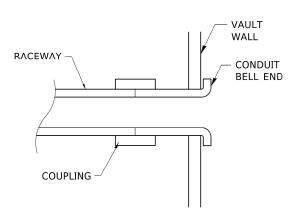
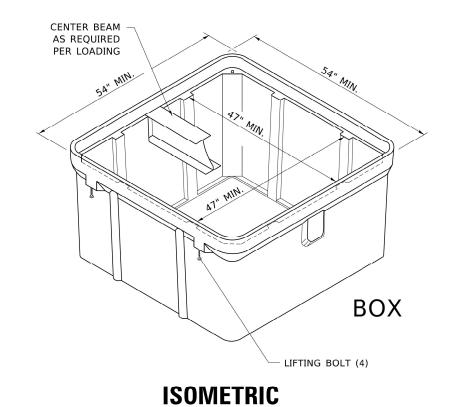
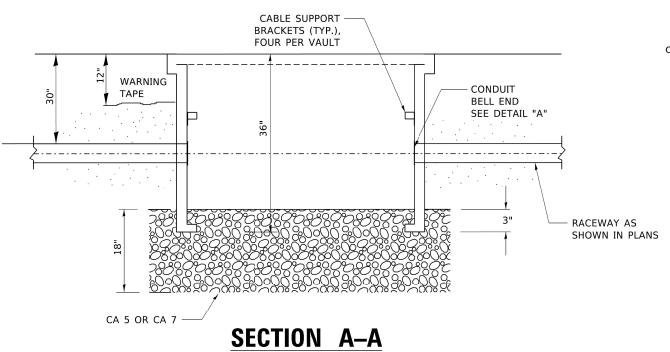
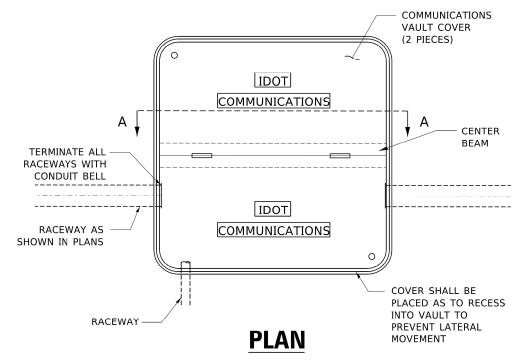
COMMUNICATIONS VAULT LOAD RATINGS							
COMPONENT	ANSI	I LOADING					
COMPONENT	TIER	DESIGN	TEST				
ВОХ	22	22,500 lbs.	37,750 lbs.				
COVER	22	22,500 lbs.	37,750 lbs.				

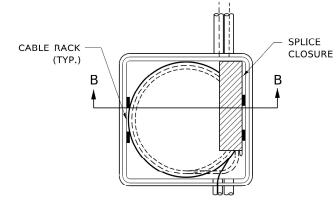


DETAIL A

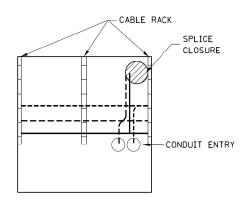








TOP VIEW



SECTION B-B

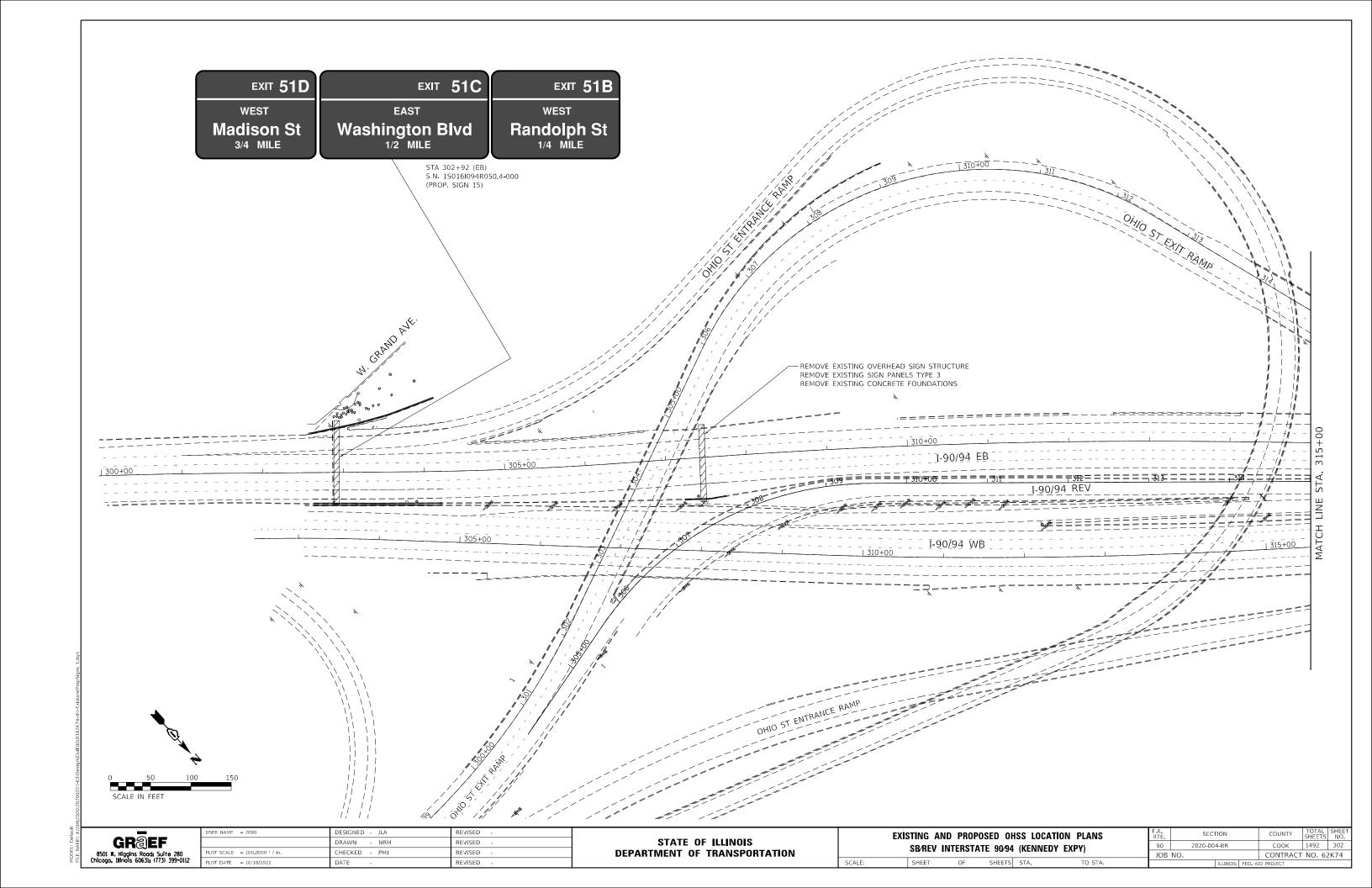
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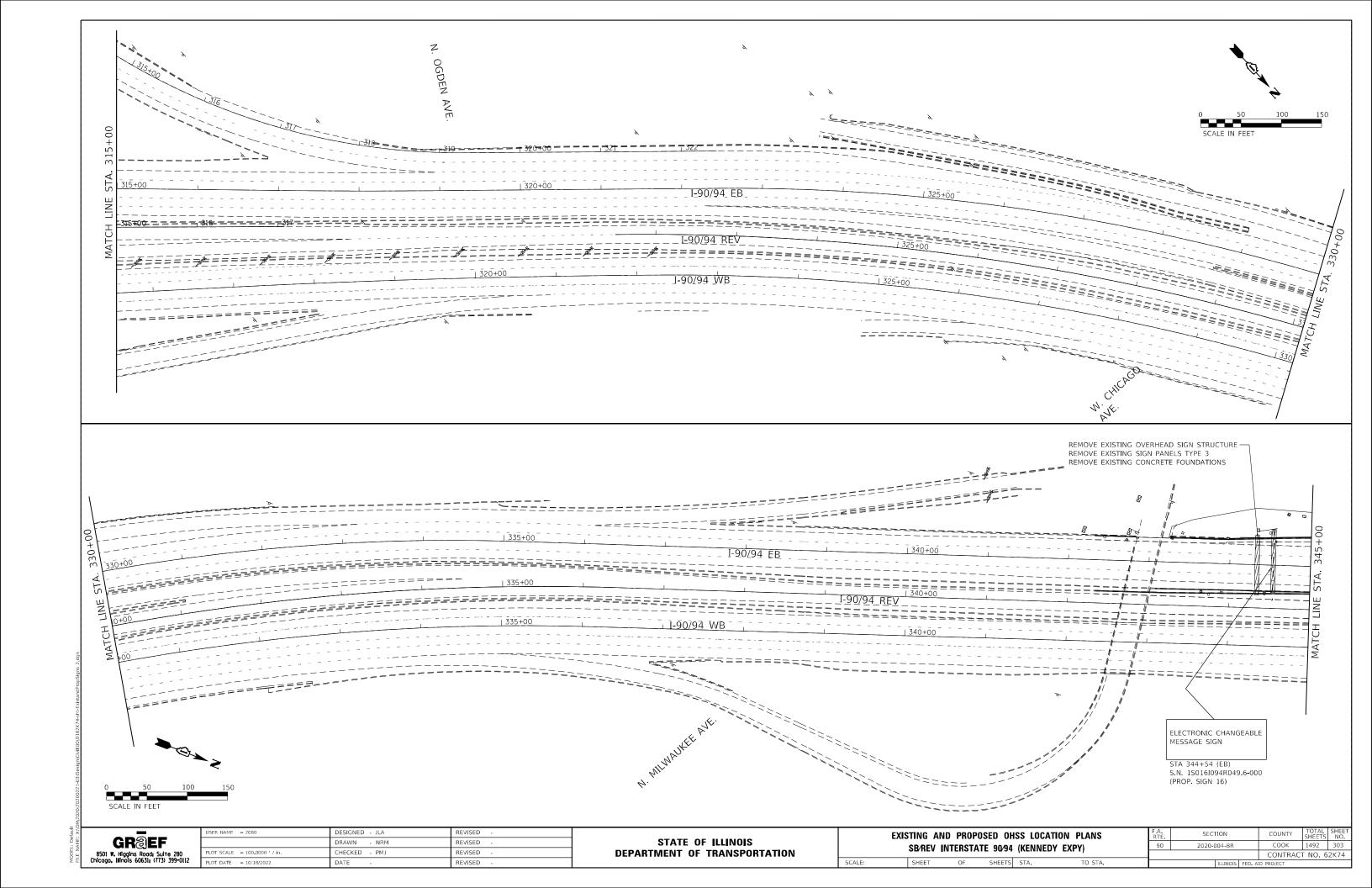
- 1. BOX SHALL HAVE AN OPEN BASE.
- ALL OPENINGS IN STRUCTURE MUST BE MACHINED AT TIME OF FABRICATION OR PUNCH DRIVEN AT TIME OF PLACEMENT. IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS.
- 3. FIELD PLACEMENT OF COMMUNICATIONS VAULT SHALL BE AS DIRECTED BY THE ENGINEER.
- 4. ALL DIMENSIONS ARE MINIMUM AND A LARGER SIZE HANDHOLE MAY BE USED, WITH THE APPROVAL OF THE ENGINEER, TO FΛCILITΛΤΕ USING Λ ΜΛΝUFΛCTURER'S STANDARD PRODUCT.

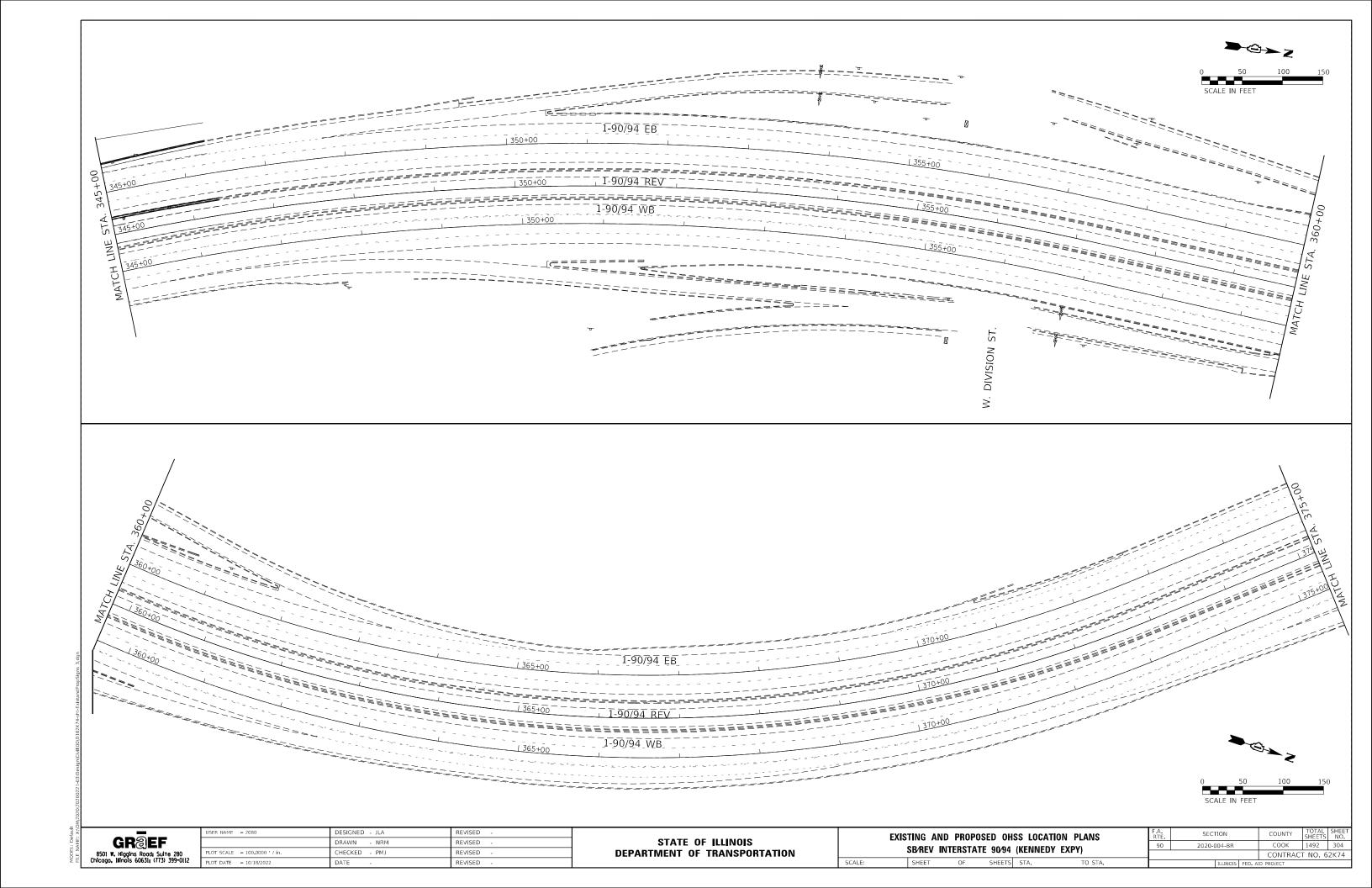
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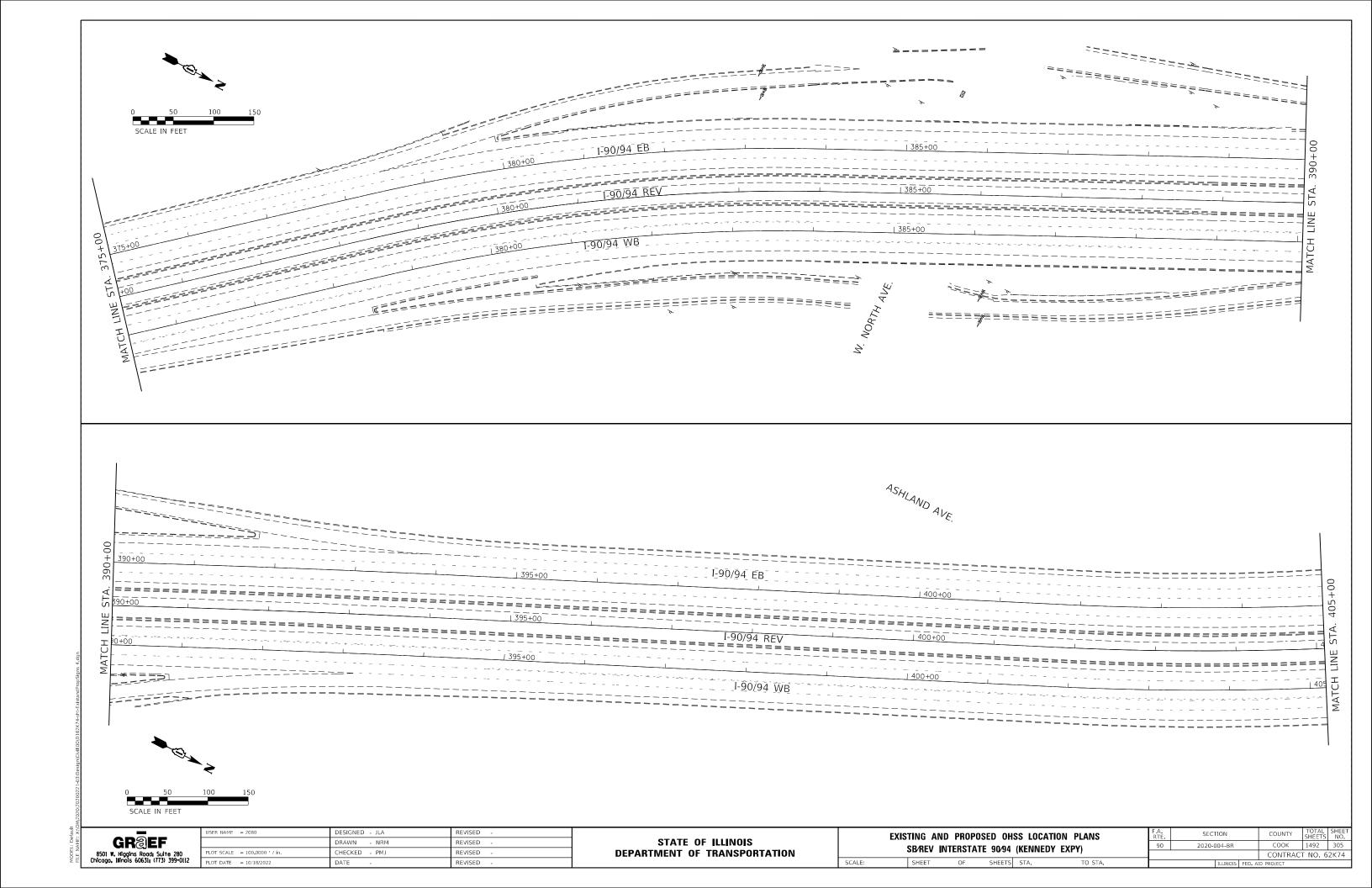
STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

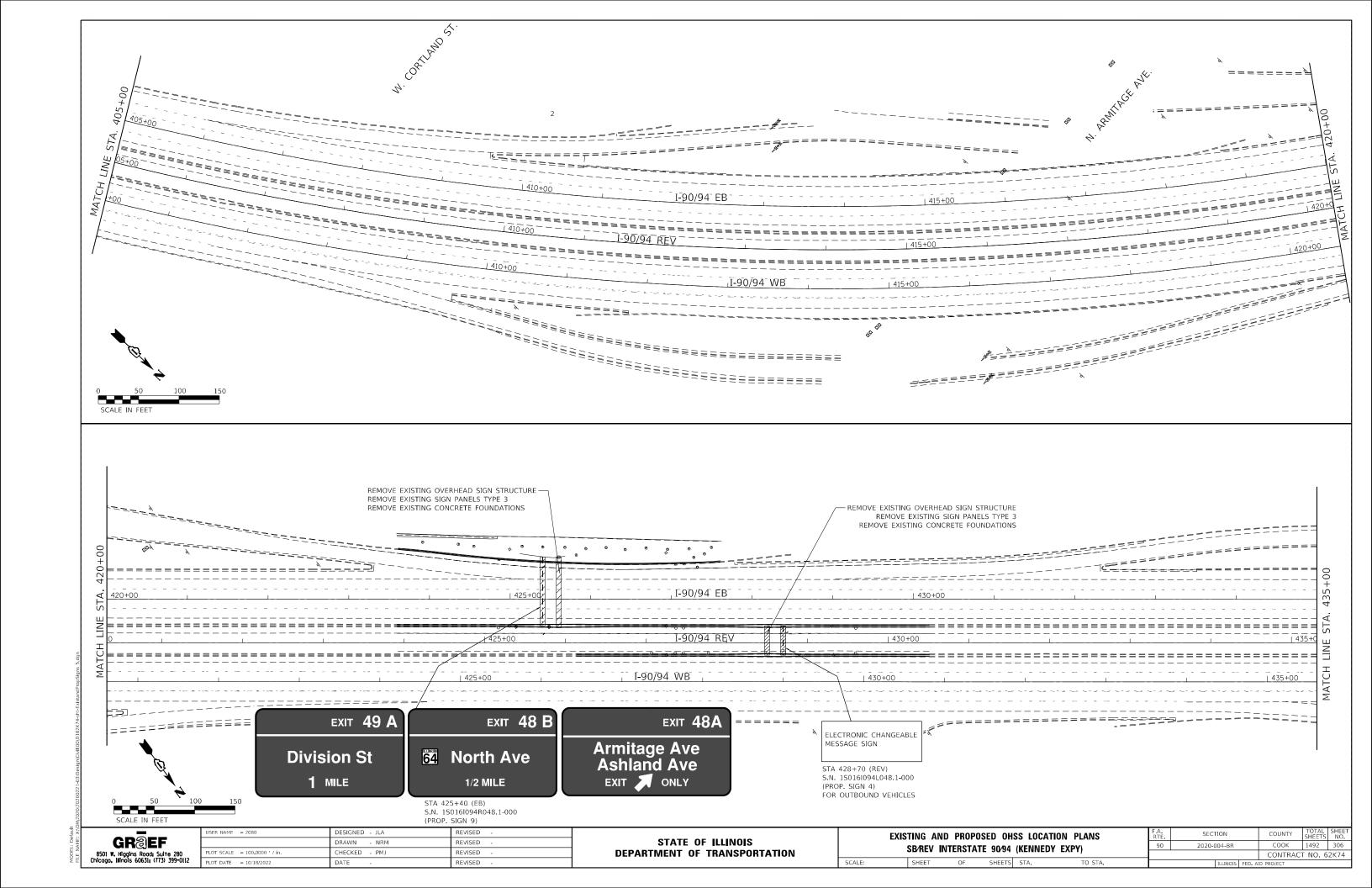
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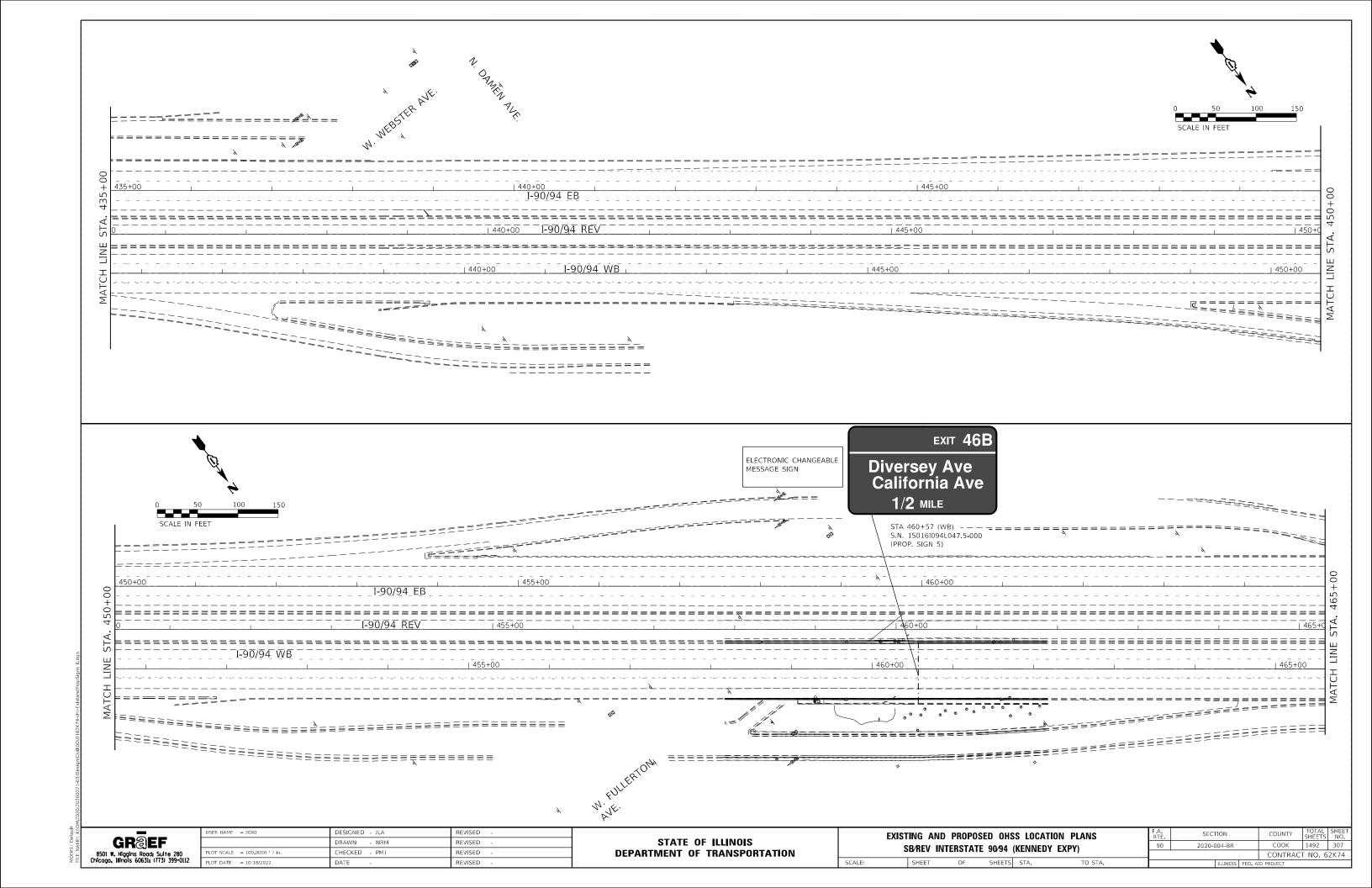


