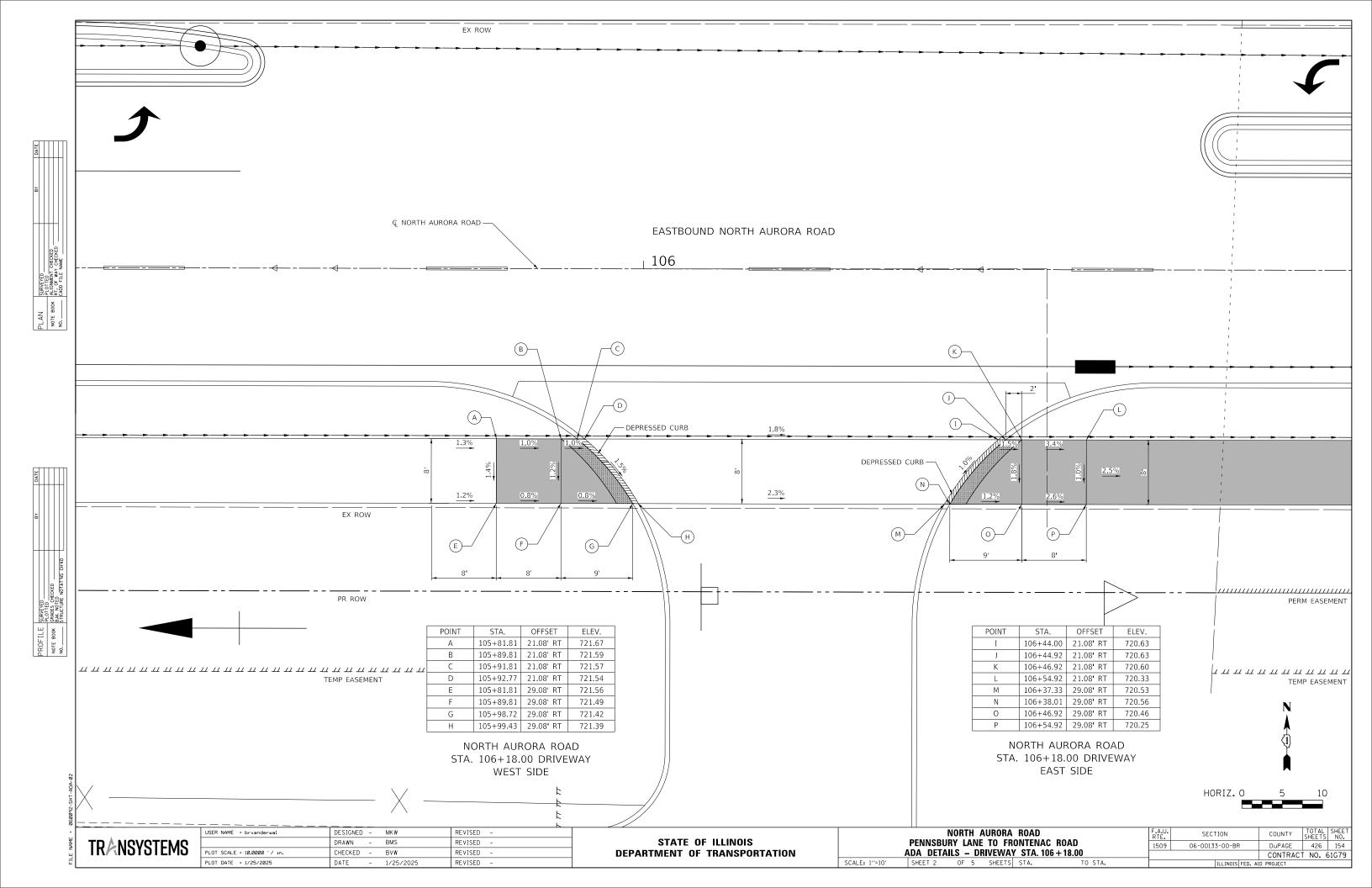
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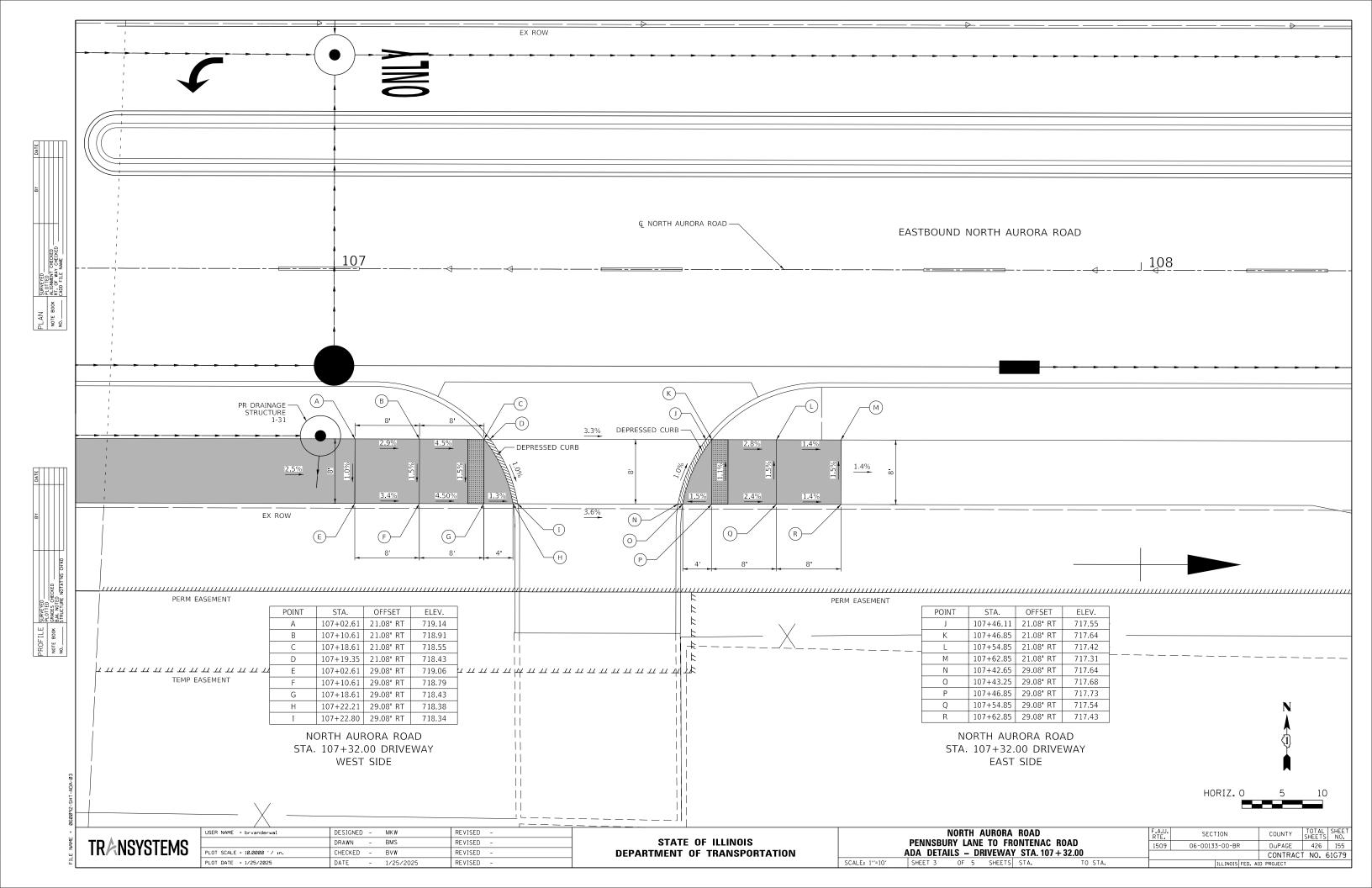


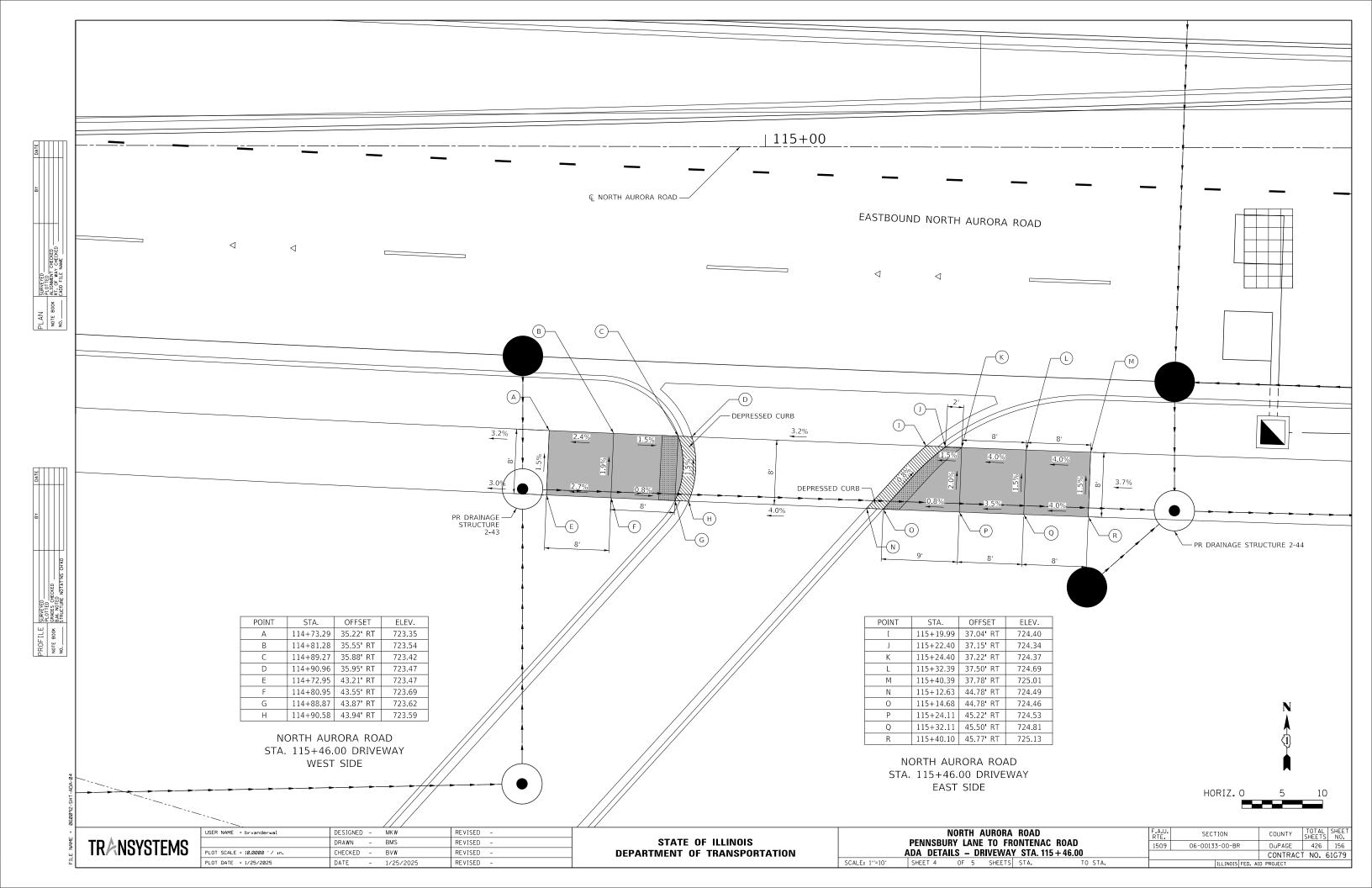


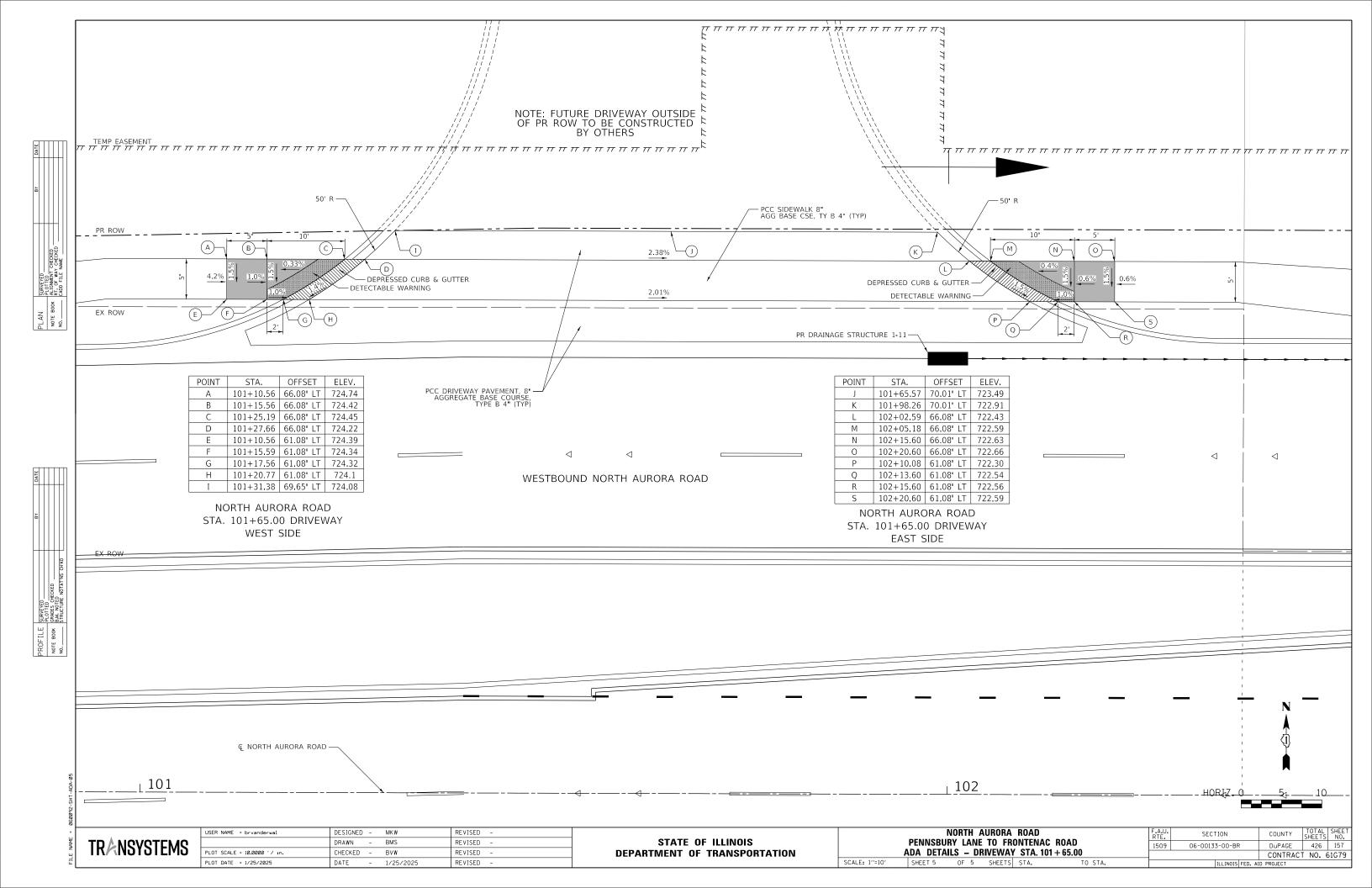


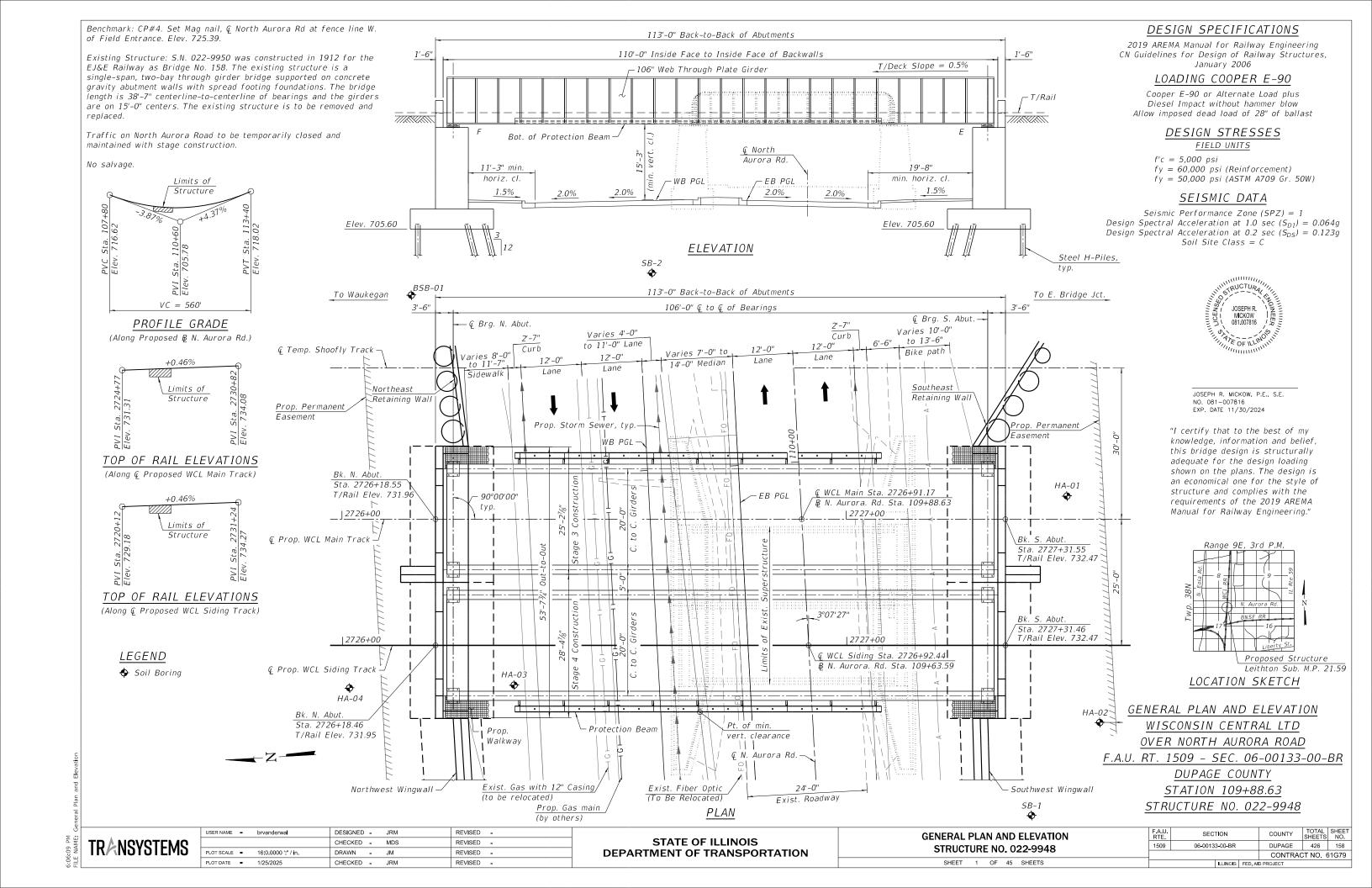












GENERAL NOTES

SPECIFICATIONS

- 1. Design is in accordance with the American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for Railway Engineering, 2019 edition as modified by CN Railway Guidelines for Design of Railway Structures, January 2006 revision. Steel and bearing design is in accordance with AREMA Chapter 15, Steel Structures. Concrete and foundation design is in accordance with AREMA Chapter 8, Concrete Structures and Foundations.
- 2. Workmanship and Materials shall be in accordance with the Standard Specifications for Road and Bridge Construction, Illinois Department of Transportation, adopted Jan. 1, 2022, and the Supplemental Specifications and Recurring Special Provisions, adopted Jan. 1, 2024. Steel fabrication shall be in accordance with the AREMA Specifications as modified by CN Railway's Specification HC05121 Structural Steel Fabrication for Railway Bridges (Modified).
- 3. All railroad related work of the contract shall be completed to the satisfaction of the Railroad Engineer. The decision of the Railroad Engineer shall be final on all questions which may arise regarding, including but not limited to, the quality and acceptability of materials and work; the manner of performance; acceptable rates of progress on the work; the interpretation of the contract plans and specifications; and the fulfillment of the contract.

STRUCTURAL STEEL

- 1. All structural steel shall be ASTM A709, Grade 50W, unless otherwise noted.
- 2. Fabrication of structural steel shall be performed by a fabricator certified under the AISC Certification Program, Category ABR, Advanced Bridges, with Fracture Critical Endorsement.
- 3. The webs and tension flanges of the through plate girders and end floor beams of the bridge are designated as "Fracture Critical Members" and shall conform to the fracture control plan for fracture critical members in the AREMA Specifications, Chapter 15 for Zone 3. These members are designated "F.C.M." on the plans.
- 4. The main load carrying components subjected to tensile stress, other than fracture critical members, shall conform to the supplemental requirements for Notch Toughness, Zone 3. These components are designated "N.T.R." on the plans.
- 5. Material noted on the plans to be corrosion-resistant (C.R.) steel shall conform to ASTM A709 Grade 50W.
- 6. The upper floor plate must have raised pattern conforming to the requirements of ASTM A786 and be corrosion resistant steel according to ASTM A709 Grade 50W.
- 7. Calculated weight of Structural Steel = 884,655 lbs. (ASTM A709 Grade 50W)
- 8. Furnishing and Erecting Structural Steel, Special shall include all steel components of the proposed through plate girder spans including bearings, anchor bolts, walkway bracketss, protection beams, all bolts and connections. See Special Provisions.
- 9. All bolted connections shall be made with high-strength bolts conforming to ASTM F3125 Grade A325, Type 3. Bolts shall be $\frac{7}{8}$ " diameter in $\frac{15}{16}$ " diameter holes, unless otherwise noted. Holes for shop fasteners shall be subpunched or subdrilled and reamed through a template in accordance with the Special Provisions. Bolts shall be tightened by the "turn-of-the-nut" method as described in the AREMA Specifications.
- 10. All bolted surfaces are designed for F3125 Grade A325 Class B (Slip Coefficient = 0.5) requiring all surfaces to be blast cleaned to a min. standard of SPSC 6.

WELDING

- 1. All welding shall be in accordance with the requirements of the current Bridge Welding Code, AWS D1.5 and the applicable provisions of Chapter 15 of the AREMA Specifications.
- All welding must be done with shielded metal-arc (SMAW) or submerged arc processes (SAW). All flange-to-web "T-Joint" welds and shop welded splices in flanges or webs shall be by approved continuous automatic feed and travel submerged arc welds. Electro-slag, electro-gas and gas metal-arc processes will not be permitted.
- 3. Preheating is required prior to flame cutting or welding:

<u> Material Thickness</u>	<u>Temperature</u>
>1½" to 2¾"	150 °F
>23/8"	225 °F

- 4. Welding procedures shall be submitted for the Engineer's review and approval prior to commencing any fabrication work. Welding procedures shall be prepared in accordance with the applicable requirements of Section 5 of the Bridge Welding Code AWS D1.5 and the AREMA Specifications.
- 5. No field welding is permitted except as specified in the contract documents.

