

EXPIRES 11-30-2008

Exist. Overtop. 185** 1,300 189.37 N/A 437.89 1.41 N/A 439.30 Max. Calc. 500 1,523 N/A 330.47 437.96 N/A 1.30

*Proposed Condition (Existing Low Grade Elevation: 439,30 ft. © Sta. 1453+44,76)
**The Proposed Condition Yielded No Overtopping through the 500-Year Frequency

10 632 168.35 289.84 436.60 0.03 -0.04 436.63 436.56

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Top of Slab Elevations Superstructure

Superstructure Details

Diaphragm Details Structural Steel

Structural Steel Details

West Abutment East Abutment

Cantilever Forming Brackets

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18 Soil Boring Logs

GENERAL NOTES Stage Construction Details Temporary Concrete Barrier

Fasteners shall be AASHTO M164 Type 1, mechanically galvanized bolts. Bolts $\frac{3}{4}$ in. ϕ , holes $\frac{15}{16}$ in. ϕ , unless otherwise noted.

ROUTE NO. SECTION

Contract #76949

58R-2

F.A.P.

COUNTY

WASHINGTON

Calculated weight of Structural Steel = 59280 lb.

No field welding is permitted except as specified in the contract documents. Reinforcement bars shall conform to the requirements of ASTM A 706 Gr 60 (IL Modified). See Special Provisions

TOTAL SHEET NO.

27

97

SHEET NO. 1

18 SHEETS

Reinforcement bars designated (E) shall be epoxy coated.

The Inorganic Zinc Rich Primer / Acrylic / Acrylic Paint System shall be used for shop and field painting of new structural steel except where otherwise noted. The color of the final finish coat for all interior steel surfaces shall be gray, Munsell No. 5B 7/1. The color of the final finish coat for the exterior and bottom flange of the fascia beams shall be Gray, Munsell No. 5B 7/1. See Special Provision for "Cleaning and Painting New Metal Structures".

Layout of slope protection system may be varied in the field to suit ground conditions as directed by the Engineer.

The Contractor shall drive test piles to 110% of the nominal required bearing specified in production locations at substructures specified or approved by the Engineer before ordering the remainder of piles.

The embankment configuration shown shall be the minimum that must be placed and compacted prior to construction of the abutments.

Slip forming of the parapets is not allowed.

Excavation behind existing abutment walls shall be performed to balance front and back soil pressure before removing the existing superstructure. The Contractor shall sawcut the upper portion of the existing abutment at the stage removal line before Stage I removal to ensure the remaining portion will not be prematurely damaged.

TOTAL BILL OF MATERIAL

ITEM	UNIT	SUPER	SUB	TOTAL
Porous Granular Embankment (Special)	Cu. Yd.		86	86
Stone Riprap, Class A4	Sq. Yd.			525
Filter Fabric	Sq. Yd.			525
Removal of Existing Structures No. 1	Each			1
Structure Excavation	Cu. Yd.		140	140
Concrete Structures	Cu. Yd.		28.8	28.8
Concrete Superstructure	Cu. Yd.	88.1		88.1
Bridge Deck Grooving	Sq. Yd.	213		213
Protective Coat	Sq. Yd.	281		281
Furnishing and Erecting	L. Sum	1		1
Structural Steel				
Stud Shear Connectors	Each	1134		1134
Reinforcement Bars, Epoxy Coated	Pound	18200	3760	21960
Bar Splicers	Each	267	18	285
Driving Piles	Foot		245	245
Test Pile Steel, HP12x53	Each		1	1
Furnishing Steel Piles, HP12x53	Foot		245	245
Name Plates	Each	1		1
Anchor Bolts, 1"	Each		24	24
Geocomposite Wall Drain	Sq. Yd.		51	51
Pipe Underdrains for Structures 4"	Foot		138	138
Floor Drains	Each	8		8
Temporary Soil Retention System	Sq. Ft.			364.3
Concrete Encasement	Cu. Yd.		4.2	4.2

LOADING HL-93

STATION BUILT 20 BY STATE OF ILLINOIS .A.S. RT. 1832 SEC.5BR-2 LOADING HL93 STRUCTURE NO. 095-0077

NAME PLATE

See Std. 515001

Allow 50#/sq. ft. for future wearing surface.

DESIGN SPECIFICATIONS

DESIGN STRESSES

FIELD UNITS

<u>SEISMIC DATA</u>

Seismic Performance Category (SPC) = 2 Bedrock Acceleration Coefficient (A) = .11a Site Coefficient (S) = 1.5



GENERAL PLAN IL. RTE. 160 OVER PLUM CREEK F.A.S. RTE. 1832 - SEC. 5BR-2 WASHINGTON CO. STATION 1453+11.50 STRUCTURE NO. 095-0077

= 50,000 psi (structural steel) = 60,000 psi (reinforcement)

CHECKED

PRI /NRB