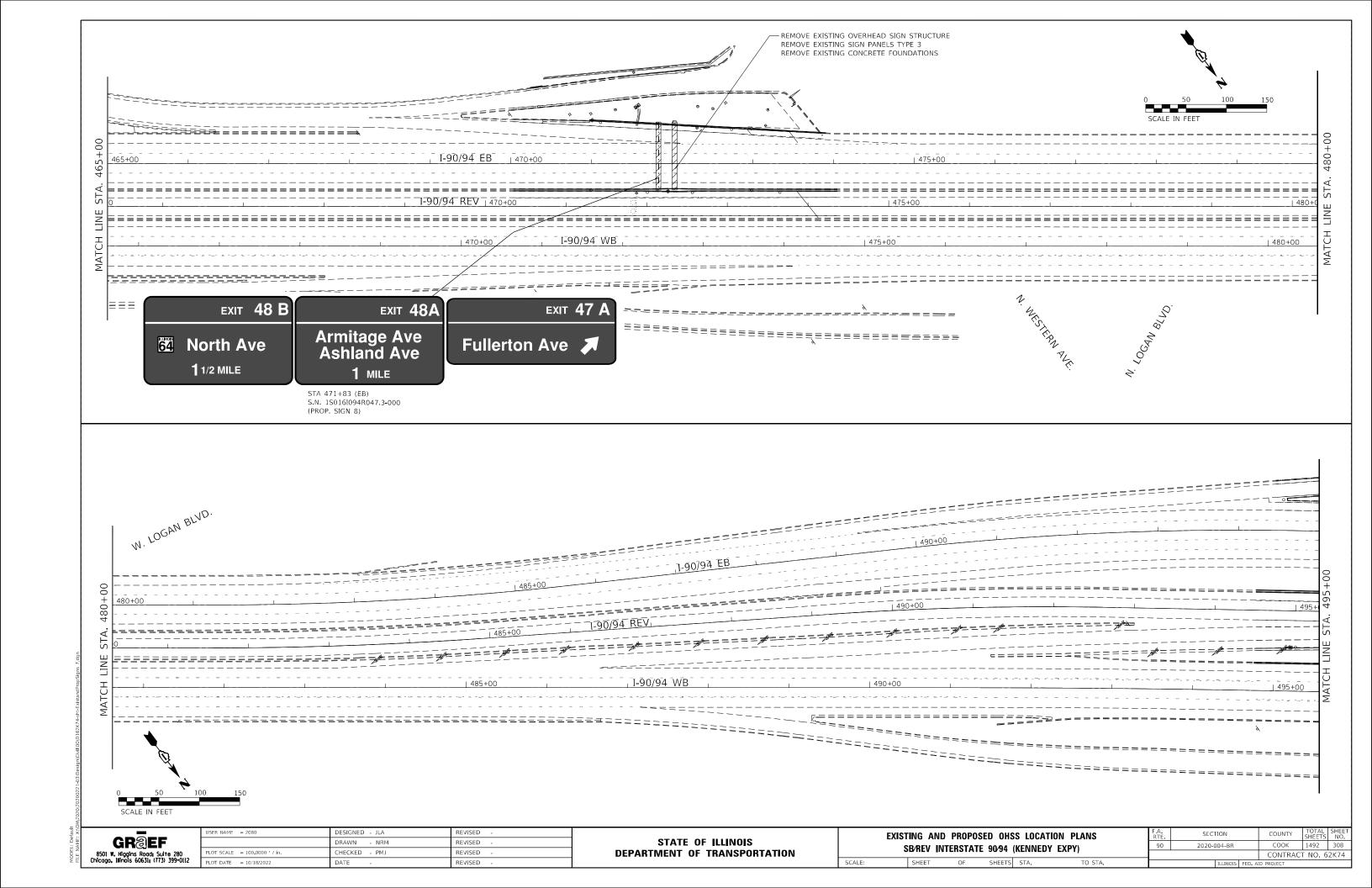


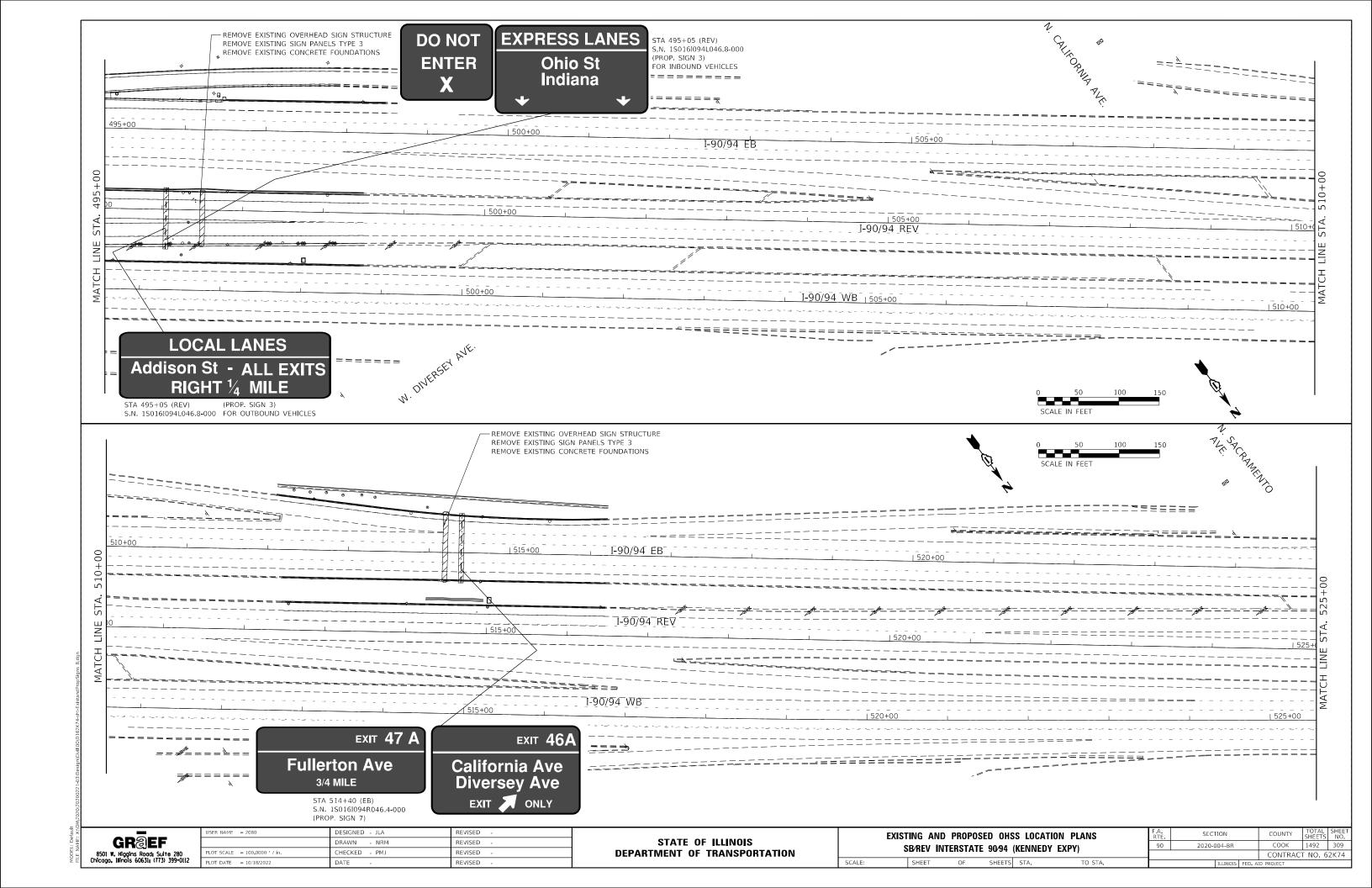


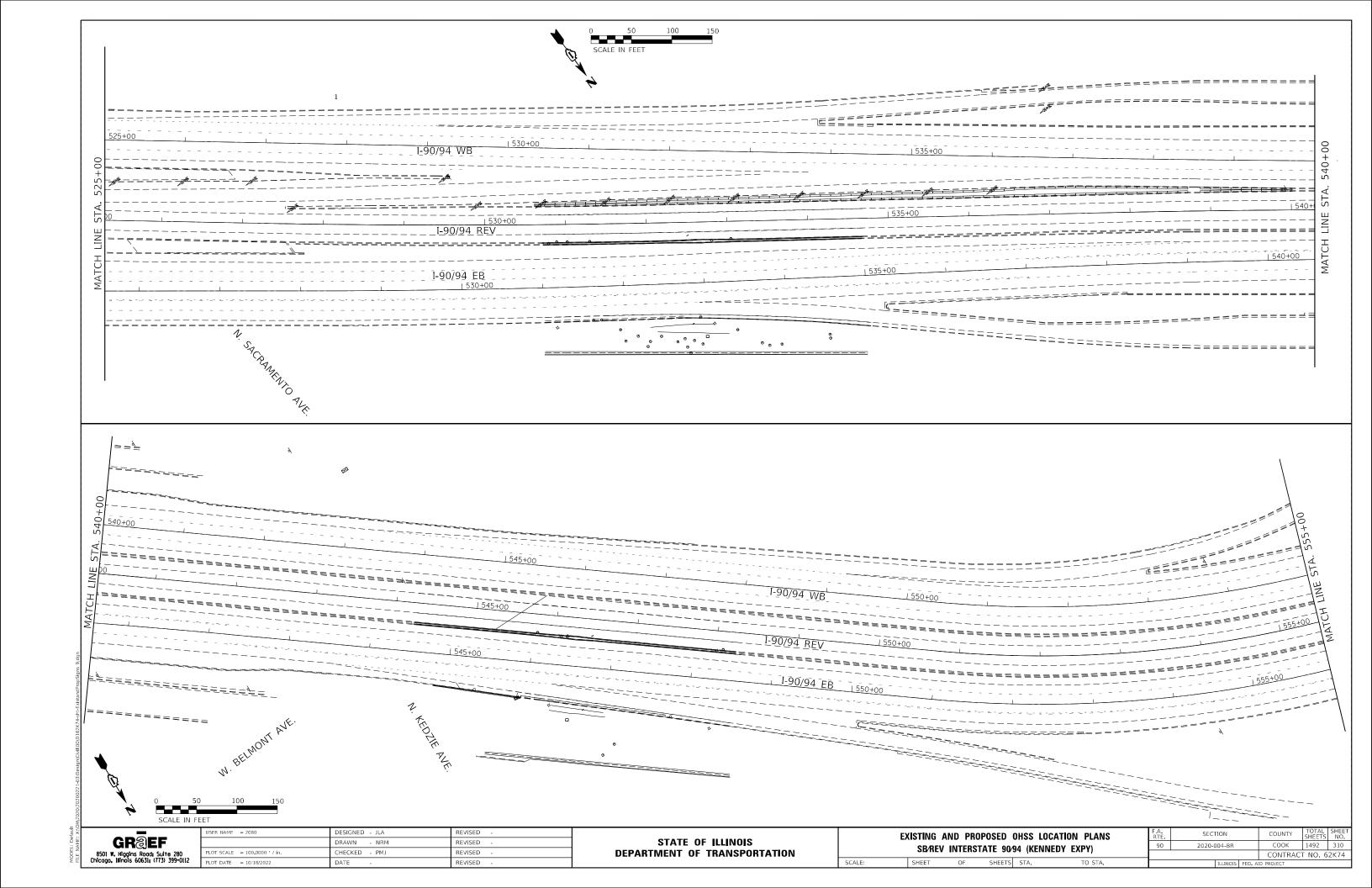


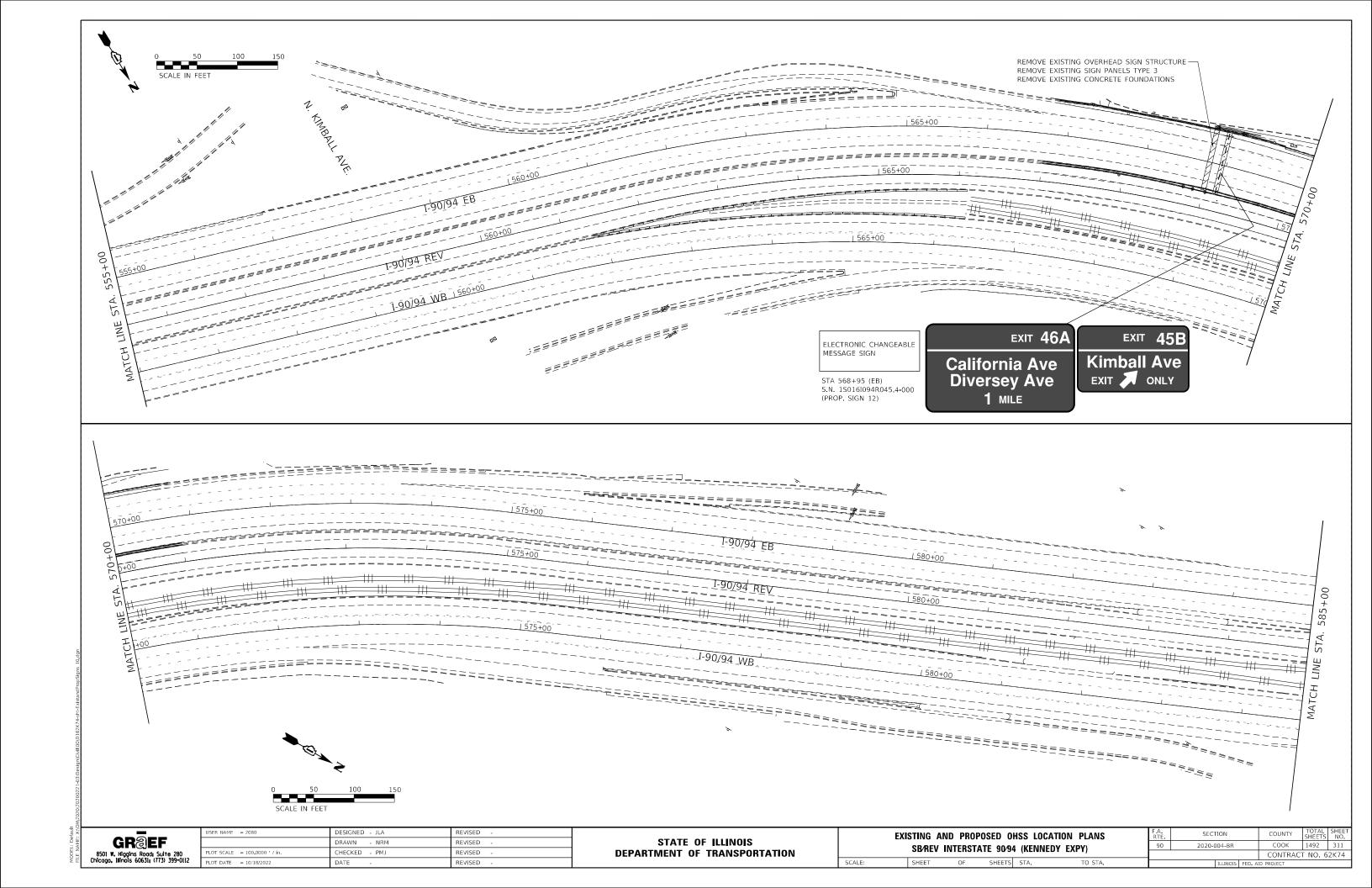


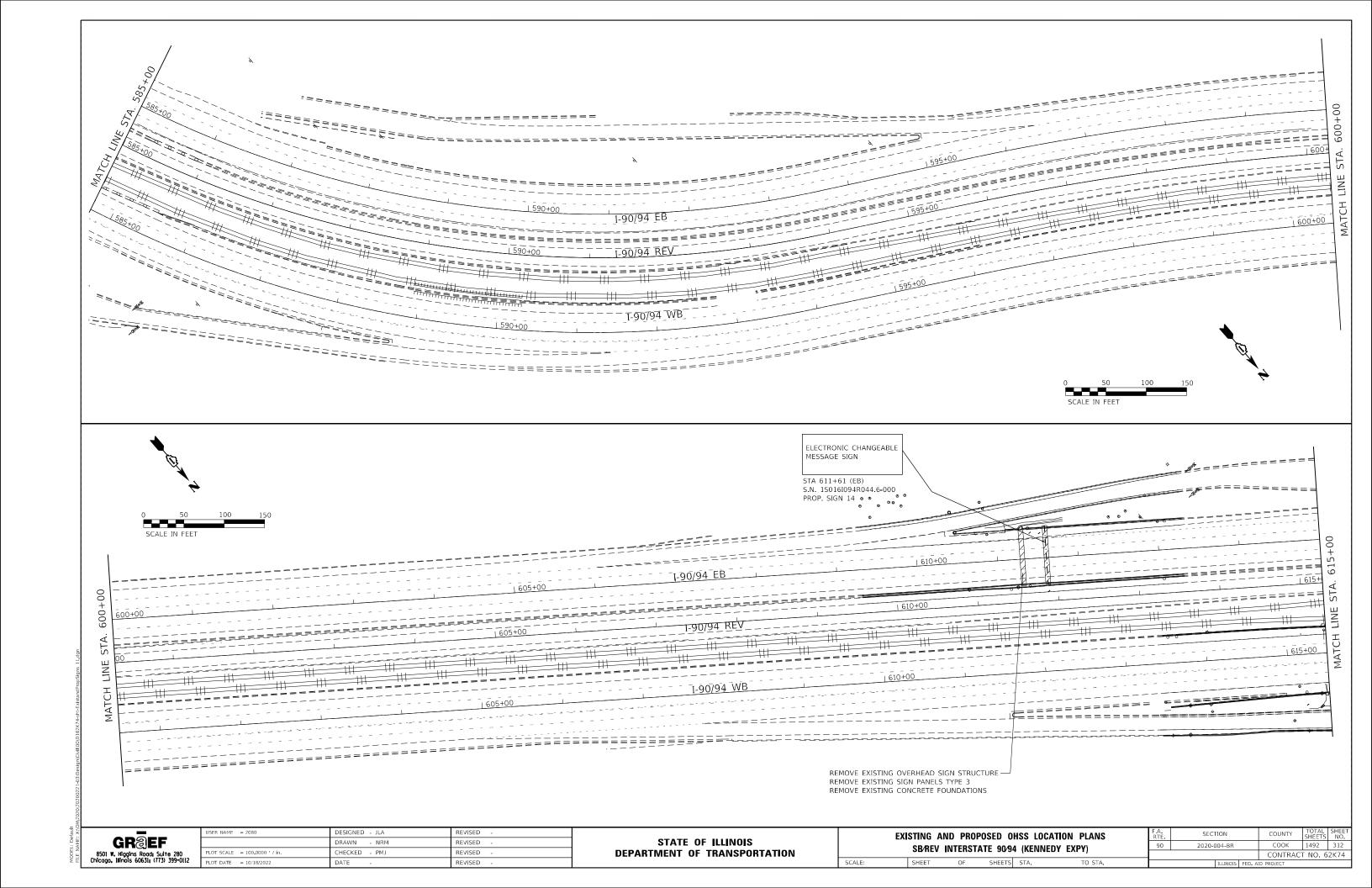


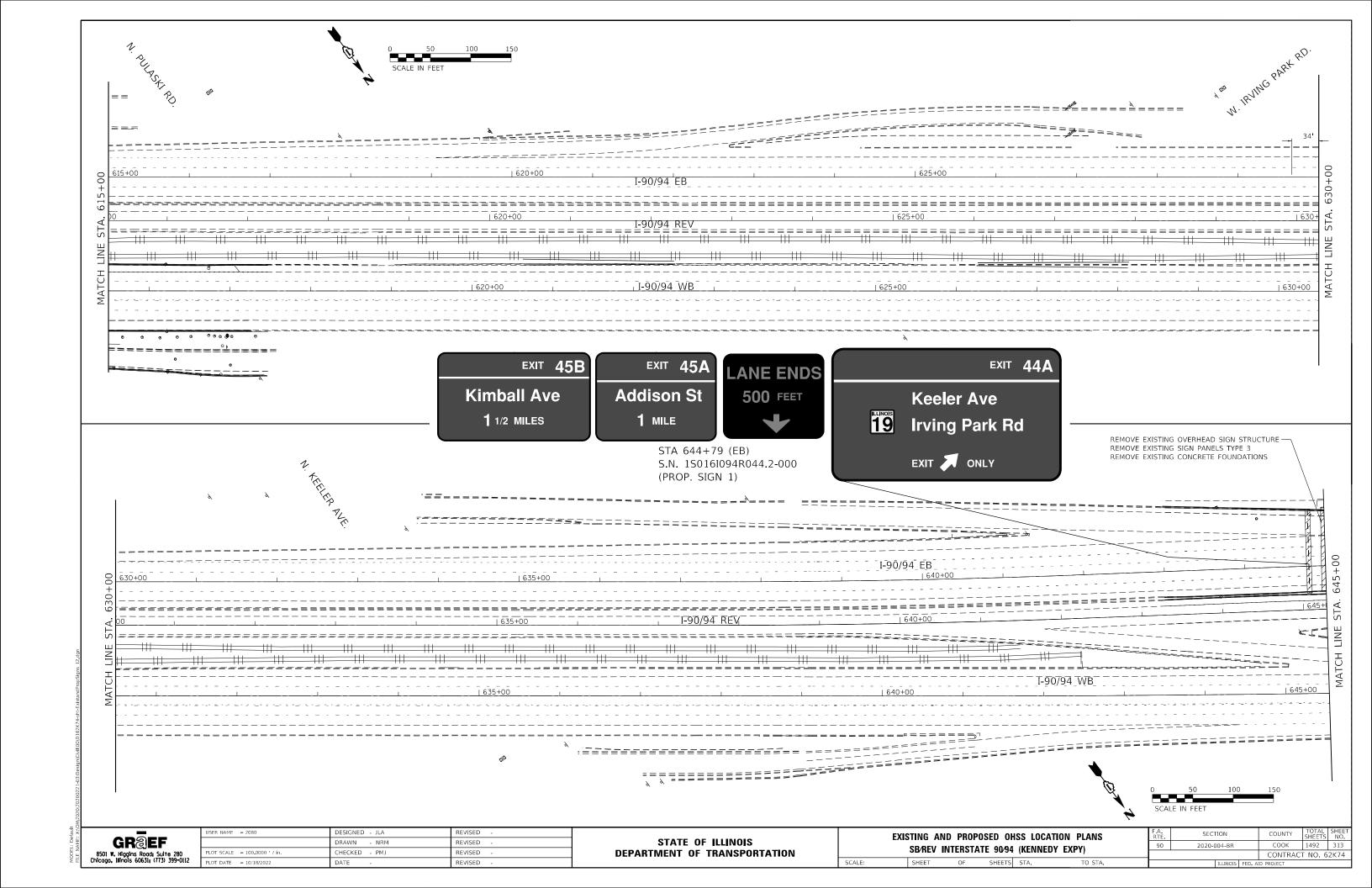


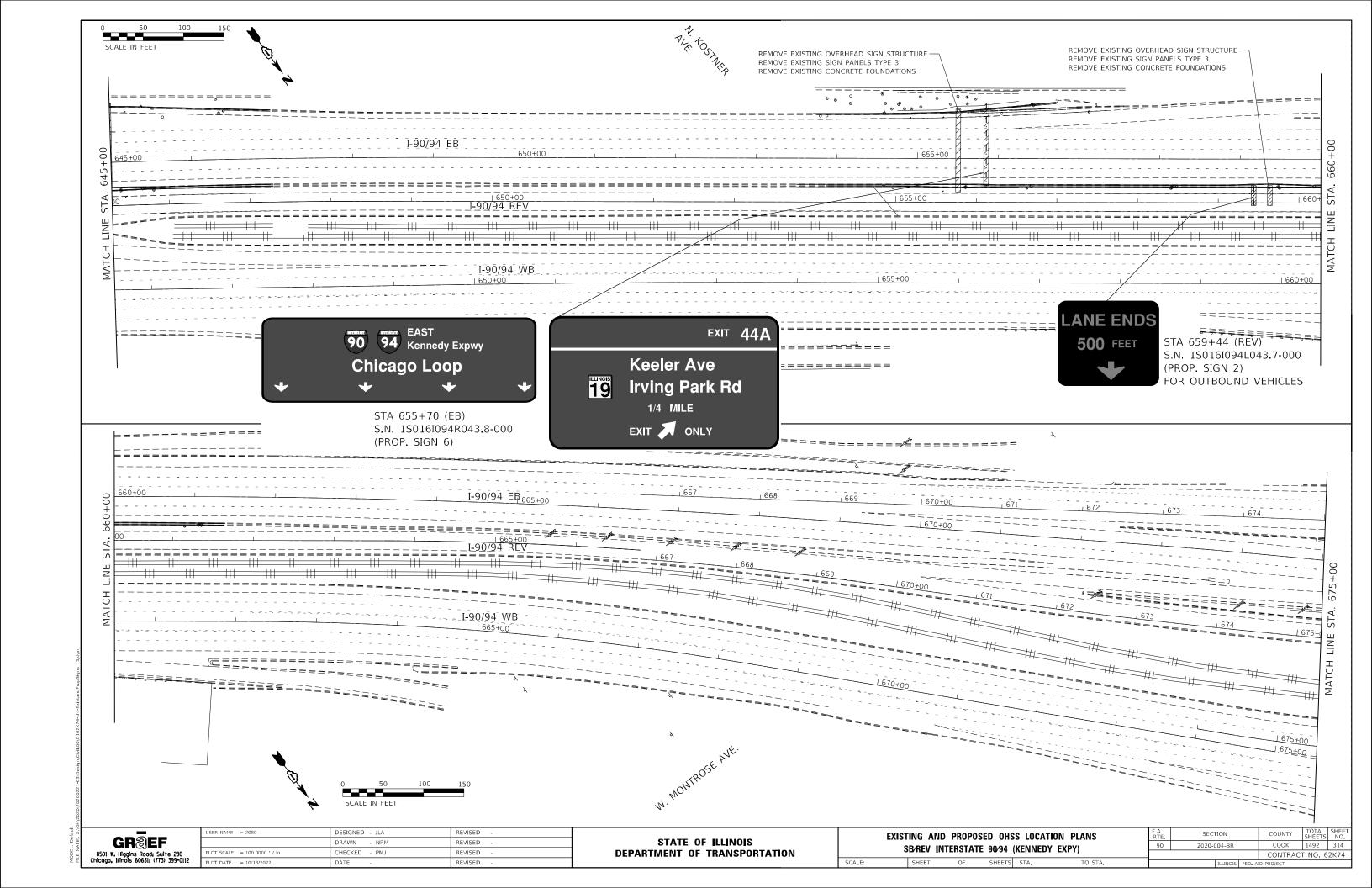


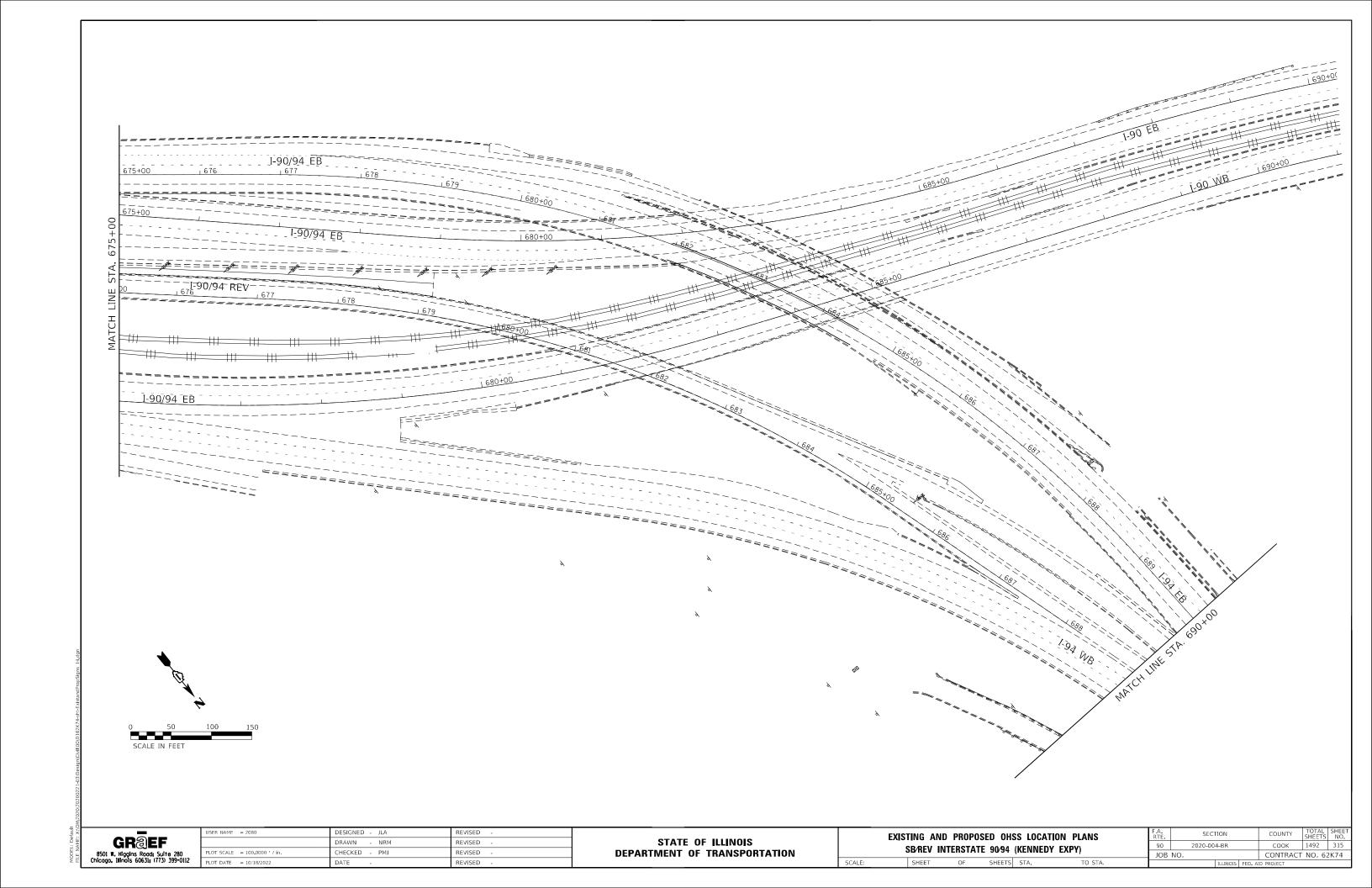


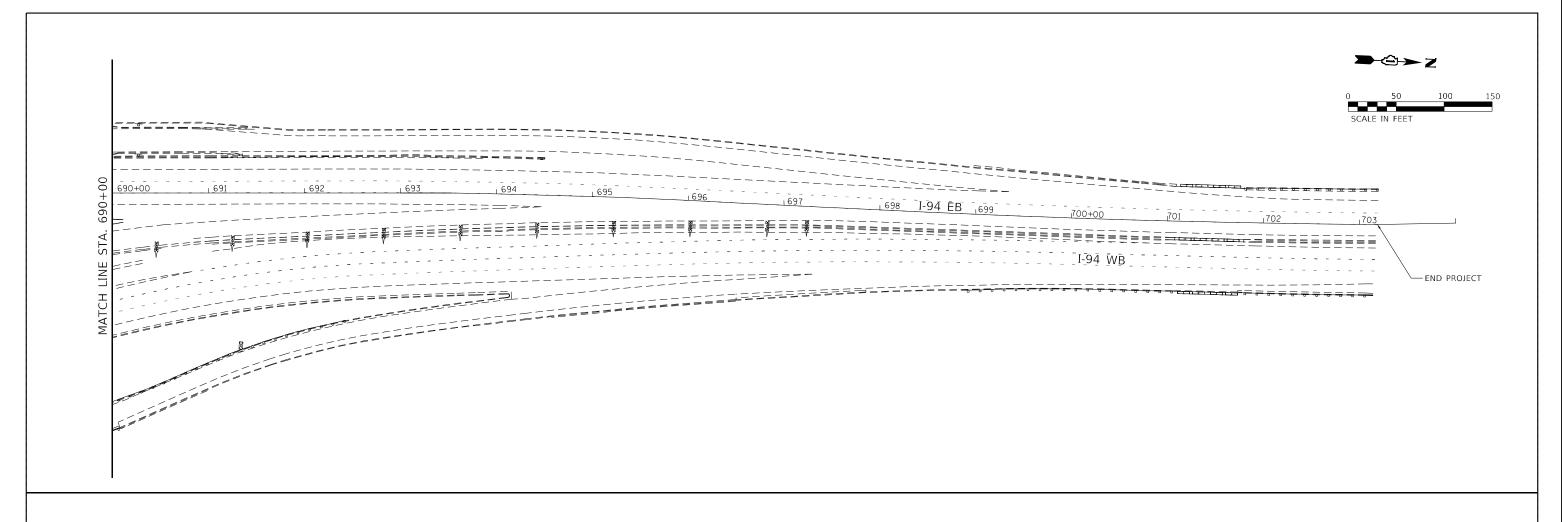












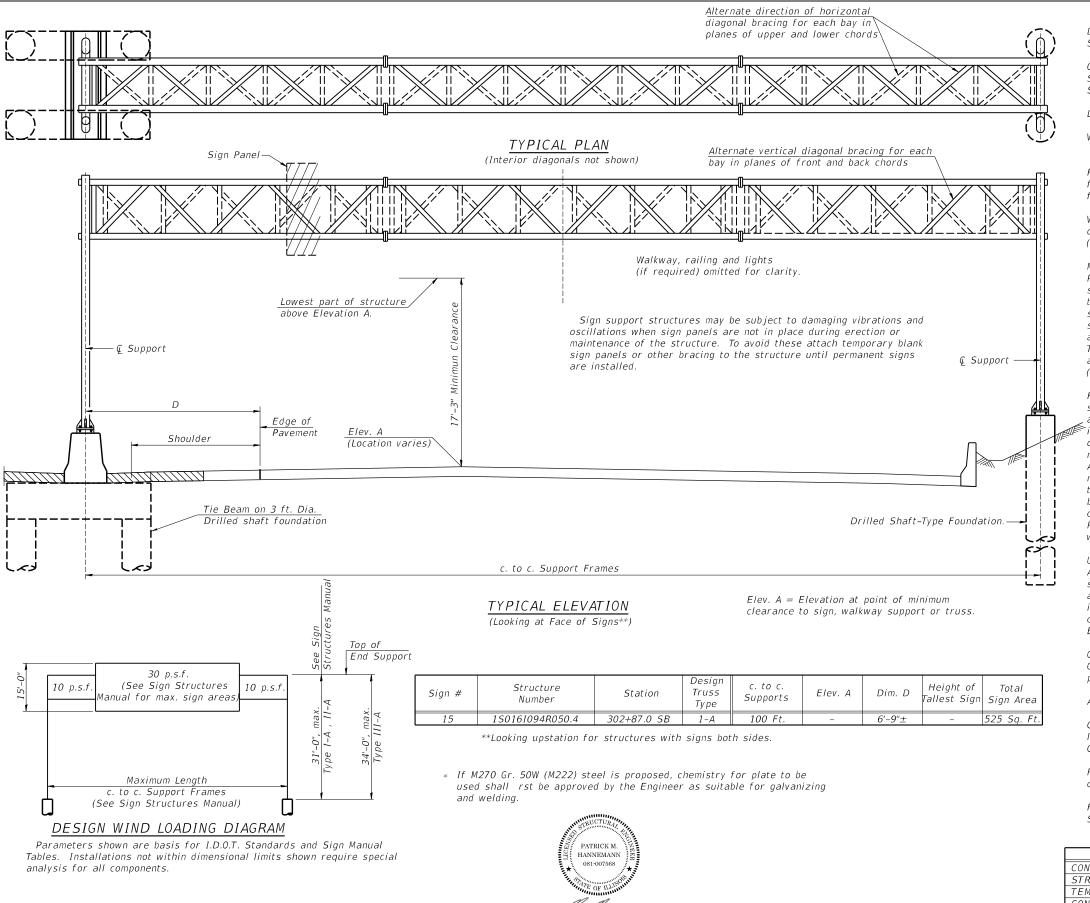
GRaEF 8501 W. Higgins Road: Suite 280 Chicago, Illinois 60631: (773) 399-0112

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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SCALE:

EXIS	EXISTING AND PROPOSED OHSS LOCATION PLANS					F.A. RTE	F.A. SECTION		COUNTY	TOTAL SHEETS		
	SB/REV INTERSTATE 90/94 (KENNEDY EXPY)						90 2020-004-BR		COOK	1492	316	
OBTIEV INTERIOTATE 3034 (REINITED) EXTT										CONTRAC	T NO. 6	2K74
	SHEET	OF	SHEETS	STA.	TO STA.			ILLINOIS	FED. A	D PROJECT		



09/09/2022 DATE LIC. EXP. DATE: 11/30/2024 GENERAL NOTES

DESIGN: AASHTO Standard Speci cations for Structural Supports for Highway Signs, Luminaires and Tra c Signals. ("AASHTO Speci cations")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Speci cations for Road and Bridge Construction, Supplemental Speci cations and Special Provisions. ("Standard Speci cations")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

DESIGN STRESSES: Field Units

f'c = 3,500 p.s.i.

fy = 60,000 p.s.i. (reinforcement)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Speci ciations.

MATERIALS: Aluminum Alloys as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer. The steel pipe and sti ening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel at washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Speci cations for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold nished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel at washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to ASTM F1554 Gr. 105.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest nal ground line at each foundation shall be cleaned and coated with Concrete Sealer in accordance with the Standard Speci cations.

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Speci cations.

FOUNDATIONS: The contract unit price for Concrete Foundations and Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
CONCRETE BARRIER REMOVAL	Foot	52
STRUCTURE EXCAVATION	Cu. Yds.	41
TEMPORARY SOIL RETENTION SYSTEM	Sq. Ft.	178
CONCRETE BARRIER TRANSITION	Foot	40
OVERHEAD SIGN STRUCTURE - SPAN, TYPE I-A (4'-0" X 4'-6")	Foot	100
DRILLED SHAFT CONCRETE FOUNDATIONS	Cu. Yds.	71.5
REMOVE OVERHEAD SIGN STRUCTURE - SPAN	Each	1

05-A-1

2-17-2017

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

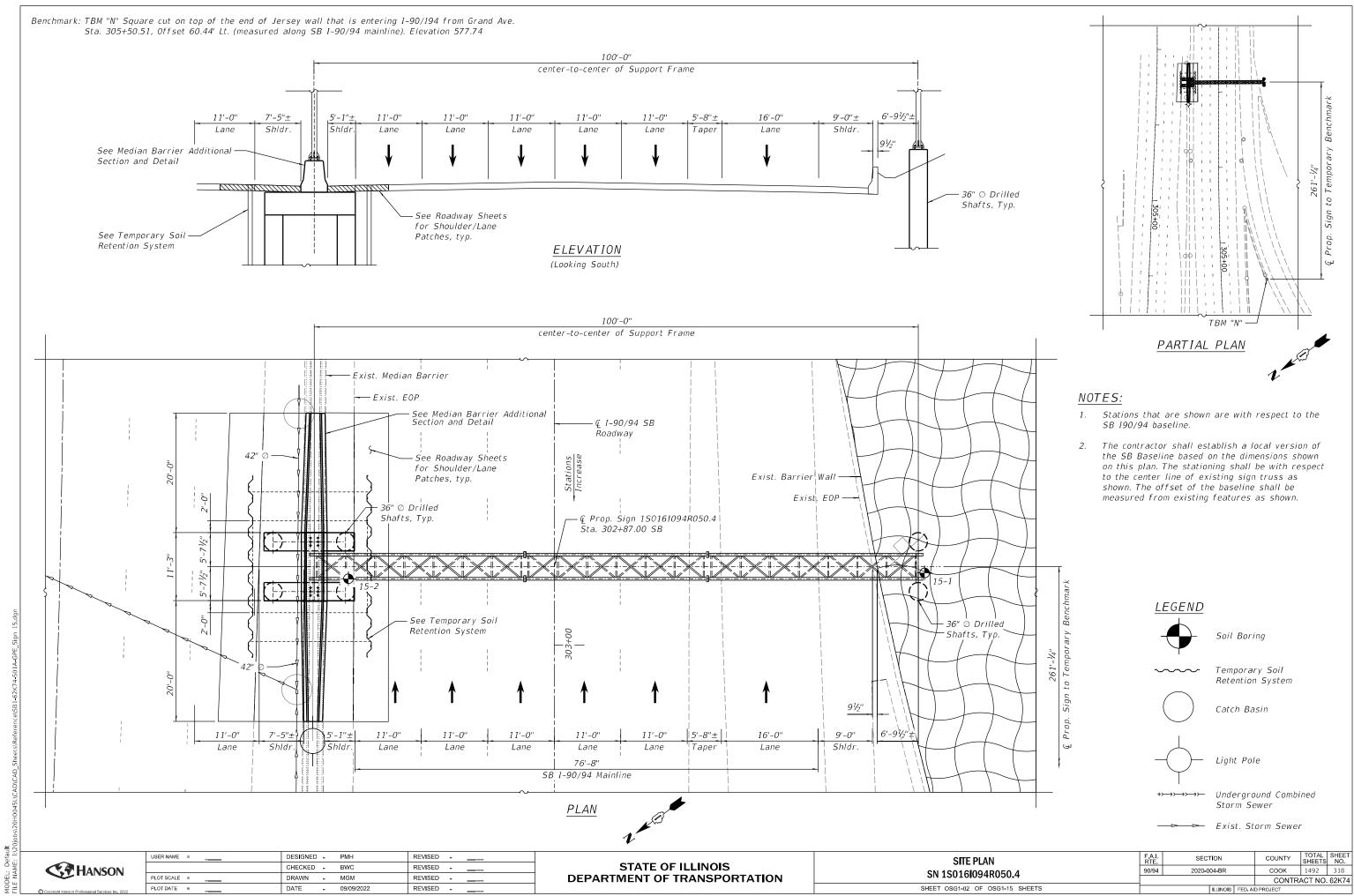
GENERAL PLAN & ELEVATION - ALUMINUM TRUSS & STEEL SUPPORT SN 1S016I094R050.4

SHEET OSG1-01 OF OSG1-15 SHEETS

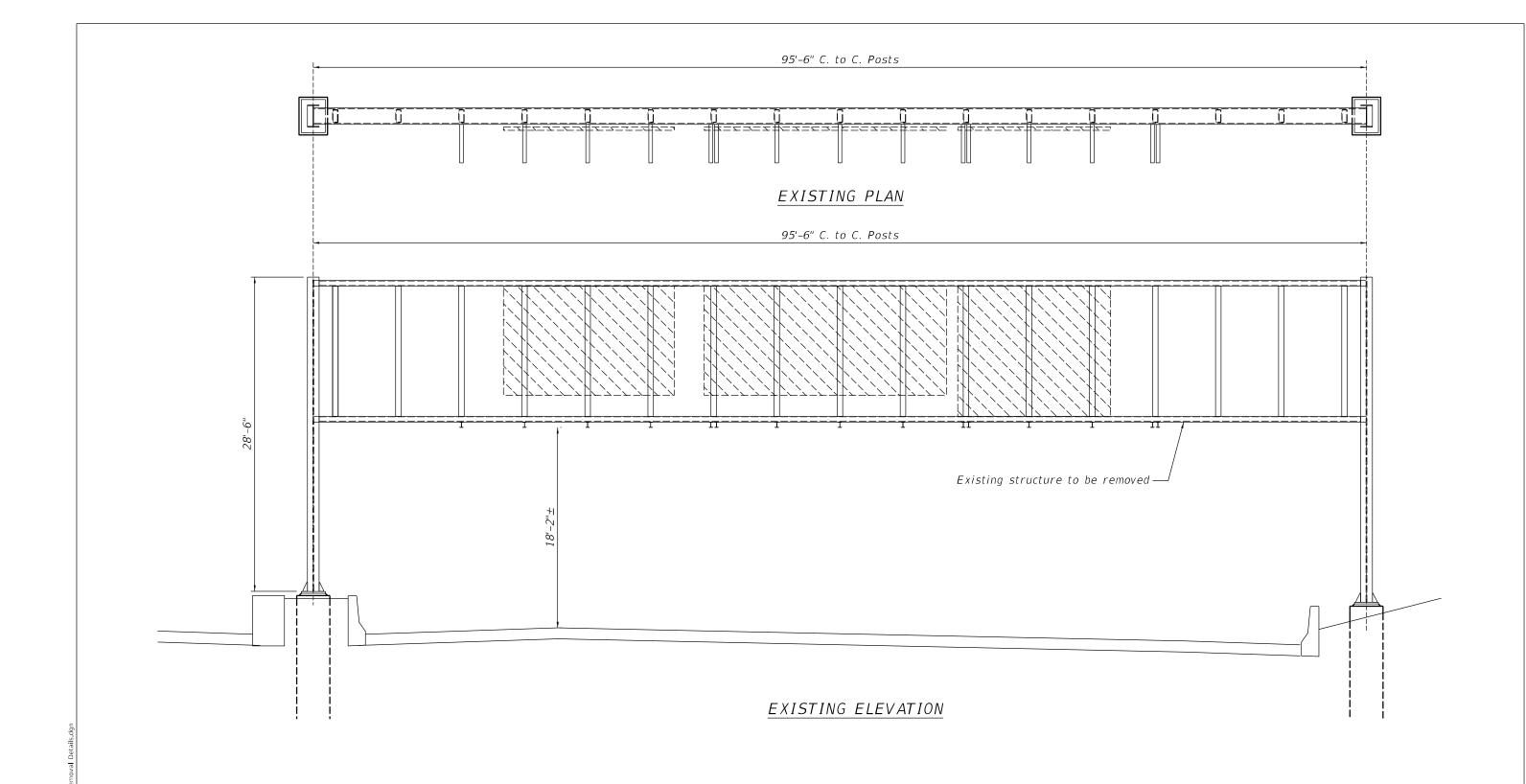