TOTAL BILL OF MATERIAL

Item	Unit	Super	Sub	Total
Porous Granular Embankment	Cu. Yd.		386	386
Removal Of Existing Structures	Each			1
Structure Excavation	Cu. Yd.		2,848	2,848
Form Liner Textured Surface	Sq. Ft.		2,493	2,493
Reinforcement Bars, Epoxy Coated	Pound		71,110	71,110
Bar Splicers	Each		178	178
Pipe Handrail	Foot		10	10
Furnishing Steel Piles HP14x89	Foot		9,090	9,090
Driving Piles	Foot		9,090	9,090
Test Pile Steel HP14x89	Each		2	2
Pile Shoes	Each		106	106
Name Plates	Each		1	1
Drainage System for Structures	L Sum		1	1
Geocomposite Wall Drain	Sq. Yd.		304	304
Pipe Underdrains For Structures 4"	Foot		84	84
Concrete Gutter, Type B	Foot		44	44
Conduit Attached to Structure,	Foot	226		226
4" Dia., Galvanized Steel				
Concrete Structures (Special)	Cu. Yd.		900.5	900.5
Furnishing And Erecting Structural Steel, Special	L. Sum.	1		1
Temporary Soil Retention System (Special)	Sq. Ft.		7,011	7,011
Membrane Waterproofing (Special)	Sq. Ft.	4,064		4,064
Chain Link Fence, 4' Attached To Structure	Foot		44	44
Furnish and Install Walkway	Foot	227		227
Anti-Graffiti Protection System	Sq. Ft.		3,215	3,215
Structural Repair of Concrete	Sq. Ft.		25	25
(Depth Greater Than 5 Inches)				

1112	<u> </u>
1	General Plan and Elevation
2	General Data
3	Foundation Layout Plan
4	Stage Construction Details
5	Construction Staging Plan - Stage 3
6	Construction Staging Plan - Stage 4
7	Temporary Soil Retention System General Notes
8	Temporary Soil Retention System Plan - Stage 3
9	Temporary Soil Retention System Plan - Stage 4
10	Temporary Soil Retention System Elev N. Abut. Stage
11	Temporary Soil Retention System Elev S. Abut. Stage
12	Temporary Soil Retention System Elev N. Abut. Stage
13	Temporary Soil Retention System Elev S. Abut. Stage
14	Temporary Soil Retention System Details
15	Existing Structure Removal Details 1
16	Existing Structure Removal Details 2
17	North Abutment Plan and Elevation
18	North Abutment Footing Plan
19	North Abutment Details 1
20	North Abutment Details 2
21	South Abutment Plan and Elevation
22	South Abutment Footing Plan
23	South Abutment Details 1
24	South Abutment Details 2
25	Deck Cross Section
26	Framing Plan
27	Girder Details 1
28	Girder Details 2
29	Design Data
30	End Floorbeam Details
31	Lateral Bracing Details
32	Floor Plate Plan
33	Ballast Stop Plate Details
34	Upper Floor Plate Details
35	Fixed Bearing Details
<i>36</i>	Expansion Bearing Details
<i>37</i>	Walkway Details
<i>38</i>	Protection Beam Details
<i>39</i>	Drainage Details
40	Bar Splicer Assembly and Mechanical Splicer Details

INDEX OF SHEETS

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Boring Logs 1

Boring Logs 2

Boring Logs 3

Boring Logs 4 Boring Logs 5

GENERAL NOTES (cont.)

1. Plan dimensions and details relative to the existing structure have been taken from existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

CAST-IN-PLACE CONCRETE

- 1. Concrete shall have a minimum compressive strength of 5,000 psi in 28 days.
- 2. Reinforcement bars designated (E) shall be epoxy coated in accordance with ASTM A775.
- 3. All exposed corners shall be chamfered $\frac{3}{4}$ ", unless otherwise noted.
- 4. Anti-Graffiti Protection System shall be applied to the exposed areas of the abutments and wingwalls.

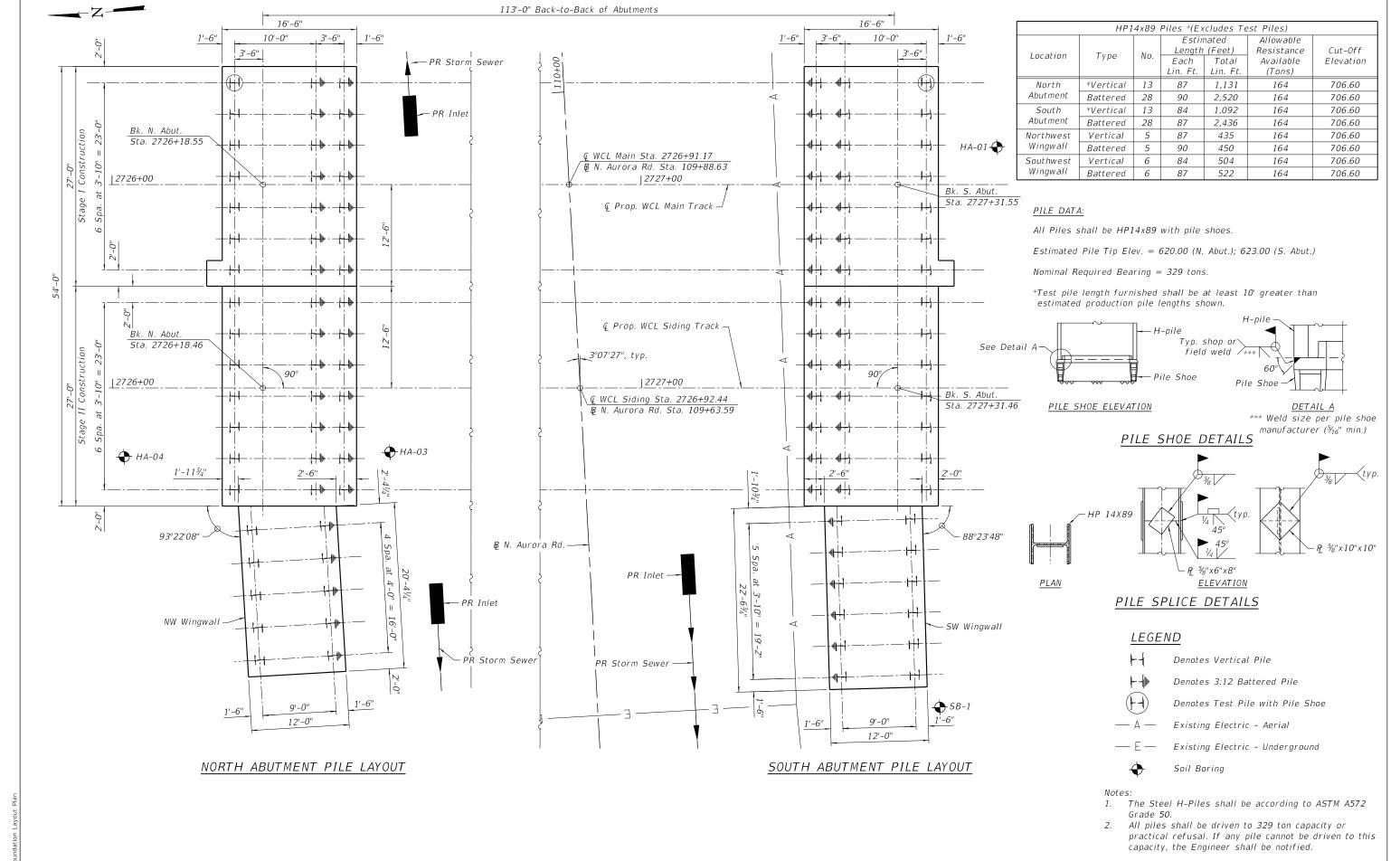
WISCONSIN CENTRAL R.R.
BUILT BY
CITY OF NAPERVILLE
F.A.U. RTE. 1509
SEC. 06-00133-00-BR
STA. 109+88.63 LOADING E90
STRUCTURE NO. 022-9948

NAME PLATE
See Std. 515001

150

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				CONTRAC	T NO. 6	31G79
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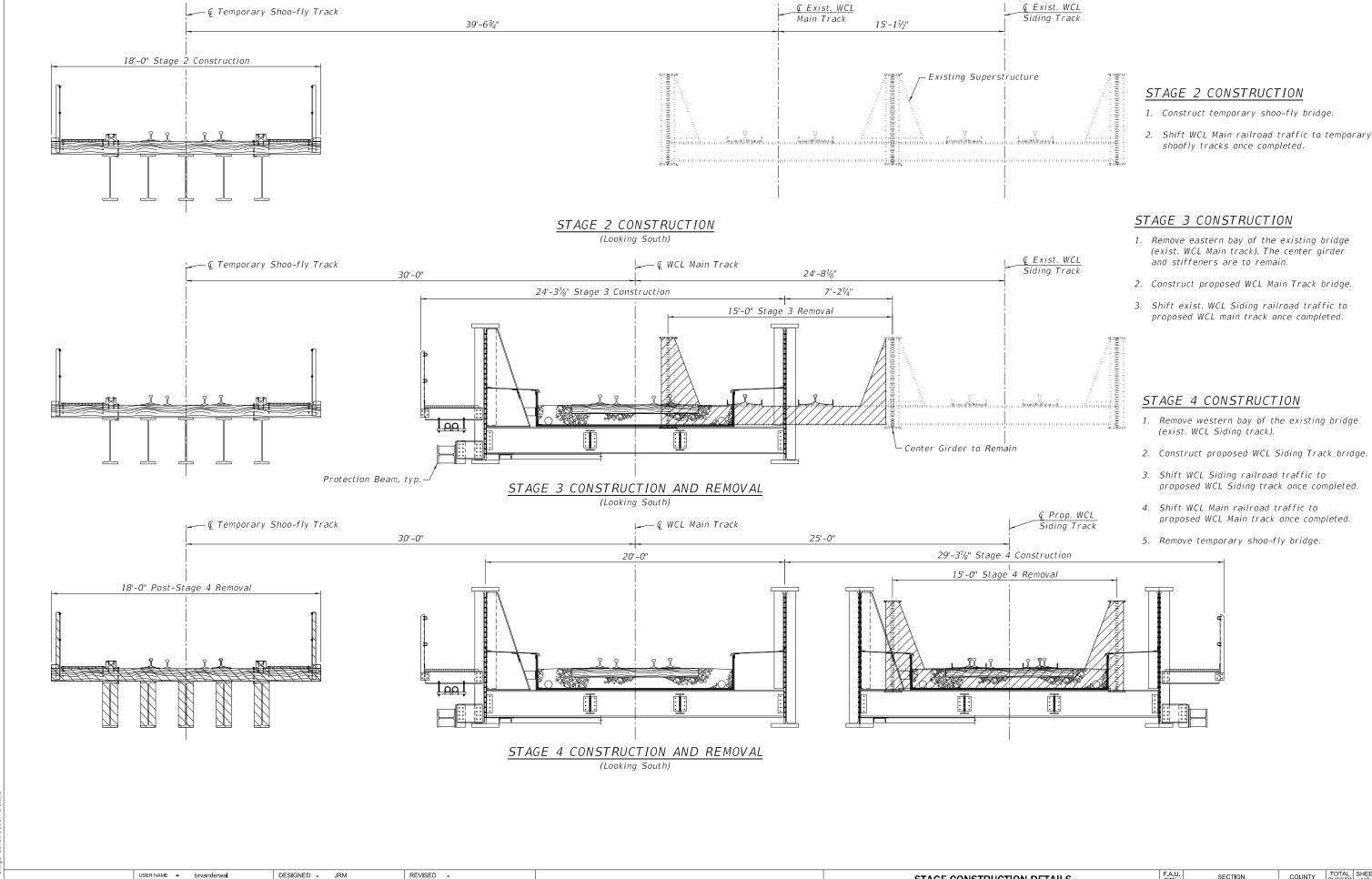


TRANSYSTEMS

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

FOUNDATION LAYOUT PLAN STRUCTURE NO. 022-9948

SHEET 3 OF 45 SHEETS



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TRANSYSTEMS

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STAGE CONSTRUCTION DETAILS
STRUCTURE NO. 022-9948

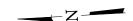
SHEET 4 OF 45 SHEETS

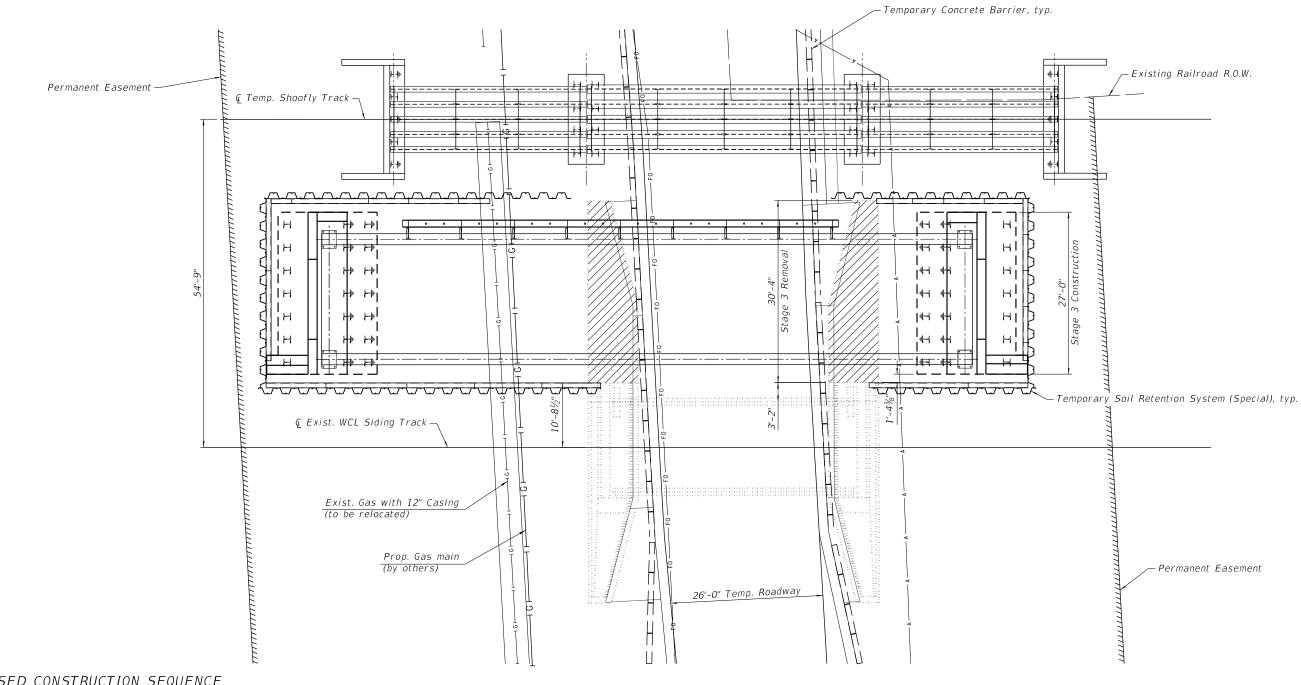
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 SECTION
 COUNTY SHEETS NO.
 SHEET NO.

 1509
 06-00133-00-BR
 DUPAGE
 426
 161

 CONTRACT NO. 61G79

 ILLINOIS FED. AID PROJECT





PROPOSED CONSTRUCTION SEQUENCE

- 1. Remove the existing main track bridge superstructure (center girder remains).
- 2. Construct the temporary retention systems.
- 3. Remove the Stage 3 portion of the existing substructure.
- 4. Excavate for each abutment.
- 5. Construct the main track bridge abutments and superstructure.
- 6. Install temporary closure plates according to Details on sheet 9 of 45.
- 7. Remove the temporary retention systems with sections to remain as shown on sheets 8 through 13.
- 8. Shift exist. WCL Siding railroad traffic to prop. WCL Main track.

PLAN - STAGE 3

For Temporary Soil Retention System (Special) Details, see Sheets 7 through 14 of 45.

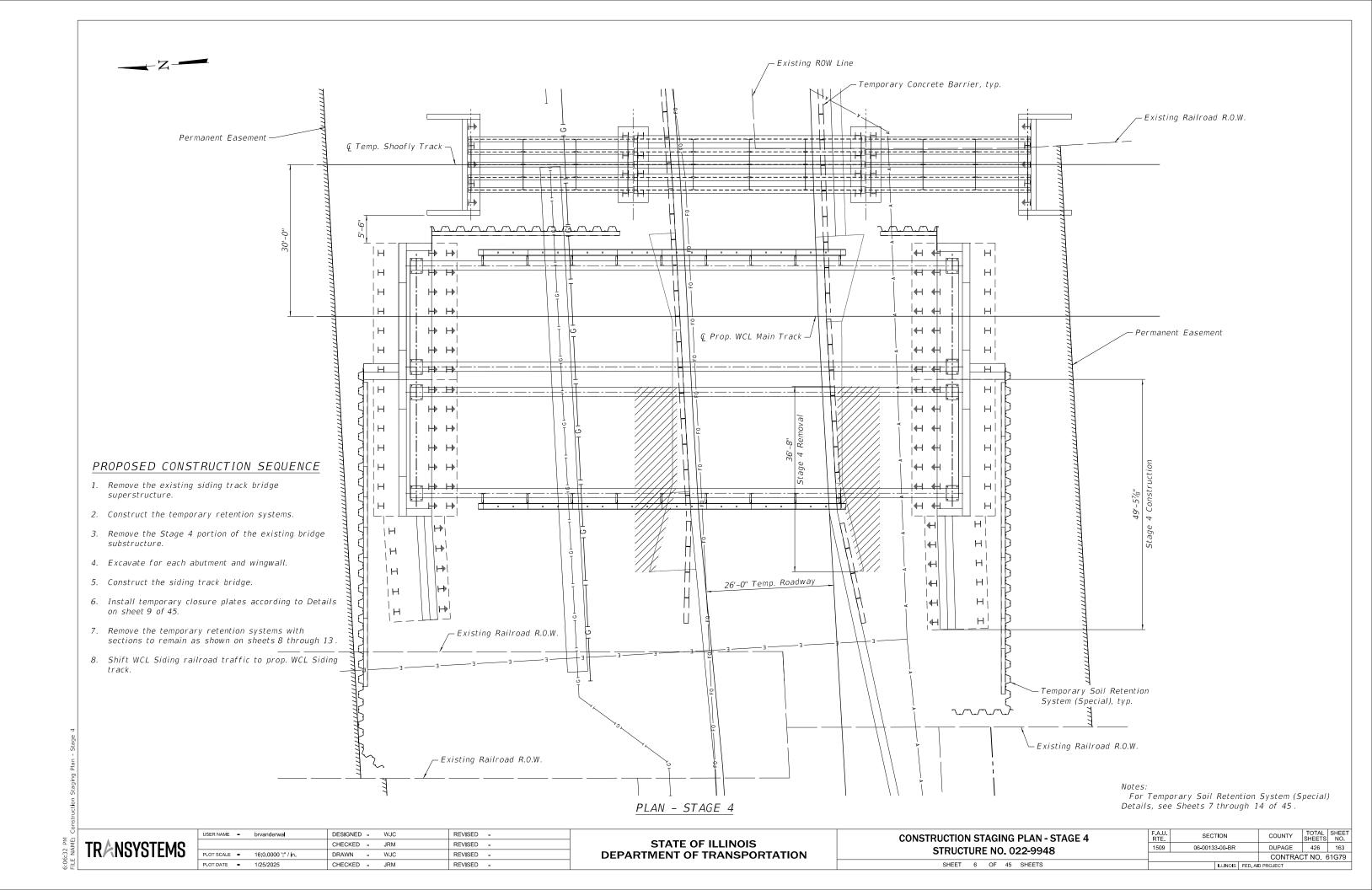
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **CONSTRUCTION STAGING PLAN - STAGE 3 STRUCTURE NO. 022-9948** SHEET 5 OF 45 SHEETS

A.U. TE	SEC.	TION	COUNTY	TOTAL SHEETS	SHE
509	06-0013	3-00-BR	DUPAGE	426	16
			CONTRAC	CT NO. 6	31G7

ILLINOIS FED. AID PROJECT



GENERAL NOTES - TEMPORARY SOIL RETENTION SYSTEM

PRE-CONSTRUCTION REQUIREMENTS:

- 1. The Contractor shall be responsible for confirming or establishing the existence of all utility facilities relevant to their exact locations, and schedule all necessary utility relocations.
- 2. Contractor must coordinate all sheet pile and pile driving activities with Nicor for the construction of the temporary soil retention system.
- 3. All elevations and dimensions must be verified in the field.

REGROUTABLE GROUND ANCHORS:

- 1. Drill approximately a minimum 8-inch diameter hole to the lengths shown on the Anchor Schedule. Anchor locations and elevations are shown on plan and elevations. Tremie grout the anchor hole. Insert tendon after the anchor hole is filled with grout.
- 2. Regrout the bond length as necessary to develop the required anchor capacity.
- Allow grout to harden a minimum of five (5) days. Then tension each anchor in accordance with the anchor testing procedures.
- 4. All anchors within the Railroad R.O.W. shall be fully removed.

MATERIALS:

Steel:

ASTM A 572, Grade 50 for steel sheet piles

ASTM A 572, Grade 50 for steel walers and bearing plates

Anchor: 11/4" diameter, 150 ksi Grade, conforming to AASHTO M275 (ASTM A722), with Class I corrosion protection (double).

Neat cement grout having a 28-day compressive strength of 3,000 psi, consisting of portland cement Type I, II or III, and conforming to Section Grout: 1001 of the Standard Specifications. Testing of grout is not required since grout will be tested with anchor stressing as part of system

performance.

Welds: E70XX weld strength level, low hydrogen electrode. Testing of welds is not required since welds will be tested with anchor stressing as part of

system performance.

Misc: Bolts, nuts and washers shall conform to the requirements of ASTM F3125 Grade A325, A563 and F436, respectively. Pipe spacers shall

conform to the requirements of ASTM A53.

Loads: Anchor Types 1, 3, and 4 were design to withstand E80 loading, while Anchor Types 2, 5, and 6 were designed for a construction load of 300

Equivalent members may be substituted for those shown. If alternate members are used, the Contractor must submit a re-design of the wall for approval.

For more information, see the Special Provisions for Temporary Soil Retention System (Special), Ground Anchors, and Helical Ground Anchors.

Lateral live load deflection shall be limited to \(^3/8\)" for shoring located within 18'-0" of centerline of track and 1/3" for shoring located outside 18'-0". The maximum allowable vertical or horizontal displacement of rail shall be \(\frac{1}{4}''.

See sheet 14 of 45 for Ground Anchor Schedule.

ANCHOR TESTING PROCEDURES

PERFORMANCE TEST

Five percent of the ground anchors or a minimum of three ground anchors, whichever is greater, shall be performance-tested according to the following procedures. The Engineer shall select the ground anchors to be performance tested. The remaining anchors shall be tested according to the proof test procedures.

The performance test shall be made by incrementally loading and unloading the ground anchors in accordance with the following schedule:

PERFORMANCE TEST SCHEDULE

LUAD	LUAD	LUAD		
AL	AL	1.00DL		
0.25DL*	0.25DL	1.20DL*		
AL	0.50DL	AL	Where:	AL = Alignment Load
0.25DL	0.75DL	0.25DL		DL = Design Load
0.50DL*	1.00DL*	0.50DL		
AL	AL	0.75DL	*Gra	ph Required
0.25DL	0.25DL	1.00DL		
0.50DL	0.50DL	1.20DL		
0.75DL*	0.75DL	1.33DL* (Max	test load)	
		Reduce to loc	ck-off load (1.0	OODL)

The load shall be raised from one increment to another immediately after recording the ground anchor movement

The ground anchor movement, on either side of the jack, shall be measured and recorded to the nearest 0.001 inch with respect to the independant fixed reference points at the alignment load and at each load increment

The load shall be monitored with a presure gauge.

The reference pressure gauge shall be placed in series with the pressure gauge during each performance test.

The maximum test load in a performance test shall be held for 10 minutes. The jack shall be repumped as necessary in order to maintain a constant load. The load-hold period shall start as soon as the maximum test load is applied and the ground anchor movement shall be measured and recorded at 1 minute, 2, 3, 4, 5, 6, and 10 minutes. If the ground anchor movements between 1 minute and 10 minutes exceeds 0.04 inch, the maximum test load shall be held for an additional 50 minutes. If the load-hold is extended, the ground anchor movement shall be recorded at 15 minutes, 20, 25, 30, 45 and 60 minutes.

A graph shall be constructed showing a plot of ground anchor movement versus load for each load increment marked with the asterisk (*) in the performance test schedule and a plot of the residual ground anchor movement of the tendon at each alignment load versus the highest previously applied load. Graph shall be approved by the Engineer prior to use.

