SUPERSTRUCTURE - STRUCTURAL STEEL:

ALL STRUCTURAL STEEL SHALL CONFORM TO AASHTO M-183 UNLESS OTHERWISE NOTED.

CALCULATED WEIGHT OF STRUCTURAL STEEL:

C. Wieczorek

2,255,500 Lss, M-183 4,064,590 Lss, M-223, Grade 50 & M-222 68,290 Lss, M-192, Grade 90

ALL SHOP AND FIELD CONNECTIONS, OTHER THAN WELDED SHALL BE MADE WITH 7/8" B HIGH STRENGTH BOLTS AND 15/16" B HOLES AND COMPORN TO AASSITO MM-164 UNLESS OTHERWISE NOTED. Shop BOLTS ARE SHOWN ON THE PLANS BY THE SYMBOL ($\frac{1}{100}$), AND FIELD BOLTS BY ($\frac{1}{100}$). Shop and FIELD CONNECTIONS MAY

FRACTURE CRITICAL MEMBER REQUIREMENTS ARE MANDATORY FOR THE GIRDERS. THE PLANS. SEE SPECIAL PROVISIONS.

SUPPLEMENTAL NOTCH TOUGHNESS REQUIREMENTS (ZONE 2) ARE MANDATORY FOR ALL STRINGERS, BINK BARS, SPLICE PLATES, AND BOTTOM FLANGES AND NEBS OF ALL FLOORBEAMS. Frances comm.
THESE COMPONENTS ARE NOTED N.T.R. ON PLANS, SEE SPECIAL PROVISIONS.

PROVISIONS HAVE BEEN MADE FOR JACKING THE TOTAL DEAD LOAD ONLY. OF THE ARCH SPAN AT BOTH PIERS.

A COMPLETE PENETRATION GROOVE WELD SHALL BE USED WHENEVER THE TERM "GROOVE WELD" IS USED ON THE PLANS. FIELD WELDING OF CONSTRUCTION ACCESSORIES WILL NOT BE PERMITTED TO ANY PART OF THE ARCH TIES, THE BOTTOM FLANGES AND NEBS OF FLOORBEAKS AND STRINGERS OR TO THE TOP FLANGE OF STRINGERS FOR A DISTANCE EQUAL TO ONE-FOURTH THE SPAN LENGTH EACH WAY FROM THE CENTER OF SUPPORT OVER WRICH THE MEMBER IS CONTINUOUS. FIELD WELDING IN OTHER AREAS WILL BE PERMITTED ONLY WHEN APPROVED BY THE ENGINEER.

GENERAL NOTES

ARCH SPAN SHALL BE FABRICATED TO THE FINAL GEOMETRIC SHAPE UNDER FULL DEAD LOAD EXCLUDING FUTURE MEARING SURFACE. A FABRICATION ALLONANCE SHALL BE MADE FOR SHORTENING OF THE ARCH RIB AND LENGTHENING OF THE TIE GIRDER AND HANGERS NDER FULL DEAD LOAD EXCLUDING FUTURE HEARING SURFACE. SEE SPECIAL PROVISIONS

HANGERS SHALL CONSIST OF MULTIPLE-WIRE BRIDGE STRAND CONFORMING TO ASTM A585 AND SHALL BE ZINC COATED. ZINC COATING SHALL BE CLASS C ON OUTER WIRES. AND CLASS A ON INNER WIRES. MINIMUM METALLIC AREA OF EACH STRAND SHALL BE 1.59 SQUARE INCHES AND HAVE A MINIMUM BREAKING STRENGTH (EACH STRAND) OF 310 KIPS. MODULUS OF ELASTICITY AFTER PRESTRESSING SHALL BE 24,000 KSI.