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 90/94
 2020-004-BR
 COOK
 1492
 317

 CONTRACT NO. 62K74



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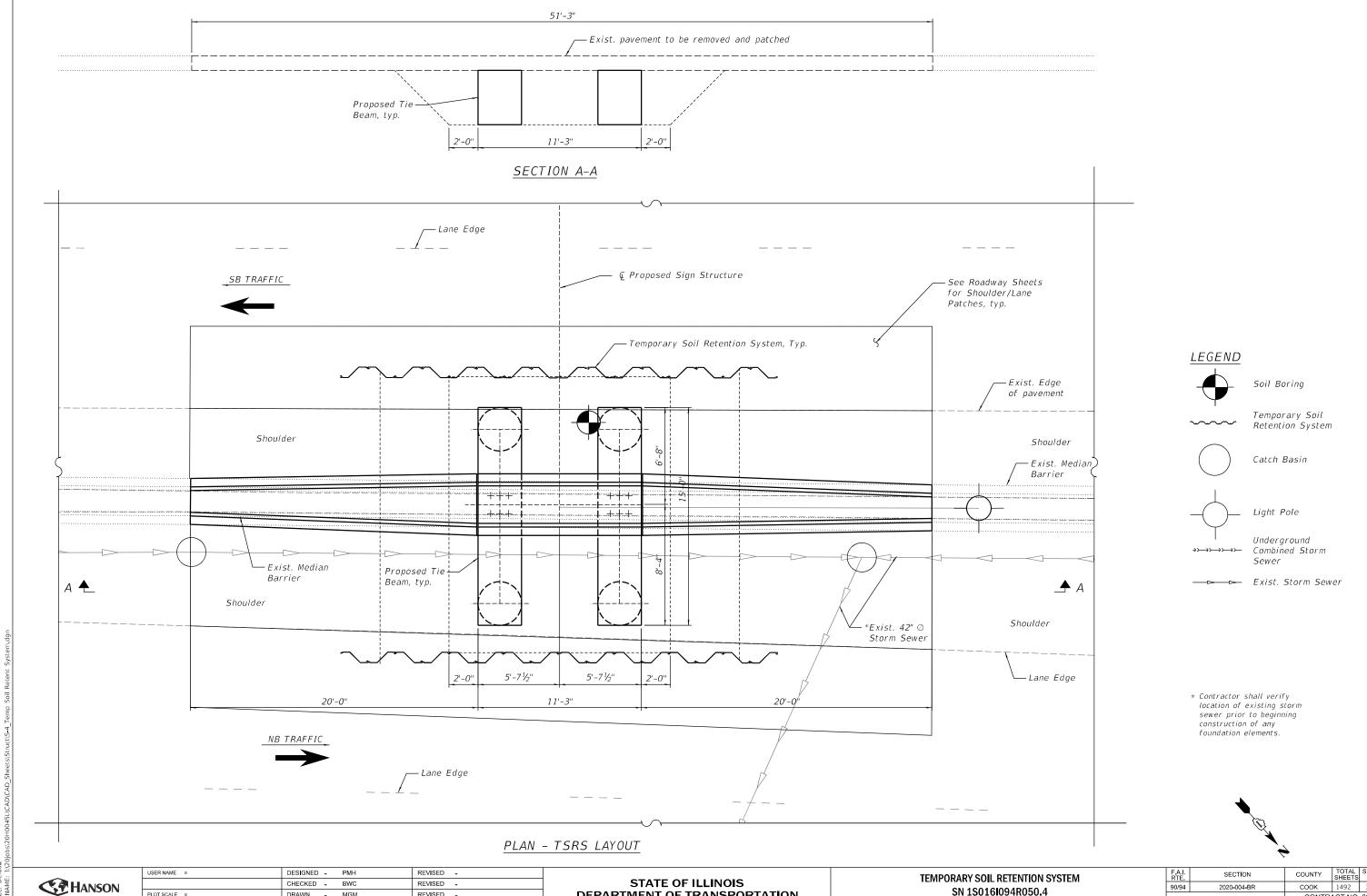


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STATE OF ILLINOIS	
DEPARTMENT OF TRANSPORTATION	

REMOVAL DETAILS								
SN 1S016I094R050.4								
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F.A.I. SECTION				COUNTY	TOTAL SHEETS	SHEE NO.
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				CONTR	ACT NO.	62K7
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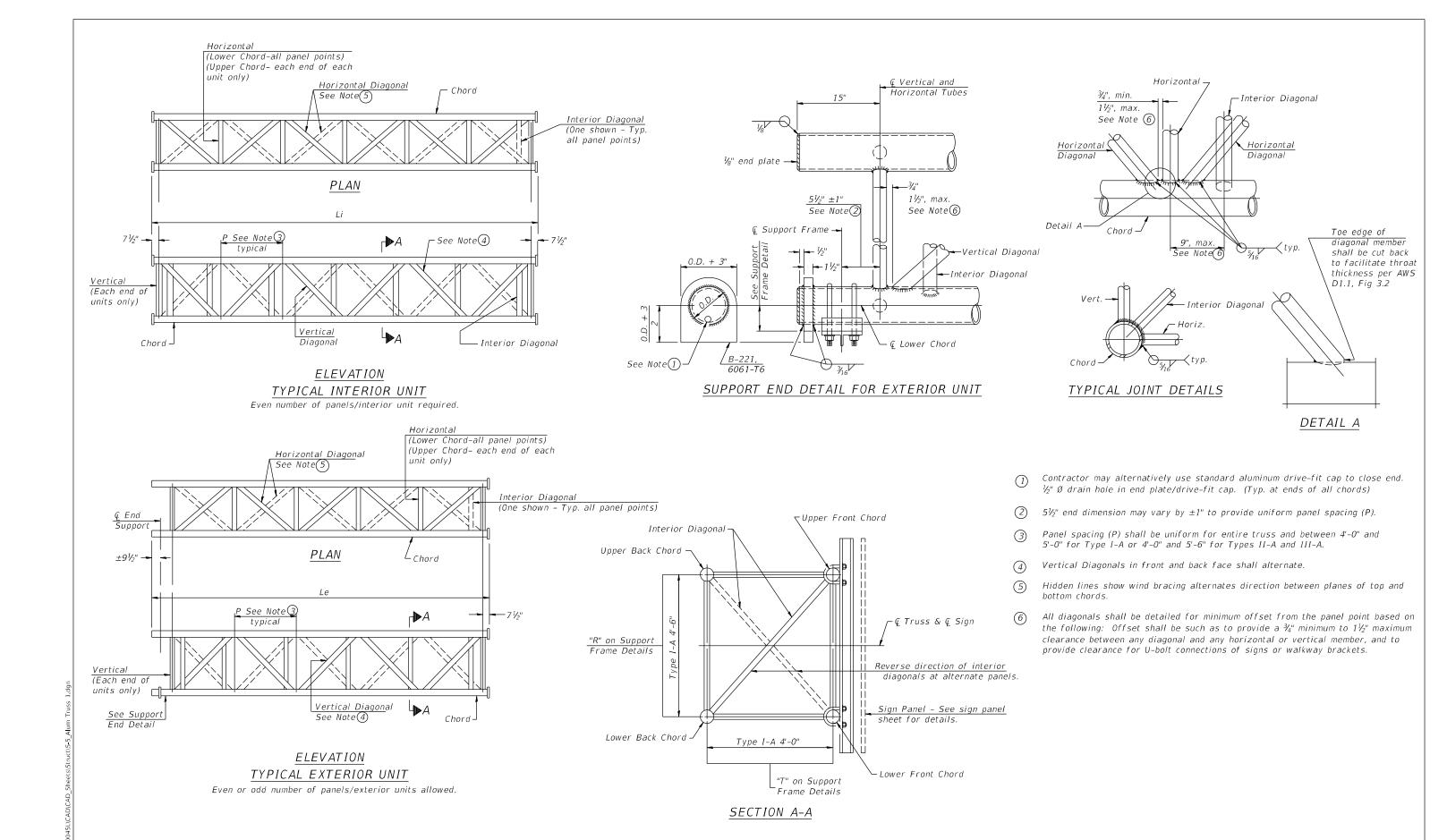
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DEPARTMENT OF TRANSPORTATION

SN 1S016I094R050.4 SHEET OSG1-04 OF OSG1-15 SHEETS COUNTY TOTAL SHEET NO.

COOK 1492 320 CONTRACT NO. 62K74



05-A-2

2-17-2017

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

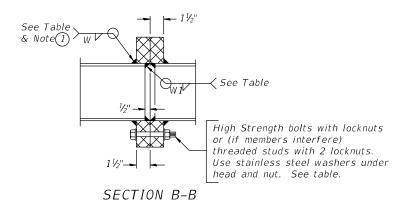
ALUMINUM TRUSS DETAILS FOR TRUSS TYPES I-A, II-A AND III-A SN 1S016I094R050.4

SHEET OSG1-05 OF OSG1-15 SHEETS

SHANSON

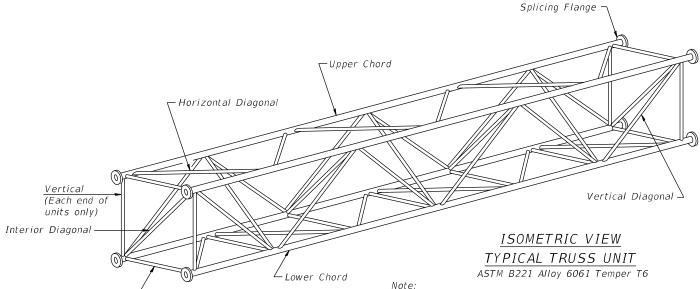
TRUSS UNIT TABLE

	Structure		Design Truss	Exte	erior Unit:	s (2)		Interio	or Unit		Upper &	⊊ Lower ord		Horizontals; Horizontal,	Camber		9	Splicing	Flang	e	
Sign #	Number	Station	Type	No. Panels per Unit	Unit Lgth.(Le)	Panel Lgth.(P)	No. Reg'd.	No. Panels per Unit	Unit Lgth.(Li)	Panel Lgth.(P)		Wall	and Interio	or Diagonals Wall	Midspan	Bolts No./Splice	Dia.	Weld W	Sizes W1	А	В
15	1S016I094R050.4	302+87.00 SB	I-A	7	35'-8 ¹ / ₂ "	4'-10"	1	6	30'-3"	4'-10"	5 ¹ / ₂ "	⁵ / ₁₆	2½"	5∕ ₁₆ "	31/4"	6	7/8"	3/8"	1/4"	91/4"	121/4"

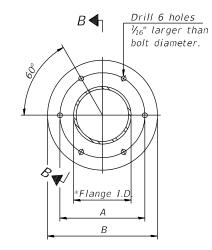


1) Splicing Flanges shall be attached to each truss unit with the truss shop assembled to camber shown. Truss units shall be in proper alignment and flange surfaces shall be shop bolted into full contact before welding. Sufficient external welds or tacks shall be made to secure flanges until remaining welds are made after disassembly. Adjacent flanges shall be "match marked" to insure

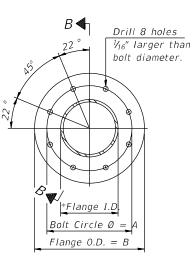
proper field assembly.



Units shall be shipped individually with adequate provision to prevent detrimental motion during transport. This may require ropes between horizontals and diagonals or energy dissipating (elastic) ties to the vehicle. The Contractor is responsible for maintaining the configuration and protection of the units.



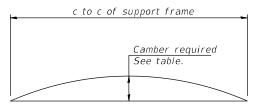
TRUSS TYPES I-A, II-A, & III-A



TRUSS TYPES II-A & III-A

SPLICING FLANGES

ASTM B221, Alloy 6061-T6 or ASTM B209, Alloy 6061-T651 *To fit O.D. of Chord with maximum gap of $\frac{1}{16}$ ".



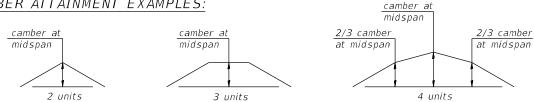
(Upper Chord - each end of each unit only)

(Lower Chord - all panel points)

CAMBER DIAGRAM

Camber curve shown is theoretical. Actual camber attained by slope changes at splices between units.