PROOF TEST

The proof test shall be performed by incrementally loading the ground anchors in accordance with the following schedule:

PROOF TEST SCHEDULE

<u>LOAD</u>	<u>LOAD</u>		
AL	1.00DL		
0.25DL	1.20DL	Where:	AL = Alignment Load
0.50DL	1.33DL (Max test load)		DL = Design Load
0.75DL	Reduce to lock-off load (1)	.0DL)	

The load shall be raised from one increment to another immediately after recording the ground anchor movement

The ground anchor movement, on either side of the jack, shall be measured and recorded to the nearest 0.001 inch with respect to the independant fixed reference points at the alignment load and at each load increment

The load shall be monitored with a presure gauge.

At load increments other than the maximum test load, the load shall be held just long enough to obtain the movement reading.

The maximum test load in a proof test shall be held for 10 minutes. The load-hold period shall start as soon as the maximum test load is applied and the ground anchor movement shall be measured and recorded at 1 minute, 2, 3, 4, 5, 6, and 10 minutes. If the ground anchor movements between 1 minute and 10 minutes exceeds 0.04 inch, the maximum test load shall be held for an additional 50 minutes. If the load-hold is extended, the ground anchor movement shall be recorded at 15 minutes, 20, 25, 30, 45 and 60 minutes.

A graph shall be constructed showing a plot of ground anchor movement versus load for each load increment in the proof test. Graph shall be approved by the Engineer prior to use.

ANCHOR LOAD TEST ACCEPTANCE CRITERIA

A tested ground anchor with a 10 minute load-hold is acceptable if:

- A. The ground anchor will carry the maximum test load with less than 0.04 inches of movement between 1 minute and 10 minutes, and
- B. The total movement at the maximum test load exceeds 80% of the theoretical elongation of the unbonded length.

A tested ground anchor with a 60 minute load-hold is acceptable if:

- A. The anchor will carry the maximum test load with a creep rate that does not exceed 0.08 inches in the last log cycle of time, and
- B. The total movement at the maximum test load exceeds 80% of the theoretical elongation of the unbonded length.

A ground anchor which has a creep rate greater than 0.08 inch per log cycle of time can be incorporated into the structure at a design load equal to one-half of its failure load. The failure load is the load resisted by the ground anchor after the load has been allowed to stabilize for 10 minutes.

When a ground anchor fails, the Contractor shall modify the design and/or the installation procedures. These modifications may include, but are not limited to, installing a replacement ground anchor, reducing the design load by increasing the number of ground anchors, modifying the installation methods, increasing the bond length or changing the ground anchor type.

Lock-Off: Upon completion of the anchor test, the load must be reduced to the lock-off load indicated on the performance and proof test schedules and transferred to the anchorage device. The anchor may be completely unloaded prior to lock-off. After transferring the load, and prior to removing the jack, a lift-off reading shall be taken. The lift-off reading shall be within 10 percent of the specified lock-off load, the anchorage shall be reset and another lift-off reading shall be made.

PERFORMANCE TEST DATA

	% DL	LOAD	PRESS.	DIAL	% DL	LOAD	PRESS.	DIAL
Ī	AL				AL			
Ī	25*				25			
Ī	AL				50			
Ī	25				75			
Ī	<i>50</i> *				100			
Ī	AL				120*			
	25				AL			
	50				25			
	<i>75</i> *				50			
Ī	AL				75			
	25				100			
	50				120*			
	75				133*			
	100*				L.O.			

Performance Test Remarks:	
Anchor No.	Design Load:

PROOF TEST DATA

% DL	LOAD	PRESS.	DIAL	DEFL.	REMARKS
AL					
25					
50					
75					
100					
120					
133					
L.O.					

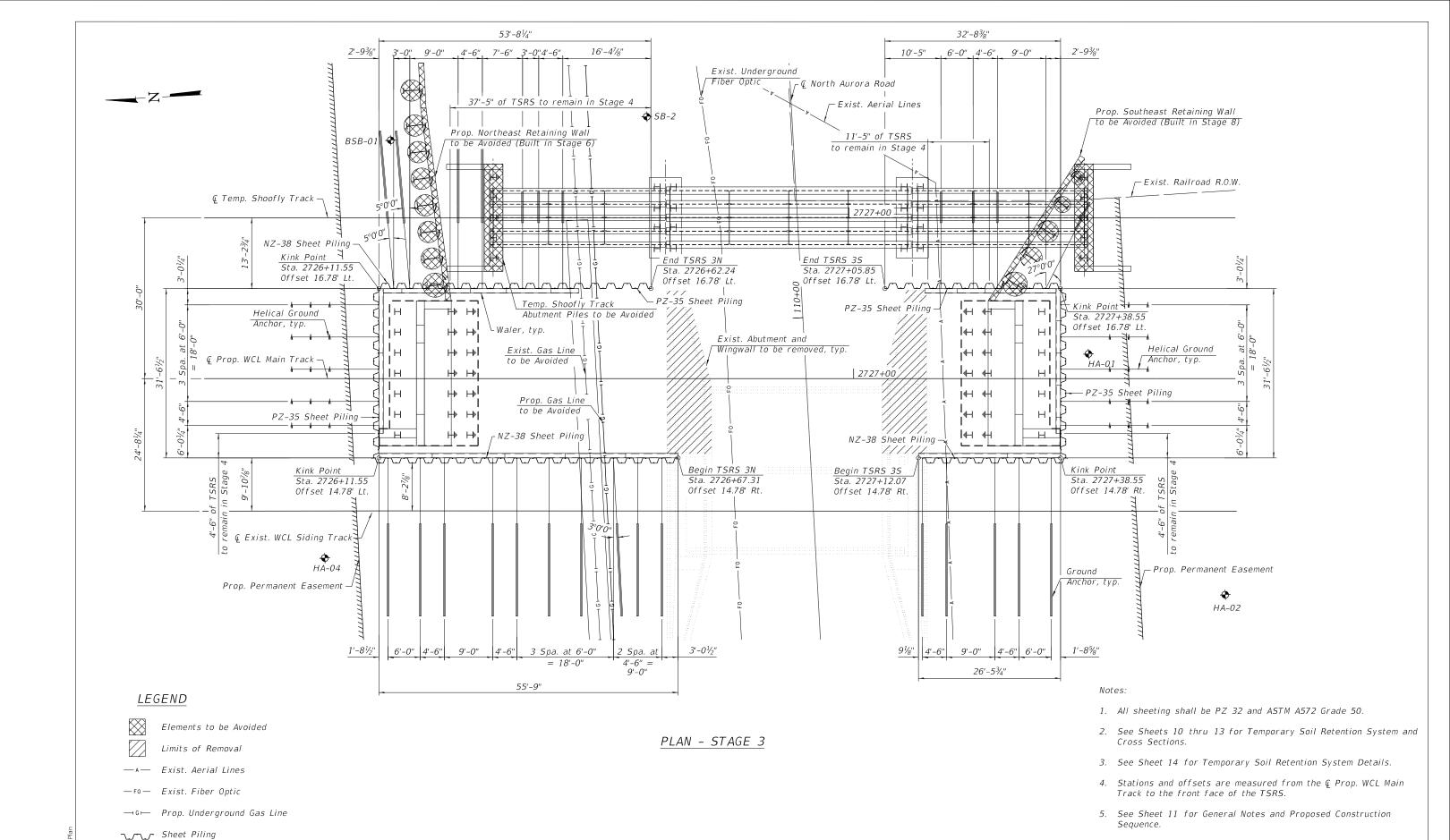
Performance Test Remarks:	
Anchor No	Design Load:

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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** TEMPORARY SOIL RETENTION SYSTEM GENERAL NOTES **STRUCTURE NO. 022-9948** SHEET 7 OF 45 SHEETS

SECTION COUNTY 1509 06-00133-00-BR DUPAGE 426 164 CONTRACT NO. 61G79 ILLINOIS | FED. AID PROJECT



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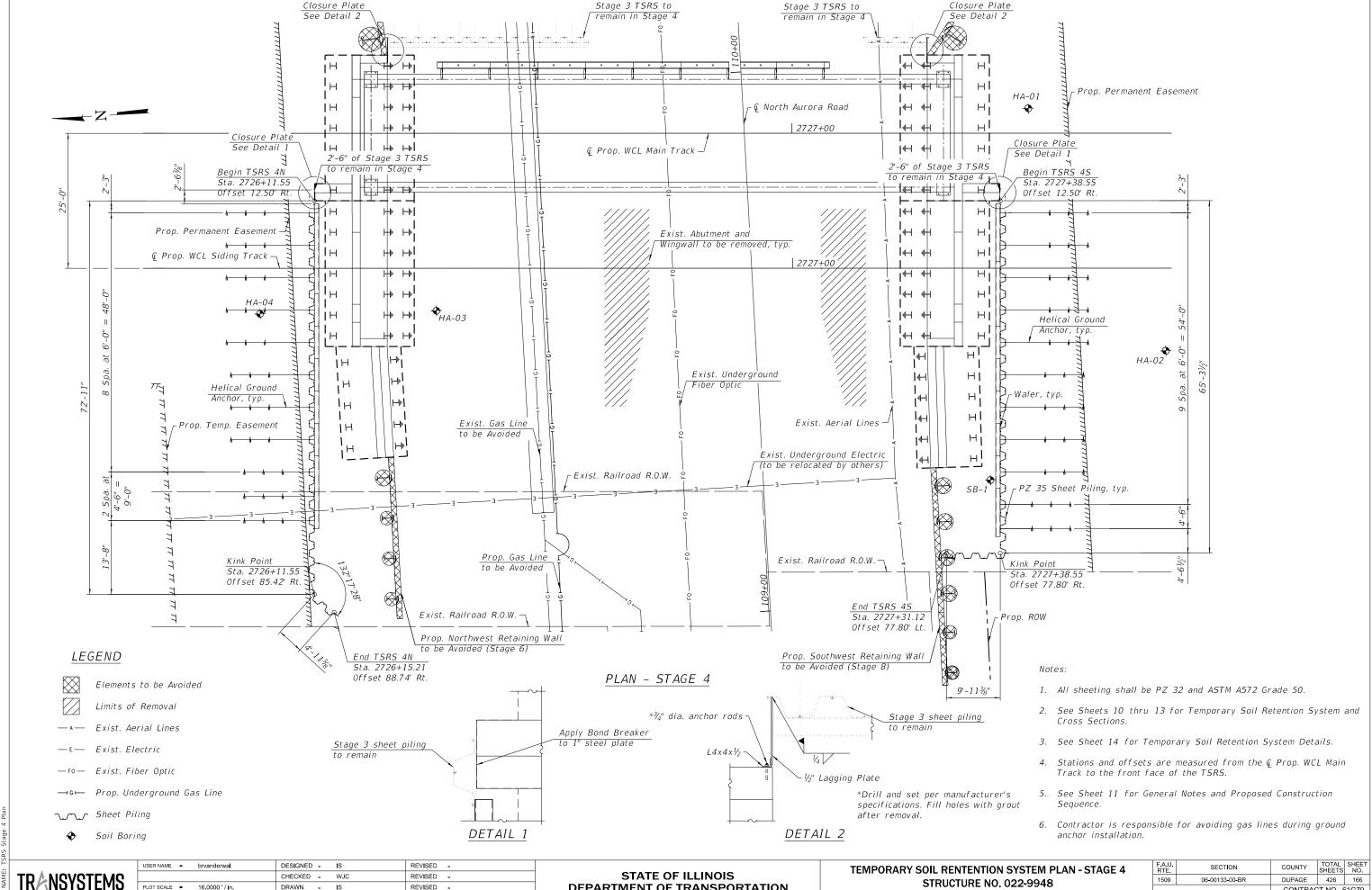
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION COUNTY 1509 06-00133-00-BR DUPAGE 426 165 CONTRACT NO. 61G79 ILLINOIS FED, AID PROJECT

6. Contractor is responsible for avoiding gas lines during ground

anchor installation

TEMPORARY SOIL RETENTION SYSTEM PLAN - STAGE 3 STRUCTURE NO. 022-9948 SHEET 8 OF 45 SHEETS



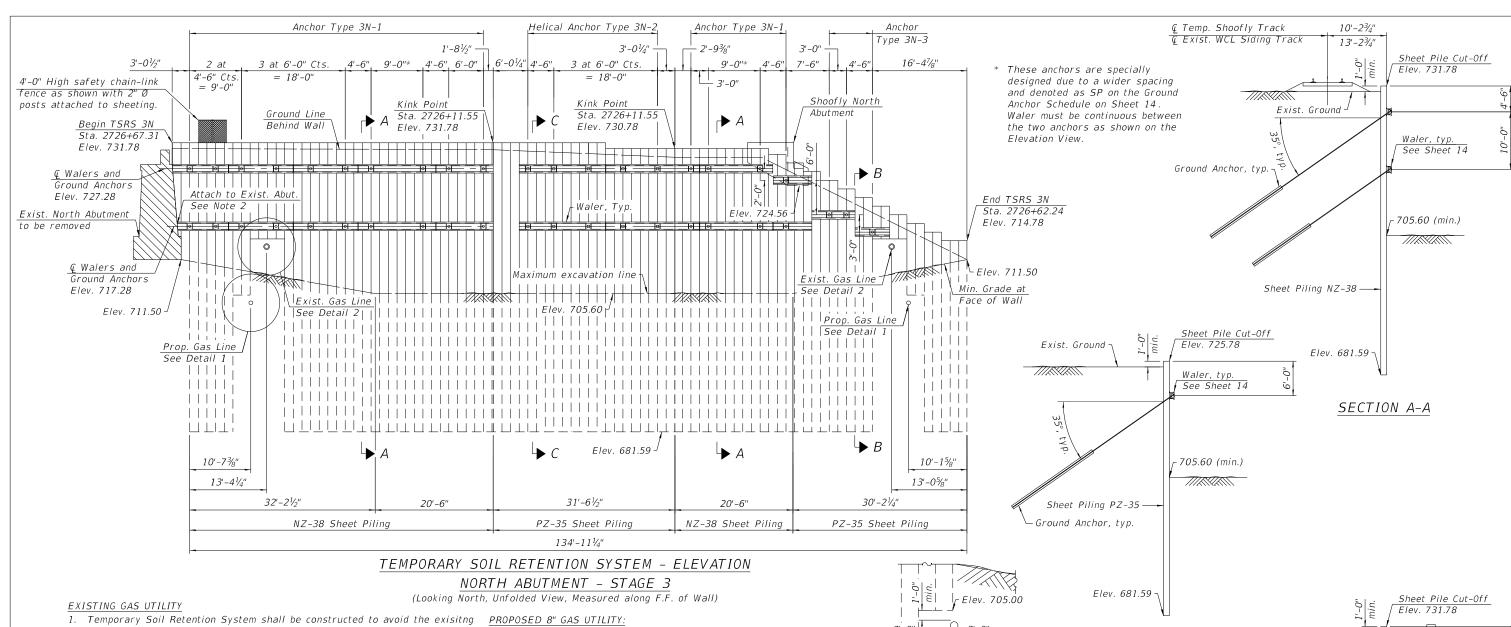
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DEPARTMENT OF TRANSPORTATION

SHEET 9 OF 45 SHEETS

CONTRACT NO. 61G79



- 1. Temporary Soil Retention System shall be constructed to avoid the exisiting gas line. After installation and excavation of the Temporary Soil Retention System, the portion of the exisiting gas line within the limits of the Temporary Soil Retention System shall be removed.
- 2. Before driving sheet piling over the existing utility structure, the Contractor shall verify the location and depth of the structure by probing.
- 3. Sheet piling shall not be driven to embedment depth shown on plans at the location of the existing gas line as shown on Sheet 8.
- 4. Once the existing gas main is removed, the gap within the Temporary Soil Retention System will then be covered with Timber Lagging.

TSRS 3N BILL OF MATERIAL

Item	Unit	Total
Temporary Soil Retention System (Special)	Sq. Ft.	2,856
*Walers	Pound	19,730
*Ground Anchors	Each	31
*Helical Ground Anchors	Each	10

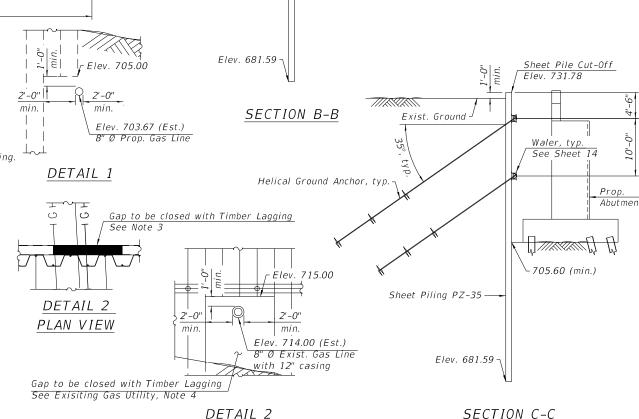
*For information only. Cost included with Temporary Soil Retention System (Special).

(Walers include weight of all structural steel related to waler system.)

- 1. The proposed gas line will be installed by utility company before the installation of TSRS. Service shall be maintained through the proposed gas line throughout the duration of the project.
- 2. Before driving sheet piling over the proposed utility structure, the Contractor shall verify the location and depth of the structure by probing.
- 3. Sheet piling shall not be driven to embedment depth shown on plans at the location of the proposed gas line as shown on Sheet 8.
- 4. Prior to driving sheet piling, the Contractor shall submit to the Engineer a contingency plan in the event the gas line is damaged or becomes unserviceable during construction. Any disruptions to the proposed gas line utility service shall be repaired at the expense of the Contractor.

Note:

- . See Sheet 11 for General Notes and Proposed Construction Sequence.
- See Sheet 14 for Backwall to Sheet Pile Connection Detail and Fence Connection Detail.
- 4. Timber lagging shall be Sourthern Pine, No. 2 with a minimum allowable bending stress of 1000 psi and no less than a 3 in. nominal rough-sawn thickness.

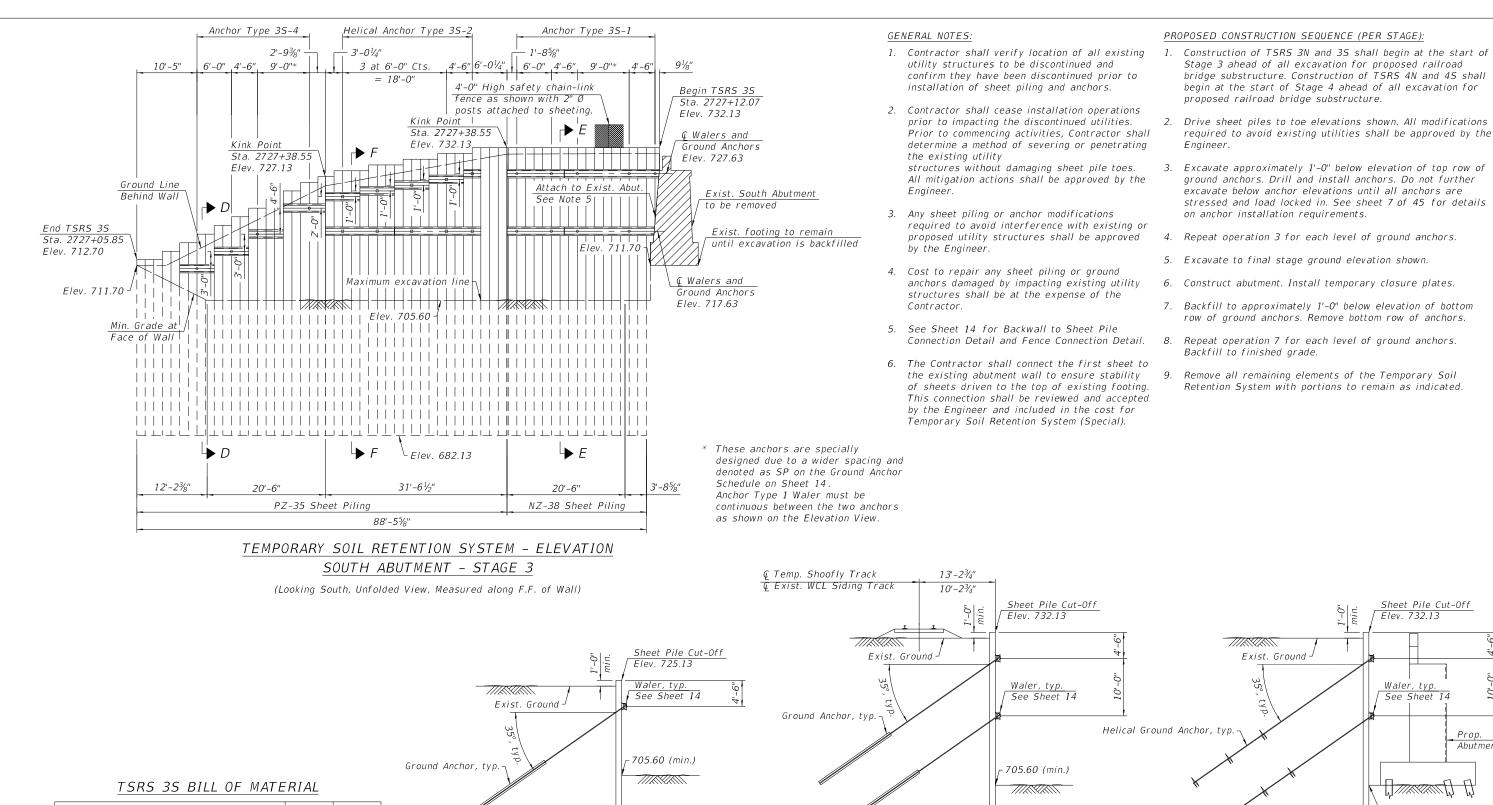




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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TEMPORARY SOIL RETENTION SYSTEM ELEV N. ABUT - STAGE 3	F.A.U. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
STRUCTURE NO. 022-9948	1509	06-00133-00-BR	DUPAGE	426	167
311(00101(E1(0), 022-3340			CONTRAC	CT NO. 6	31G79
SHEET 10 OF 45 SHEETS		ILLINOIS F	ED, AID PROJECT		



Item	Unit	Total
Temporary Soil Retention System (Special)	Sq. Ft.	1,723
*Walers	Pound	11,780
*Ground Anchors	Each	13
*Helical Ground Anchors	Each	10

*For information only. Cost included with Temporary Soil Retention System (Special).

(Walers include weight of all structural steel related to waler system.)

SECTION D-D

Sheet Piling PZ 35

Elev. 682.13 -

SECTION E-E

Sheet Piling NZ-38

Elev. 682.13 -

SECTION F-F

TRANSYSTEMS

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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

TEMPORARY SOIL RETENTION SYSTEM ELEV S. ABUT - STAGE 3	F
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SHEET 11 OF 45 SHEETS

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509	06-0013	3-00-BR		DUPAGE	426	168		
			CONTRAC	T NO. 6	31G79			

Sheet Pile Cut-Off

Elev. 732.13

Waler, typ.

See Sheet 14

-705.60 (min.)

Prop. Abutment

Exist. Ground

Sheet Piling PZ-35 -

Elev. 682.13 -

TEMPORARY SOIL RETENTION SYSTEM - ELEVATION NORTH ABUTMENT - STAGE 4

(Looking North, Unfolded View, Measured along F.F. of Wall)

TSRS 4N BILL OF MATERIAL

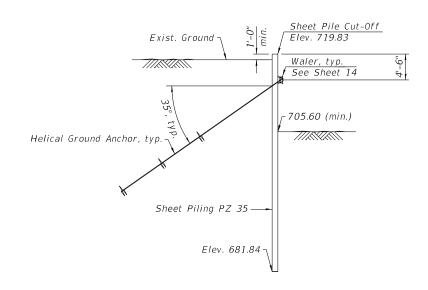
Item	Unit	Total
Temporary Soil Retention System (Special)	Sq. Ft.	1,163
*Walers	Pound	9,190
*Helical Ground Anchors	Each	19

*For information only. Cost included with Temporary Soil Retention System (Special).