THE CONTRACTOR SHALL SUBHIT TO THE ENGINEER COMPLETE PLANS OF PROPOSED EREC-TION SCHEMES FOR THE WORK SHOWING ERECTION LOAD STRESSES. ANY MATERIAL ORDERED PRIOR TO THE REVIEW OF THE ERECTION SCHEME BY THE ENGINEER SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ERECTION STRESSES SHALL BE COMPOSED OF THE STEEL DEAD LOAD OF THE STRUCTURE AND ERECTION EQUIPMENT PLUS THE STRESS FROM WIND OF 45 POUNDS PER SQUARE FOOT ON THE STRUCTURE AND ERECTION EQ. MENT IN ELEVATION. ERECTION STRESSES SHALL NOT EXCEED NORMAL UNIT STRESS BY MORE THAN 33-1/3 PERCENT. NO PAYMENT WILL BE MADE FOR ANY EXTRA MATERIAL REQUIRED DUE TO ERECTION CONDITIONS. REVIEW OF THE ERECTION PLANS SHALL NOT RELIEVE THE CONTRACTOR FROM HIS FULL RESPONSIBILITY FOR THE SAFETY OF THE ERECTION SCHEMES.

NOTE: DO NOT SCALE THIS DRAWING. FOLLOW DIMENSION:

SEE SPECIAL PROVISIONS FOR NAVIGATIONAL CONSIDERATIONS AND CONSTRUCTION IN

THE ZINC-SILICATE AND YINYL PAINT SYSTEM SHALL BE USED FOR SHOP PAINTING AND MELD PRINTING OF STRUCTURAL STEEL. SEE SPECIAL PROVISIONS.

ALL CONTACT SURFACES OF JOINTS WITH FRICTION TYPE BOLTS SHALL BE FREE OF PAINT OR LACQUER.

ALL LENGTHS SHOWN ON PLANS ARE AT A NORMAL TEMPERATURE OF 50° F, under total dead load excluding FMS, except as noted

In the application of the Standard Specifications, the Special Provisions and the Supplementary requirements stated Merein, the following structural PARTS SHALL BE CLASSIFIED AS MAIN MEMBERS: STRINGERS, FLOORBEAMS, ARCH MEMBERS (TIES, HANGERS, AND RIBS), UPPER BRACING BETWEEN THE ARCH RIBS AND THE COMPONENTS OF THE LOWER LATERAL BRACING SYSTEM.

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7-11-83

TIED ARCH SPAN GENERAL NOTES, ANCHOR BOLT PLAN, QUANTITIES AND INDEX

FA-412 OVER ILLINOIS RIVER SECTION 50-48(F&E) PROJECT EBF-412-4(6) STA. 863+1600 (FA-412) LASALLE CO.

SHEET NO. 2 OF 24

EXISTING TIED ARCH GENERAL NOTES ABRAHAM LINCOLN MEMORIAL BRIDGE OVER THE THE MOTE DIVED IDEAL TO WATERON

St Pier & & Brg., Tied Arch Span Le Pier & & Brg., Tied Arch Spon - t / 3" Dia Bearing Base Pl. Limits (6'-6"x6'-0")-€ 1-4" Dio. Ancher Belt. & Tied Arch Span Bose Costing Limits (5'-0"x5'-0") Maintaining Engineers Field Office 4. I ³4" Dio. Anchor BoltS Live Load Supp Base Pl. Limits (1'-6"x 2'-72") — Live Load Support Base Pl. Limits - E Live Load Approach Span Bridge Seat -6 194" Dia -Approach Spar Bridge Seat Anchor Bolt - Symm, obt. + FA-412 C. Wieczorek PIER 35(28) R.L. Olson G.J. Roufa PIER 34 (27) ANCHOR BOLT PLAN D. Smithpete

FREPARED BY: SVERDRUP & PARCEL AND ASSOCIATES, Inc

DESIGNED -	VH
CHECKED -	HMA
CHECKED	ПМН
DRAWN -	VH
CHECKED -	HMA

FOR INFORMATION ONLY



Alfred Benesch & Company 205 North Michigan Avenue, Suite 2400 Chicago, Illinois 60601 312-565-0450 J Job No. 3938.05

SUPERSTRUCTURE - STRUCTURAL STEEL

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SHEET NO. S4	F.A.I. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
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SHEETS S6			CONTRACT	NO. 66	SA34

FED. ROAD DIST. NO. | ILLINOIS FED. AID PROJECT