CAMBER ATTAINMENT EXAMPLES:



Camber shown is for fabrication only, measured with truss fully supported. (No-load condition)

054-A-2

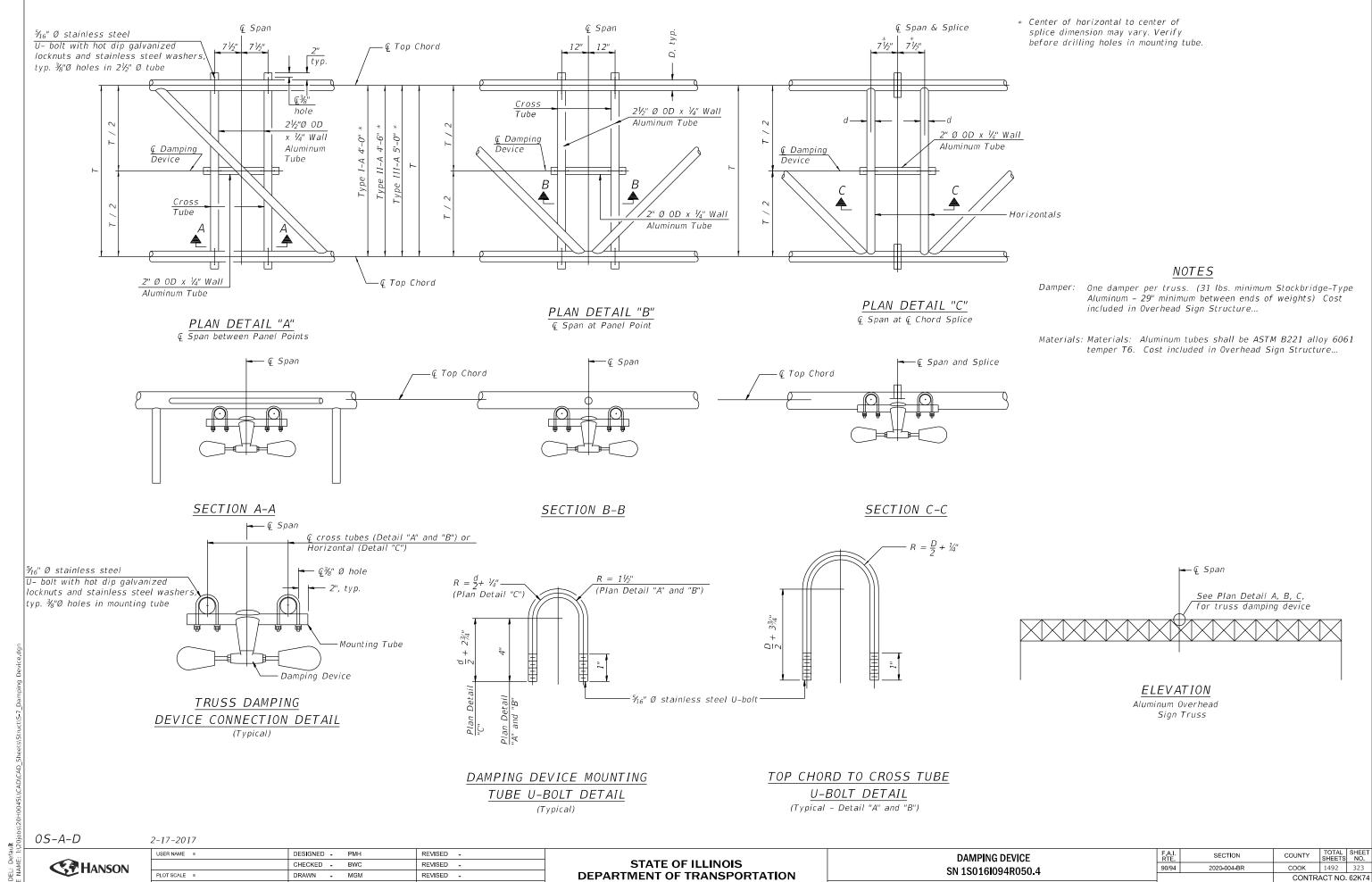
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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** ALUMINUM TRUSS DETAILS FOR TRUSS TYPES I-A, II-A AND III-A SN 1S016I094R050.4 SHEET OSG1-06 OF OSG1-15 SHEETS

TOTAL SHEET NO. SECTION COUNTY 2020-004-BR COOK 1492 322 90/94 CONTRACT NO. 62K74



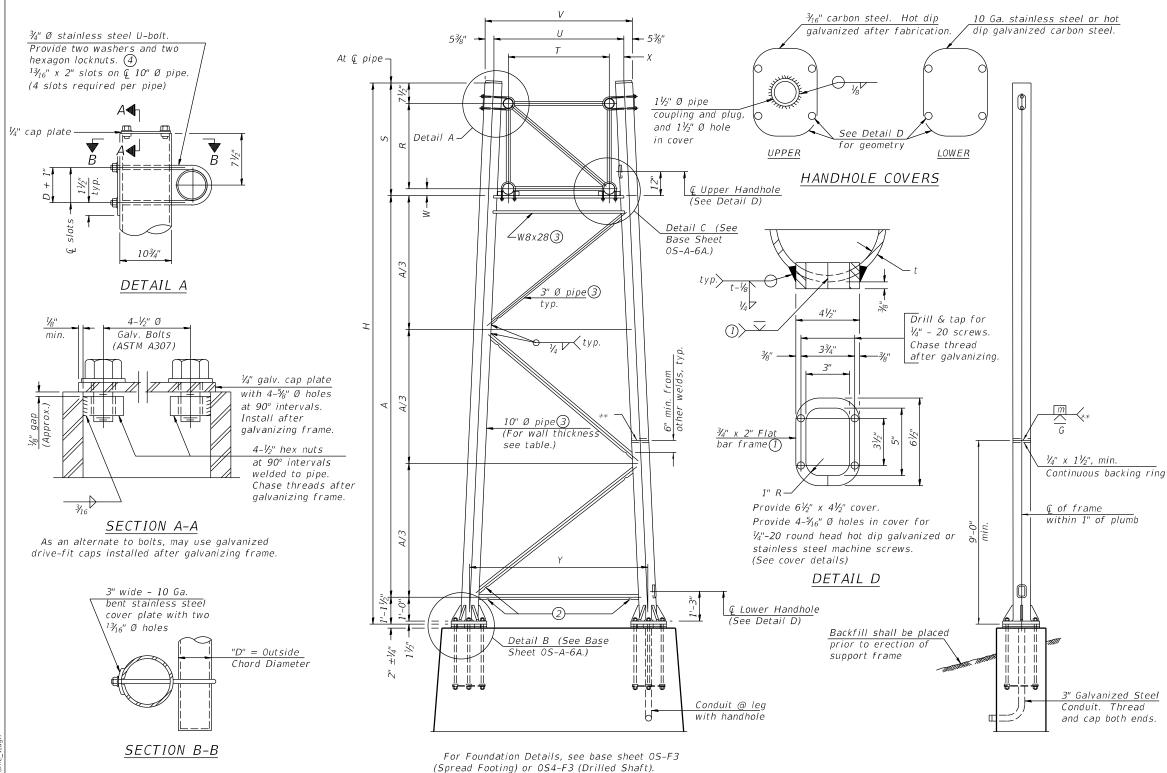
SHEET OSG1-07 OF OSG1-15 SHEETS

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DATE - 09/09/2022

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SIDE ELEVATION

Support Design Loads: See Base Sheet OS-A-1 for design and loading criteria.

Load combinations checked include deadload plus:

a) 100% wind normal to sign, 20% parallel to sign

b) 60% wind normal to sign, 30% parallel to sign

- In lieu of fabricated handhole frame as shown, may cut from 2" plate (rolling direction vertical). All cut faces to be ground to ANSI Roughness of 500μ in or less.
- Galvanizing vent holes of adequate size shall be provided on underside at each end of bracing pipes. Alternately, holes may be provided in wall of pipe column. All vent holes shall be drilled and de-burred, typ.
- (3) Steel pipe, plate, carbon steel handhole covers and rolled sections shall be hot dip galvanized after fabrication. Painting is not permitted. See Base Sheet OS-A-1.
- (4) See General Notes for fasteners.
- Dimensions shown are based on selection criteria in the Sign Structures Manual. Nonstandard applications must have dimensions verifed or amended as appropriate.
- 6 "H" based on 15'-0" or actual sign height, whichever is

END ELEVATION

Truss		Dimensions									
Туре	R	S	T	U	V	W	Х	Y			
I-A	4'-6"	5'-5½"	4'-0"	5'-6"	6'-4¾"	4"	9"	8'-3"			

10" Ø PIPE TRUSS SUPPORT FRAME

** One butt welded joint is allowed only on one post per support frame. If used, weld procedure must be preapproved by Engineer and joint shall receive 100% RT or UT (tension criteria) at Contractor's expense.

	Sign #	Structure	Station	Sup	port	Truss	Pipe Wall	Н	4	
	Jigii #	Number	Station	Left	Right	Type	Thickness	6	A	
ĺ	15	1S016I094R050.4	302+87.00 SB	Χ		I-A	0.279	25'-1 ¹ / ₄ "	18'-6 ¹ / ₄ "	
	15	1S016I094R050.4	302+87.00 SB		Χ	I-A	0.279	20'-5 ³ /8"	13'-10 ³ / ₈ "	

05-A-6

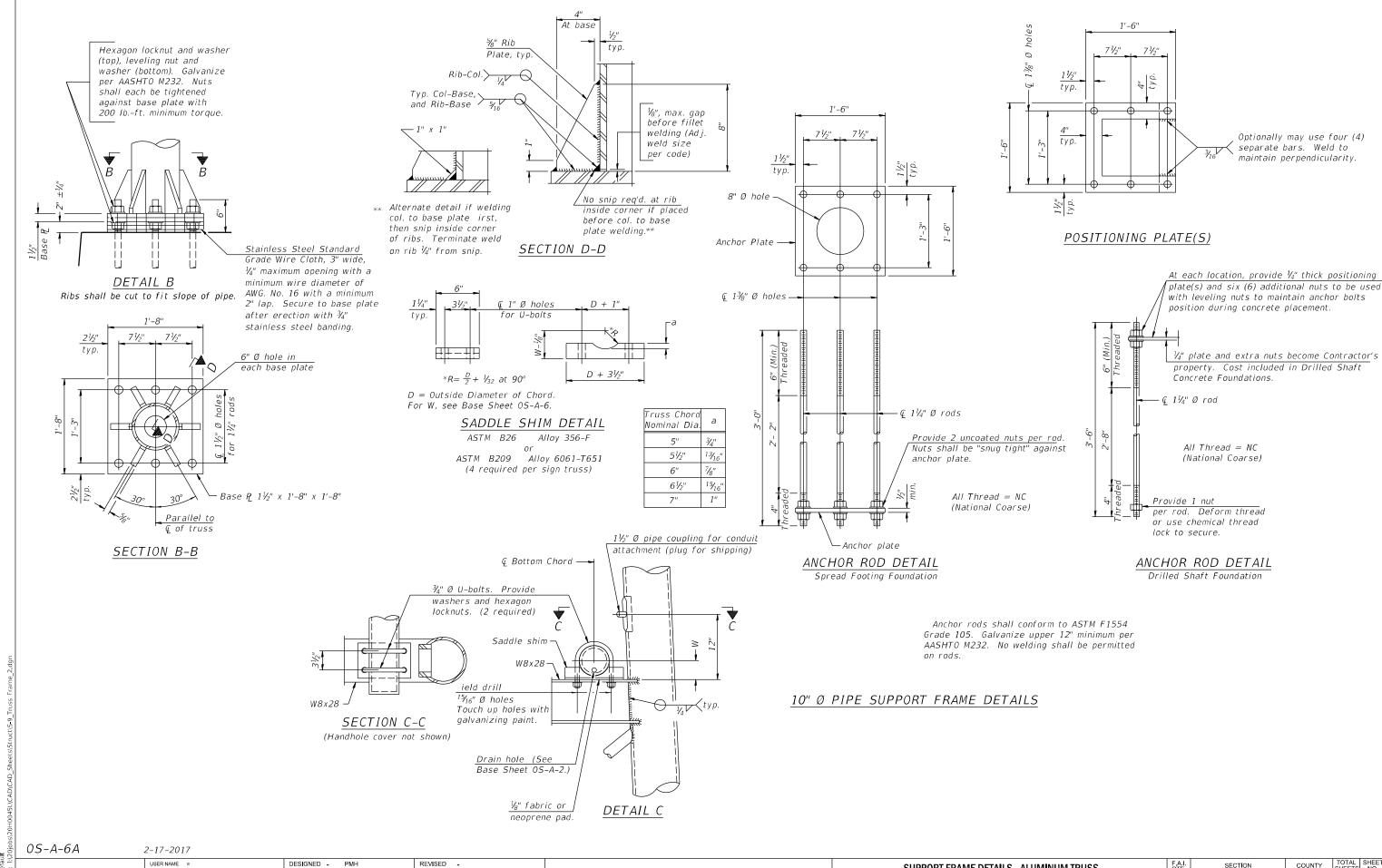
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PLOT SCALE =	DRAWN	-	MGM	REVISED -
PLOT DATE =	DATE	-	09/09/2022	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

SUPPORT FRAME FOR ALUMINUM TR SN 1S0161094R050.4	USS
SHEET OSG1-08 OF OSG1-15 SHEETS	

TOTAL SHEET SHEETS NO. SECTION COUNTY 2020-004-BR COOK 1492 324 90/94 CONTRACT NO. 62K74



MODEL: Default

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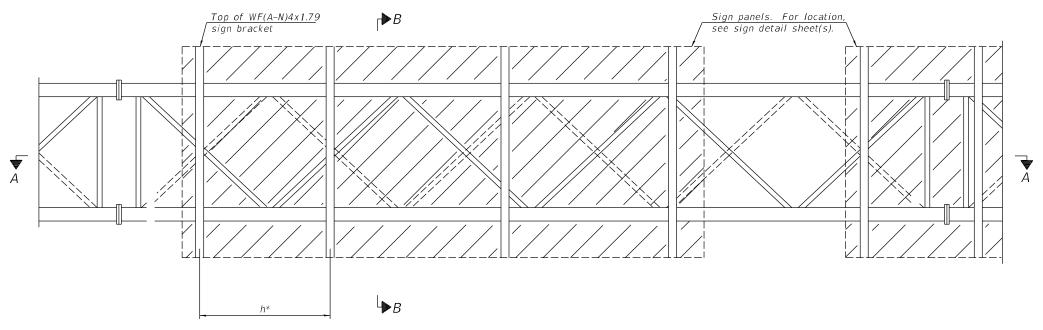
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 DATE
 09/09/2022
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STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

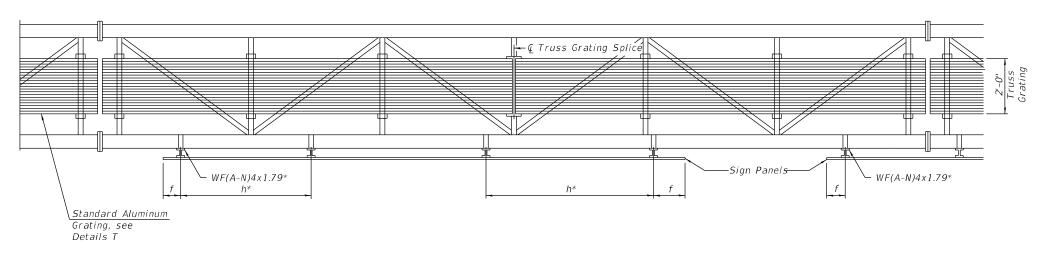
 SUPPORT FRAME DETAILS - ALUMINUM TRUSS
 F.A.I. RTE.
 SECTION
 COUNTY SHEETS
 TOTAL SHEETS NO.

 SN 1S016I094R050.4
 90/94
 2020-004-BR
 COOK
 1492
 325

 SHEET OSG1-09 OF OSG1-15 SHEETS



TYPICAL FRONT ELEVATION



BRACKET TABLE

ASTM B	1-T6						
Sign \	Number						
Greater Than	Less Than or Equal To	Brackets Required					
	8'-0"	2					
8'-0"	14'-0"	3					
14'-0"	20'-0"	4					
20'-0"	26'-0"	5					
26'-0"	32'-0"	6					

SECTION A-A

Place all sign brackets as close to panel points as practical.

* Space sign brackets WF(A-N)4x1.79 for efficiency and within limits shown:

f=12" maximum, 4" minimum (End of sign to $\mathcal Q$ of nearest bracket) h=6'-0" maximum ($\mathcal Q$ to $\mathcal Q$ sign support brackets, WF(A-N)4x1.7

Notes:

For Detail T and Section B-B, see Base Sheet OS-A-10-NW.
Truss grating to facilitate inspection shall run full length
(center to center of support frames) 12"± on overhead trusses.
Cost of truss grating is included in "Overhead Sign Structure".

Truss Grating width dimensions are nominal and may vary 16"±".

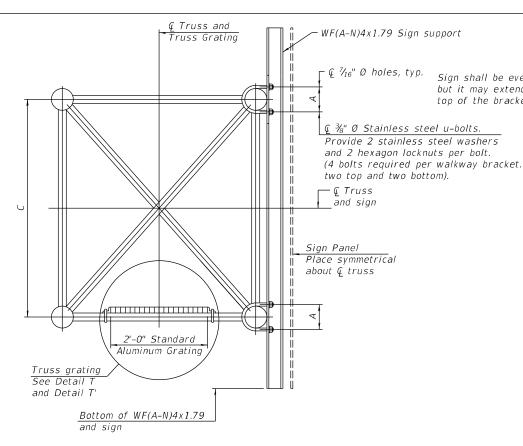
Truss Grating width dimensions are nominal and may vary $\frac{1}{2}$ " \pm based on available standard widths.

05-A-9-NW

4-1-2020



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PLOT DATE =	DATE	-	09/09/2022	REVISED	-



SECTION B-B

SPECIFICATIONS FOR STANDARD ALUMINUM GRATING

Main Bearing Bars shall be $\sqrt[3]{_6}$ " x $1\sqrt[4]{_2}$ " on $1\sqrt[3]{_6}$ " centers and conform to ASTM B221 Alloy 6061-T6.

Cross bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on 4" centers and conform to ASTM B221 Alloy 6063-T5 or 6061-T6.

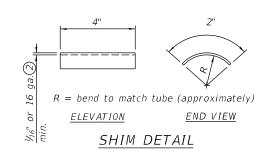
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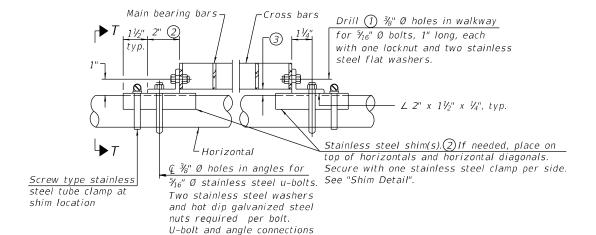
Aluminum Grating with modified "t" sections for main bearing bars shall meet the following requirements:

Main bars shall conform to ASTM B221 Alloy 6061–T6 and have a minimum section modulus equal to 0.0705 in.³ per bar, a depth of $1\frac{1}{2}$ ", spaced on $1\frac{3}{16}$ " centers.

Cross bars shall conform to ASTM B221 Alloy 6063-T5 or T-42 and spaced on $4^{\prime\prime}$ centers.

Sign #	Structure Number	Station	А	С
15	15016I094R050.4	302+87.00 SB	6"	4'-6"



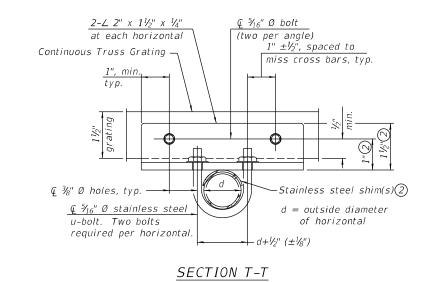


DETAIL T

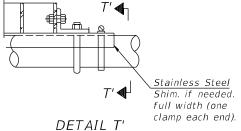
required at horizontals only.

Sign shall be even with the top of the bracket, but it may extend no more than 6" above the top of the bracket for field adjustments.

(Continuous Truss grating)

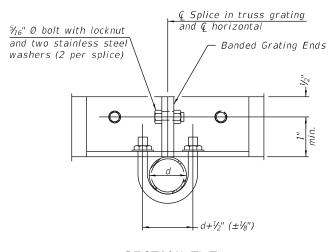


- Drilling holes in grating may be done in shop or field, based on Contractor's preference and subject to accurate alignment.
- 2 Stainless steel shims shall be placed as shown in Detail T if needed to compensate for alignment variations between horizontal and diagonal pipes beyond adjustment provided by angles. Thicker shims may be used subject to shims performing properly.
- 3 Tube to grating gap may vary from 0 to ½", max. to align walkway, allow for camber, etc.



(Truss grating splice)

Details not shown same as Detail T. Alternate materials may be used subject to the Engineer's review and approval.



SECTION T'-T'

0S-A-10-NW

4-1-2020



USER NAME =	DESIGNED	-	PMH	REVISED -
	CHECKED	-	BWC	REVISED -
PLOT SCALE =	DRAWN	-	MGM	REVISED -
PLOT DATE =	DATE	-	09/09/2022	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

ALUMINUM WALKWAY DETAILS SN 1S016I094R050.4 SHEET OSG1-11 OF OSG1-15 SHEETS
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 90/94
 2020-004-BR
 COOK
 1492
 327

 CONTRACT NO. 62K74

BAR LIST - EACH FOUNDATION

L	Bar	Number	Size	Length	Shape
v	/4(E)	24	#9	F less 5"	
7	#4 ba	ar spiral	(E) - see	Side Eleva	tion

NOTE.

3" Ø Galvanized Steel

Conduit. Thread

and cap both ends.

The foundation dimensions shown are based on the presence of mostly cohesive soils with an average Unconfined Compressive Strength (Qu) of at least 1.25 tsf, which must be determined by previous soil investigations at the jobsite. When other conditions are indicated, the boring data will be included in the plans and the foundation dimensions shown will be the result of site specific designs.

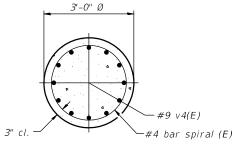
If the conditions encountered are different than those indicated, the Contractor shall notify the Engineer to determine if the foundation dimensions need to be modified. If dimensions "B" or "F" are revised by more than 12" by the Contractor, "as-built" plans shall be prepared and submitted to the District Bureau of Operations for future reference.

No sonotubes or decomposable forms shall be used below the lower conduit entrance. Permanent metal forms or other shielding may not be left in place below that elevation without the Engineer's written permission.

Concrete shall be placed monolithically, without construction joints.

Backfill shall be placed per Article 502 of Standard Specification and prior to erection of support column.

A normal surface finish followed by a Concrete Sealer application will be required on concrete surfaces above the lowest elevation 6" below finished ground line. Cost included in Drilled Shaft Concrete Foundation.



#9 v4(E #4 bar sp

DETAILS FOR 10" Ø SUPPORT FRAME TYPE I-A or II-A TRUSS

Sign # Structure Number Station Elevation A B F Elevation A B Top Bottom A B		Concrete
	F	(Cu. Yds.)
15	37'-6"	19.7

0S4-F3

2-17-2017

71/2"-

3'-0" Ø



spiral (E) at 6"

12-#9 v4(E) bars-

3 hoops minimum top and bottom

USER NAME =	DESIGNED	-	PMH	REVISED	-
	CHECKED	-	BWC	REVISED	-
PLOT SCALE =	DRAWN	-	MGM	REVISED	-
PLOT DATE =	DATE	-	09/09/2022	REVISED	-

8'-3" G to G

Approved clamps for grounding*

> #6 copper wire or cable

> > 3'-0" Ø

7½"--

¾" Ø x 10'-0" copper weld ground rod driven into ground 9'-0". Cost of rod, cable, conduit, caps and clamps

shall be included in Drilled Shaft Concrete Foundations.

11'-3"

8'-3"

PLAN

SIDE ELEVATION

Elevation (Top)

6" -

END VIEW

For anchor rod size and placement, see Support Frame Detail Sheet.

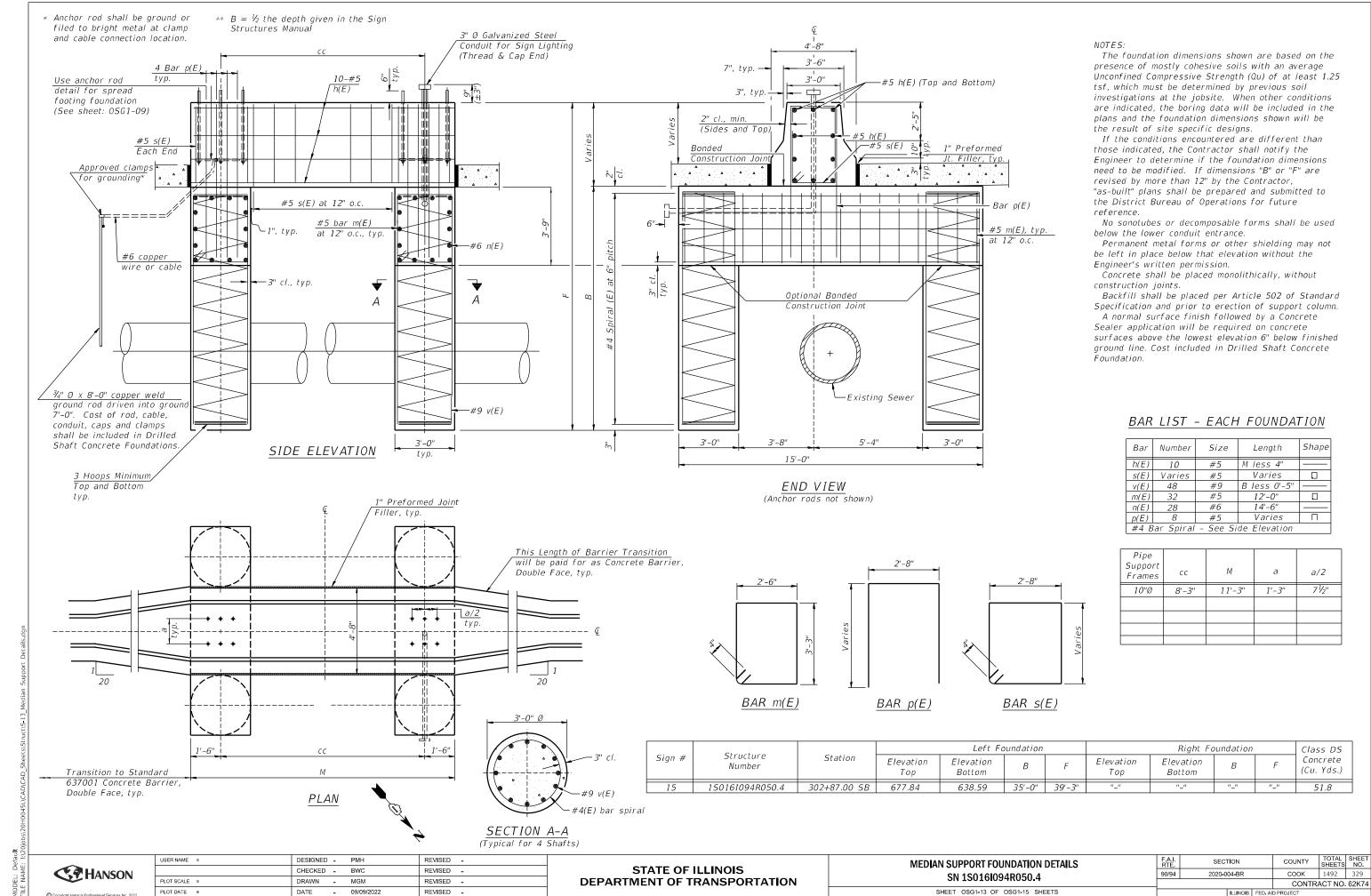
* Anchor rod shall be ground or filed to bright metal at clamp and cable connection location.