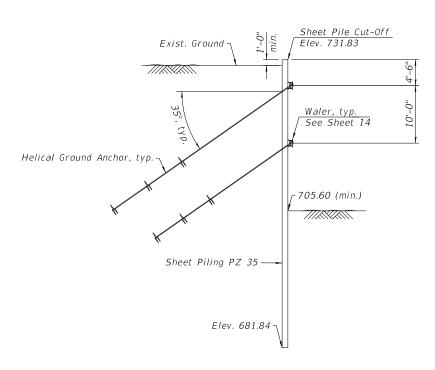
(Walers include weight of all structural steel related to waler system.)

Note:

- 1. See Sheet 11 for General Notes and Proposed Construction Sequence.
- 2. See Sheet 9 for Bond Breaker Details.
- 3. See Sheet 14 for Fence Connection Detail.



SECTION E-E



SECTION F-F

TRANSYSTEMS

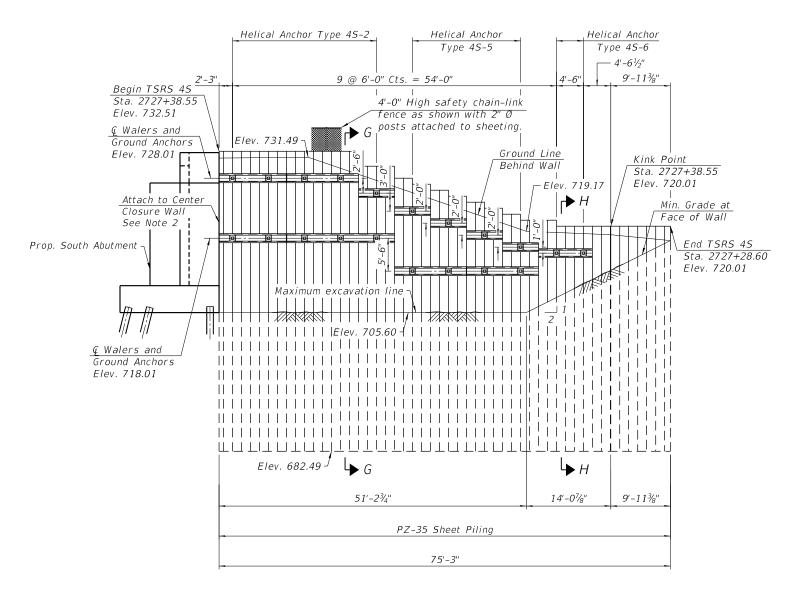
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TEMPORARY SOIL RETENTION SYSTEM ELEV N. ABUT - STAGE 4 STRUCTURE NO. 022-9948							
SHEET	12	OF	45	SHEETS			

F.A.U. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
1509	06-00133-00-BR		DUPAGE	426	169
			CONTRAC	T NO. 6	31G79
	ILLINOIS	FED, AI	D PROJECT		

JAME: TSRS Stage 4N Elevation and Cross Sections



TEMPORARY SOIL RETENTION SYSTEM - ELEVATION SOUTH ABUTMENT - STAGE 4

(Looking South, Unfolded View, Measured along F.F. of Wall)

TSRS 4S BILL OF MATERIAL

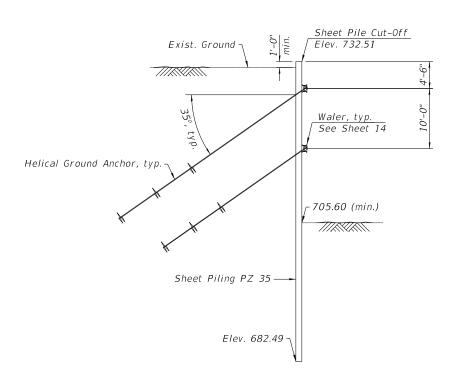
Item	Unit	Total
Temporary Soil Retention System (Special)	Sq. Ft.	1,269
*Walers	Pound	9,780
*Helical Ground Anchors	Each	20

*For information only. Cost included with Temporary Soil Retention System (Special).

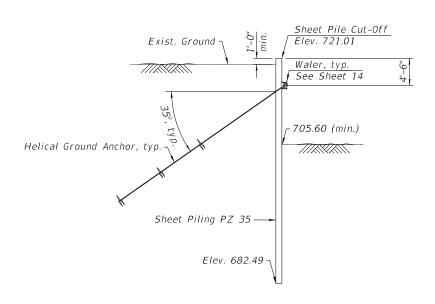
(Walers include weight of all structural steel related to waler system.)

Noto

- 1. See Sheet 11 for General Notes and Proposed Construction Sequence.
- 2. See Sheet 9 for Bond Breaker Details.
- 3. See Sheet 14 for Fence Connection Detail.



SECTION G-G



SECTION H-H

TRANSYSTEMS

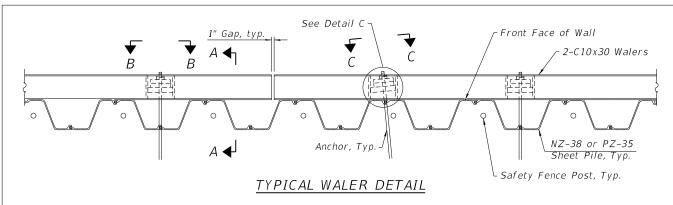
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PLOT DATE	-	1/25/2025	CHECKED	-	WJC	REVISED	-

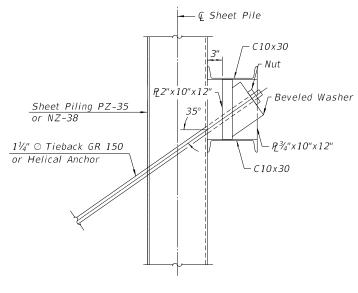
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TEMPORARY SOIL RETENTION SYSTEM ELEV S. ABUT - STAGE 4 STRUCTURE NO. 022-9948							
SHEET	13	OF	45	SHEETS			

A.U.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
509	06-00133-00-BR		DUPAGE	426	170
		SECTION COUNTY SHEETS			61G79
ILLINOIS FED. AID PROJECT					

NAME: TSRS Stage 45 Elevation and Cross Section



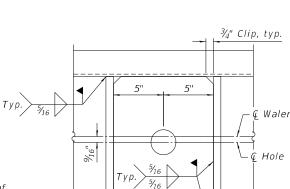


C10×30 10" — R³/₄"×10"×12" — Q Waler — Q Hole VIEW B-B

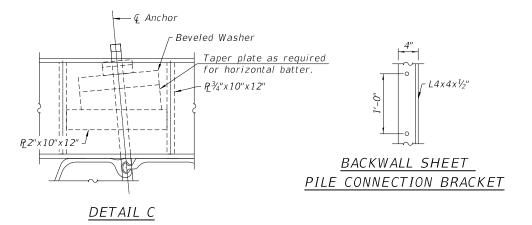
<u>SECTION A-A</u> (Vertically Angled Anchors)

Notes:

- 1. All structural steel shall meet the requirements of ASTM A572 Grade 50.
- 2. All bolts, nuts and washers shall meet the requirements of ASTM F3125 Grade A325, ASTM A563 and ASTM F436, respectively.
- 3. Anchors and walers must be installed when bottom of excavation is a maximum of 1'-0" below the level of walers. The removal process shall be the reverse of the installation process.
- 4. Sheeting is to be removed after backfilling is complete or temporary embankment is removed. Walers and ground anchor system must be removed during backfilling.
- 5. All sheet piling, walers, ground anchors, miscellaneous steel shapes, and connecting hardware required for temporary sheet piling will be paid for under the "Temporary Soil Retention System (Special)" item.
- 6. All elevations and dimensions must be verified in the field.
- 7. Contractor shall verify that each ground anchor does not interfere with existing or proposed utility structures prior to installation. Any anchor modifications required to avoid interference with existing or proposed utility structures shall be approved by the Engineer.
- 8. Any excavation beyond limits shown must be approved by the Engineer.
- 9. If the temporary soil retention system outlined in these plans is not used, the Contractor must provide design calcuations and working drawings of the soil retention system stamped by a licensed Illinois Structural Engineer.
- 10. See Special Provision "Ground Anchors" and "Helical Ground Anchors" for information regarding ground anchors for "Temporary Soil Retention System (Special)".
- 11. Safety fence shall be according to Article 664 in the Standard Specifications. Cost included with "Temporary Soil Retention System (Special)". See Hwy. Std. 664001.



VIEW C-C



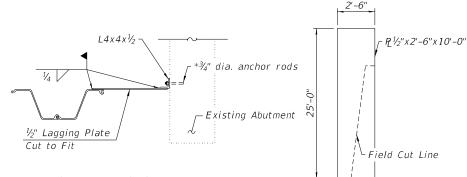
GROUND ANCHOR SCHEDULE

	Туре	Design Load (kips)	Elevation (feet)	Angle (degrees)	Min. Unbonded Length (feet)	Est. Bonded Length (feet)	Total Length (feet)
	3N-1T	48.13	727.28	35	15	22	37
	3N-1T-SP	53.63	727.28	35	15	24	39
	3N-1T-2	48.13	725.28	35	15	22	37
	3N-1B	48.41	717.28	35	15	22	37
	3N-1B-SP	53.11	717.28	35	15	24	39
	3N-2T	28.14	727.28	35	11	9	20
	3N-2B	31.60	717.28	35	13	9	22
	3N-3T	27.14	719.28	35	15	12	27
	3N-3T-2	27.14	719.28	35	15	12	27
	3S-1T	48.13	727.63	35	15	22	37
	3S-1T-SP	53.63	727.63	35	15	24	39
	3S-1B	48.41	717.63	35	15	22	37
	3S-1B-SP	53.11	717.63	35	15	24	39
	3S-2T	28.14	727.63	35	11	9	20
	3S-2T-2	28.14	726.63	35	11	9	20
	3S-2T-3	28.14	725.63	35	11	9	20
	3S-2T-4	28.14	724.63	35	11	9	20
	3S-2T-5	28.14	723.63	35	11	9	20
	3S-2B	31.60	717.63	35	13	9	22
	3S-4T-SP	17.39	721.63	35	15	8	23
	3S-4T-2-SP	17.39	717.13	35	15	8	23
	3S-4T-3	15.55	714.13	35	15	7	22
	3S-4T-4	15.55	711.13	35	15	7	22
	4N-2T	28.14	727.33	35	11	9	20
	4N-2T-2	28.14	725.33	35	11	9	20
	4N-2T-3	28.14	723.33	35	11	9	20
	4N-2B	31.60	717.33	35	13	9	22
	4N-5T	24.88	721.33	35	10	9	19
	4N-5T-2	24.88	718.33	35	10	9	19
	4N-5T-3	24.88	716.33	35	10	9	19
	4N-5B	22.20	711.33	35	10	9	19
	4N-6T	17.97	715.33	35	7	9	16
	4S-2T	28.14	728.01	35	11	9	20
	4S-2T-2	28.14	725.51	35	11	9	20
	4S-2B	31.60	718.01	35	13	9	22
	4S-5T	24.88	722.51	35	10	9	19
	4S-5T-2	24.88	720.51	35	10	9	19
	4S-5T-3	24.88	718.51	35	10	9	19
	4S-5T-4	24.88	716.51	35	10	9	19
	4S-5B	22.20	712.51	35	10	9	19
or	45-6T	17.97	715.51	35	7	9	16
	Noto:						

SP = Special Anchor

T = Top AnchorB = Bottom Anchor

Estimated bonded and unbonded lengths are specified in the ground anchor schedule; actual length and depth required to develop the design load specified shall be determined by the anchor supplier. Minimum ultimate load capacity shall be 2 x design load. Changes in bonded and unbonded length shall be approved by the Engineer.



BACKWALL TO SHEET
PILE CONNECTION DETAIL

*Drill and set per manufacturer's specifications

CONNECTION PLATE

TRANSYSTEMS

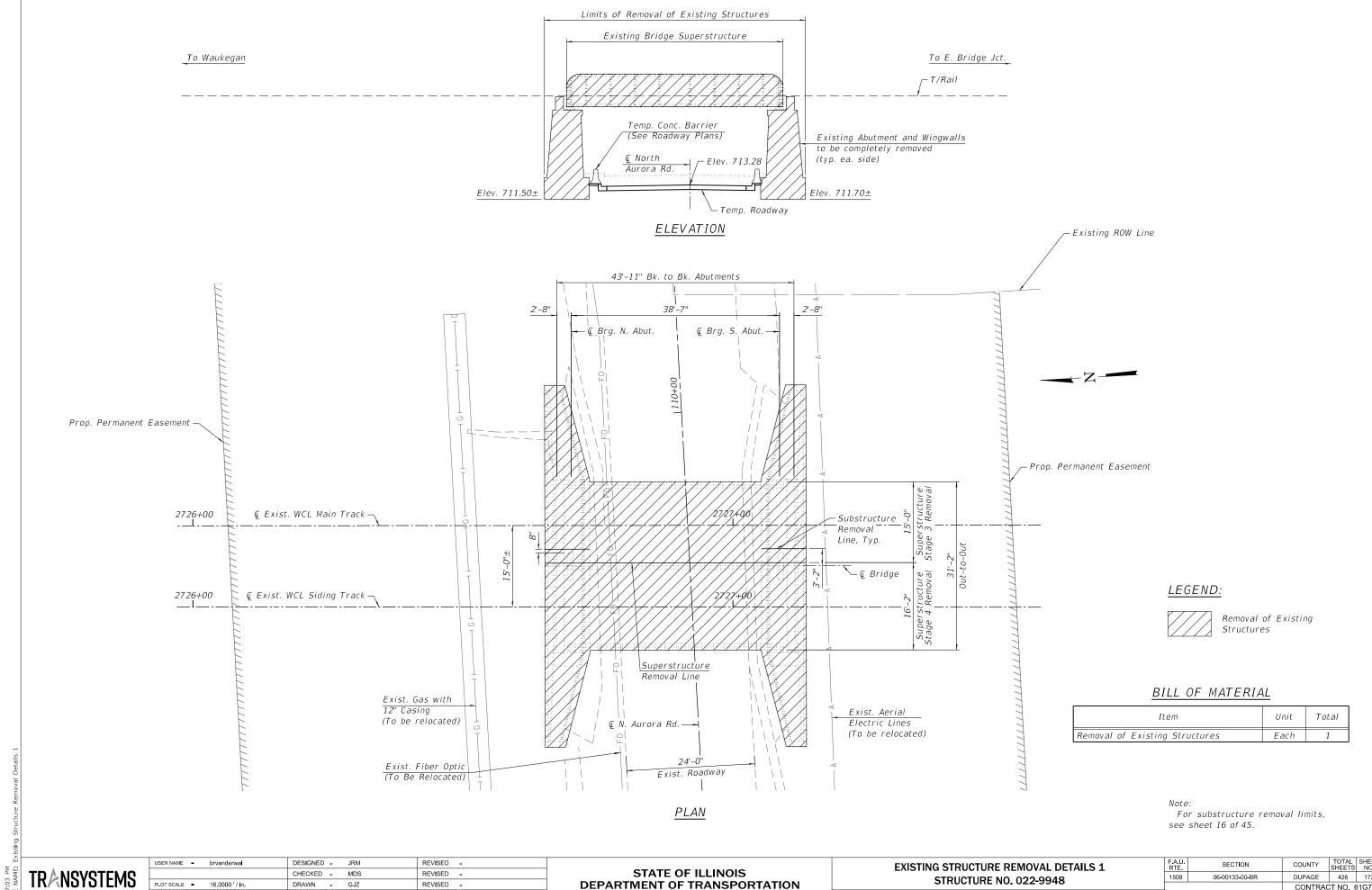
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		CHECKED -	MJC	REVISED -
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PLOT DATE =	1/25/2025	CHECKED -	WJC	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

TEMPORARY SOIL RETENTION SYSTEM DETAILS
STRUCTURE NO. 022-9948

SHEET 14 OF 45 SHEETS

F.A.U. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.	
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		CONTRAC	CT NO. 6	31G79		

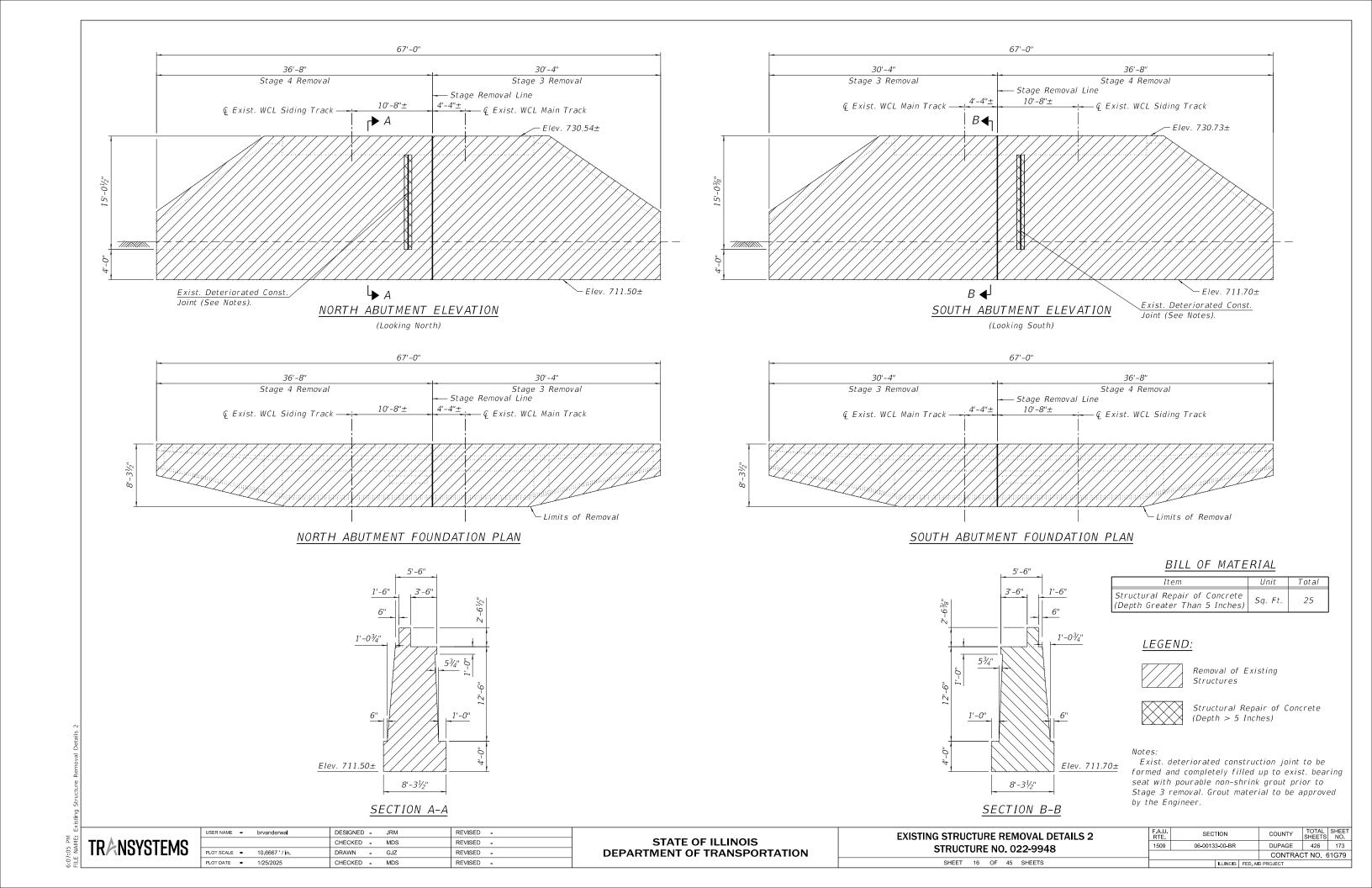


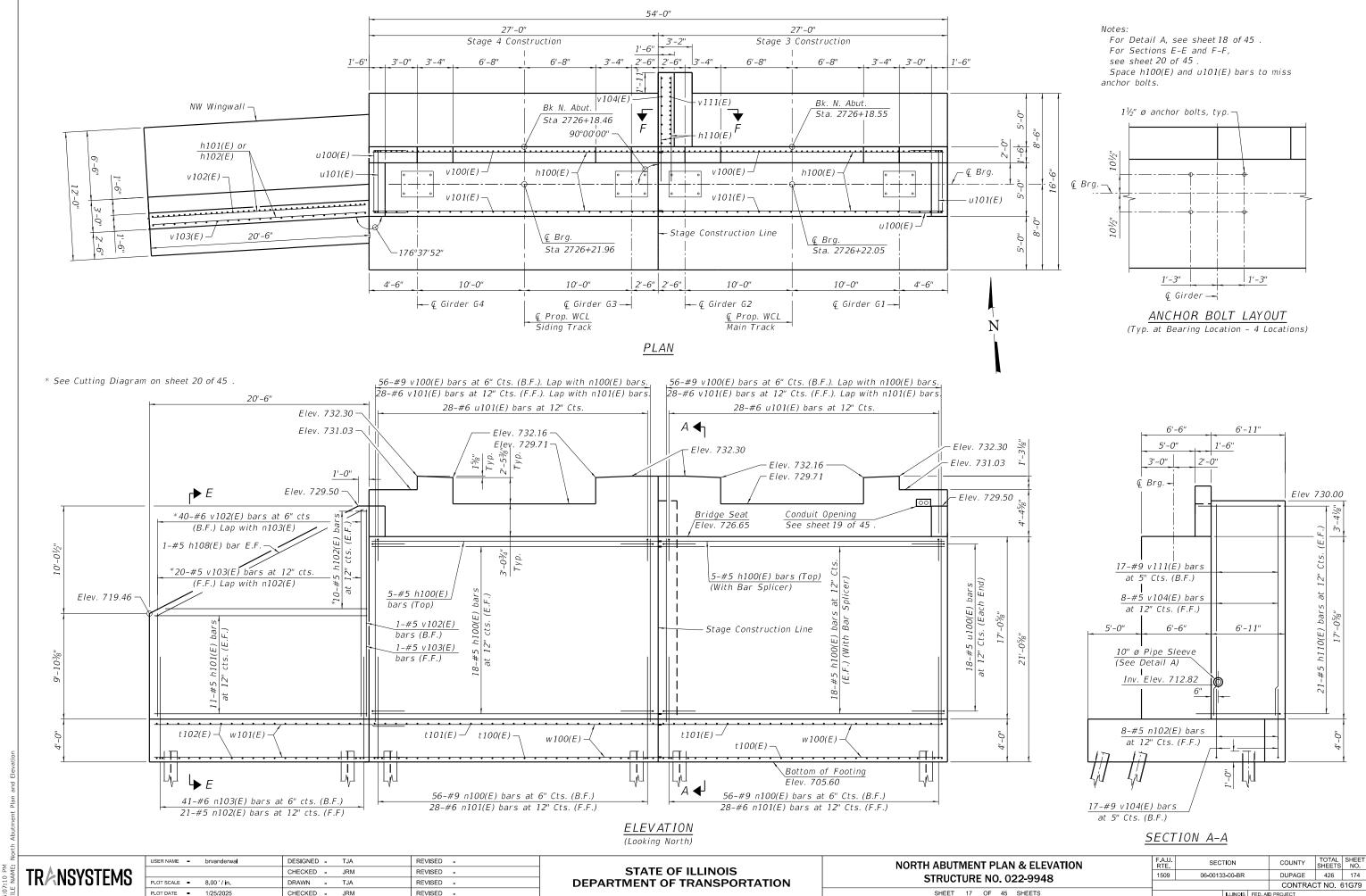
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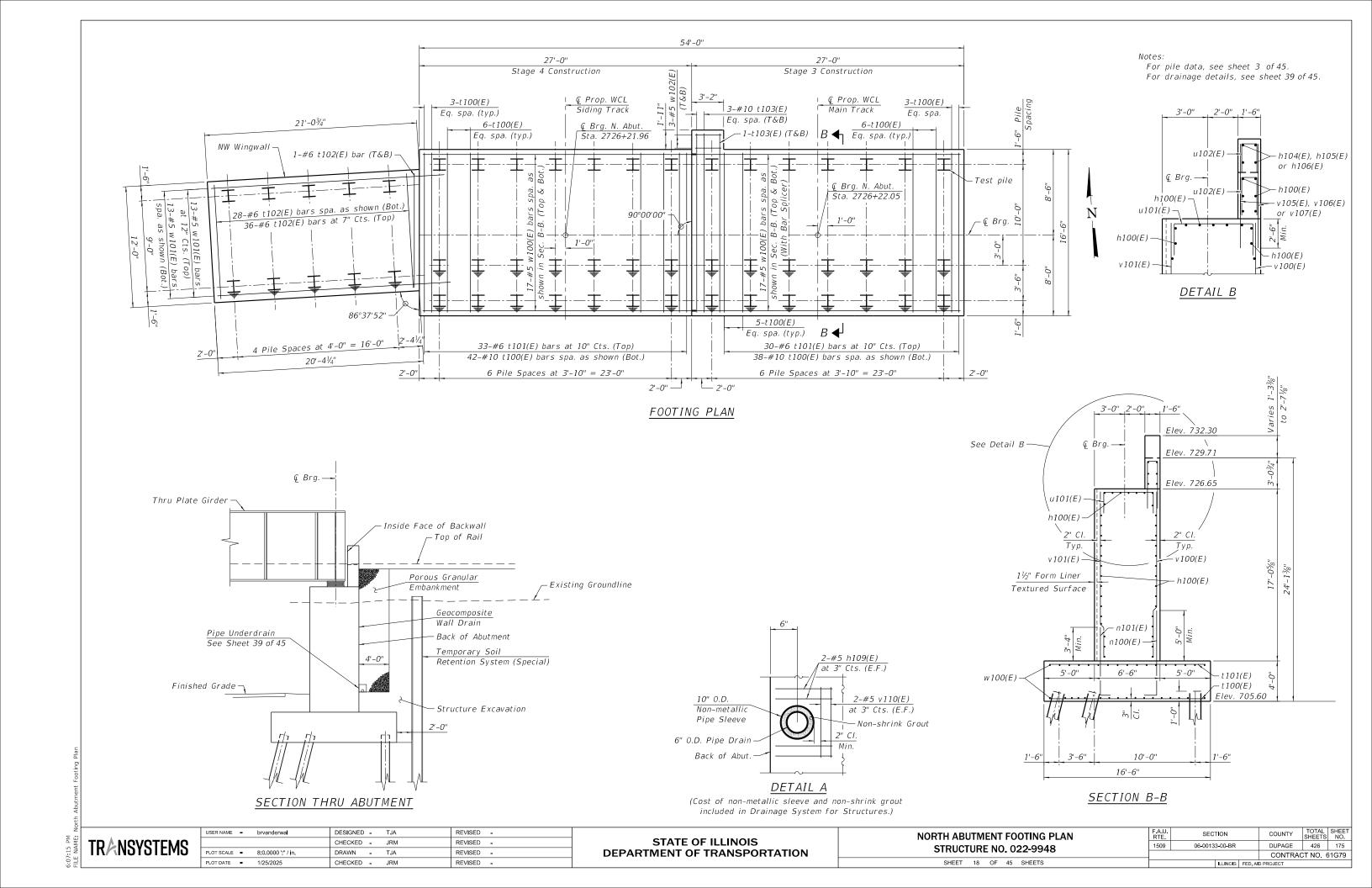
DEPARTMENT OF TRANSPORTATION

