

	0.4 Span 1	W. Pier	0.5 Span 2	M. Pier	0.5 Span 3	E. Pier	0.6 Span 4
Is (in #)	1600	1600	1600	1600	1600	1600	1600
Ss (in #)	151	151	151	151	151	151	151
Q (K/ft.)	0.865	0.865	0.865	0.865	0.865	0.865	0.865
M _Q (K)	39	-57	23	-64	41	-82	46
M _L (K)	128	-121	121	-121	151	-136	151
M (Imp) (K)	38.4	-36.3	36.3	-36.3	45.3	-40.8	45.3
S ₃ (M _L + I) (K)	277	-262	262	-262	327	-295	327
Mo (K)	411	-415	371	-424	479	-491	485
fs @ non-comp (K.s.i.)	3.1	-4.5	1.8	-5.1	3.3	-6.5	3.7
fs S ₃ (L + I) (K.s.i.)	22.0	-20.8	20.8	-20.8	26.0	-23.4	26.0
fs (Overload) (K.s.i.)	25.1	-25.3	22.6	-25.9	29.3	-29.9	29.7
fs (Total) (K.s.i.)	32.6	-32.9	29.4	-33.7	38.0	-38.9	38.6
VR (K)	23	65	23	69	23	68	25

Flange Splice P
 $5_8 \times 8 \frac{1}{4} \times 3'-2"$
 Each Flange

$1 \frac{1}{2}"$ Max.

$7 \frac{1}{8}" \phi$ H.S. Bolts

$2"$

$4"$

$2"$

$2"$

$5 \text{ sp. at } 3" \text{ cts.}$

$4"$

$5 \text{ sp. at } 3" \text{ cts.}$

$2"$

$1 \frac{1}{2}"$

$7 \frac{1}{8}" \phi$ H.S. Bolts

$5 \text{ sp. at } 3" \text{ cts.}$

$1 \frac{1}{2}"$

Web Splice P
 $3_8 \times 1'-1" \times 1'-6"$
 Each Side

$1 \frac{1}{2}"$

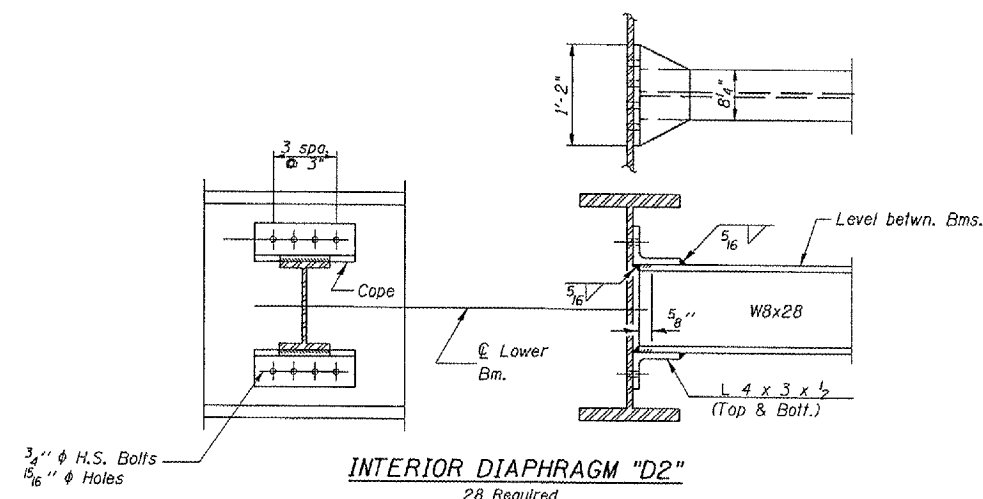
$3" \text{ cts.}$

$4"$

$3" \text{ cts.}$

$1 \frac{1}{2}"$

Location	STATION	Girder 1	Girder 2	Girder 3	Girder 4	Girder 5
☺ Bearing W. Abutment	9+44.63	99.197	99.066	98.936	98.742	98.548
0.25 L	9+50.60	99.197	99.069	98.942	98.753	98.564
0.5 L	9+56.58	99.197	99.073	98.948	98.766	98.585
0.75 L	9+62.56	99.199	99.077	98.955	98.781	98.607
☺ Bearing W. Pier	9+68.54	99.199	99.080	98.961	98.794	98.628
0.25 L	9+75.39	99.171	99.070	98.969	98.811	98.653
0.5 L	9+82.25	99.143	99.059	98.976	98.827	98.678
0.75 L	9+89.10	99.113	99.048	98.982	98.842	98.701
☺ Bearing M. Pier	9+95.96	99.085	99.038	98.990	98.858	98.726
0.25 L	10+04.00	99.036	99.015	98.994	98.870	98.746
0.5 L	10+12.04	98.971	98.983	98.994	98.875	98.756
0.75 L	10+20.09	98.908	98.951	98.995	98.881	98.767
☺ Bearing E. Pier	10+28.13	98.843	98.919	98.995	98.886	98.777
0.25 L	10+34.96	98.843	98.919	98.995	98.890	98.786
0.5 L	10+41.80	98.842	98.918	98.994	98.894	98.794
0.75 L	10+48.63	98.842	98.918	98.994	98.898	98.802
☺ Bearing E. Abutment	10+55.46	98.843	98.919	98.995	98.904	98.813



Note: over all

5" Constant dimension. Slope with beams.

2 5/8" dia. 3" x 6"

3'-0"

2'-0"

1 1/2"

1 1/2"

6" ϕ

12x25

1/2" ϕ

4 Sides

L6"x4"x1/2"

6 3/4" ϕ H.S. bolts

5/8" ϕ holes

4 Sides

A

A

DSGN	K.J. Hoffmann								
DR	K.J. Hoffmann								
CHK	J.R. Wolf								
APVD	J.A. Frauenhoffer	NO.	DATE		REVISION			BY	APVD

Frauenhoffer and Associates, P.C. Consulting Engineers
3002 Crossing Court Champaign, Il. 61822 217-351-6268

STEEL FRAMING DETAILS (SHEET 2 OF 2)
FAS 1384 (C.H. 27) OVER M. BRANCH OF COPPERAS CREEK
SECTION 07-00117-03-BR
FULTON COUNTY

SHEET	17
DWG NO.	7022-STL.dgn
DATE	OCT 2007
PROJ NO.	7022