Elevation

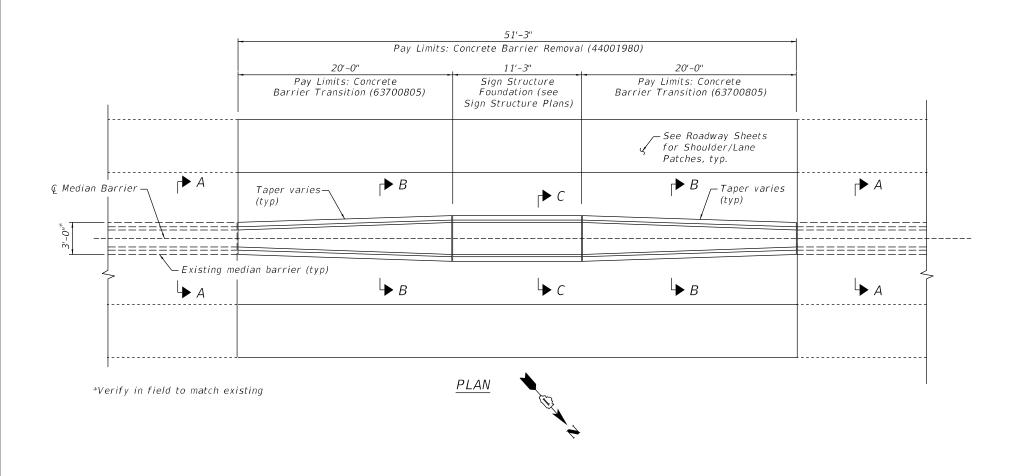
F.A.I. RTE	SEC.	TION	COUNTY	TOTAL SHEETS	SHEE NO.	
90/94	2020-0	04-BR	соок	1492	328	
			CONTRA	ACT NO.	62K7	
ILLINOIS FED. AI				D PROJECT		

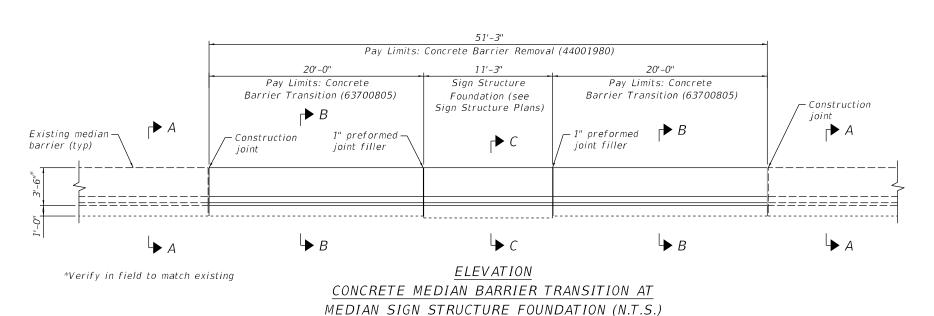
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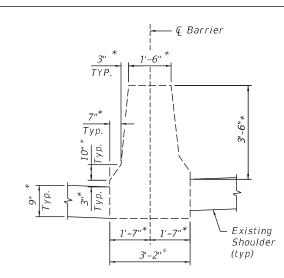
Sheets\Struct\S-12_Drilled Shaft Det-1.dgn



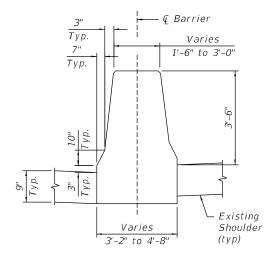
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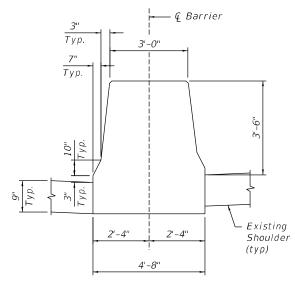




*Verify in eld to match existing SECTION A-A



SECTION B-B



SECTION C-C



USER NAME =	DESIGNED	-	PMH	REVISED -
	CHECKED	-	BWC	REVISED -
PLOT SCALE =	DRAWN	-	MGM	REVISED -
PLOT DATE =	DATE	-	09/09/2022	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

CONCRETE MEDIAN BARRIER TRANSITION DETAILS
SN 1S016I094R050.4

SHEET OSG1-14 OF OSG1-15 SHEETS



GSI Job No. 19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date 10/26/21

PTB 185-012, WO #32 PROJECT I-90 & I-94 Tollway LOCATION COUNTY DRILLING METHOD Hollow Stem Auger CME Automatic D B U M Surface Water Elev. CLIENT **HBM** E L C O P O S I CO POSI T W S H S Qu T BORING NO. OSB-29 1903749 H S Qu T Dry ft 1169777 Easting Dry ft **Upon Completion** Ground Surface Elev. 573.2 Hrs. (ft) (/6") (tsf) (%) (pcf) (ft) (/6") (tsf) (%) (pcf) CRUSHED STONE-medium dense (Fill) CLAY-gray-soft to medium stiff (continued) 2 0.60 23 4 B CLAY-gray-soft to medium stiff CLAY LOAM-gray-stiff 1 0.50 25 5 2 B 3 1.00 16 3 B 2 0.50 25 1.20 18 1 B 5 B 1 0.50 25 4 1.30 19 -10 ² B 6 B End Of Boring @ -30.0'. Boring backfilled with cuttings. 0 0.50 25 0 B -15 2 P 2 0.50 25 2 B 2 1.40 23

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



GSI Job No. 19079-B

SOIL BORING LOG

Page 1 of 1

LOCATION	I-90 & I-94	Tollway													_
COUNTY	Cook	DRILLING	3 ME	тно	D _		Hol	llow Stem Auger	HAMMER T	YPE	(CME	Auto	mati	С
CLIENT	НВМ		D E P	B L O	UCS	M 0 1	B	Surface Water Elev. Stream Bed Elev.		ft ft	D E P	B L O	UCS	M 0 I	
BORING NO Northing Easting	1903784 116987	5	T H	w s	Qu	s	DENS-+-Y	Groundwater Elev.: First Encounter Upon Completion	Dry Dry		Н	w s	Qu	s	
Ground Surface		74.7 ft	(ft)	(/6")	(tsf)	(%)	(pcf)	The state of the s	1000	ft	(ft)	(/6")	(tsf)	(%)	(p
10.0" CONCRE	TE	573.9	-					CLAY-gray-soft to med (continued)	lium stiff		182				
CLAYEY GRAV		373.3	_	2	-	-	-	(100,111,111,111,111,111,111,111,111,111				2			
STONE-brown 8	& gray-loose			3		15					4		0.60	20	-
			-	2			_				92	2	В		
			-	-											
			-	6							19-	0			
				5		26		İ					1.00	21	
			5	3			-				25	3	В		_
CLAY-gray-soft	to medium stit	569.2	_	-				CLAY LOAM-gray-med	dium stiff to	549.2	_				
		1270		3				stiff			_	2			
			_		0.50	24							1.00	16	
			_	3	Р		-				-	4	Р		_
				1							_				
				1							1	3			
					0.50	24						3 5	2.30	20	
			10	2	В		-	End Of Boring @ -30.0	P Boring	544.7	-30	5	В		-
			-	1				backfilled with cuttings	. Builing		1				
				0											
			_	1 2	0.50	24					_				
			1	-	В	_	-				9				
			_	1							_				
				1			_								
				4	0.25	23									
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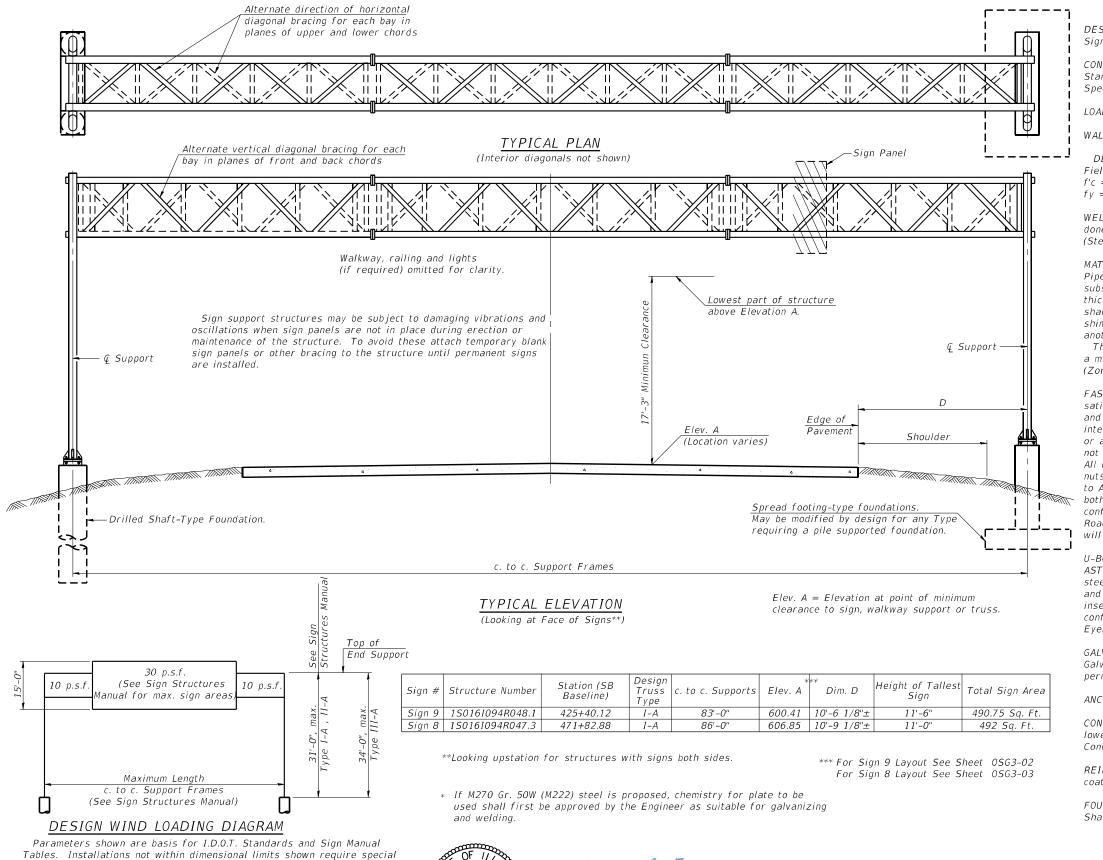
The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)

HANSON

USER NAME = DESIGNED - PMH CHECKED - BWC REVISED -PLOT SCALE = MGM REVISED -PLOT DATE = DATE - 09/09/2022 REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

TOTAL SHEET NO. **SOIL BORING LOGS** SECTION COUNTY 2020-004-BR COOK 1492 331 SN 1S016I094R050.4 90/94 CONTRACT NO. 62K74 SHEET OSG1-15 OF OSG1-15 SHEETS



GENERAL NOTES

DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions. ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

DESIGN STRESSES: Field Units f'c = 3,500 p.s.i.

fy = 60,000 p.s.i. (reinforcement)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Specificiations.

MATERIALS: Aluminum Alloys as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer.

The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to ASTM F1554 Gr. 105.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final ground line at each foundation shall be cleaned and coated with Concrete Sealer in accordance with the Standard Specifications.

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

FOUNDATIONS: The contract unit price for Concrete Foundations and Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Concret	Foot	88
Concret	Foot	66
Overhe	Foot	169
Drilled \$	ih Cu Yd	67
Remove (Ove Each	2.0

5-A-1 2-17-2017

analysis for all components.

HBM ENGINEERING GROUP LIC

USER NAME =	DESIGNED	-	SK	REVISED -
	CHECKED	-	MI	REVISED -
PLOT SCALE =	DRAWN	-	SK	REVISED -
PLOT DATE =	DATE	-	12/5/2022	REVISED -

MOUSSA A

ISSA 081-005738 CHICAGO•

ILL INDIS

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

(TOTAL OF 18 SHEETS)

FOR SHEETS OSG3-01 THRU OSG3-18

Moussa H. ISSac

12/05/2022

Dr. Moussa A. Issa, S.E. II. Lic. No. 081-005738 Expires 11-30-2024

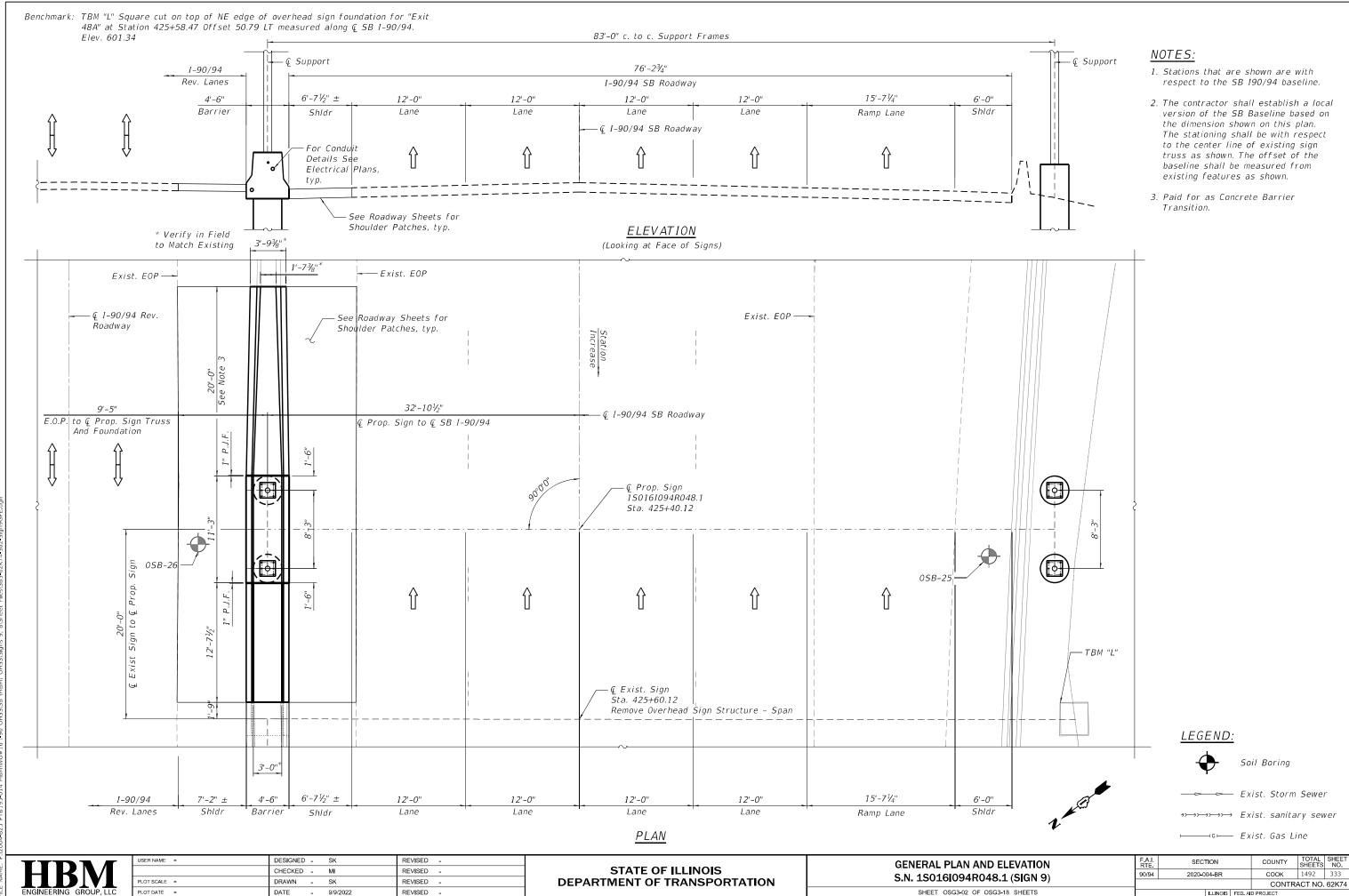
OVERHEAD SIGN STRUCTURES - GENERAL PLAN & ELEVATION - ALUMINUM TRUSS & STEEL SUPPORTS

SHEET OSG3-01 OF OSG3-18 SHEETS

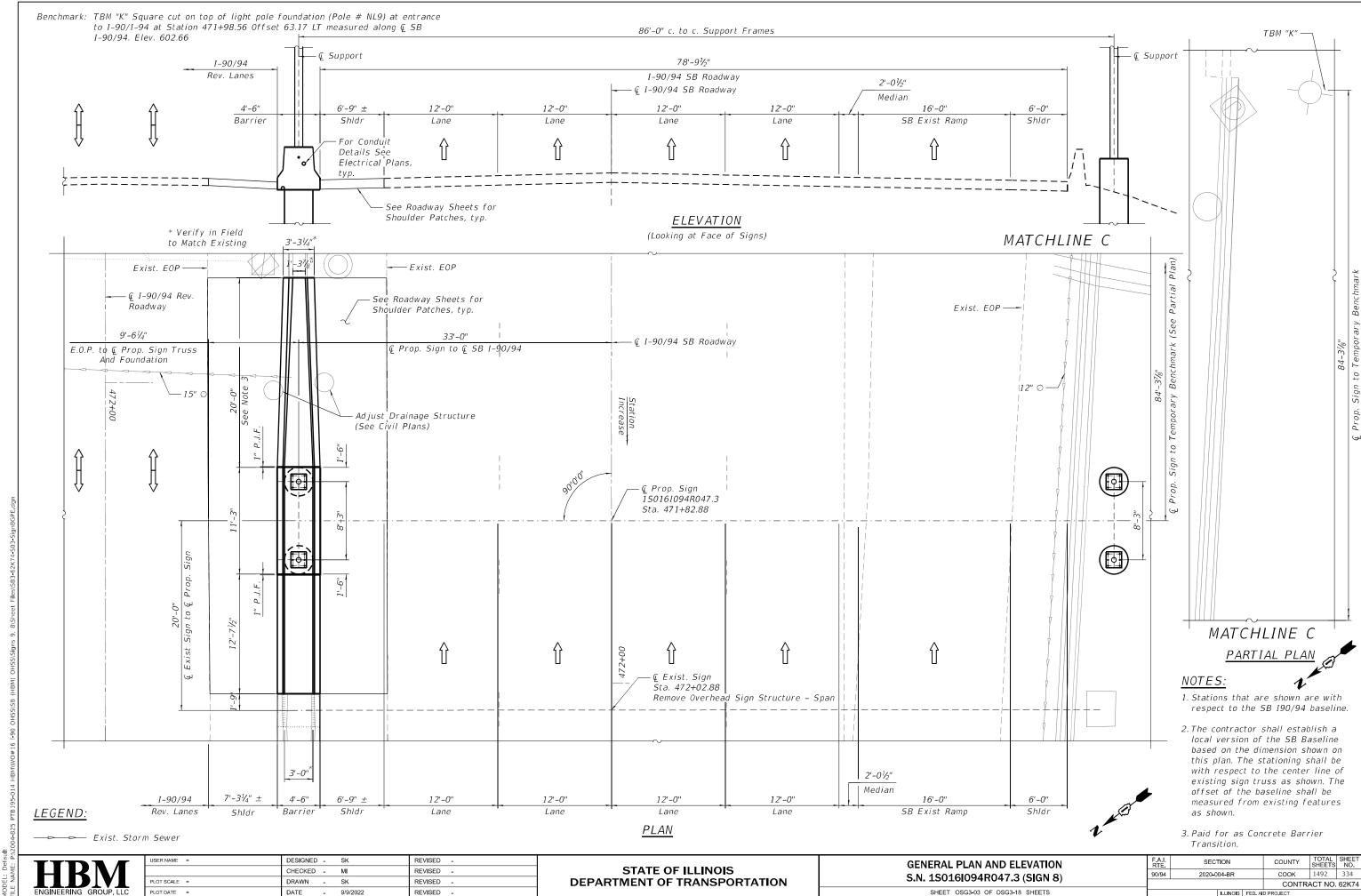
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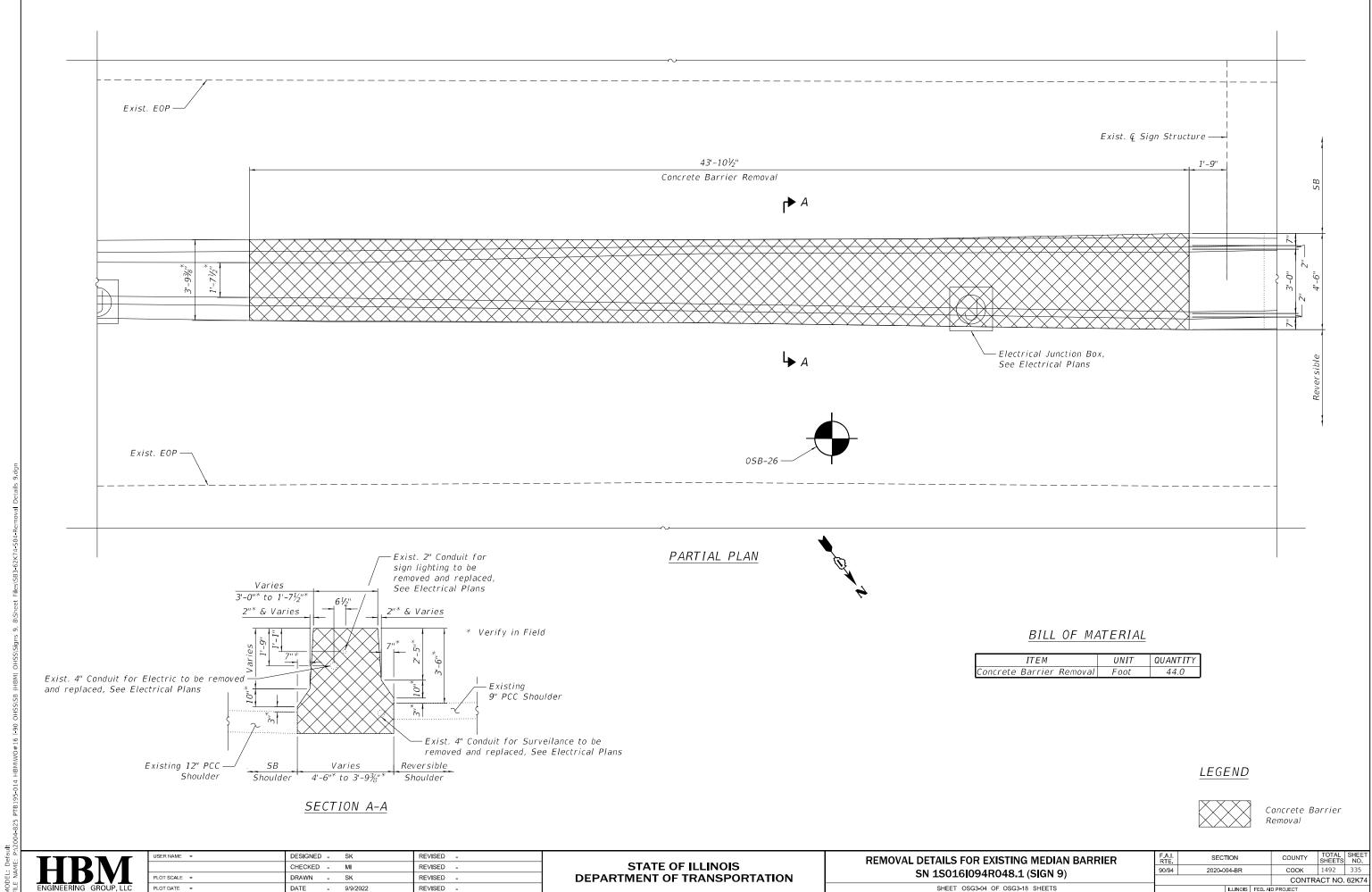
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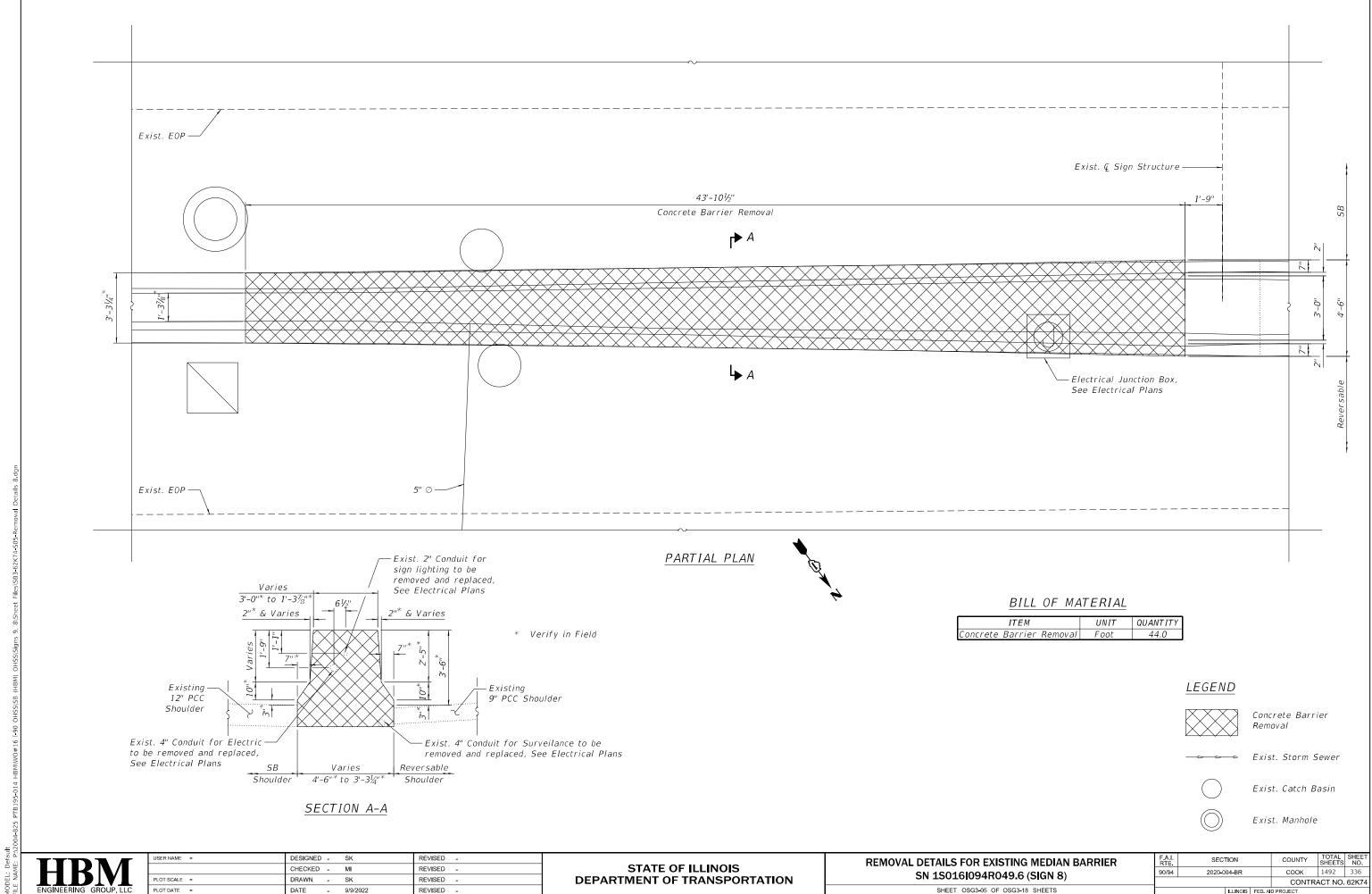


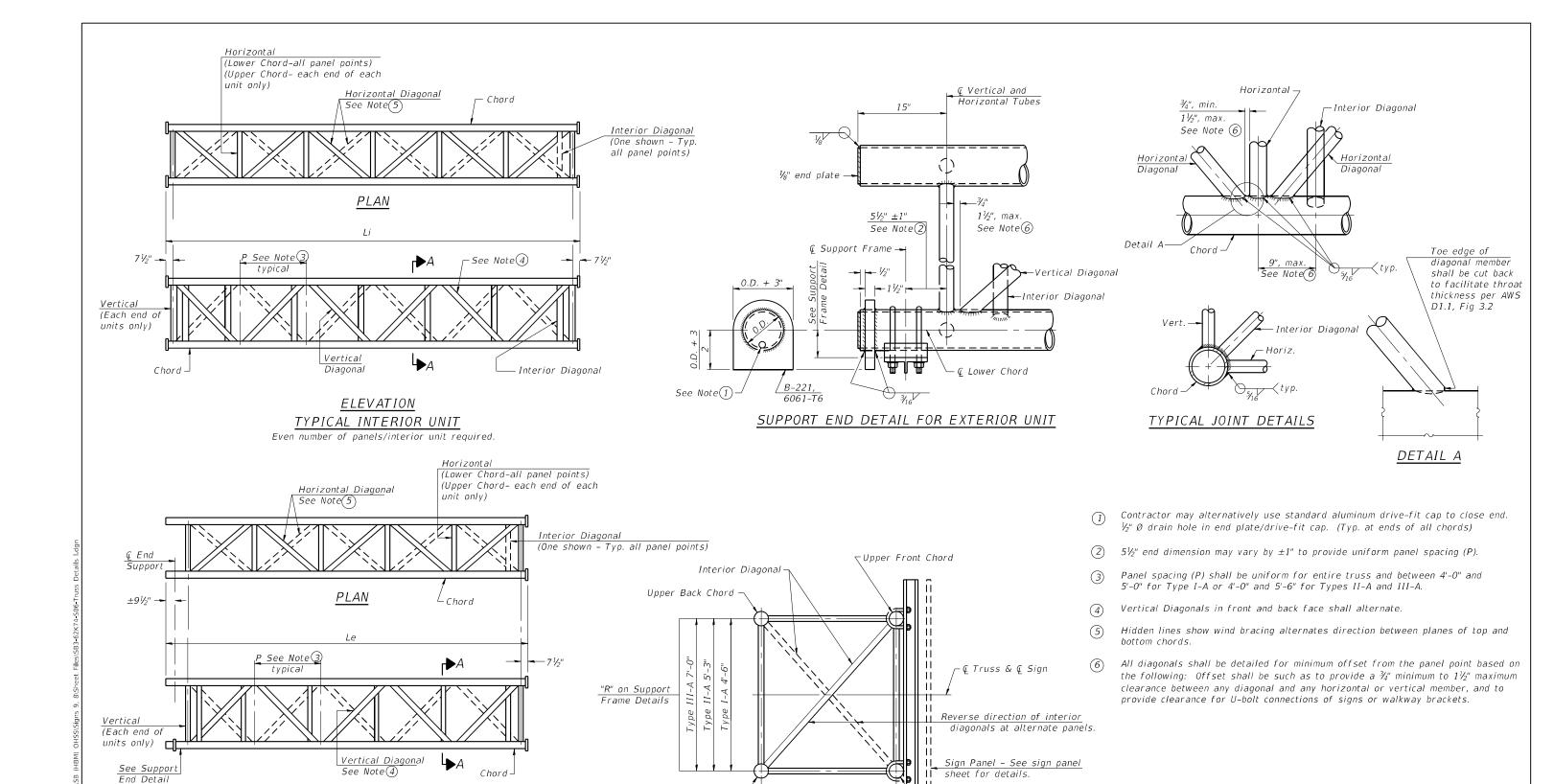
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SECTION A-A

Type I-A 4'-0"

Type II-A 4'-6"

Type III-A 5'-0"

L"T" on Support Frame Details

- Lower Front Chord

2-17-2017

-	USER NAME =	DESIGNED	-	SK	REVISED	-
		CHECKED	-	MI	REVISED	-
	PLOT SCALE =	DRAWN	-	SK	REVISED	-
5	PLOT DATE =	DATE	-	9/9/2022	REVISED	-

ELEVATION

TYPICAL EXTERIOR UNIT

Even or odd number of panels/exterior units allowed.