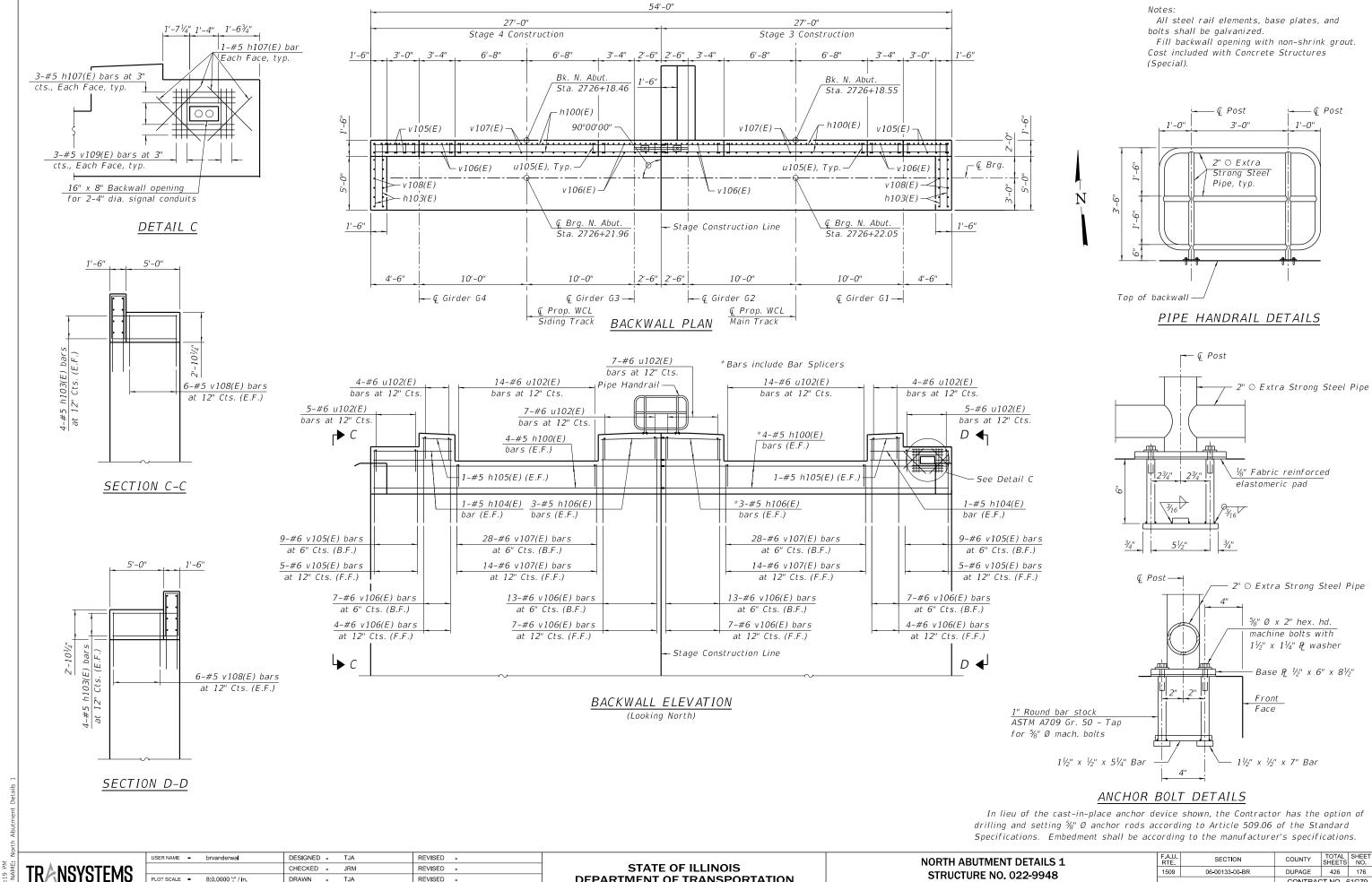
STRUCTURE NO. 022-9948 SHEET 15 OF 45 SHEETS

DUPAGE 426 172 CONTRACT NO. 61G79





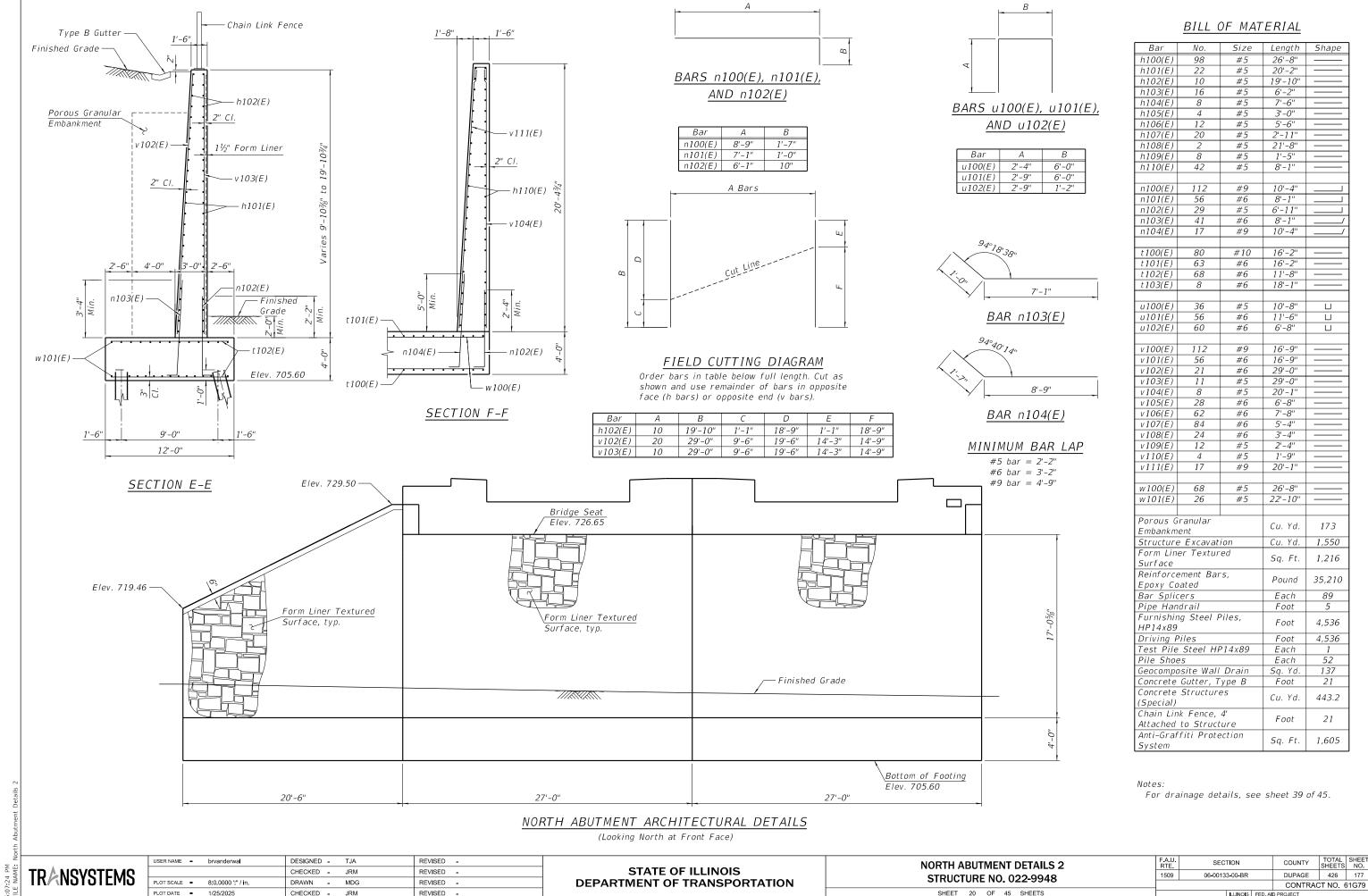


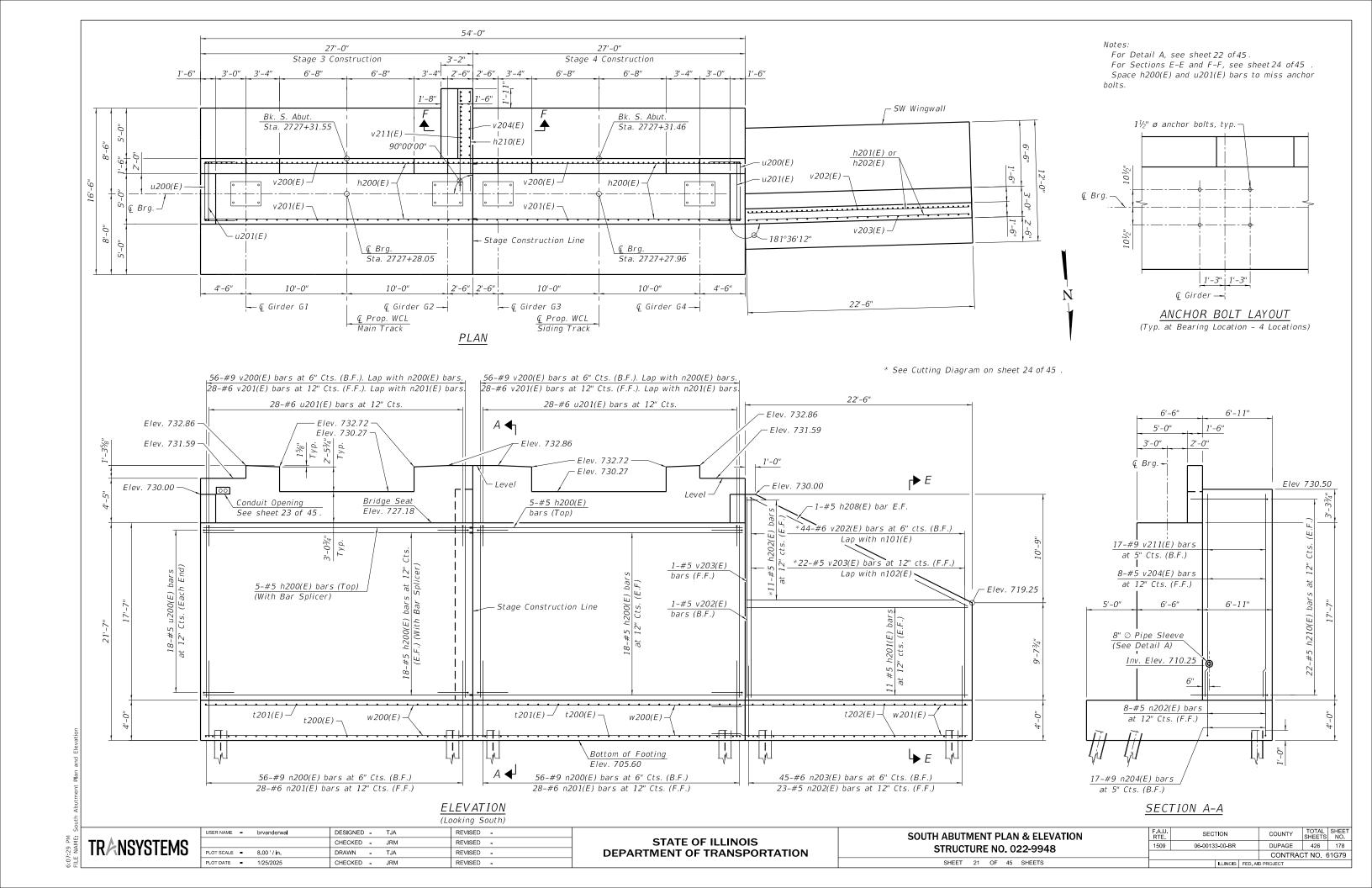


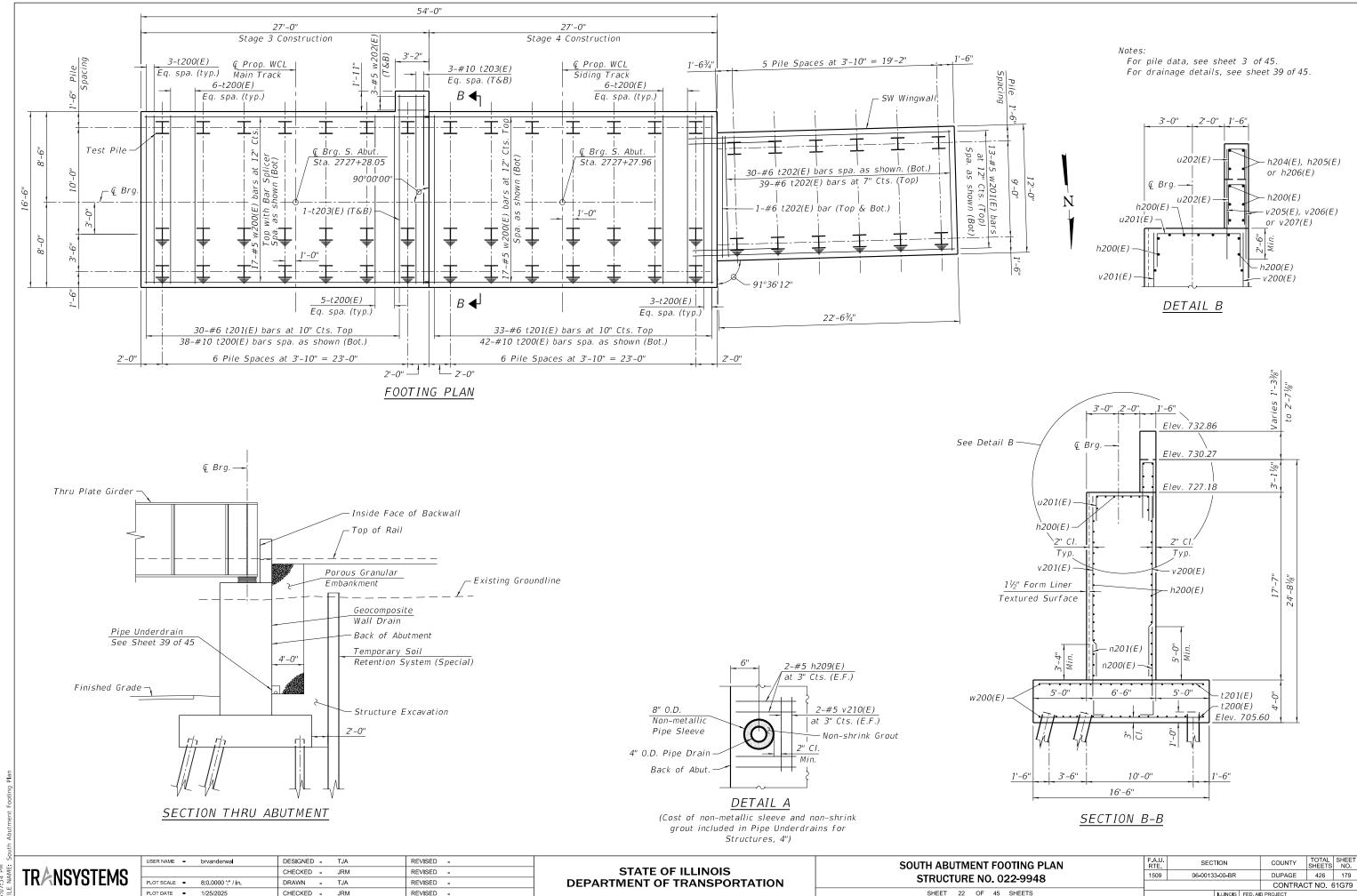
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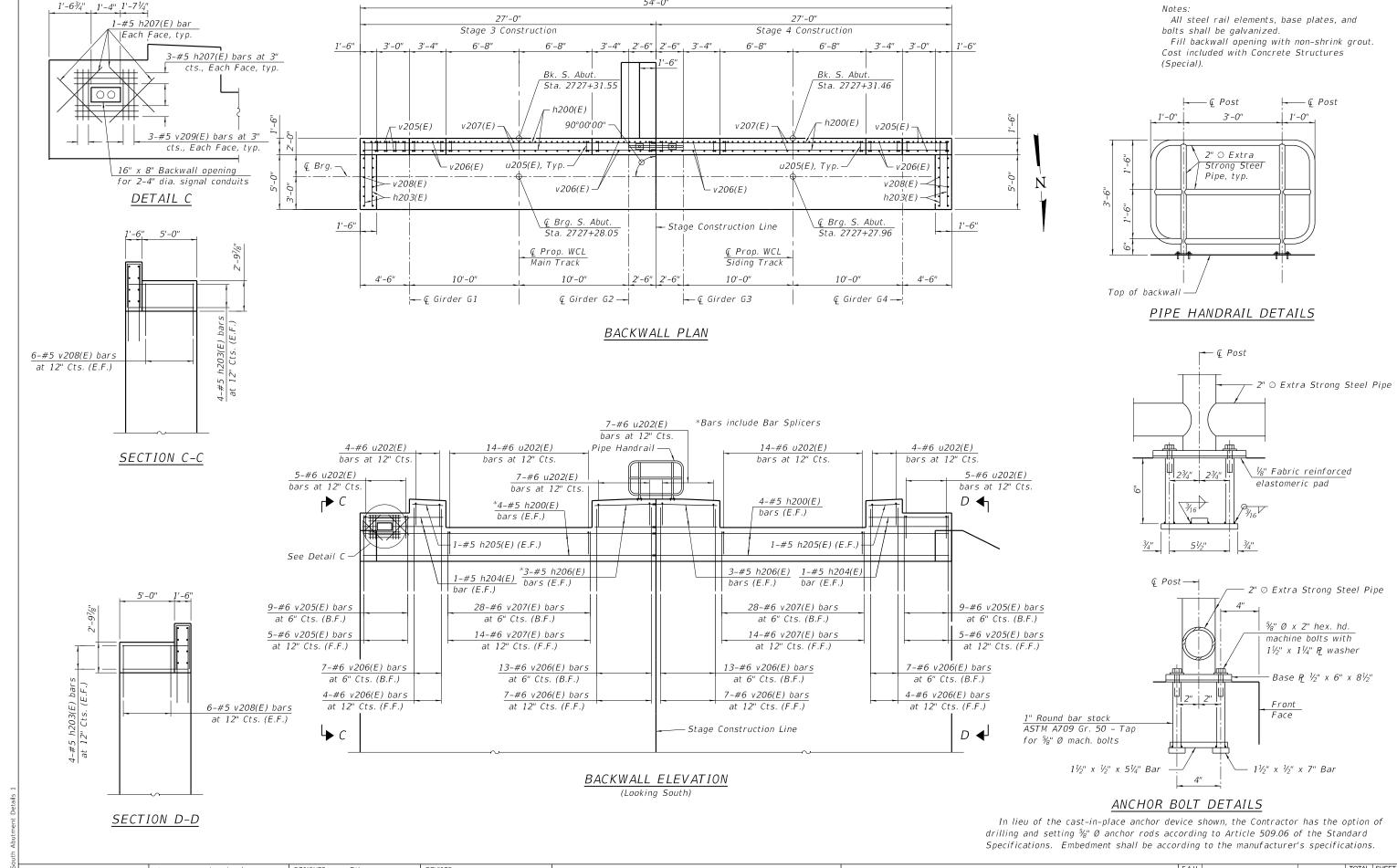
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **STRUCTURE NO. 022-9948** SHEET 19 OF 45 SHEETS

1509 06-00133-00-BR DUPAGE 426 176 CONTRACT NO. 61G79









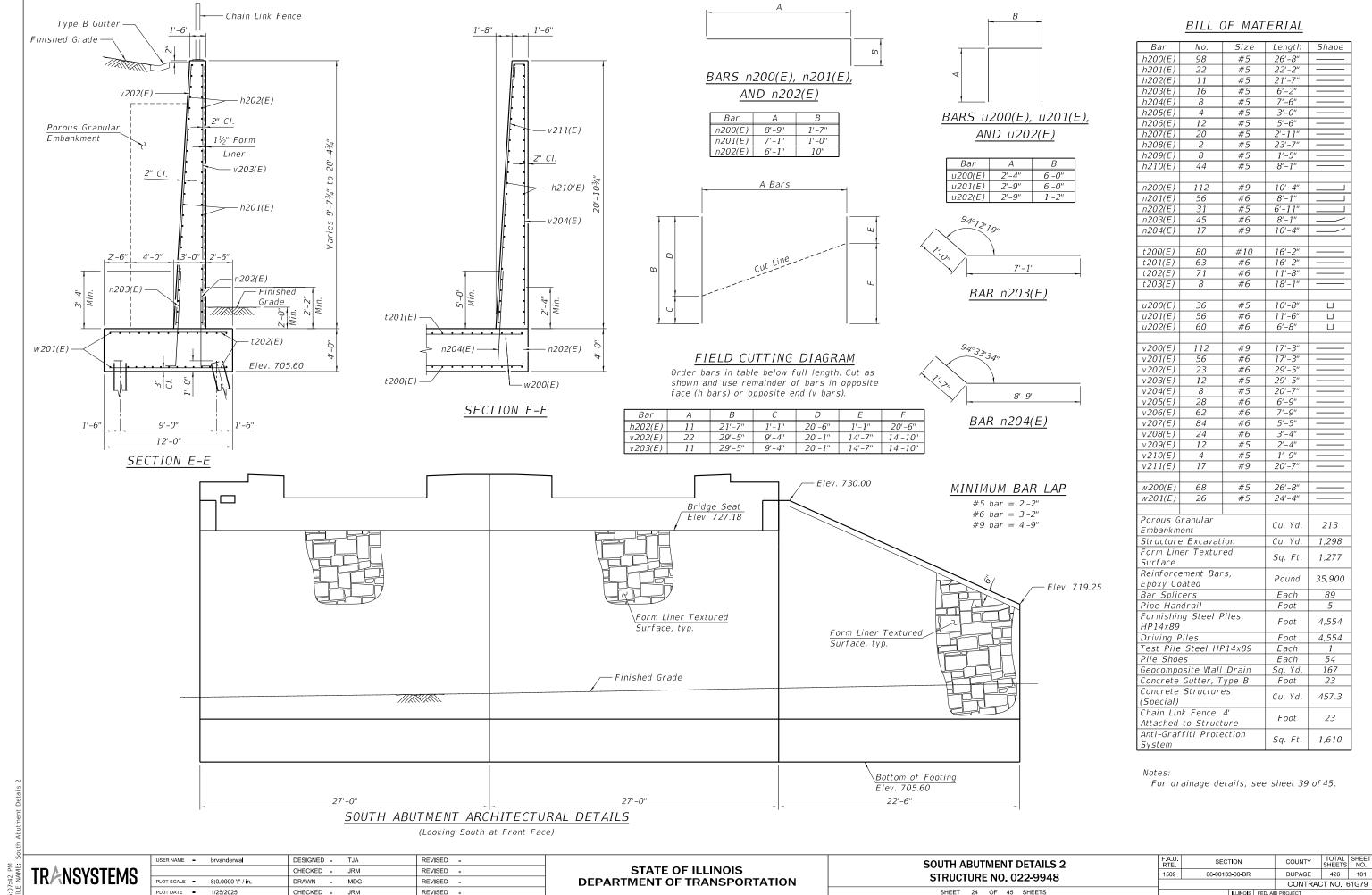
54'-0"

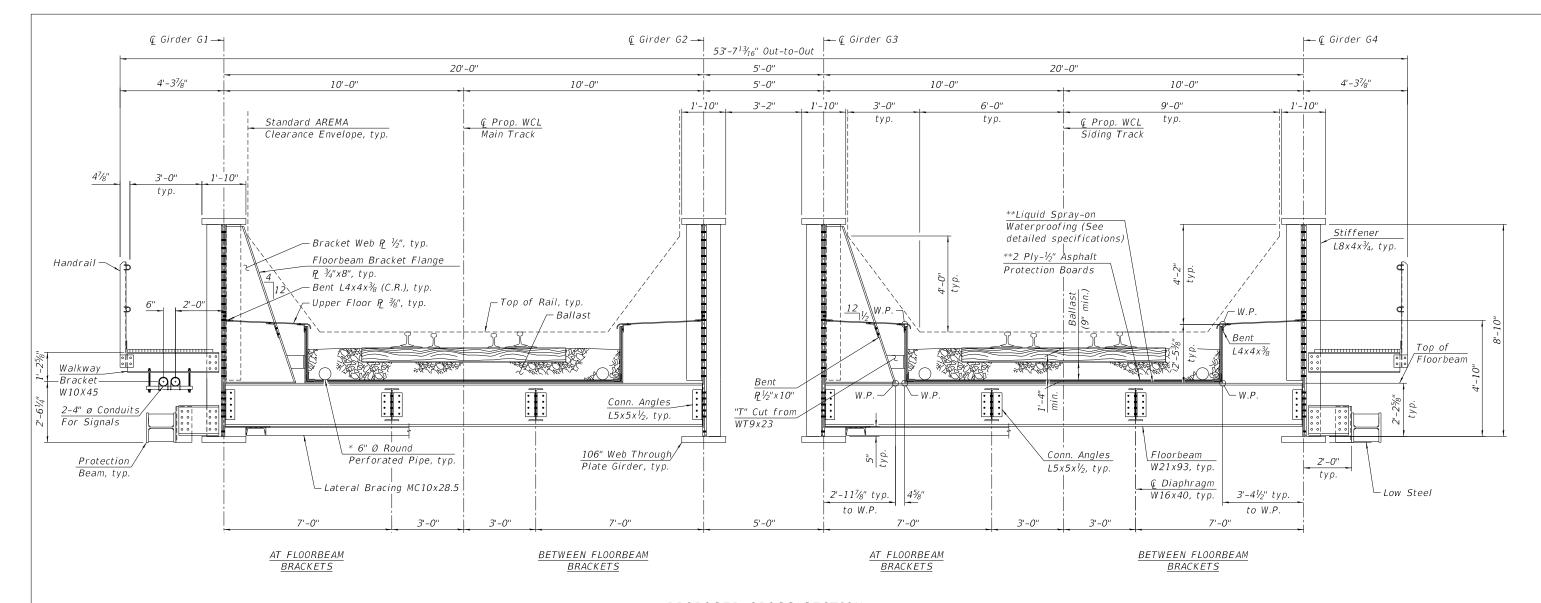
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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **SOUTH ABUTMENT DETAILS 1 STRUCTURE NO. 022-9948** SHEET 23 OF 45 SHEETS

SECTION COUNTY 1509 06-00133-00-BR DUPAGE 426 180 CONTRACT NO. 61G79





PROPOSED CROSS SECTION

(Looking South)

CONSTRUCTION DEPTH: 4'-73/8" MIN.

8½16" - Rail & Tie Plate

7" - Tie

9" - Ballast (min.)

1" - Asphalt Protection Boards

½16" - Waterproofing

5⁄8" - Floor Plate

21%" - Floorbeam

5" - Top of Girder Bottom Flange to bottom of Floorbeam

* Cost included with Drainage System for Structures. ** Cost Included with Membrane Waterproofing (Special).

1. T/Rail to $T/Tie = 8\frac{1}{16}$ ". Dimension includes

of 13" tie plate of $\frac{3}{4}$ ".

height of 136 lb. rail at $7\frac{5}{16}$ ", and thickness

3" - 1

Girder	Flange	

F.A.U. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.	
1509	06-00133-00-BR	DUPAGE	426	182	ı

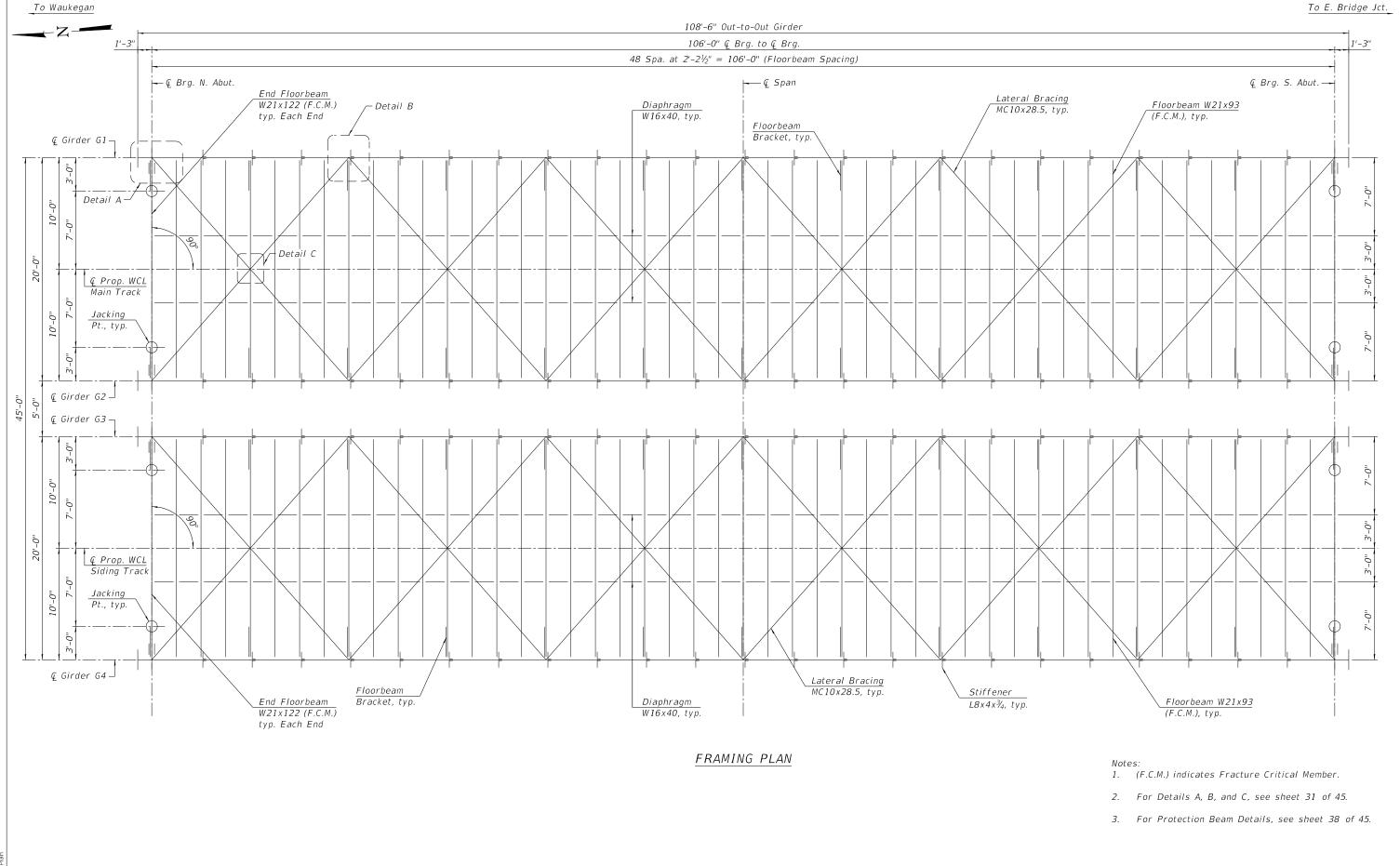
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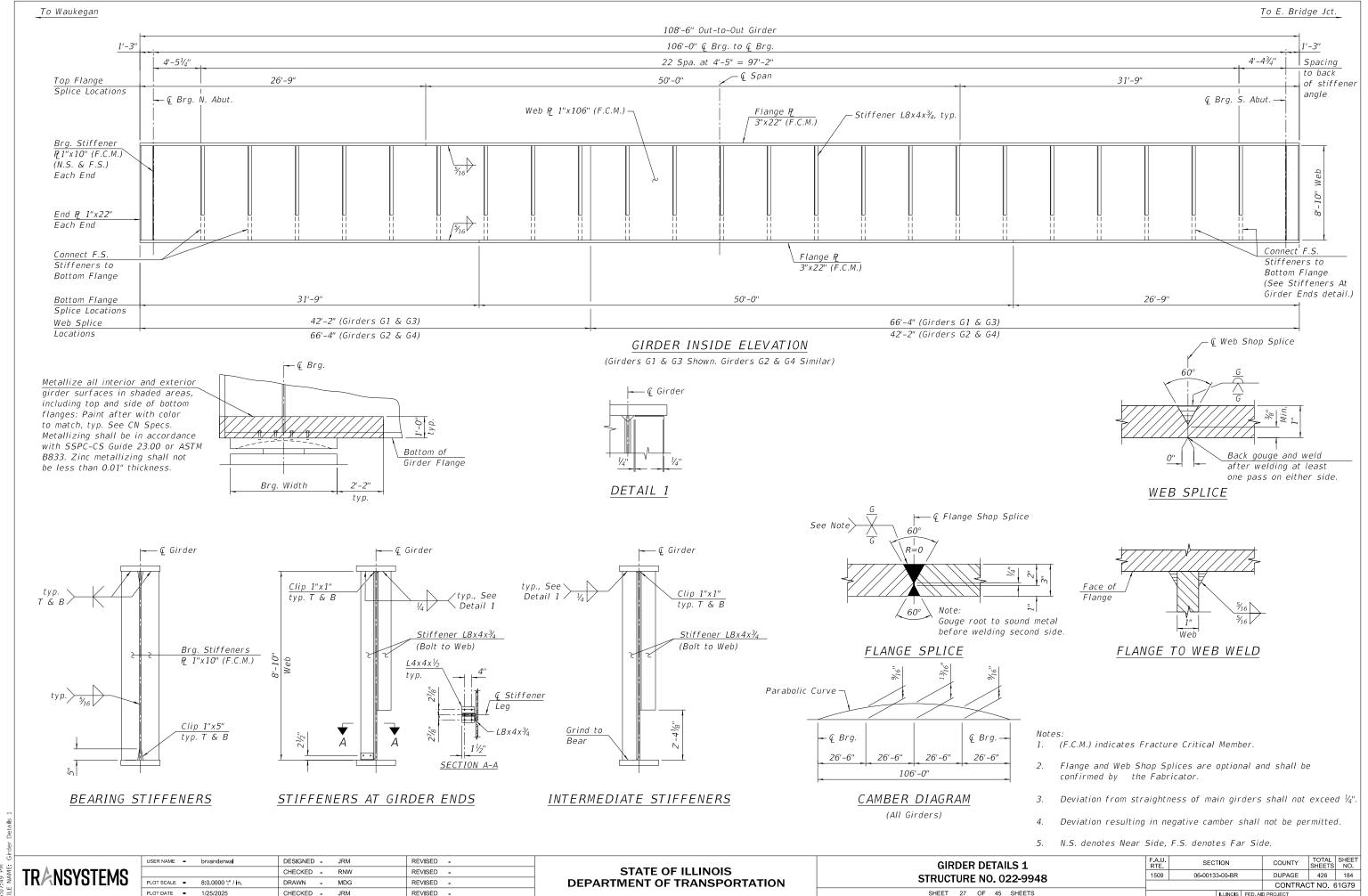
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

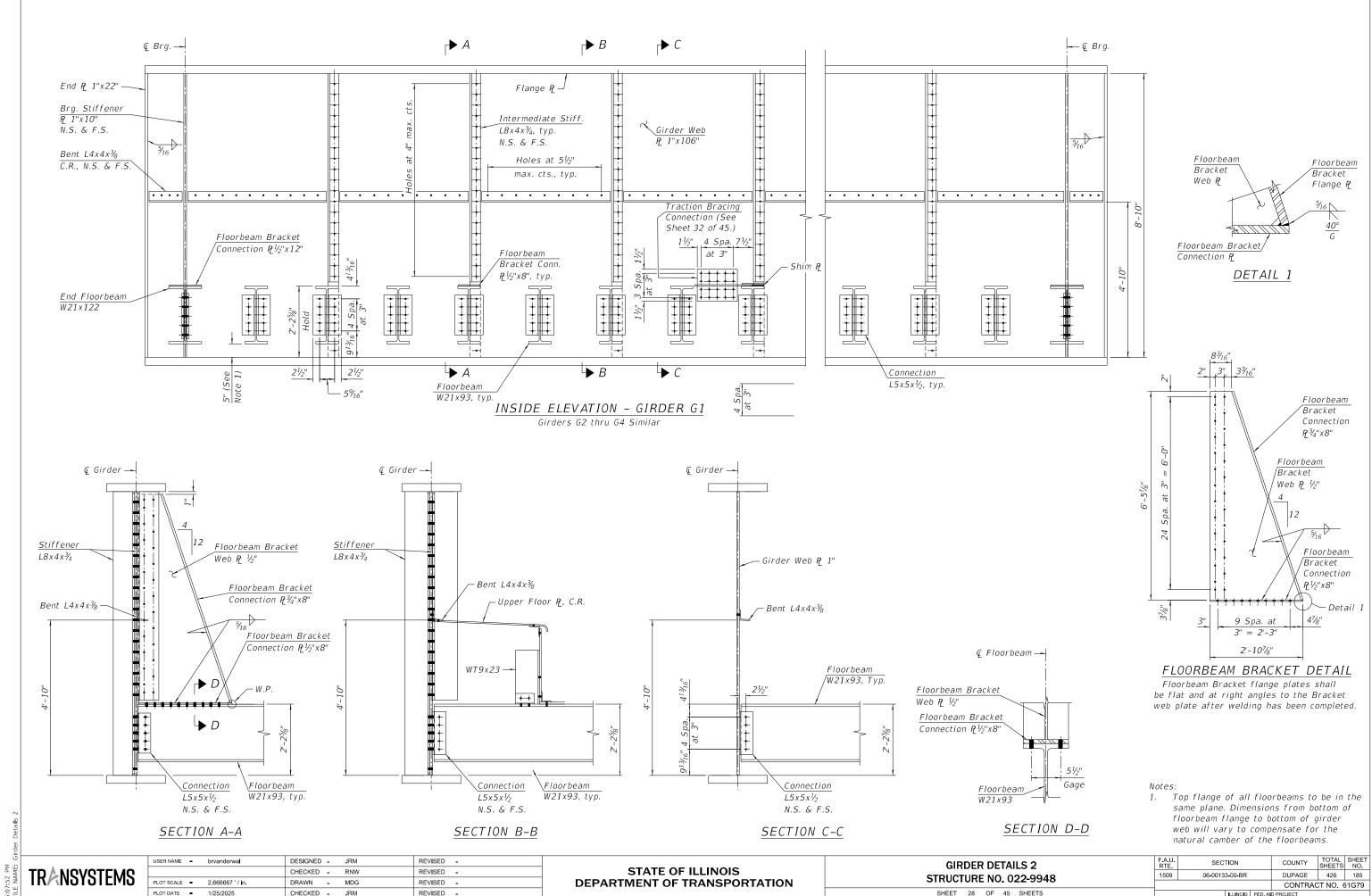
DECK CROSS SECTION STRUCTURE NO. 022-9948 SHEET 25 OF 45 SHEETS

CONTRACT NO. 61G79



TRANSYSTEMS





MOMENT AND SHEAR PER GIRDER

Girders G1 Thru G4

	Giraers G	1 Thru G4
DESCR	IPTION	AT 0.5L
	МОМ	ENT
Dead Load	('k)	5,685
Live Load (E90)	('k)	8,088
Impact	('k)	2,103
Total	('k)	15,876
	Web	1"×106"
Section	Flanges	3"x22"
Gross I Furnishe	d (in⁴)	491,423
Net I Furnished	(in⁴)	491,423
Net Section Modu	ılus (in³)	8,775
Allow Max. Compo Stress in Flange	ressive (ksi)	27.5
Actual Max. Comp Stress in Flange	oressive (ksi)	21.7
Allow Max. Tensi Stress in Flange	le (ksi)	27.5
Actual Max. Tensile Stress in Flange (ksi)		21.7
Allow Max. Deflection Live Load + Impact (in)		1.70
Actual Max. Deflo Live Load + Impa		1.46
	SHI	EAR
Dead Load	(k)	214.5
Live Load (E90)	(k)	353.8
Impact	(k)	92.0
Total	(k)	660.3
Allow Max. Web Shear Stress	(ksi)	17.5
Actual Max. Web Shear Stress (ksi)		6.2
End Stiffener Column Area Req'd (in²)		27.7
Section (2 Plates)	1"×10"
End Stiffener Column Area Fur	n. (in²)	32.0
	FAT	IGUE
Allowable Stress Fatigue S _{RFAT} (Ca	-	16.0
Actual Stress Ra Fatigue S _{RFAT}	nge (ksi)	12.1

MOMENT AND SHEAR PER GIRDER

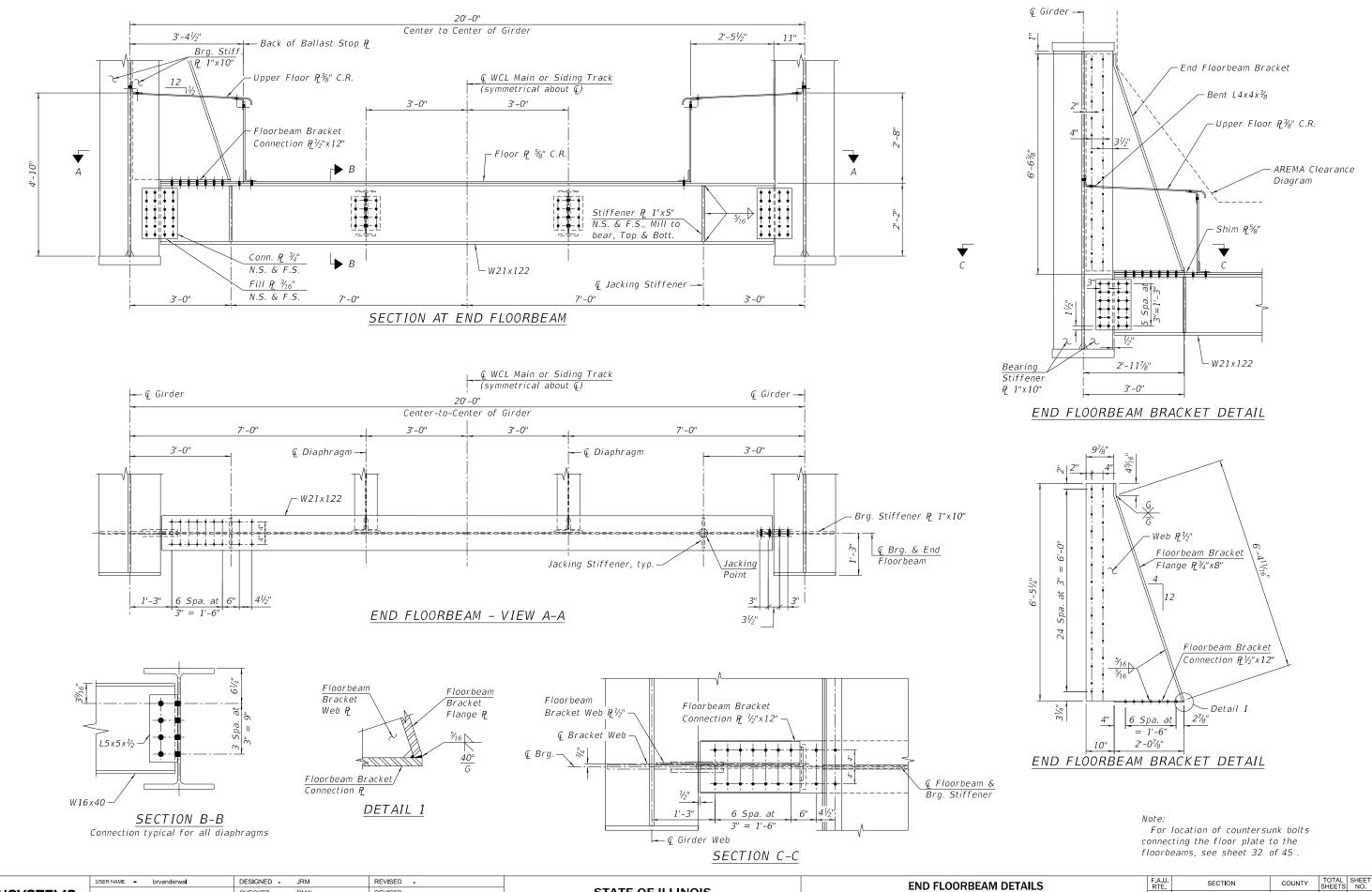
Girders G1 Thru G4

DESCRIPTION		FLOORBEAM	END FLOORBEAM		
			BRIDGE	JACKING	
		MOMENT	T	I	
Dead Load	('k)	39.1	54.6	653.7	
Live Load (E90)	('k)	190.5	267.7		
Impact	('k)	76.8	108.0		
Total	('k)	306.4	430.3	653.7	
Section	('k)	W21x93	W21	x122	
Gross I Furnished	(in⁴)	2,070	2,9	960	
Net I Furnished	(in⁴)	2,070	2,7	745	
Net Section Modulus	(in³)	192	20	67	
Allow Max. Compressive Stress in Flange	(ksi)	27.5	27.5	41.3	
Actual Max. Compressive Stress in Flange	(ksi)	19.2	18.9	28.7	
Allow Max. Tensile Stress in Flange	(ksi)	27.5	27.5	41.3	
Actual Max. Tensile Stress in Flange	(ksi)	19.2	19.4	32.6	
Allow Max. Deflection Live Load + Impact	(in)	0.32	0.32		
Actual Max. Deflection Live Load + Impact	(in)	0.30	0.30		
		SHEAR			
Dead Load	(k)	8.3	11.6	227.8	
Live Load (E90)	(k)	25.4	35.7		
Impact	(k)	10.3	14.4		
Total	(k)	44.0	61.7	227.8	
Allow Max. Web Shear Stress	(ksi)	17.5	17.5	26.3	
Actual Max. Web Shear Stress	(ksi)	3.8	5.2	19.2	
		FATIGUE			
Allowable Stress Range Fatigue S _{RFAT} (Cat. B)	(ksi)	16.0	16.0		
Actual Stress Range Fatigue S _{RFAT}	(ksi)	14.0	14.1		