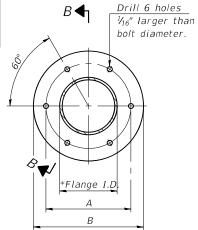
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

Lower Back Chord -

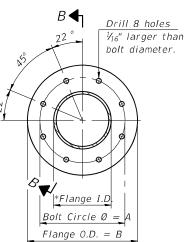
OVERHEAD SIGN STRUCTURES - ALUMINUM TRUSS DETAILS FOR TRUSS TYPE I-A SHEET OSG3-06 OF OSG3-18 SHEETS

SECTION COUNTY 90/94 2020-004-BR COOK 1492 337 CONTRACT NO. 62K74

Sign #	Structure Number	Station	Design Truss Type	,	Exterior Units (2	?)			Interior Unit			& Lower ord	Horiza Vert Horizor	icals; ontals; tical, otal, and Diagonals	Camber at Midspan			Splicing	Flange		
				No. Panels per Unit	Unit Lgth. (Le)	Panel Lgth. (P)	No. Reg'd	No. Panels per Unit	Unit Lgth. (Li)	Panel Lgth. (P)	0.D.	Wall	0.D.	Wall		Bolt No./Splice	Dia.	Weld S W	izes W1	Α	В
Sign 9	15016I094R048.1	425+40.12	I-A	5	26'-9 1/4"	4'-11 3/4"	1	6	31'-1 1/2"	4'-11 3/4"	5"	5/16"	2 1/2"	5/16"	2.40"	6	7/8"	5/16"	1/4"	8 3/4"	11 3/4"
Sign 8	150161094R047.3	471+82.88	I-A	6	29'-4 1/2"	4'-7"	1	6	28'-9"	4'-7"	5"	5/16"	2 1/2"	5/16"	2.55"	6	7/8"	5/16"	1/4"	8 3/4"	11 3/4"



TRUSS TYPES I-A, II-A, & III-A



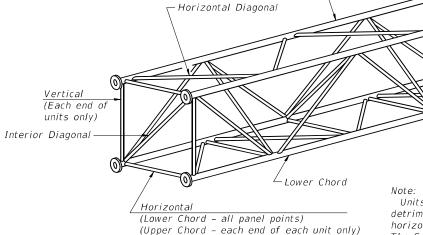
TRUSS TYPES II-A & III-A SPLICING FLANGES

ASTM B221, Alloy 6061-T6 or ASTM B209, Alloy 6061-T651 *To fit 0.D. of Chord with maximum gap of $\frac{1}{16}$ ".

See Table & Note(1) WVQ ≺ See Table High Strength bolts with locknuts or (if members interfere) threaded studs with 2 locknuts. Use stainless steel washers under head and nut. See table.

SECTION B-B

1) Splicing Flanges shall be attached to each truss unit with the truss shop assembled to camber shown. Truss units shall be in proper alignment and flange surfaces shall be shop bolted into full contact before welding. Sufficient external welds or tacks shall be made to secure flanges until remaining welds are made after disassembly. Adjacent flanges shall be "match marked" to insure proper field assembly.



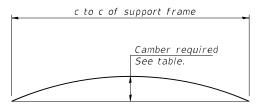
ISOMETRIC VIEW TYPICAL TRUSS UNIT ASTM B221 Alloy 6061 Temper T6

Splicing Flange

Units shall be shipped individually with adequate provision to prevent

detrimental motion during transport. This may require ropes between horizontals and diagonals or energy dissipating (elastic) ties to the vehicle. The Contractor is responsible for maintaining the configuration and protection of the units.

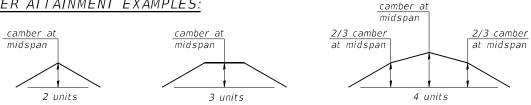
Vertical Diagonal $\stackrel{ ext{ iny }}{=}$



CAMBER DIAGRAM

Camber curve shown is theoretical. Actual camber attained by slope changes at splices between units.

CAMBER ATTAINMENT EXAMPLES:



Camber shown is for fabrication only, measured with truss fully supported. (No-load condition)

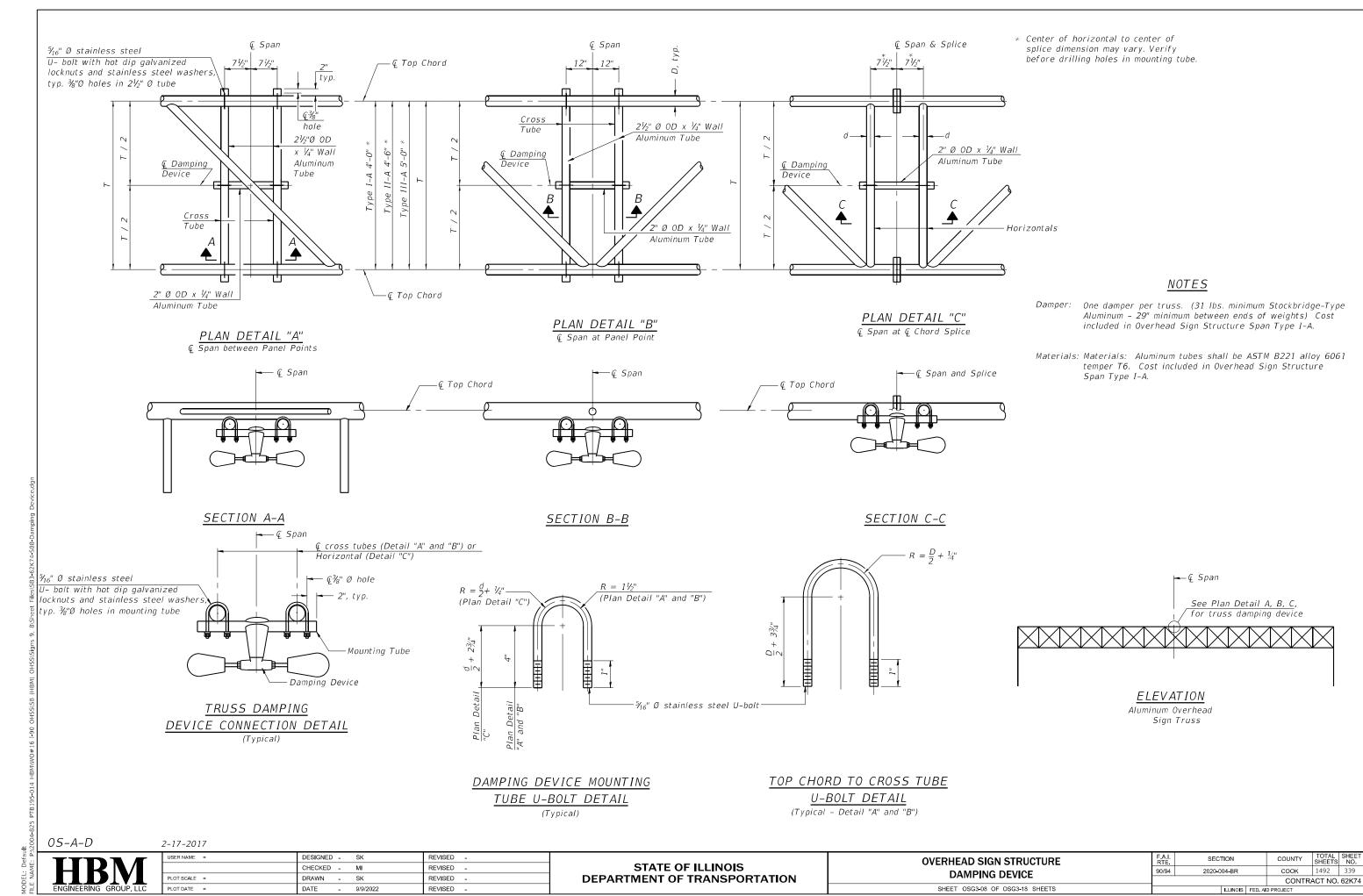
┌Upper Chord

2-17-2017

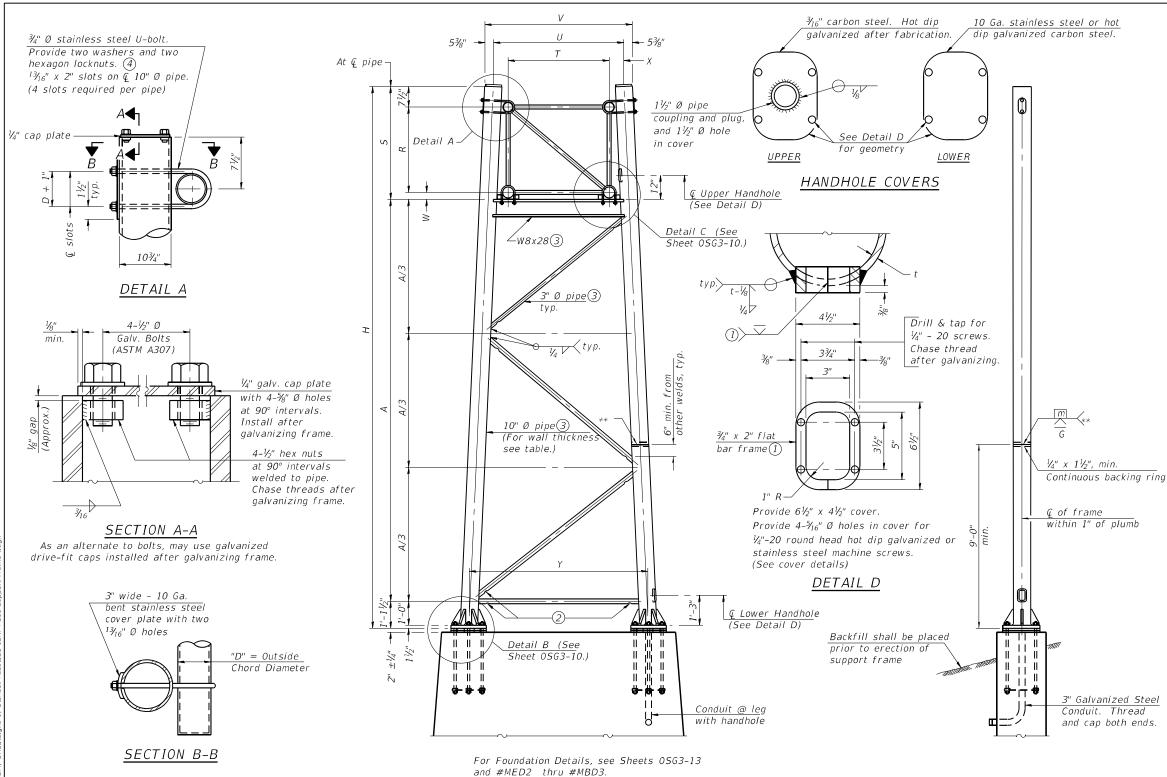
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PLOT SCALE =	DRAWN	-	SK	REVISED	-
PLOT DATE =	DATE	-	9/9/2022	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES - ALUMINUM TRUSS	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
DETAILS FOR TRUSS TYPE I-A		2020-004-BR	соок	1492	338
DETAILS FOR TROOS THE LA			CONTR	RACT NO	62K74
SHEET OSG3-07 OF OSG3-18 SHEETS		ILLINOID FED	ALD DDO JECT		



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SIDE ELEVATION

Support Design Loads: See Sheet OSG3-Ddr design and loading criteria.

Load combinations checked include deadload plus:

- a) 100% wind normal to sign, 20% parallel to sign
- b) 60% wind normal to sign, 30% parallel to sign
- In lieu of fabricated handhole frame as shown, may cut from 2" plate (rolling direction vertical). All cut faces to be ground to ANSI Roughness of 500µ in or less.
- 2) Galvanizing vent holes of adequate size shall be provided on underside at each end of bracing pipes. Alternately, holes may be provided in wall of pipe column. All vent holes shall be drilled and de-burred, typ.
- 3 Steel pipe, plate, carbon steel handhole covers and rolled sections shall be hot dip galvanized after fabrication. Painting is not permitted. See Sheet 05G3-01.
- (4) See General Notes for fasteners.
- Dimensions shown are based on selection criteria in the Sign Structures Manual. Nonstandard applications must have dimensions verified or amended as appropriate.
- 6 "H" based on 15'-0" or actual sign height, whichever is

END ELEVATION

Truss	Dimensions												
Type	R	5	T	U	V	W	X	Y					
I-A	4'-6"	5'-5½"	4'-0"	5'-6"	6'-4¾''	4"	9"	8'-3"					
11-A (5)	5'-3"	6'-31/4"	4'-6"	6'-1"	6'-11¾"	4¾"	91/2"	8'-3"					

10" Ø PIPE TRUSS SUPPORT FRAME

** One butt welded joint is allowed only on one post per support frame. If used, weld procedure must be preapproved by Engineer and joint shall receive 100% RT or UT (tension criteria) at Contractor's expense.

Sian +	Sign #	Structure Number	Station	Support		Truss	Pipe Wall	H (6)	4
31911 #		Structure Number	Station	Left	Right	Туре	Thickness		
Sign 9	Cian O	1S016I094R048.1	425+40.12	Х		I-A	0.279"	24'-9 1/8"	18'-2 1/8"
Jigii s	′				X			26'-0 5/8"	19'-5 5/8"
Sian 8	,	15016I094R047.3	471+82.88	Х		I-A	0.279"	25'-2 3/4"	18'-7 3/4"
Jigii 6	,				Χ			26'-5 1/2"	19'-10 1/2

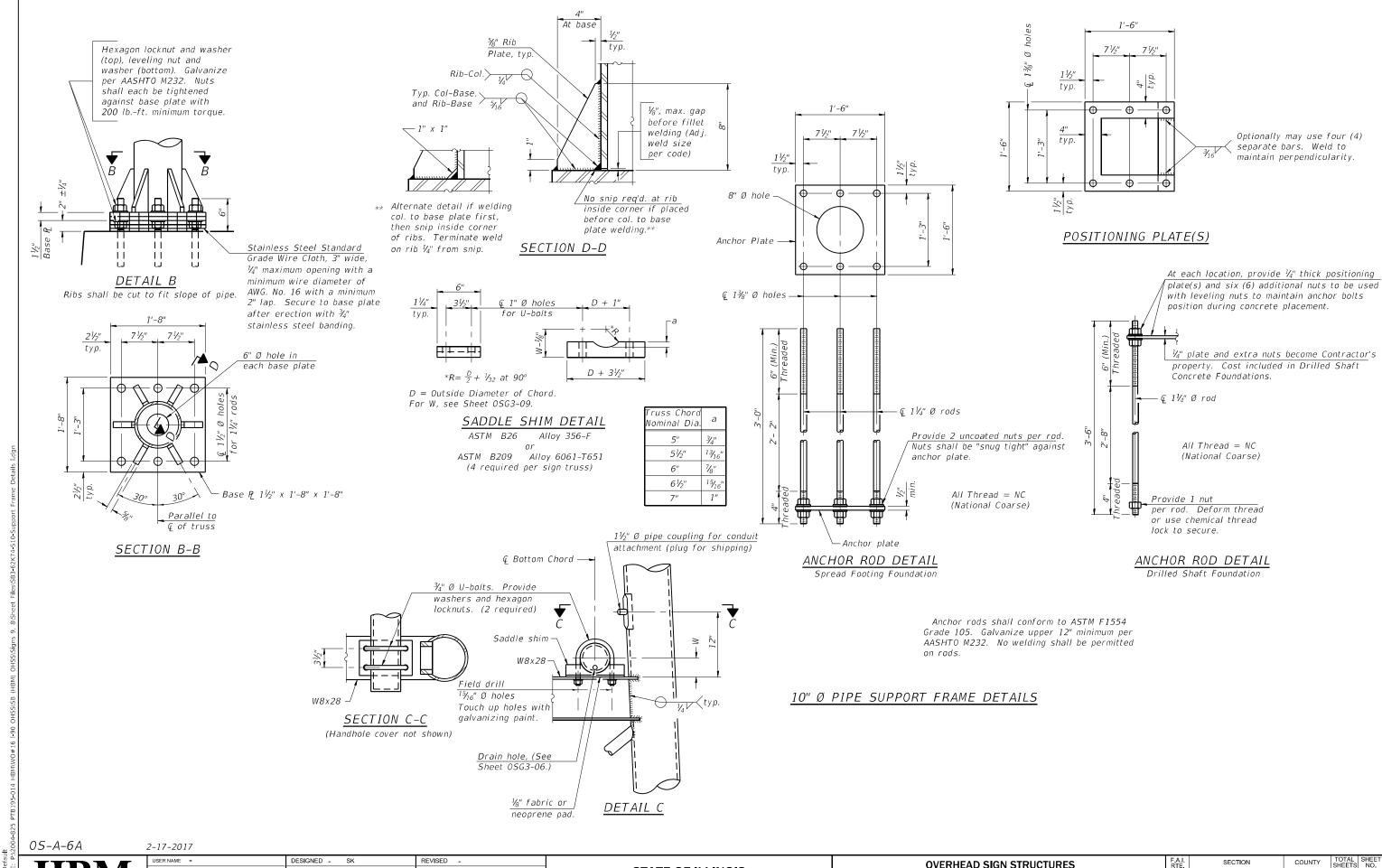
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PLOT SCALE =	DRAWN	-	SK	REVISED -
PLOT DATE =	DATE	-	9/9/2022	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES	F.A.I. RTE.					
SUPPORT FRAME FOR TYPE I-A ALUMINUM TRUSS						
SHEET OSG3-09 OF OSG3-18 SHEETS						

F.A.I. RTE	SEC	TION	COUNTY	TOTAL SHEETS	SHEET NO.	
90/94	2020-	004-BR	соок	1492	340	
				CONTR	ACT NO.	62K74
		ILLINOIS	FED. Al	D PROJECT		

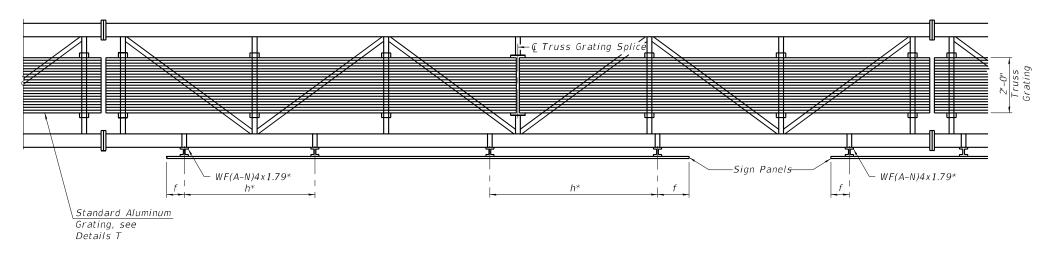


	USER NAME =	DESIGNED	-	SK	REVISED	-
		CHECKED	-	MI	REVISED	-
	PLOT SCALE =	DRAWN	-	SK	REVISED	-
.LC	PLOT DATE =	DATE	-	9/9/2022	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES 90/94 2020-004-BR COOK 1492 341 SUPPORT FRAME DETAILS FOR TYPE I-A - ALUMINUM TRUSS CONTRACT NO. 62K74 SHEET OSG3-10 OF OSG3-18 SHEETS

TYPICAL FRONT ELEVATION



BRACKET TABLE

WF(A-N)4x1.79 ASTM B308, Alloy 6061-T6										
Sign \	Number									
Greater Than	Less Than or Equal To	Brackets Required								
	8'-0"	2								
8'-0"	14'-0"	3								
14'-0"	20'-0"	4								
20'-0"	20'-0" 26'-0"									
26'-0"	32'-0"	6								

SECTION A-A

Place all sign brackets as close to panel points as practical.

* Space sign brackets WF(A-N)4x1.79 for efficiency and within limits shown:

f=12" maximum, 4" minimum (End of sign to $\mathcal Q$ of nearest bracket) h=6'-0" maximum ($\mathcal Q$ to $\mathcal Q$ sign support brackets, WF(A-N)4x1.7

Notes:

For Detail T and Section B-B, see Base Sheet OS-A-10-NW. Truss grating to facilitate inspection shall run full length (center to center of support frames) 12"± on overhead trusses. Cost of truss grating is included in "Overhead Sign Structure".

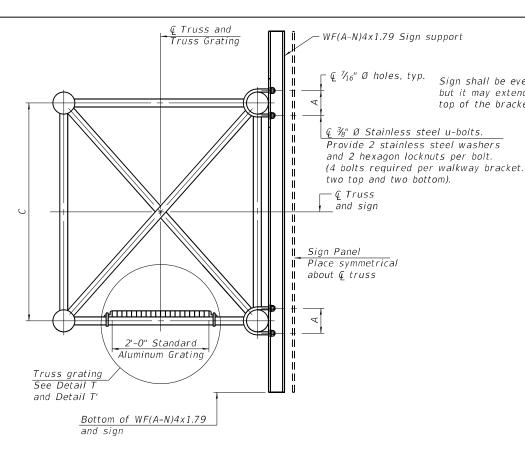
Truss Grating width dimensions are nominal and may vary $\frac{V_2}{2}$ based on available standard widths.

05-4-9-NW

4-1-2020



USER NAME =	DESIGNED	-	SK	REVISED -
	CHECKED	-	MI	REVISED -
PLOT SCALE =	DRAWN	-	SK	REVISED -
PLOT DATE =	DATE	-	9/9/2022	REVISED -



SECTION B-B

SPECIFICATIONS FOR STANDARD ALUMINUM GRATING

Main Bearing Bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on $1\frac{3}{16}$ " centers and conform to ASTM B221 Alloy 6061-T6.

Cross bars shall be \aleph_{16} " x $1\frac{1}{2}$ " on 4" centers and conform to ASTM B221 Alloy 6063-T5 or 6061-T6.

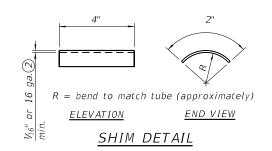
ΩR

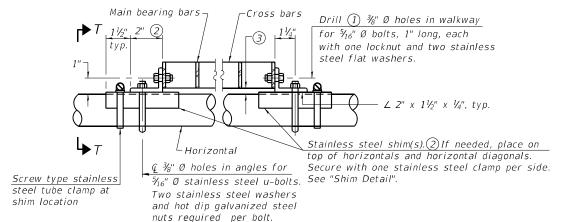
Aluminum Grating with modified "t" sections for main bearing bars shall meet the following requirements:

Main bars shall conform to ASTM B221 Alloy 6061–T6 and have a minimum section modulus equal to 0.0705 in.³ per bar, a depth of $1\frac{1}{2}$ ", spaced on $1\frac{3}{2}$ 16" centers.

Cross bars shall conform to ASTM B221 Alloy 6063-T5 or T-42 and spaced on 4" centers.

Sign #	Structure Number	Station	Α	С
Sign 9	15016I094R048.1	425+40.12	5.5"	4'-6"
Sian 8	15016I094R047 3	471+8288	5 5"	4'-6"





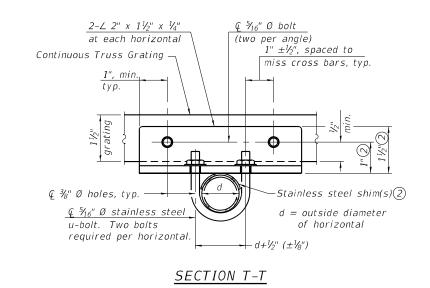
DETAIL T

U-bolt and angle connections

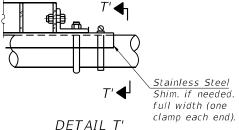
required at horizontals only.

Sign shall be even with the top of the bracket, but it may extend no more than 6" above the top of the bracket for field adjustments.

(Continuous Truss grating)

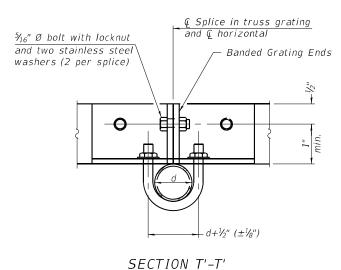


- Drilling holes in grating may be done in shop or field, based on Contractor's preference and subject to accurate alignment.
- 2 Stainless steel shims shall be placed as shown in Detail T if needed to compensate for alignment variations between horizontal and diagonal pipes beyond adjustment provided by angles. Thicker shims may be used subject to shims performing properly.
- 3 Tube to grating gap may vary from 0 to ½", max. to align walkway, allow for camber, etc.



DETAIL

(Truss grating splice)
Details not shown same as Detail T.
Alternate materials may be used subject to the
Engineer's review and approval.



05-A-10-NW

4-1-2020

03 /1 1	0 1000
	K 1
ENGINEER	NG GROUP, LLC

USER NAME =	DESIGNED	-	SK	REVISED	-
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PLOT DATE =	DATE	-	9/9/2022	REVISED	-

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES
ALUMINUM WALKWAY DETAILS
SHEET OSG3-12 OF OSG3-18 SHEETS

 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS
 SHEETS NO.

 90/94
 2020-004-BR
 COOK
 1492
 343

 CONTRACT NO. 62K74

The foundation dimensions shown are based on the presence of mostly cohesive soils with an average Unconfined Compressive Strength (Qu) of at least 1.25 tsf, which must be determined by previous soil investigations at the jobsite. When other conditions are indicated, the boring data will be included in the plans and the foundation dimensions shown will be the result of site specific designs.

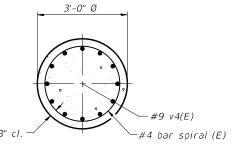
If the conditions encountered are different than those indicated, the Contractor shall notify the Engineer to determine if the foundation dimensions need to be modified. If dimensions "B" or "F" are revised by more than 12" by the Contractor, "as-built" plans shall be prepared and submitted to the District Bureau of Operations for future reference.

No sonotubes or decomposable forms shall be used below the lower conduit entrance. Permanent metal forms or other shielding may not be left in place below that elevation without the Engineer's written permission.

Concrete shall be placed monolithically, without construction joints.

Backfill shall be placed per Article 502 of Standard Specification and prior to erection of support column.

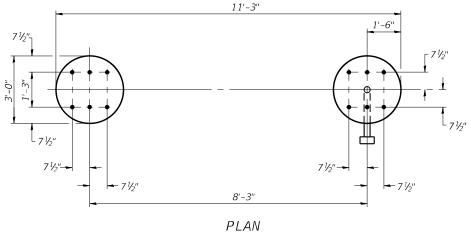
A normal surface finish followed by a Concrete Sealer application will be required on concrete surfaces above the lowest elevation 6" below finished ground line. Cost included in Drilled Shaft Concrete Foundation.



SECTION A-A

DETAILS FOR 10" Ø SUPPORT FRAME TYPE I-A or II-A TRUSS

	#6 copper wire or cable ground rod driven into ground ground driven into ground ground conduit, caps and clamps shall be included in Drilled Shaft Concrete Foundations. #6 copper wire or cable Ø Galvanized Steel Induit. Thread Ind cap both ends.	
	Shaft Concrete Foundations.	
dgn	111 211	



For anchor rod size and placement, see Support Frame Detail Sheet.

* Anchor rod shall be ground or filed to bright metal at clamp and cable connection location.