BEARING ON CONCRETE

DESCRIPTION	Girders G1 Thru G4
Total Reaction	661 k
Net Bearing Area Furnished	1,044 in²
Average Bearing Pressure	632 psi

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	CHECKED - RNW	REVISED -
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PLOT DATE = 1/25/2025	CHECKED - JRM	REVISED -



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TRANSYSTEMS

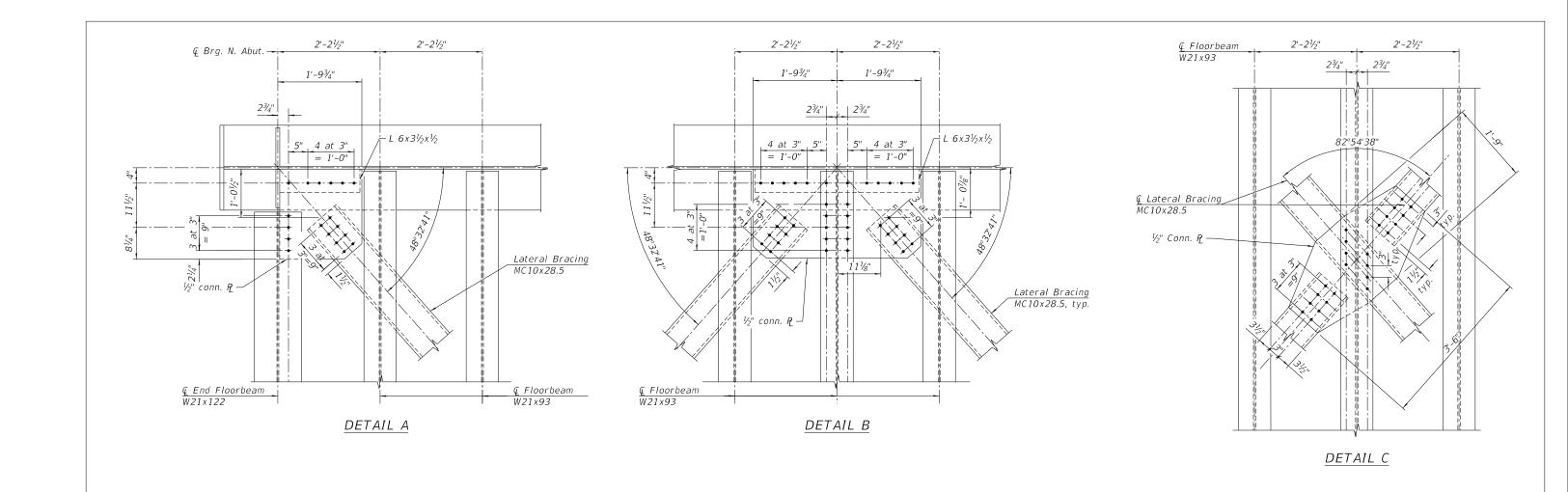
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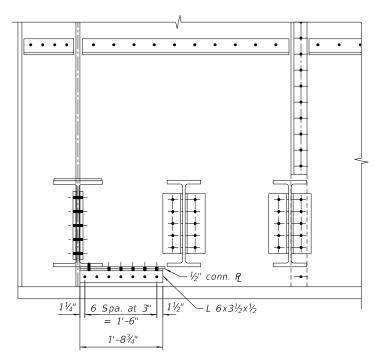
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

STRUCTURE NO. 022-9948

SHEET 30 OF 45 SHEETS





DETAIL A ELEVATION

(Lateral bracing not shown for clarity.)

• • •			• •	•	•	• •	•
1½" conn. 1½" L 6x3½x½	***	at 4½" "-6" 3'-6½"	5" 4 Sp	a. at 1'-0"	11/2"	======================================	>
}	•				1		

DETAIL B ELEVATION

(Lateral bracing not shown for clarity.)

TRANSYSTEMS

USER NAME =	brvanderwa l	DESIGNED -	JRM	REVISED -
		CHECKED -	RNW	REVISED -
PLOT SCALE =	2:0.0000 ':" / in.	DRAWN -	MDG	REVISED -
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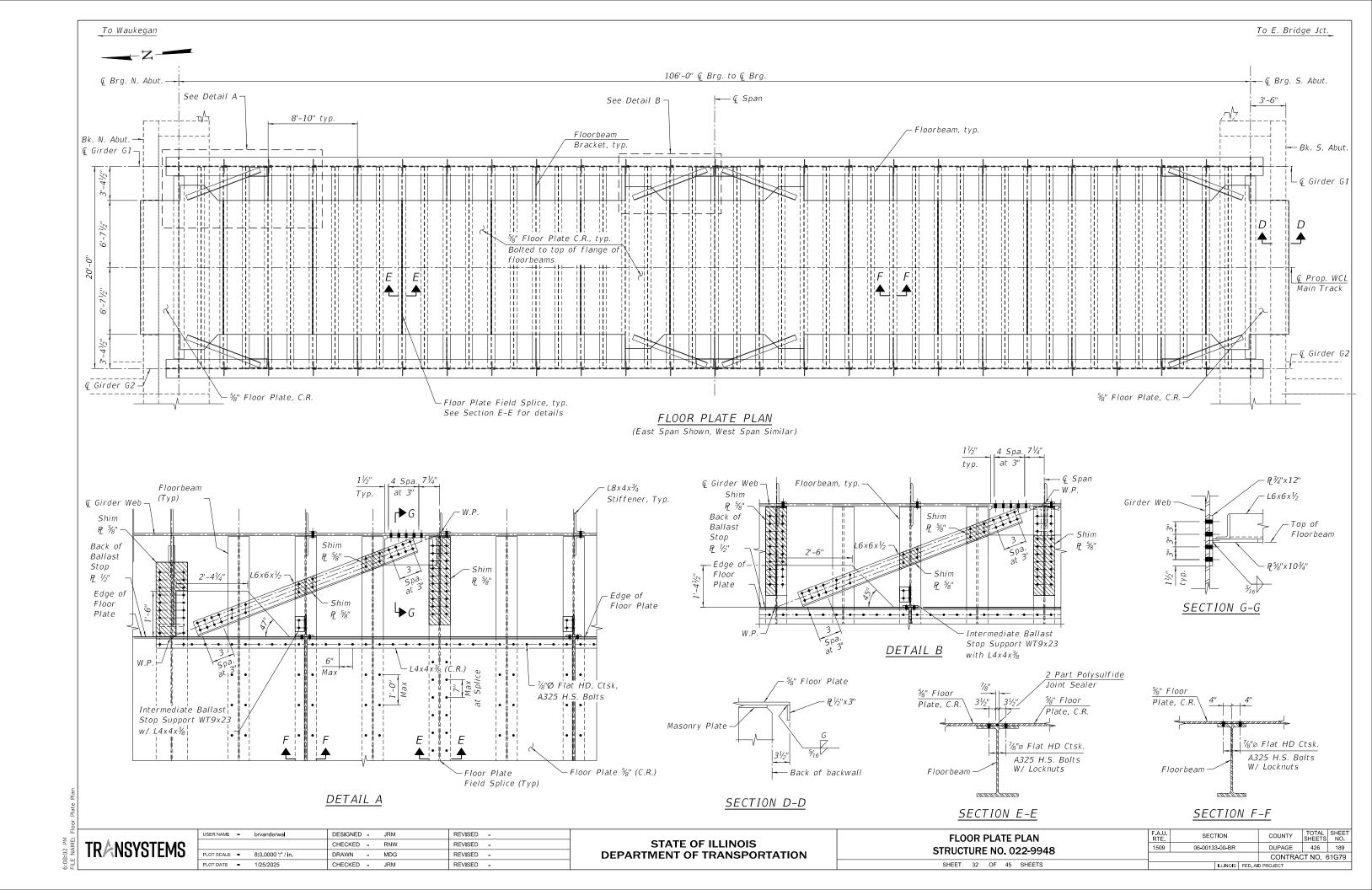
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

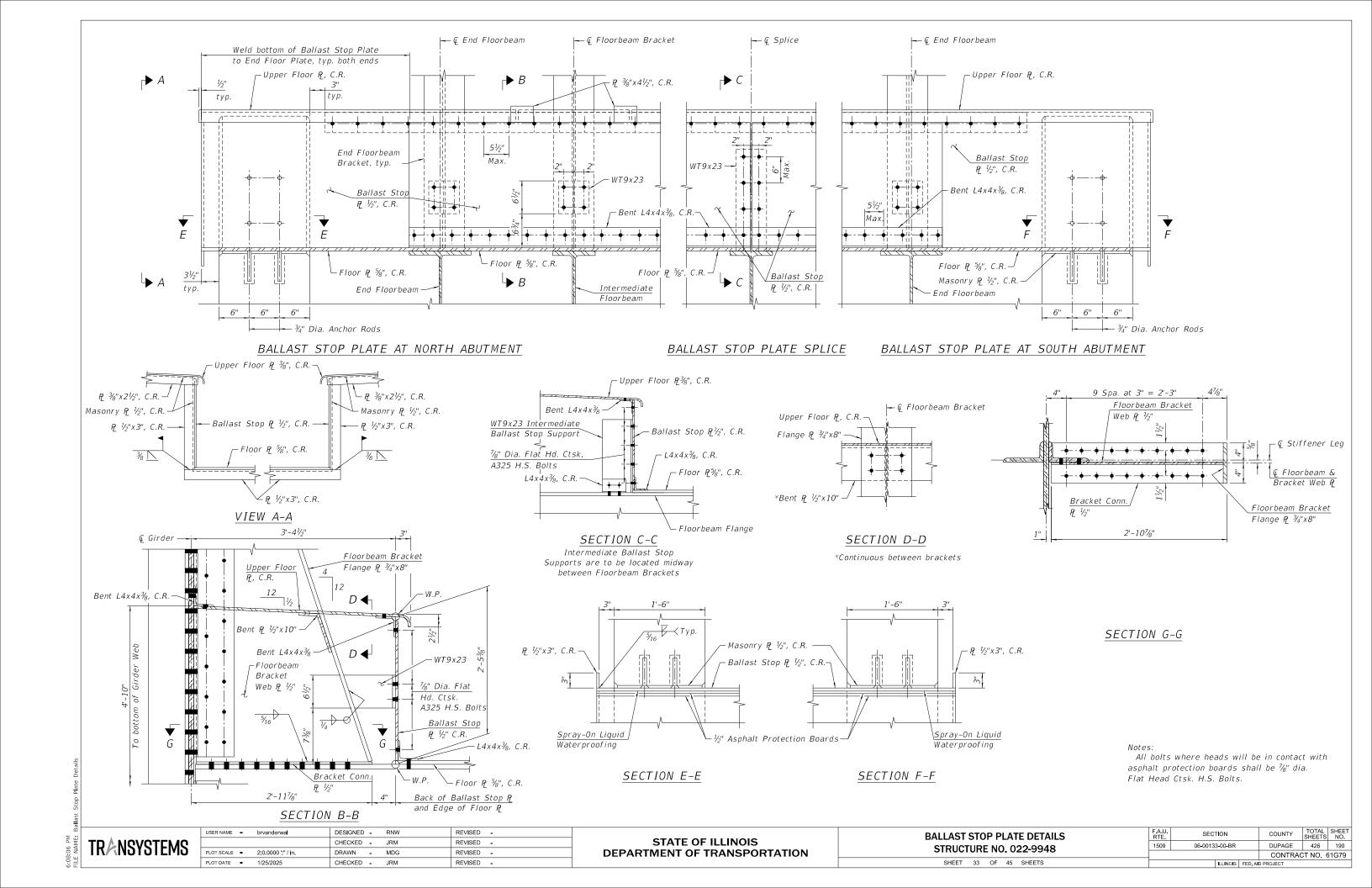
					DETAILS 22-9948	
9	HEET	31	OF	45	SHEETS	

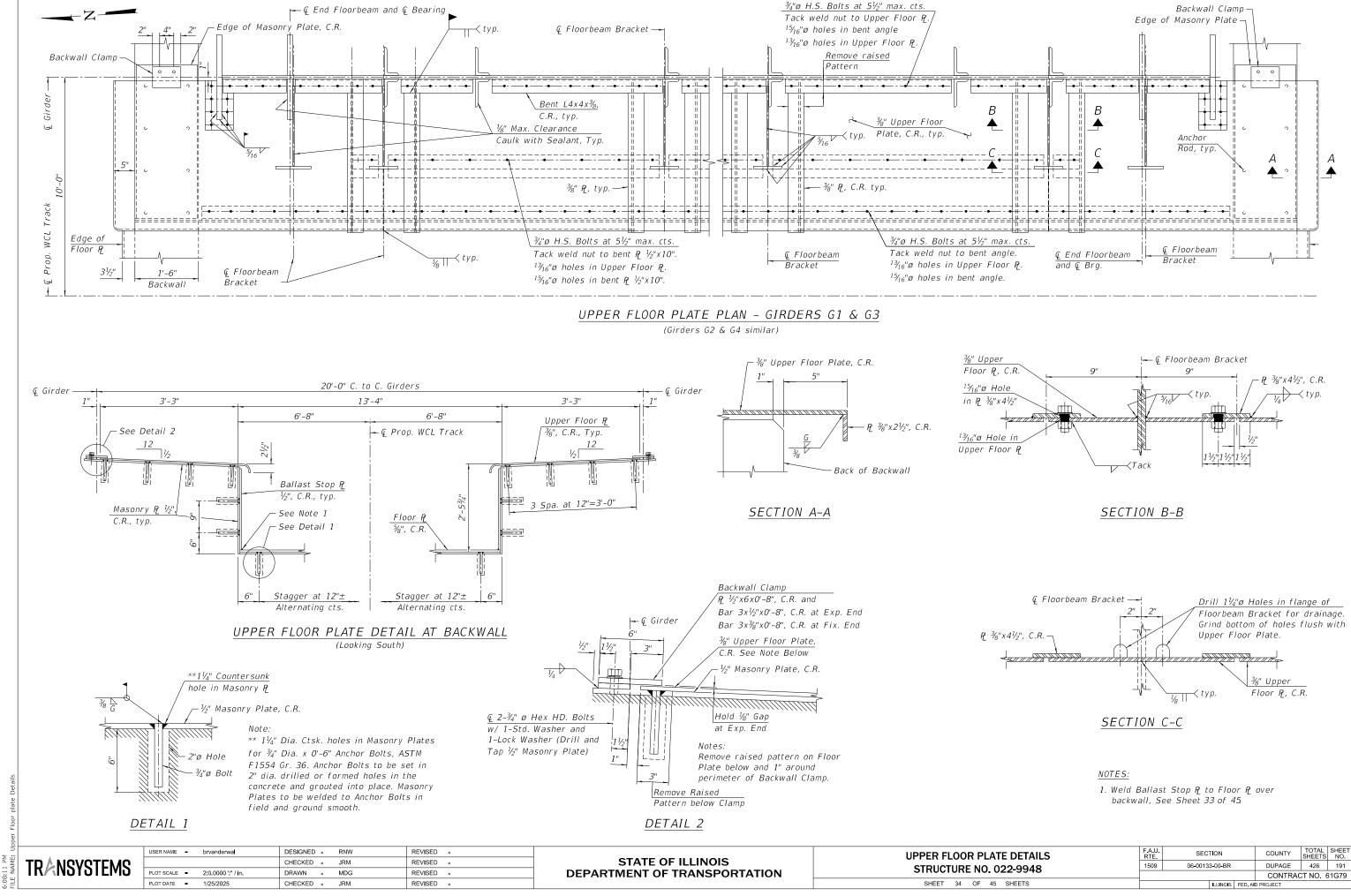
F.A.U. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
1509	06-00133-00-BR	DUPAGE	426	188	
			CONTRAC	T NO. 6	61G79
	ILLINOIS EE	D A	D DDO IECT		

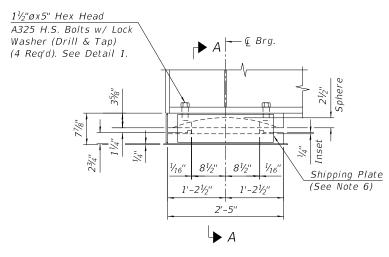
es:

1. All holes shall be $^{15}\!\!/_{16}$ " Ø for $^{7}\!\!/_{8}$ " Ø A325 bolts.

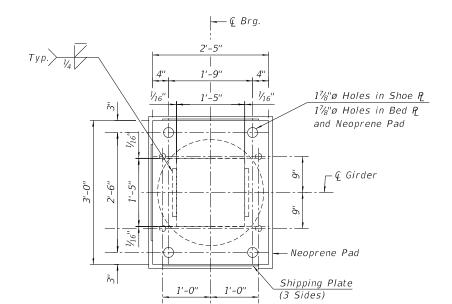






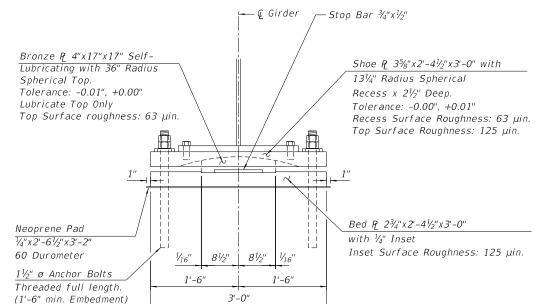


BEARING ELEVATION (North Abutment)

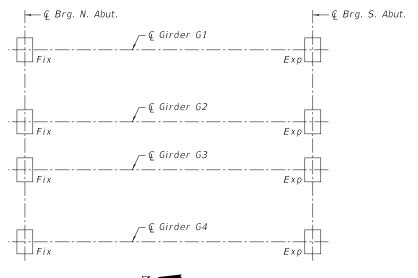


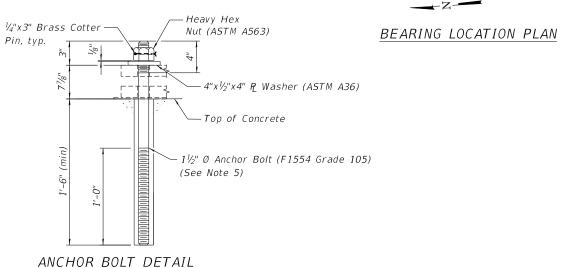
BEARING PLAN

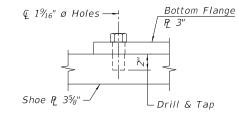
(North Abutment)



SECTION A-A







DETAIL 1

BEARING LOADS

	APPLIED	ALLOWABLE
V_{MIN}^{DL}	149 kips	-
V_{MAX}^{DL}	215 kips	-
V _{MIN} ^{LL+IM}	337 kips	-
V _{MAX}	446 kips	-
V^{MIN}	486 kips	-
V ^{MAX}	661 kips	865 kips
H _{LONG}	119 kips	173 kips
H _{LAT}	20 kips	173 kips

Notes:

- 1. Design and Workmanship shall be in accordance with the current AREMA Manual Chapter 15.
- 2. Material shall be in accordance with the following specifications:

ASTM A709 Grade 50W for Bed and Shoe Plates. Structural Steel:

Bronze Plates: ASTM B22-08 Copper Alloy UNS No. C86300.

AWS D1.5. Welding:

ASTM F1554, Gr. 105 Anchor Bolts:

High Strength Bolts: ASTM F3125 Grade A325 Type 1.

- All holes shall be drilled or sub-punched and reamed.
- All non-sliding surfaces of bearing plates shall be zinc-metallized in accordance with ASTM B833. Zinc coating shall not be less than 0.01".
- 5. All Anchor Bolts, nuts, and washers shall be galvanized in accordance with ASTM F2329. Concrete to be drilled after determining bolt location. Bolts to be grouted using non-shrink grout.
- 6. Bearings shall be shipped assembled with plates 5/16"x61/2"x2'-0" and 5%"ø Machine Bolts. Plates shall be removed after Anchor Bolts have been installed (3 Plates per Bearing).
- 7. Two $\frac{1}{8}$ " adjusting, galv. shim plates shall be provided for each bearing in addition to all other plates or shims and they shall be placed between the Bed Plate and the Neoprene Pad if required. Shim plates shall match the footprint of the Bed Plate.
- 8. Estimated weight of structural steel = 1,195 pounds per bearing. (2 req'd). Cost of fixed bearings and bronze plates included with Furnishing and Erecting Structural Steel, Special.
- 9. Surface roughness of sliding and rotational elements shall not exceed the limits indicated (RMS) in accordance with ANSI/ASME B46.1, Surface Texture.

TRANSYSTEMS

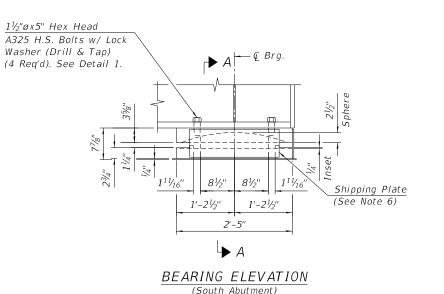
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PLOT DATE = 1/25/2025	CHECKED - JRM	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

FIXED BEARING DETAILS	
STRUCTURE NO. 022-9948	

SECTION COUNTY DUPAGE 426 192 1509 06-00133-00-BR CONTRACT NO. 61G79

SHEET 35 OF 45 SHEETS



- € Brg.

5"x17%" Slotted Holes in Shoe R $1^{7}/\!\!/8$ "Ø Holes in Bed P and

Neoprene Pad

G Girder

1/4"x3" Brass Cotter

ANCHOR BOLT DETAIL

- Neoprene Pad

Shipping Plate

Pin, typ.

2'-5"

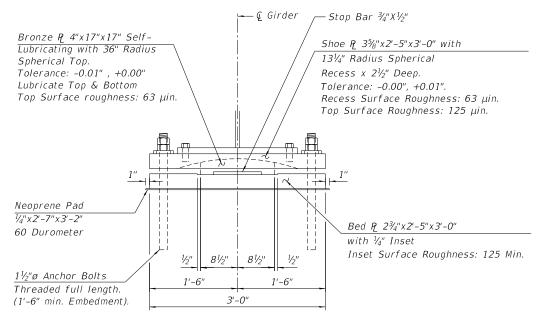
1'-9"

1'-0''

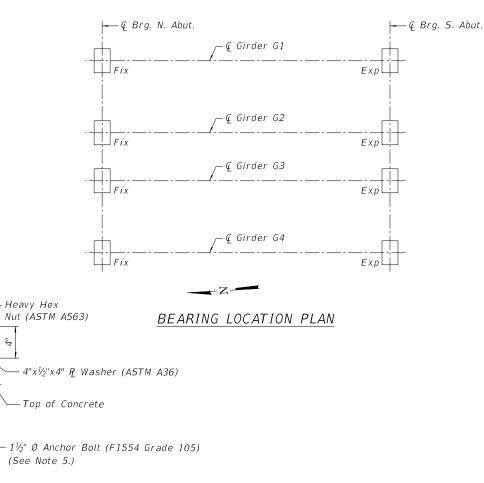
BEARING PLAN

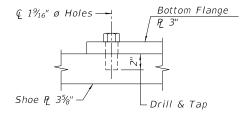
(South Abutment)

1'-0"



SECTION A-A





DETAIL 1

BEARING LOADS

	APPLIED	ALLOWABLE
V_{MIN}^{DL}	149 kips	-
V_{MAX}^{DL}	215 kips	-
V_{MIN}^{LL+IM}	337 kips	-
V_{MAX}^{LL+IM}	446 kips	-
V ^{MIN}	486 kips	-
V ^{MAX}	661 kips	865 kips
H _{LONG}	66 kips	173 kips
H _{LAT}	20 kips	173 kips

Notes:

- 1. Design and Workmanship shall be in accordance with the current AREMA Manual Chapter 15.
- 2. Material shall be in accordance with the following specifications:

Structural Steel: ASTM A709 Grade 50W for Bed and Shoe Plates.