																	Left F	oundation				Right Found	ation		
Sign #	Structure Number	Station	Elevation Top	Elevation Bottom	А	В	F	Elevation Top	Elevation Bottom	А	В	F	Class DS Concrete (Cu. Yds.)												
Sign 9	15016I094R048.1	425+40.12						602.31	568.81	3'-6"	30'-0"	33'-6"	17.6												
Sign 8	150161094R047.3	471+82.88						608.35	588.35	3'-6"	16'-6"	20'-0"	10.5												

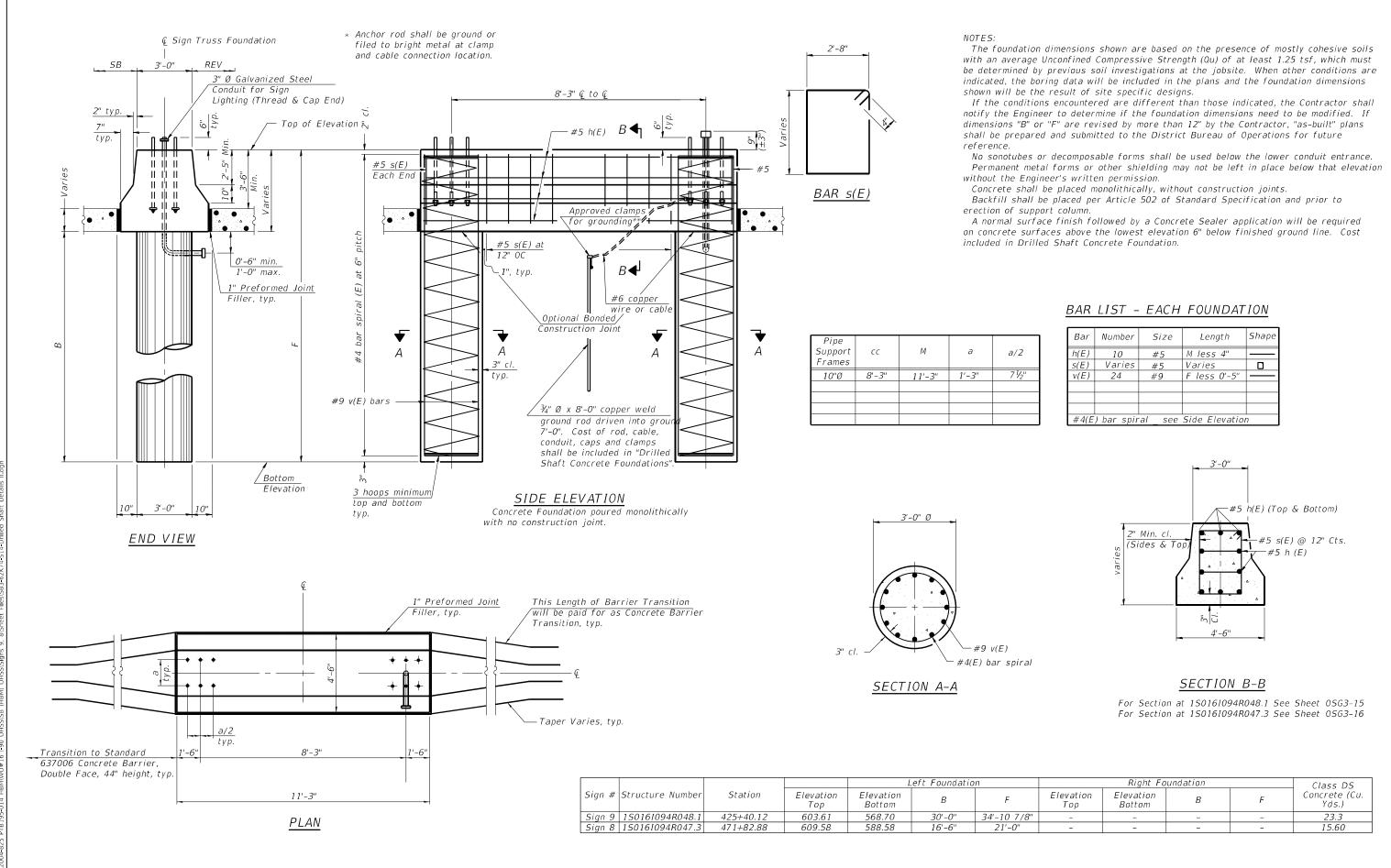
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200000		CHECKED - MI	REVISED -
PLOT DATE = DATE 9/9/2022 REVISED	PLOT SCALE =	DRAWN - SK	REVISED -
DATE = 3/3/2022 REVISED =	PLOT DATE =	DATE - 9/9/2022	REVISED -

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

OVERHEAD SIGN STRUCTURES DRILLED SHAFT DETAILS SN 1S016I094R048.1 (SIGN 9) & SN 1S016I094R047.3 (SIGN 8) SHEET OSG3-13 OF OSG3-18 SHEETS

SECTION COUNTY 2020-004-BR COOK 1492 344 CONTRACT NO. 62K74

2-17-2017



MODEL: Default

 USER NAME
 =
 DESIGNED
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 REVISED

 PLOT SCALE
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 DRAWN
 #DSD2DR
 REVISED

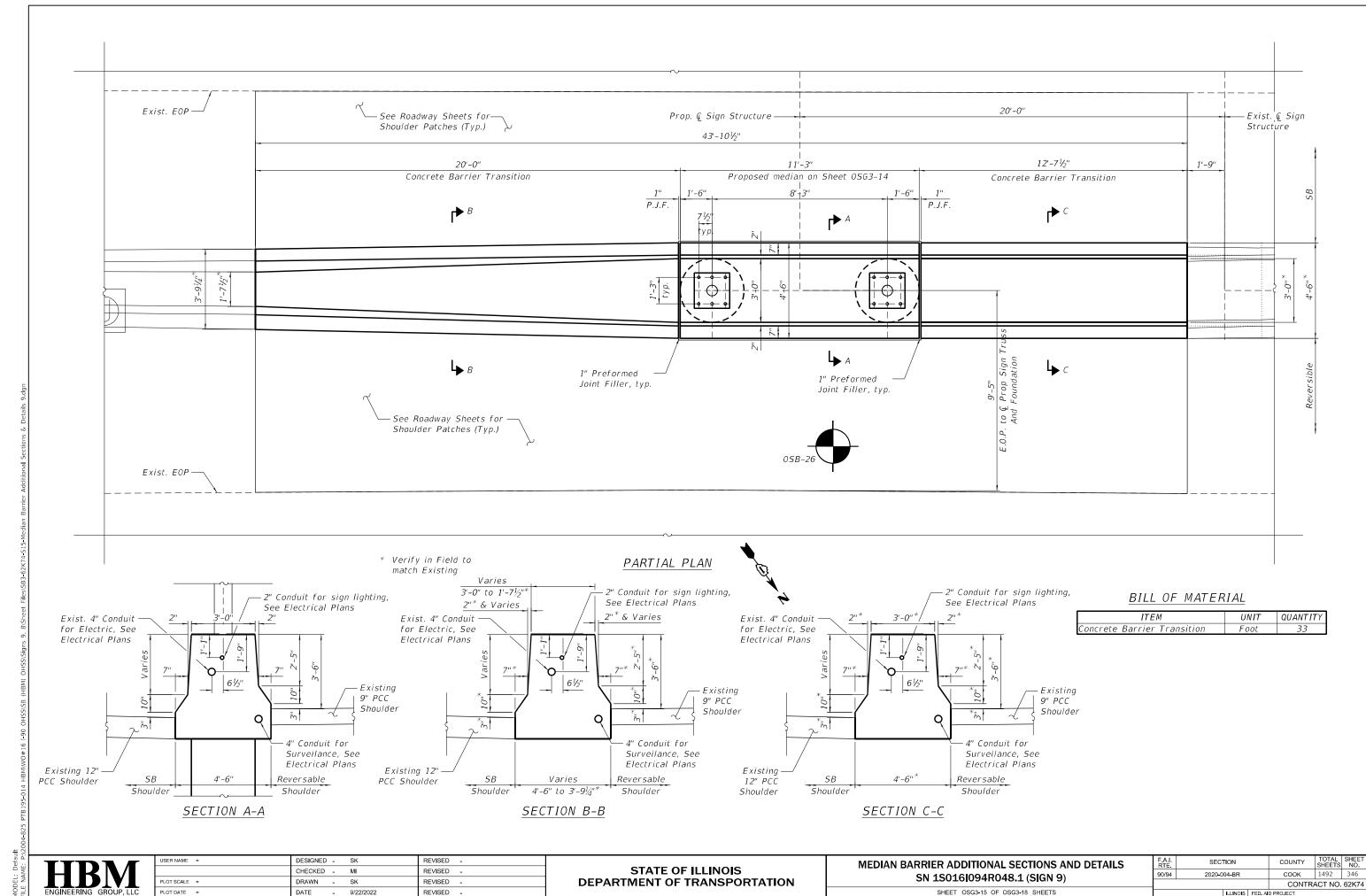
 PLOT DATE
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 DATE
 9/22/2022
 REVISED

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

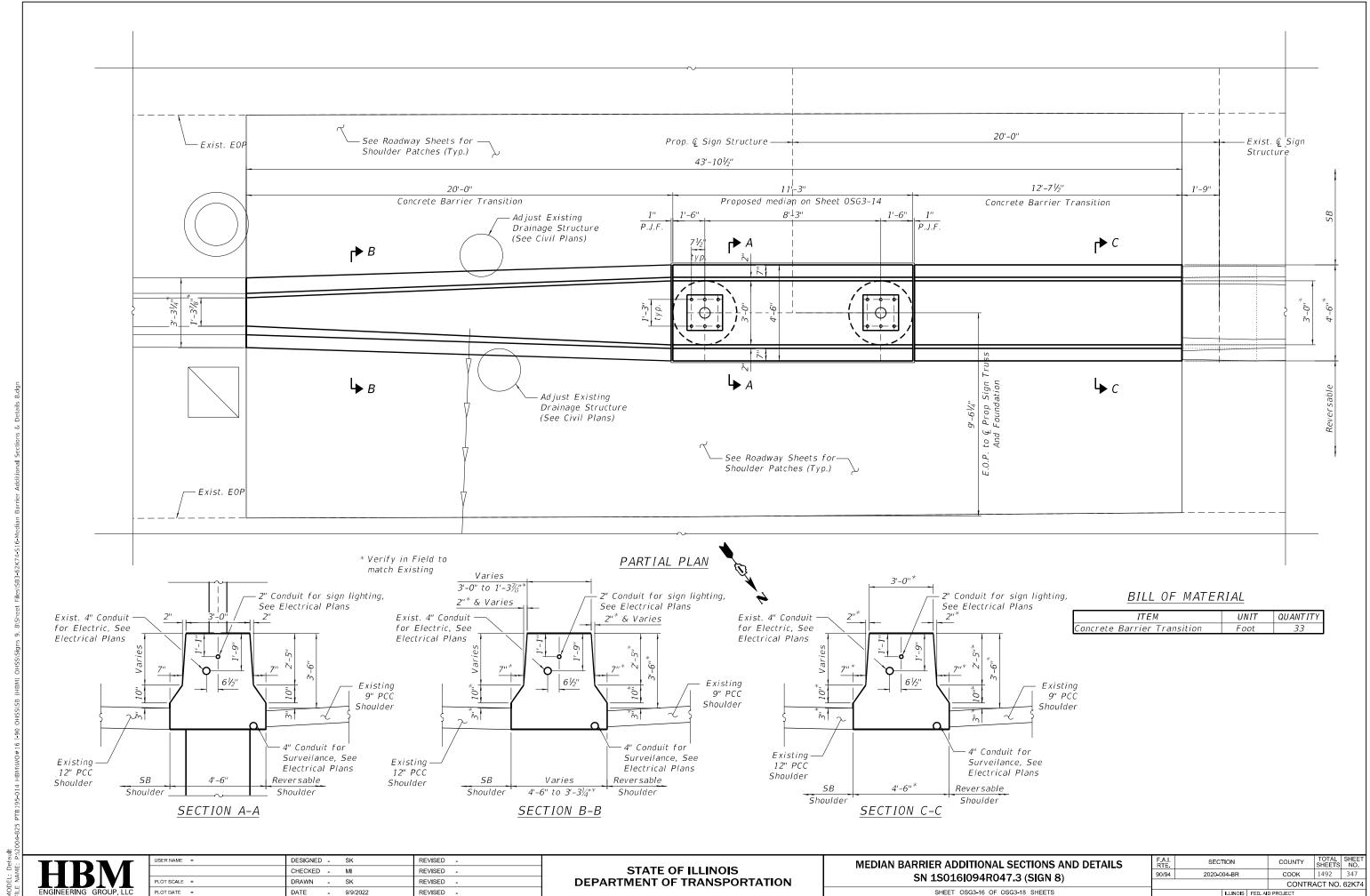
 OVERHEAD SIGN STRUCTURES
 F.A.I. RTE.
 SECTION
 COUNTY
 TOTAL SHEETS NO.

 MEDIAN SUPPORT FOUNDATION DETAILS II
 90/94
 2020-004-BR
 COOK
 1492
 345

 SHEET OSG3-14 OF OSG3-18 SHEETS



9/22/2022 10:39:03 AM



9/9/2022 6:48:38 PM



STONE-medium dense (Fill)

g SILTY CLAY-black-stiff (Fill)

TOPSOIL-black-medium stiff

CLAY LOAM-brown & gray-very

GSI Job No. __19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

0.50 26

Date 11/9/21 PTB 185-012, WO #32 **PROJECT** I-90 & I-94 Tollway LOCATION COUNTY **DRILLING METHOD** Hollow Stem Auger **HAMMER TYPE** CME Automatic UM Surface Water Elev. CLIENT **HBM** Ε CO С E 0 Stream Bed Elev. 0 SI S BORING NO. OSB-25 TW S W Groundwater Elev.: Northing 1913889 H S Qu T 571.9 **ft** ▼ H S First Encounter Qu T 1163840 **Easting Upon Completion** Ground Surface Elev. 599.4 (ft) (/6") (tsf) (%) (pcf) After Hrs. ft (ft) (/6") (tsf) (%) (pcf) 12.0" CONCRETE 578.9 CLAY-gray-medium stiff SAND & GRAVEL-brown-loose (Fill) 2 0.80 25 В CRUSHED STONE-loose (Fill) 1 0.80 26 В CRUSHED BRICK &

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)

3 3.80 14 6 B

² 2.10 20 ⁴ B

4 2.10 19 5 B



GSI Job No. __19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date <u>10/21/21</u>

PROJECT PTB 185-012, W														_
LOCATION I-90 & I-94 Tolly	vay													
COUNTY Cook DE	RILLING	ME	тно	D _		Hol	low Stem Auger	HAMMER	TYPE	(CME	Auto	mati	С
CLIENT HBM		DE	B L	U	M O	730	Surface Water Elev. Stream Bed Elev.		_ ft _ ft	D E	BL	D 0	M O	DRY C
BORING NO. OSB-26 Northing 1913955 Easting 1163891 Ground Surface Elev. 599.4	 ft	P T H	O W S		S T	DEEZO-TY (pcf)	Groundwater Elev.: First Encounter Upon Completion After Hrs.		_ ft <u>▼</u> _ ft ∑ ft		O W S (/6")	S Qu	I S T (%)	DHZW-I-A
9.0" CONCRETE		(11)	(/6")	(tSI)	(70)	(pci)	CLAY LOAM-brown &	gray-hard	578.9		(/6")	(LSI)	(/0)	(pc
CINDERS-black-medium dense	598.6	_					(continued) CLAY-gray-medium st		370.5					
(Fill)		_	8 16		22	1 2	CLAY-gray-medium st	III to still		-	2	0.50	27	
			4		22						3	0.30 В	21	
	596.4									*				
CLAY LOAM-dark brown & gray-very stiff (Fill)		_	3								4			
				2.00	21		,				450	0.50	25	
CINDERS, BRICK & STONE-medium dense to dense (Fill)		-5	17	В						-25	2	В		
CINDERS, BRICK &	593.9													
STONE-medium dense to dense			10								3			
(Fill)		_	15		23					_		0.80	25	
		-	24								1	В		
	-													
	_	-	5							_	3			
		-	10		21					7	3	1.00 B	26	
	588.9	-10					End Of Boring @ -30.0	D'. Boring	569.4	-30	-			
CLAY LOAM-brown & gray-stiff to							backfilled with cuttings	š.		07				
very stiff (Fill)			9		11		5			-				
			2		11									
	586.4	Z												
CLAY LOAM-brown & gray-stiff to very stiff (Fill) SILTY SAND & STONE-black-very dense (Fill)		_	50/0"							19				
,			00.0		26									
		-15								-35				
		_								-				
			50/1"											
CLAY LOAM-brown & gray-hard					NR					_				
	F04 4	-								-				
CLAY LOAM-brown & gray-hard	581.4		-											
			7											
			8	5.00 B	17					ST Testing				
		V -20		ם						-40	L			oxdot

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



USER NAME =	DESIGNED -	SK	REVISED -
	CHECKED -	MI	REVISED -
PLOT SCALE =	DRAWN -	SK	REVISED -
PLOT DATE =	DATE -	9/9/2022	REVISED -



GSI Job No. __19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date ___11/8/21__

PTB 185-012, WO #32 **PROJECT** I-90 & I-94 Tollway LOCATION COUNTY **DRILLING METHOD** Hollow Stem Auger **HAMMER TYPE** CME Automatic UM Surface Water Elev. CLIENT **HBM** Ε CO СО E Stream Bed Elev. 0 SI S BORING NO. OSB-20 TW S W Groundwater Elev.: Northing 1916702 H S Qu T Qu T First Encounter H S 1160083 **Easting Upon Completion** Ground Surface Elev. 606.6 (ft) (/6") (tsf) (%) (pcf) After Hrs. ft (ft) (/6") (tsf) (%) (pcf) 12.0" CONCRETE 586.1 CLAY-gray-soft to medium stiff 605.6 CRUSHED STONE-loose to medium dense (Fill) 3 0.60 26 В 2 0.60 26 20 4 B CLAY LOAM-dark brown & gray-very stiff (Fill) 4 3.10 16 2 0.25 22 В 4 2.50 17 2 0.25 26 4 B End Of Boring @ -30.0'. Boring backfilled with cuttings. 5 3.50 16 В SILTY CLAY LOAM-dark gray-stiff (Fill) 1.50 21 50 B CLAY LOAM-brown & gray-stiff to ⊭ very stiff 4 2.20 19 В 1.80 20 В

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



GSI Job No. <u>19079-B</u>

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date <u>10/20/21</u>

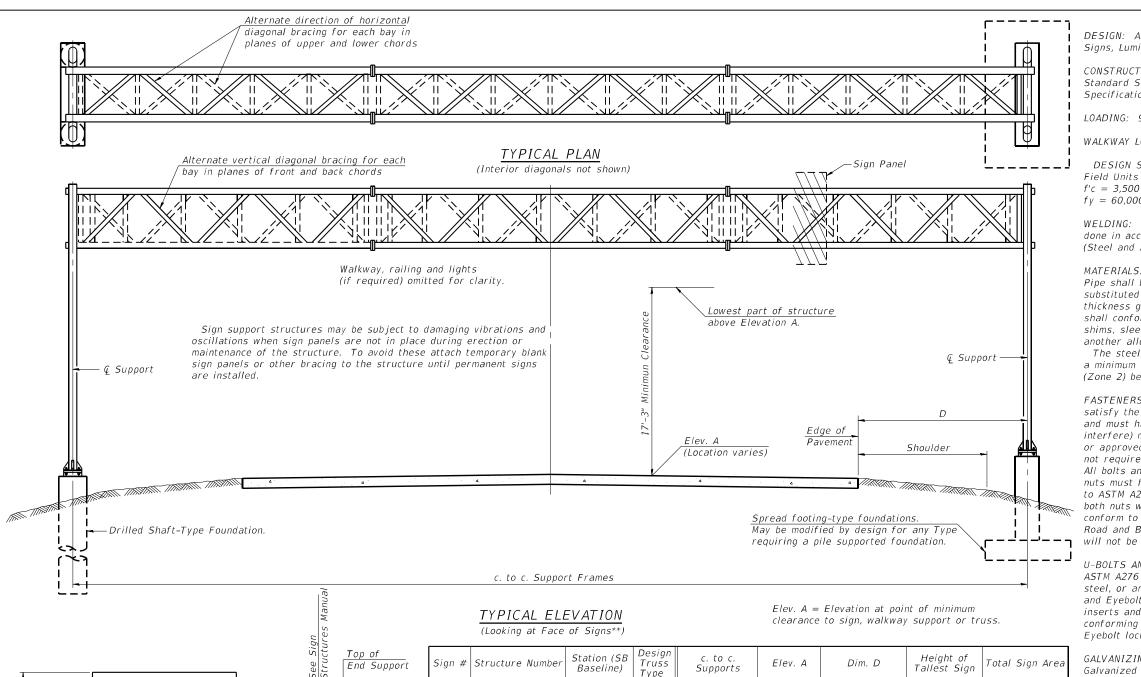
PROJECT	PTB 185-012, W	/O #32													
LOCATION	I-90 & I-94 Tollw	/ay													
COUNTY	Cook DF	RILLING	ME	тно	D _			HSA/Rotary	_ HAMMER T	YPE	(CME	Auto	mati	<u>c</u>
Northing Easting			D E P T H	B L O W S	U C S Qu	M O I S T	DRY DWZS-TY (pcf)	Upon Completion		_ ft _ ft	D E P T H	B L O W S	U C S Qu (tsf)	M O I S T	DEY DWZW-FY (pcf)
10.0" CONCR	RETE			,,,,	()	(() ·)	. ,	CLAY LOAM-brown &	gray-stiff to		_	(, 0)	()	17	
CRUSHED S	TONE	605.1	-	5				very stiff (continued)				4			
SCREENING	S-loose (Fill)		=	7		4					-		3.30	19	
			_	2							_	9	В		
% CLAY LOAM-	dark brown &	602.9						CLAY-gray-medium s	tiff	582.9					
gray-very stiff				3				,				3			
0000			_	4 5	2.00 B	23						2	0.80 B	18	
979-B			5								25				
38/190				4								2			
9019			_		4.60	17					-		0.90	24	\vdash
SORIN				7	В							3	В		
79-B															
4/190				3							_	3			
862K			10	11	3.40 B	14				575.9		2 1	0.80 B	22	
62K73			10		_			End Of Boring @ -30.	0'. Boring	373.8	-30		_		
CTS				6				backfilled with cuttings	S.						
ROA			_	1	3.00	18					-				
ZEDY			<u>-</u>	7	В										
SANDY CLAY	LOAM-dark brown	592.9													
∜ & black-stiff (F				5											
N-200			_	7	1.00 P	21					_				
1192		590.4	15								35				
CLAY LOAM	with Stone-gray-stiff			,											
표 (m)			-	4	1.50	22					*				
19079				6	Р										
CLAY LOAM-1 gray-very stifff CLAY LOAM-100-100-100-100-100-100-100-100-100-10	brown & gray-stiff to	587.9													
very stiff	Sionii a giay-siii to		-	3							-				
ROJE					1.00	23									
Z:\F			-20	2	Р						-40				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



USER NAME =	DESIGNED - SK	REVISED -
	CHECKED - MI	REVISED -
PLOT SCALE =	DRAWN - SK	REVISED -
PLOT DATE =	DATE - 9/9/202	22 REVISED -

FILE NAME: P:\2004-825 P



Sign #	Structure Number	Station (SB Baseline)	Design Truss Type	c. to c. Supports	Elev. A	Dim. D	Height of Tallest Sign	Total Sign Area
Sign 16	15016I094R049.6	344+53.87	III-A	79'-0"	580.29	16'-9 3/4"	7'-11"	231 Sq. Ft.
Sign 12	1S016I094R045.4	568+94.52	III-A	81'-0"	597.18	9'-5"	11'-3 5/8"	597.3 Sq. Ft.
Sign 14	1S016I094R044.6	611+60.67	III-A	73'-6"	607.27	16'-4 1/8"	7'-11"	231 Sq. Ft.

**Looking upstation for structures with signs both sides.

* If M270 Gr. 50W (M222) steel is proposed, chemistry for plate to be used shall first be approved by the Engineer as suitable for galvanizing and welding.

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Concrete Barrier Removal	Foot	130
Concrete Barrier Transition	Foot	83
Overhead Sign Structure – Span, Type III-A (5'-0" X 7'-0")	Foot	234
Overhead Sign Structure Walkway, Type A	Foot	119
Drilled Shaft Concrete Foundations	Cu Yd	142.4
Remove Overhead Sign Structure - Span	Each	3

GENERAL NOTES

DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions. ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

DESIGN STRESSES:

f'c = 3.500 p.s.i.

fy = 60,000 p.s.i. (reinforcement)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Specificiations.

MATERIALS: Aluminum Alloys as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer.

The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Evebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to ASTM F1554 Gr. 105.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final ground line at each foundation shall be cleaned and coated with Concrete Sealer in accordance with the Standard Specifications.

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

FOUNDATIONS: The contract unit price for Concrete Foundations and Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.



SIGNED DR. MOUSSA A. ISSA, S.E., IL. LIC. NO. 081-005738 EXPIRES 11-30-2024

12/05/2022

FOR SHEETS OSG2-01 THRU OSG2-25 (TOTAL OF 25 SHEETS)

analysis for all components.

10 p.s.i

DES**I**GNED - SK REVISED CHECKED - MI, LAB REVISED REVISED REVISED . - 12/5/2022

34'-0", max Type III-A

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

OVERHEAD SIGN STRUCTURES - GENERAL PLAN & ELEVATION - ALUMINUM TRUSS & STEEL SUPPORTS

SHEET OSG2-01 OF OSG2-25 SHEETS

SECTION COUNTY 90/94 2020-004-BR COOK 1492 350 CONTRACT NO. 62K74

10 p.s.f

30 p.s.f. (See Sign Structures

lanual for max. sign areas

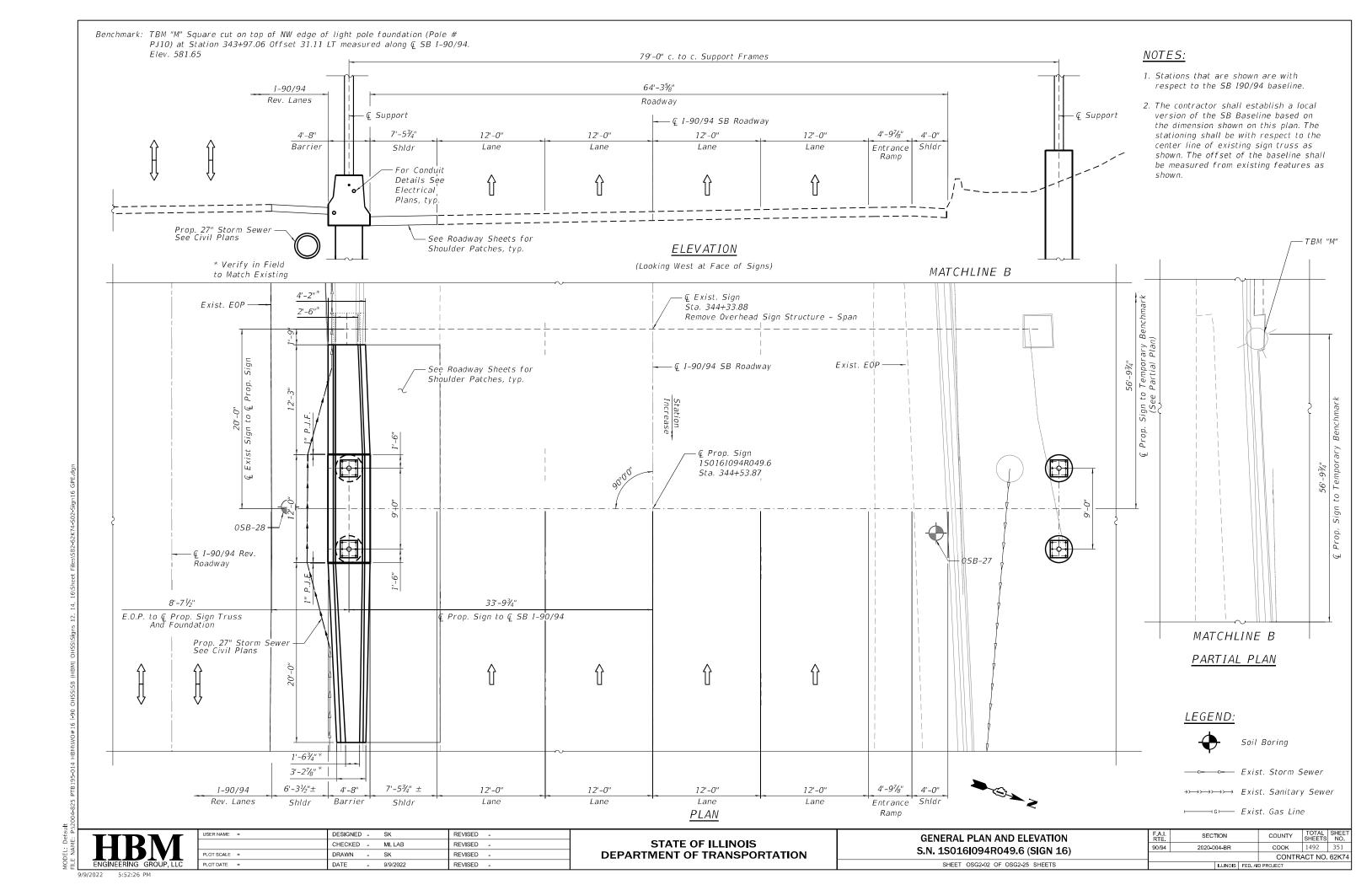
Maximum Length c. to c. Support Frames

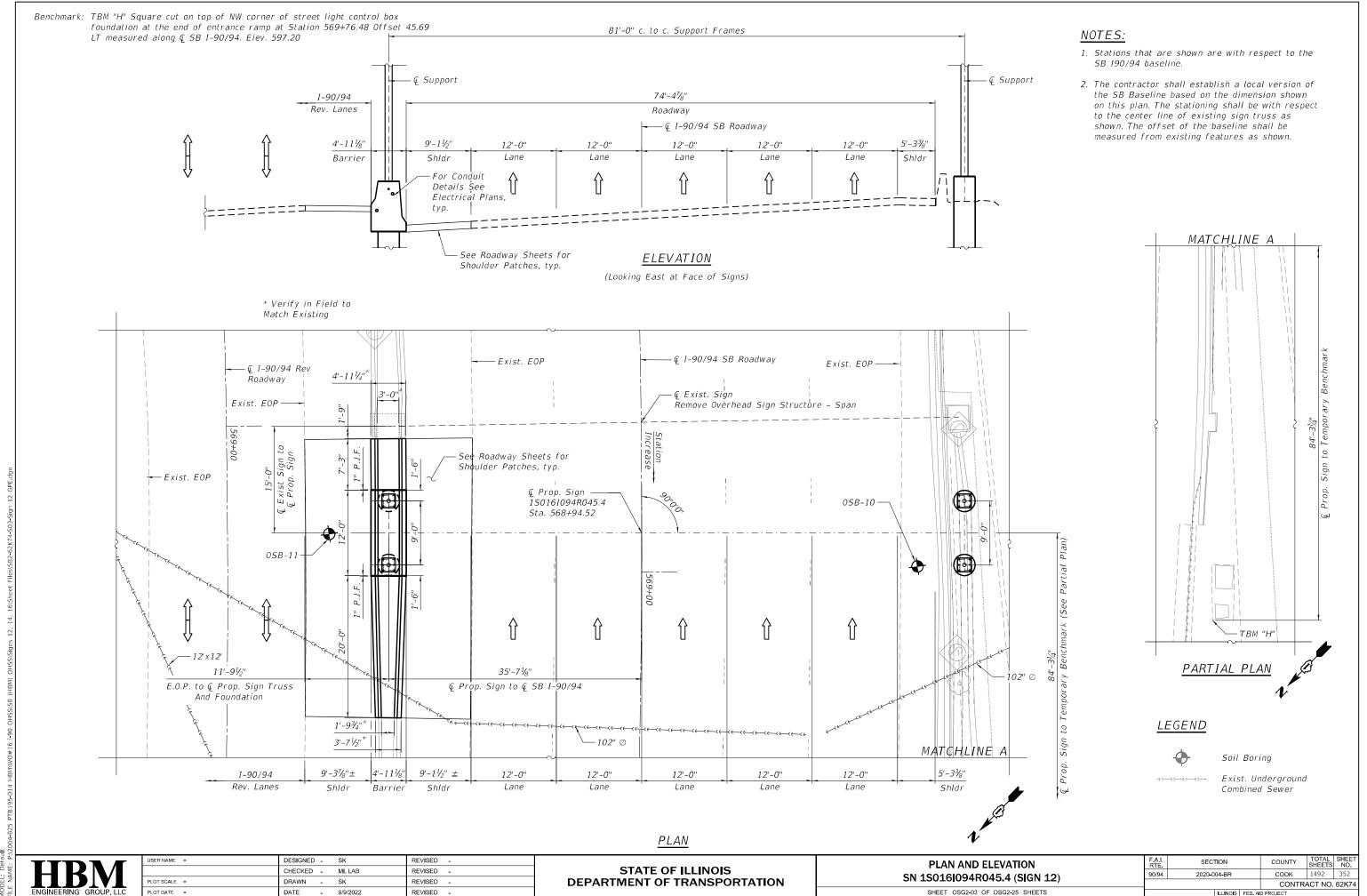
(See Sign Structures Manual)