Bronze Plates: ASTM B22-08 Copper Alloy UNS No. C86300.

AWS D1.5 Welding:

ASTM F1554, Grade 105 Anchor Bolts:

High Strength Bolts: ASTM F3125 Grade A325 Type 1.

- 3. All holes shall be drilled or sub-punched and reamed.
- All non-sliding surfaces of bearing plates shall be zinc-metallized in accordance with ASTM B833. Zinc coating shall not be less than 0.01".
- All Anchor Bolts, nuts, and washers shall be galvanized in accordance with ASTM F2329. Concrete to be drilled after determining bolt location. Bolts to be grouted using non-shrink grout.
- 6. Bearings shall be shipped assembled with plates $\frac{5}{16}$ " $x6\frac{1}{2}$ " x2'-0" and 5%"ø Machine Bolts. Plates shall be removed after Anchor Bolts have been installed (3 Plates per Bearing).
- 7. Two 1/8" adjusting, galv. shim plates shall be provided for each bearing in addition to all other plates or shims and they shall be placed between the Bed Plate and the Neoprene Pad if required. Shim plates shall match the footprint of the Bed Plate.
- 8. Estimated weight of structural steel = 2,070 pounds per bearing. (2 reg'd). Cost of expansion bearings and bronze plates included with Furnishing and Erecting Structural Steel, Special.
- Surface roughness of sliding and rotational elements shall not exceed the limits indicated (RMS) in accordance with ANSI/ASME B46.1, Surface Texture.

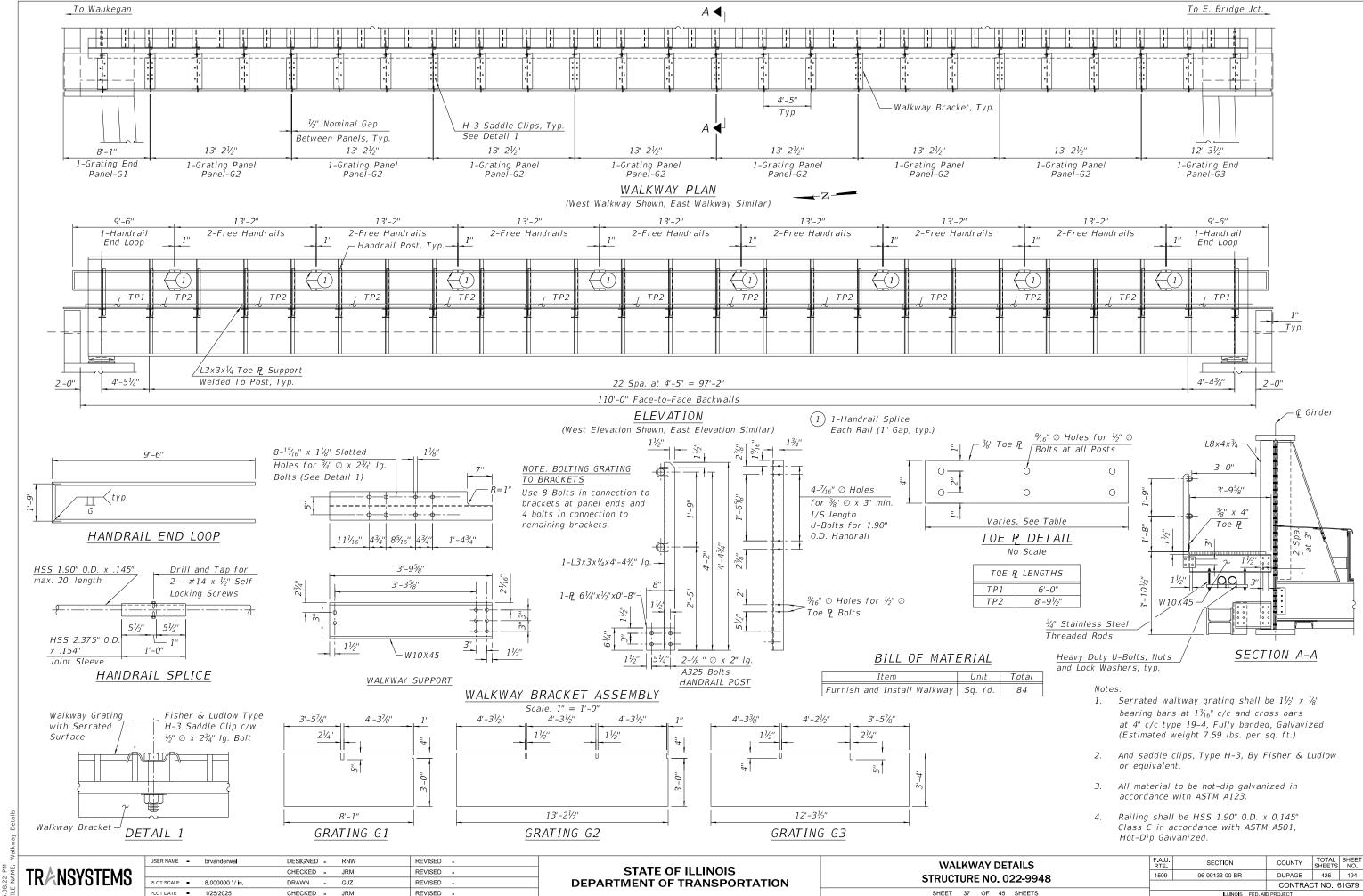
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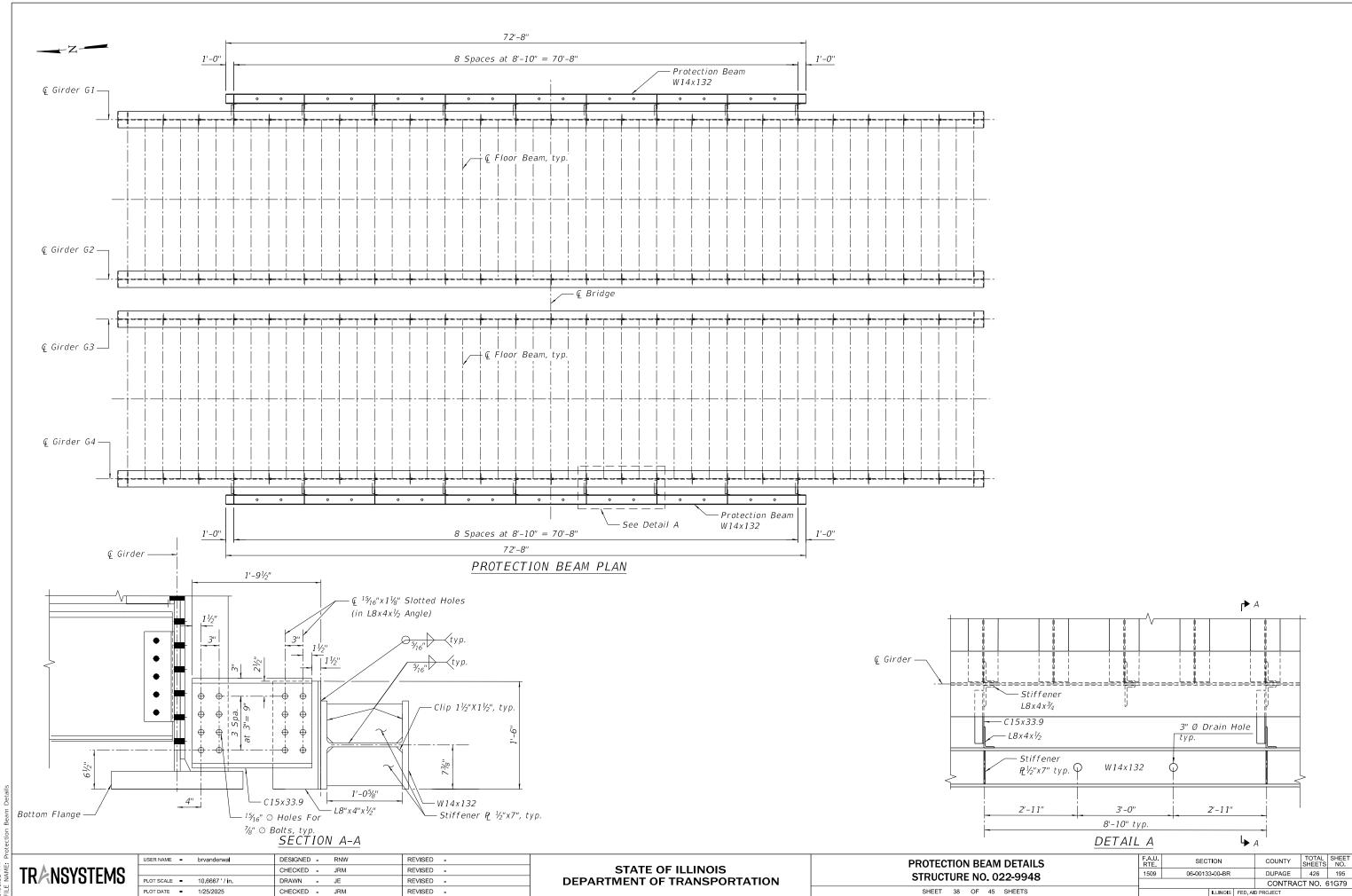
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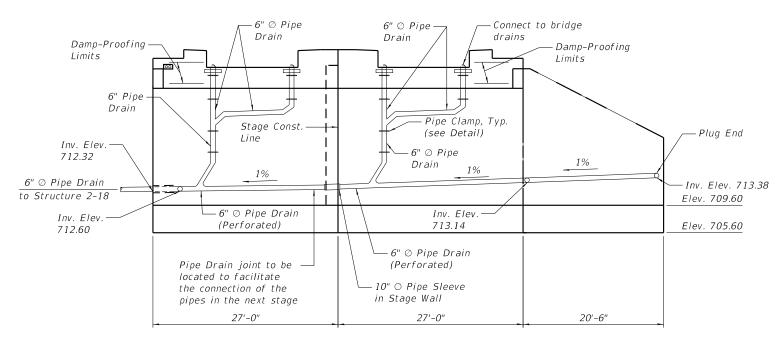
STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

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SECTION COUNTY DUPAGE 426 193 1509 06-00133-00-BR CONTRACT NO. 61G79

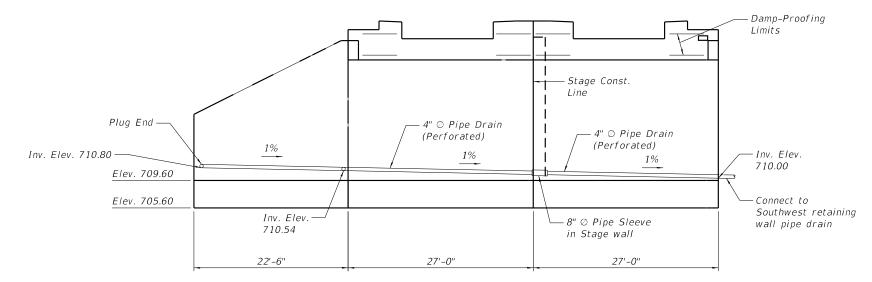






NORTH ABUTMENT ELEVATION

(Looking South at Back Face) (Pipes, Sleeves, Plugs, and Clamps included with Drainage System for Structures)

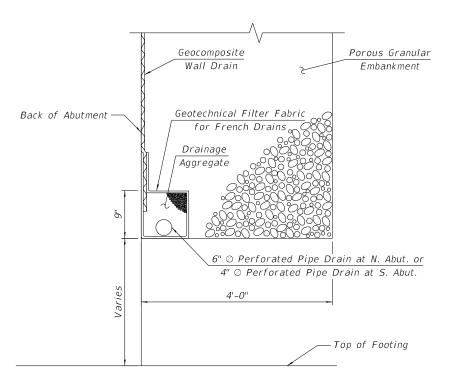


SOUTH ABUTMENT ELEVATION

(Looking North at Back Face) (Pipes, Sleeves, and Plugs included with Pipe Underdrains for Structures, 4")

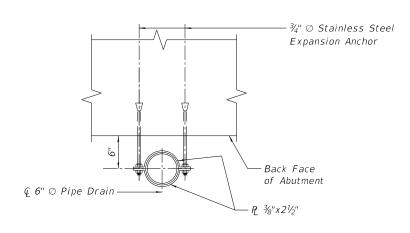
BILL OF MATERIAL

Item	Unit	Total
Drainage System for Structures	L. Sum	1
Pipe Underdrains for Structures, 4"	Foot	84



PIPE UNDERDRAIN DETAIL

Included in the cost of Drainage System for Structures (N. Abut.) or Pipe Underdrains for Structures, 4" (S. Abut.).



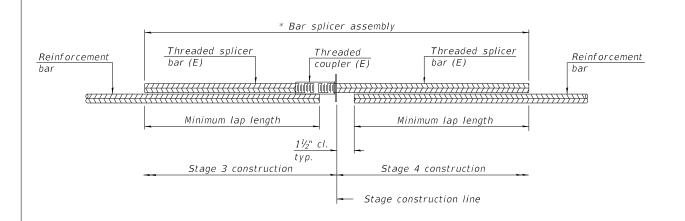
PIPE CLAMP DETAIL

Notes:

- 1. See Drainage Plans for location of drainage structures.
- 2. All pipe drains shall consist of solid pipe, unless stated otherwise.

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F.A.U. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
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			CONTRAC	T NO. 6	31G79
	ILLINOIS	FED. A	D PROJECT		



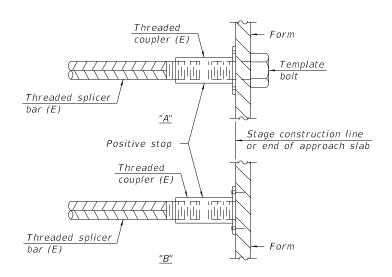
STANDARD BAR SPLICER ASSEMBLY PLAN

(All components shall be provided from one supplier)

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

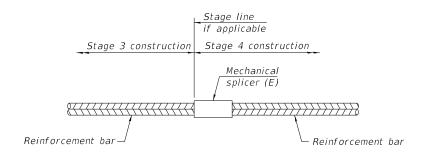
Location	Bar size	No. assemblies required	Minimum lap length
North Abutment Backwall	#5	14	2'-2"
North Abutment Stem	#5	36	2'-2"
North Abutment Stem Top	#5	5	3'-0"
North Abutment Footing	#5	34	3'-0"
South Abutment Backwall	#5	14	2'-2"
South Abutment Stem	#5	36	2'-2"
South Abutment Stem Top	#5	5	3'-0"
South Abutment Footing	#5	34	3'-0"



INSTALLATION AND SETTING METHODS

"A" : Set bar splicer assembly by means of a template bolt "B" : Set bar splicer assembly by nailing to wood forms or

cementing to steel forms. (E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required

Notes:

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

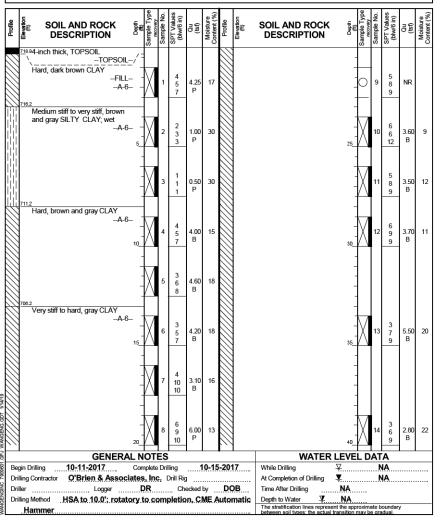
All reinforcement shall be lapped and tied to the splicer bars. Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for

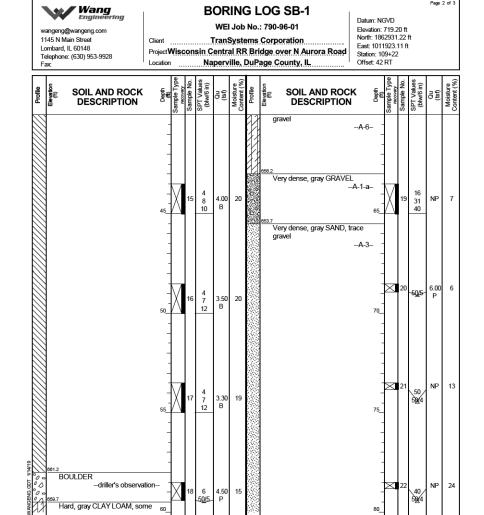
alternatives.

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While Drilling

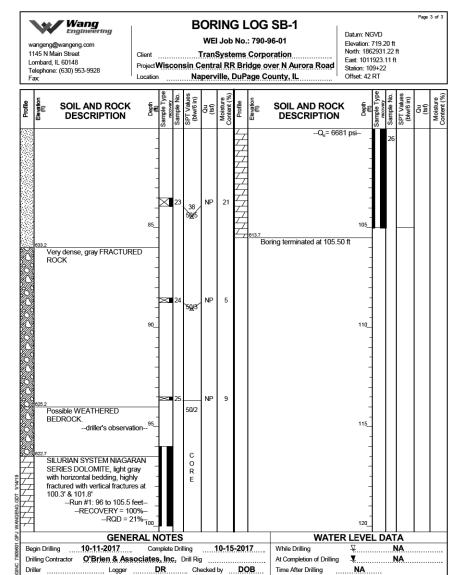
At Completion of Drilling

Time After Drilling

Depth to Water

WATER LEVEL DATA

V NA



Drilling Method .HSA to 10.0"; rotatory to completion, CME Automatic The stratification li



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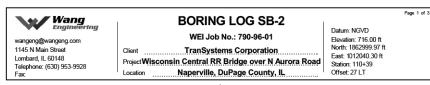
GENERAL NOTES

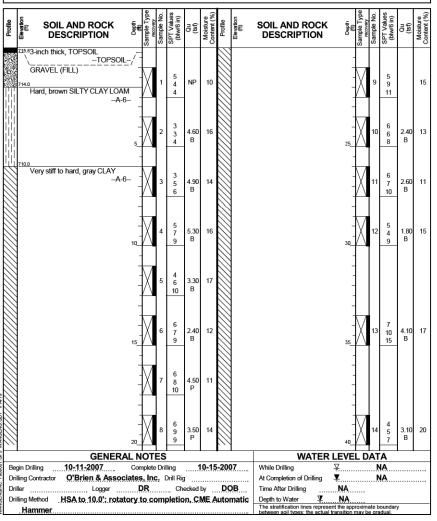
Complete Drilling

DR Checked by DOB

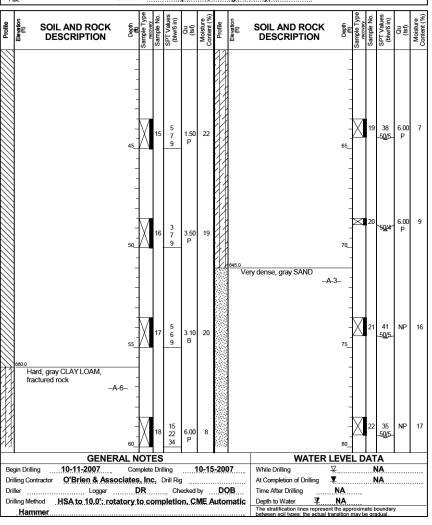
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Drilling Contractor O'Brien & Associates, Inc. Drill Rig

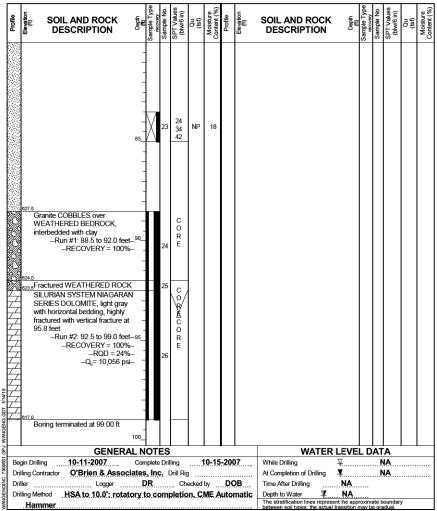




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11 Lo Te	145 N M ombard,	Engineering @wangeng.com lain Street IL 60148 e: (630) 953-9928	Client Project V	Vis		sin C	WE Fran	l Job Syste	No. ems	: 790 Corr	96-01 poration over N Aurora Road County, IL	Datum: No Elevation: North: 186 East: 101: Station: 1 Offset: 27	716.0 52999 2040.	.97 30 ft	ft		
Profile	Elevation (ft)	SOIL AND ROCK DESCRIPTION	Depth	Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture Content (%)	Profile	Elevation (ft)	SOIL AND ROODESCRIPTION		Sample Type	Sample No.	SPT Values (blw/6 in)	Qu (tsf)	Moisture
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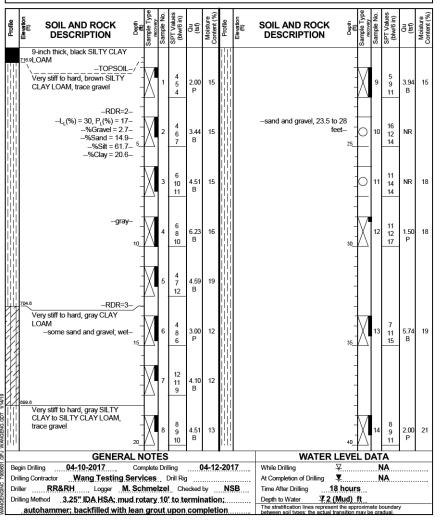


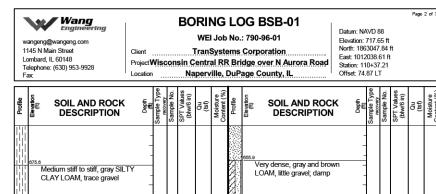


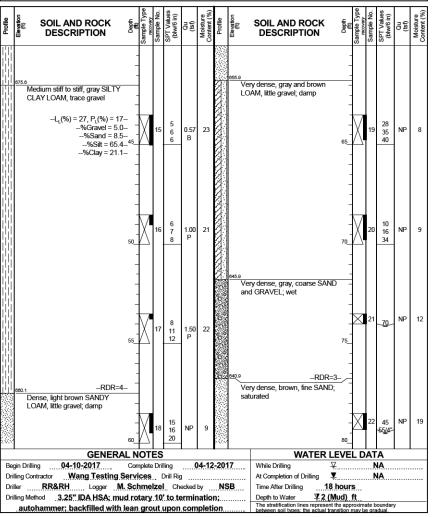


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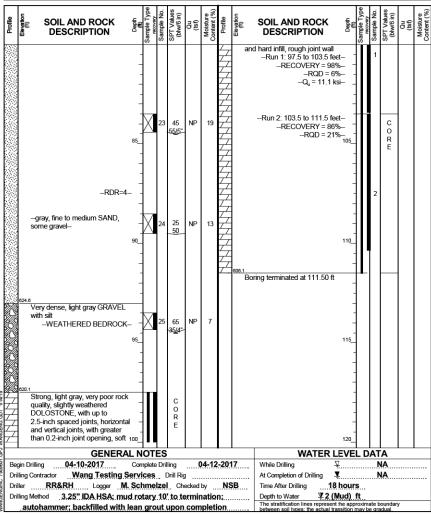












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