DESIGN WIND LOADING DIAGRAM

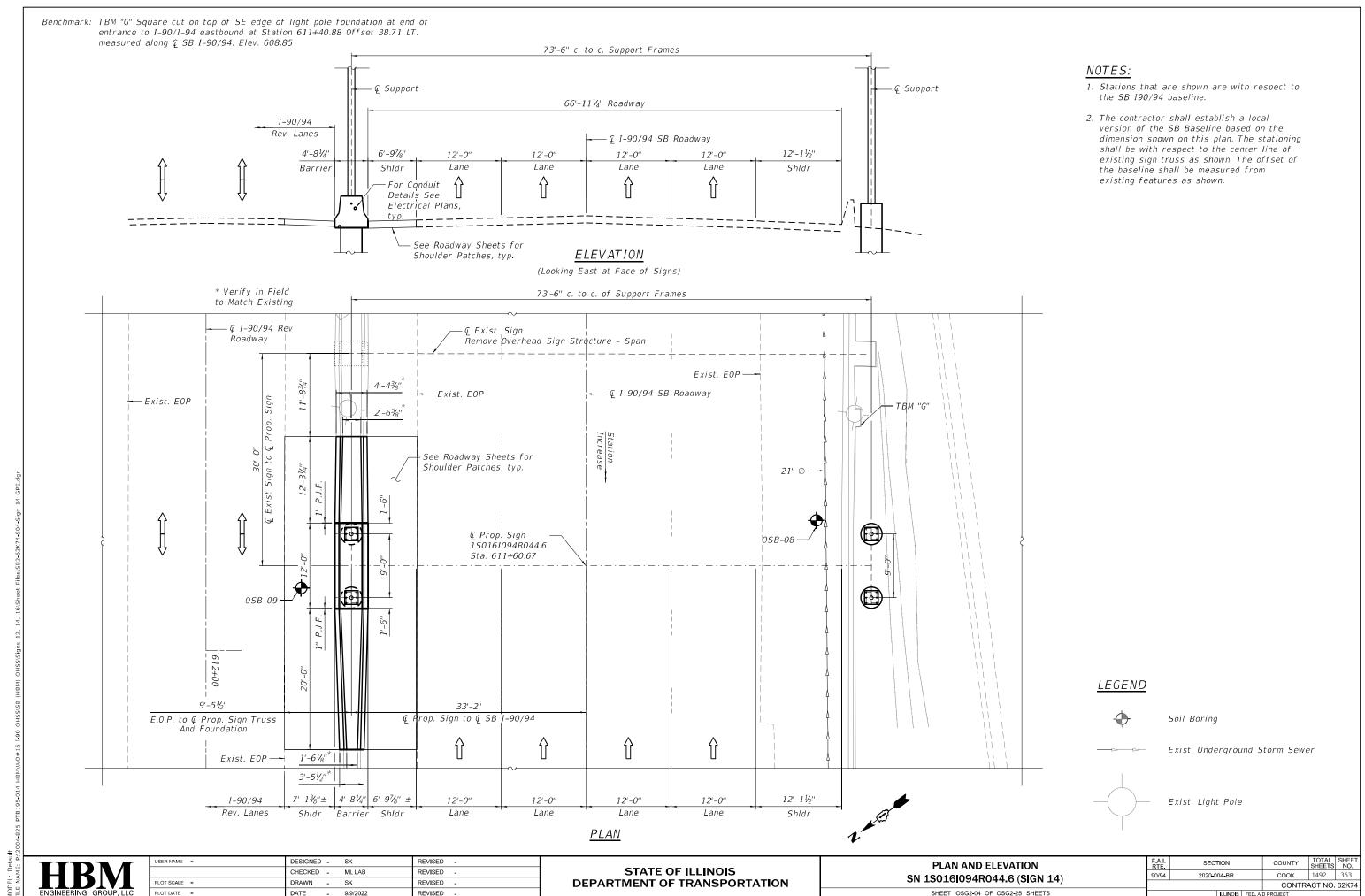
2-17-2017

Parameters shown are basis for I.D.O.T. Standards and Sign Manual Tables. Installations not within dimensional limits shown require special

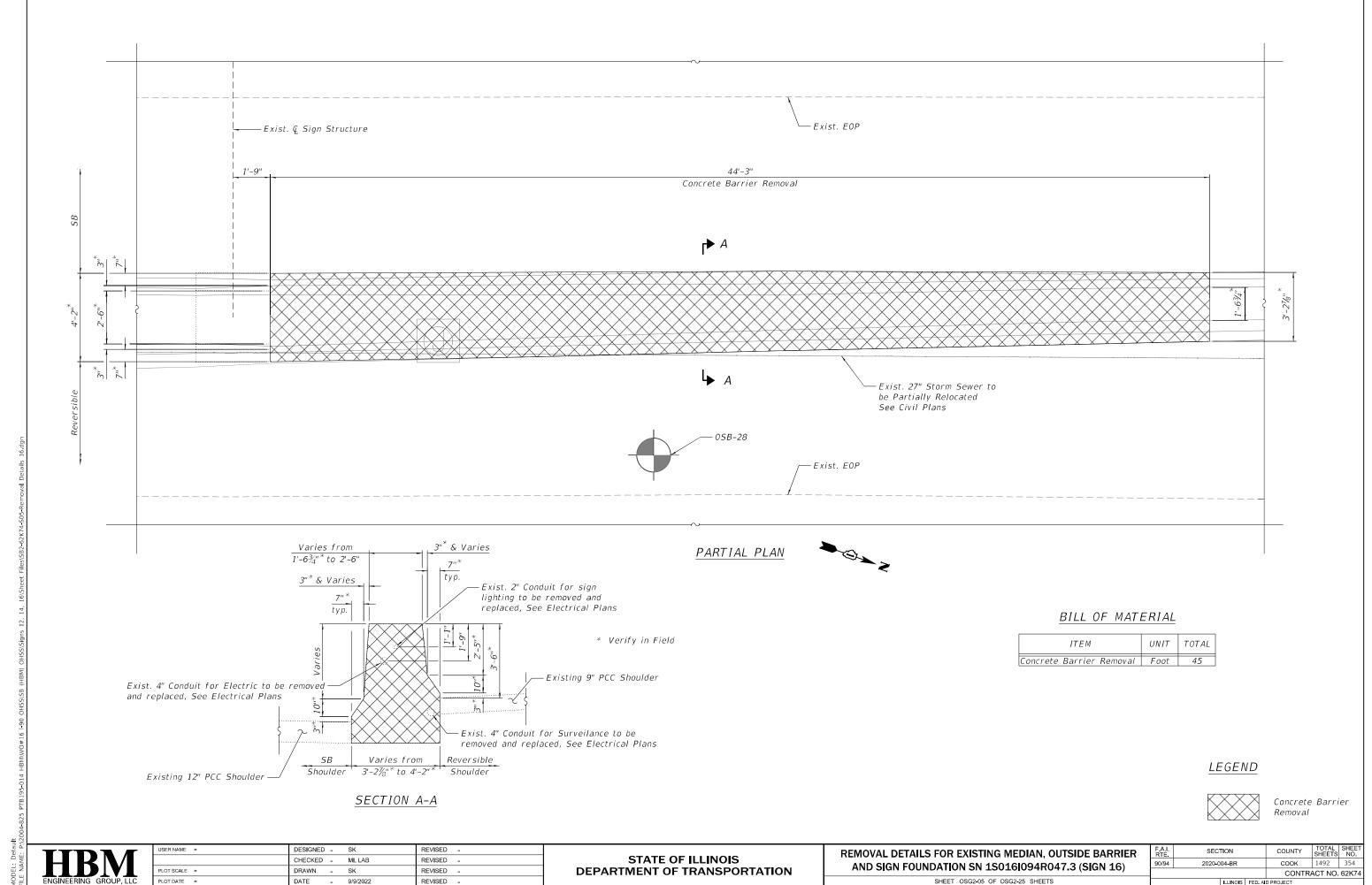




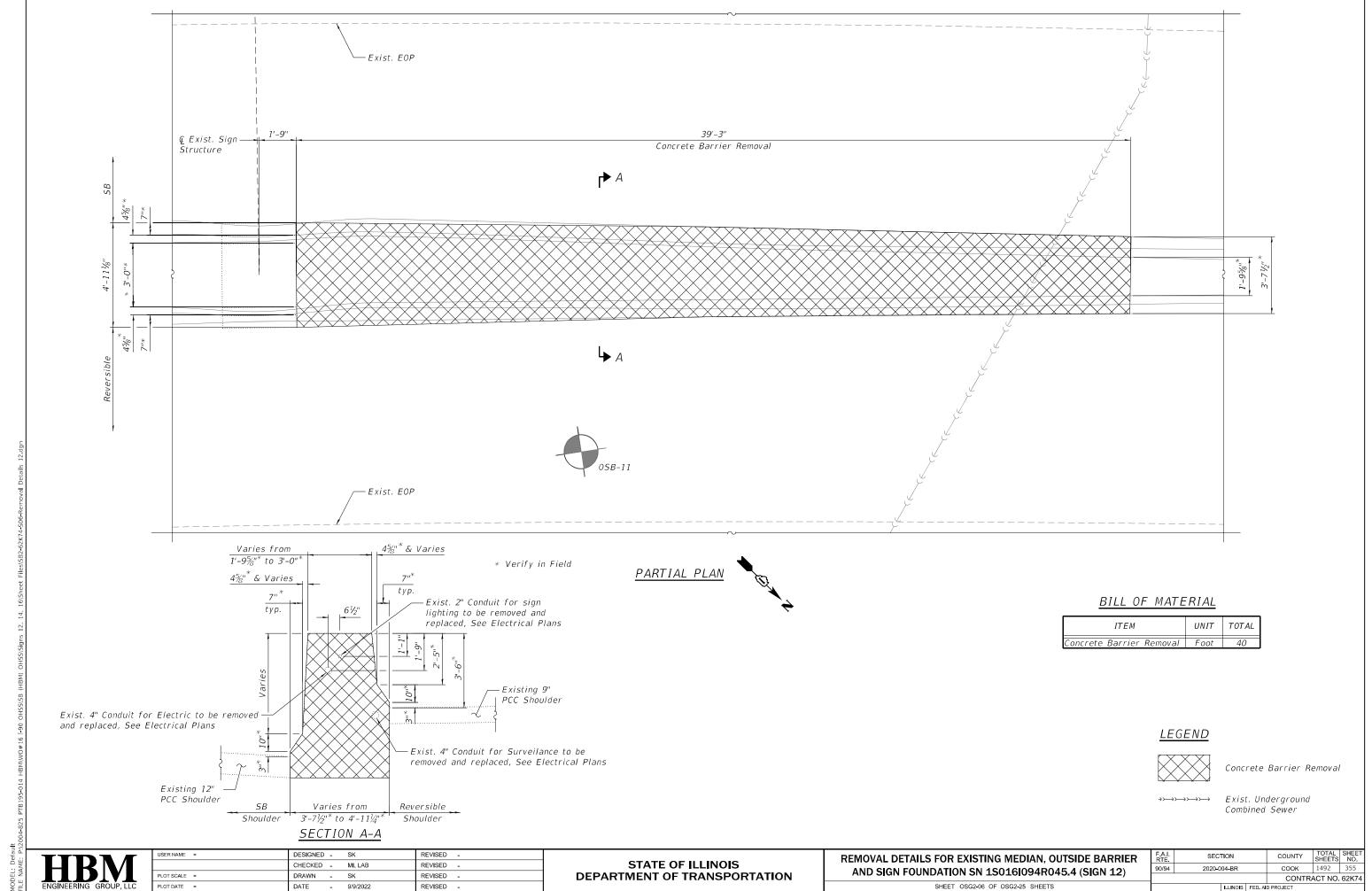
9/9/2022 5:52:28 PM



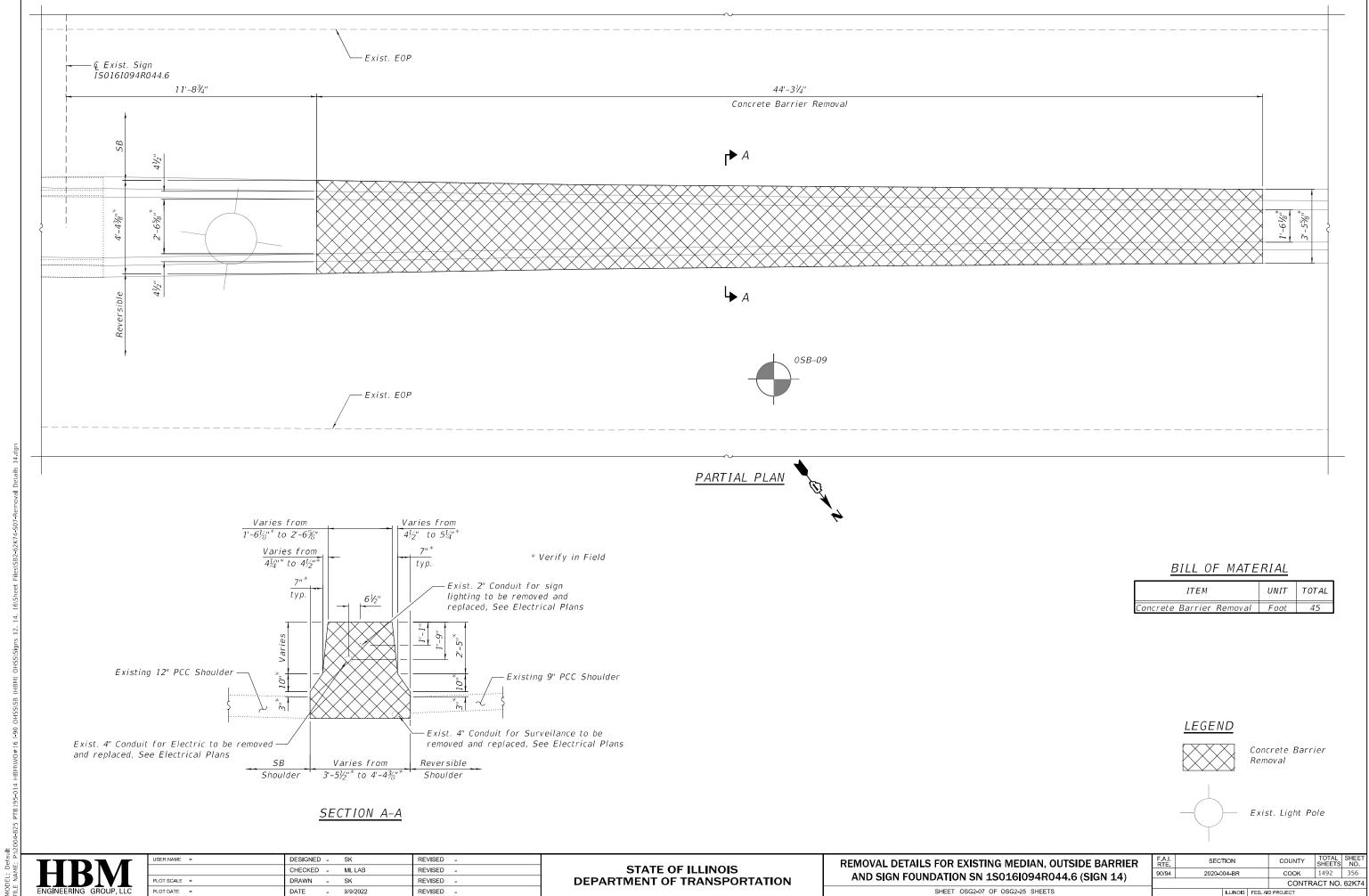
9/9/2022 5:52:31 PM



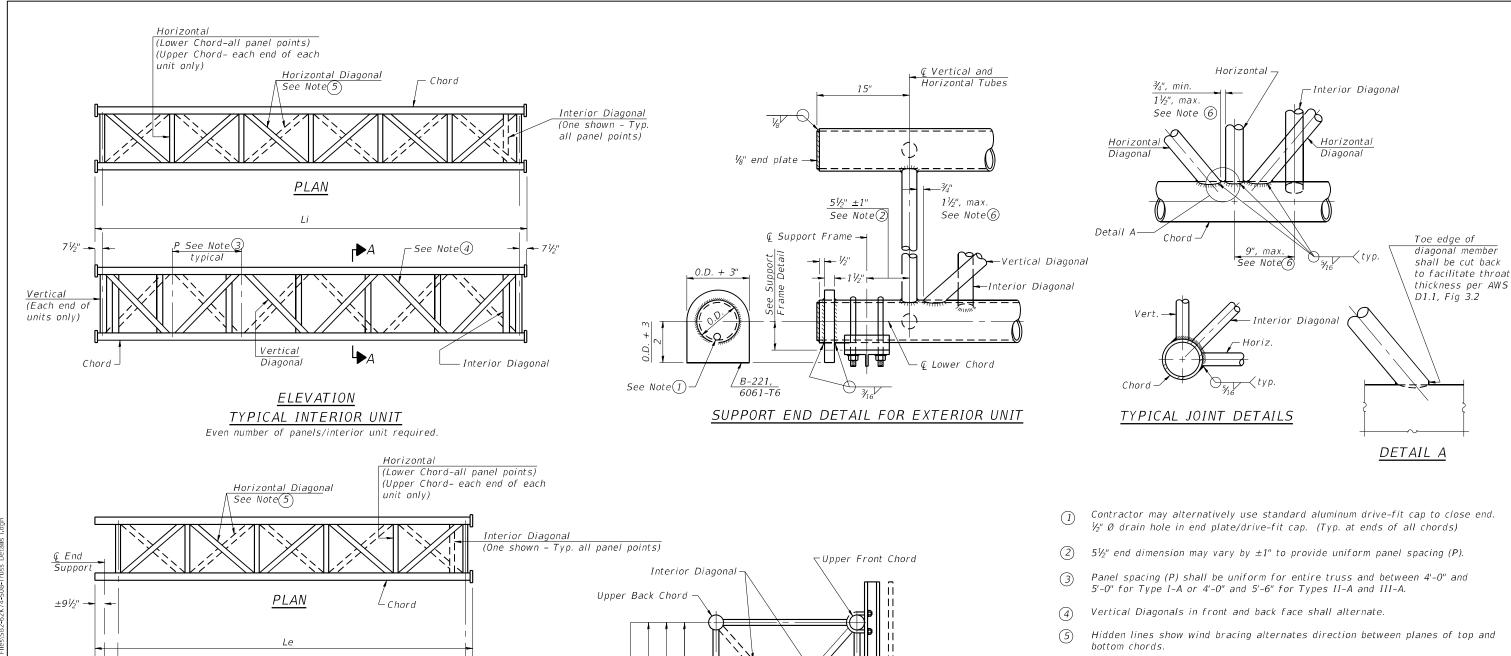
9/9/2022 5:52:34 PI



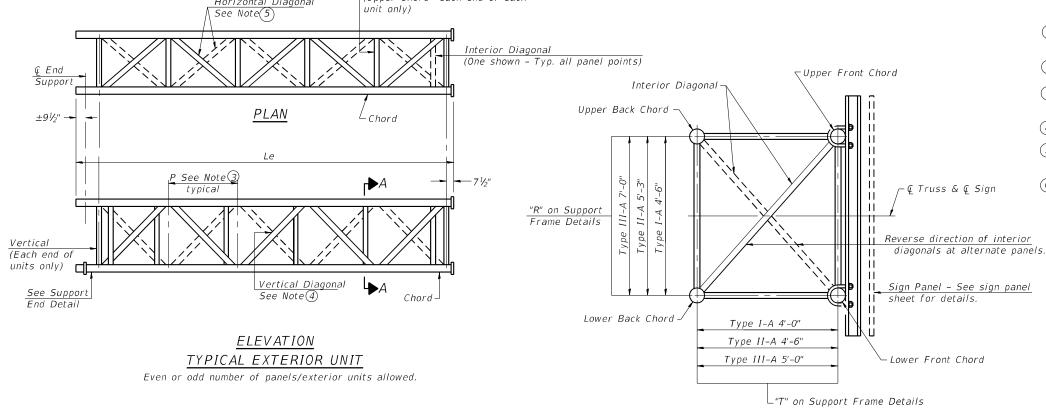
9/9/2022 5:52:36 PM



9/9/2022 5:52:38 PM



All diagonals shall be detailed for minimum offset from the panel point based on the following: Offset shall be such as to provide a $\frac{3}{4}$ " minimum to $1\frac{1}{2}$ " maximum clearance between any diagonal and any horizontal or vertical member, and to provide clearance for U-bolt connections of signs or walkway brackets.



SECTION A-A

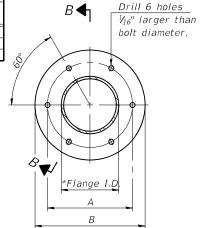
2-17-2017

DESIGNED - SK REVISED CHECKED - MI, LAB REVISED REVISED REVISED . 9/9/2022

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **OVERHEAD SIGN STRUCTURES - ALUMINUM TRUSS DETAILS FOR TRUSS TYPE III-A** SHEET OSG2-08 OF OSG2-25 SHEETS

SECTION COUNTY 90/94 2020-004-BR COOK 1492 357 CONTRACT NO. 62K74

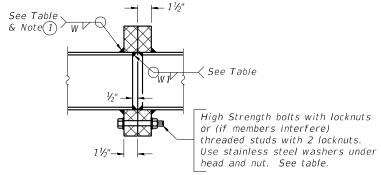
Sign #	Structure Number	Station	Design Truss Type	1	Exterior Units (2	?)		Interior Unit			Verticals; Upper & Lower Chord Horizontal, Horizontal, and Interior Diagonals Verticals; Camber at Midspan		Splicing Flange								
				No. Panels per Unit	Unit Lgth. (Le)	Panel Lgth. (P)	No. Req'd		Unit Lgth. (Le)	Panel Lgth. (P)	0.D.	Wall	0.D.	Wall		Bolt No./Splice	ts Dia.	Weld S	Sizes W1	А	В
Sign 16	150161094R049.6	344+53.87	III-A	5	28'-10 1/4"	5'-4 3/4"	1	4	22'-10"	5'-4 3/4"	7"	5/16"	3 1/4"	5/16"	0.94"	6	1"	7/16"	5/16"	11 1/2"	15"
Sign 12	2 1S016I094R045.4	568+94.52	III-A	5	26'-1 3/4"	4'-10 1/4"	1	6	30'-4 1/2"	4'-10 1/4"	7"	5/16"	3 1/4"	5/16"	0.97"	6	1"	7/16"	5/16"	11 1/2"	15"
Sian 14	1 150161094R0446	611+60 67	III_Δ	7	37'-6 1/2"	5'-1 1/8"	_	_	_	_	7"	5/16"	3 1/4"	5/16"	0.88"	6	1"	7/16"	5/16"	11 1/2"	15"



TRUSS TYPES I-A, II-A, & III-A

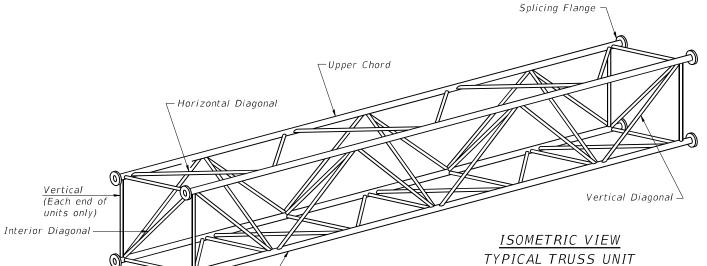
Drill 8 holes ⅓₁₆" larger than bolt diameter.

 $B \blacktriangleleft_1$



SECTION B-B

1) Splicing Flanges shall be attached to each truss unit with the truss shop assembled to camber shown. Truss units shall be in proper alignment and flange surfaces shall be shop bolted into full contact before welding. Sufficient external welds or tacks shall be made to secure flanges until remaining welds are made after disassembly. Adjacent flanges shall be "match marked" to insure proper field assembly.



Lower Chord

ASTM B221 Alloy 6061 Temper T6

Note: Units shall be shipped individually with adequate provision to prevent detrimental motion during transport. This may require ropes between horizontals and diagonals or energy dissipating (elastic) ties to the vehicle. The Contractor is responsible for maintaining the configuration and protection of the units.

TRUSS TYPES II-A & III-A SPLICING FLANGES

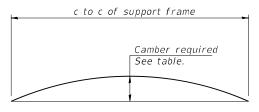
*Flange I.D.

Bolt Circle Ø =

Flange O.D. = B

ASTM B221, Alloy 6061-T6 or ASTM B209, Alloy 6061-T651

*To fit O.D. of Chord with maximum gap of $\frac{1}{16}$ ".



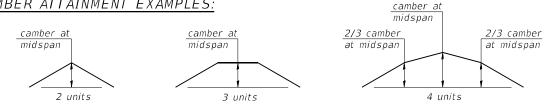
(Upper Chord - each end of each unit only)

(Lower Chord - all panel points)

CAMBER DIAGRAM

Camber curve shown is theoretical. Actual camber attained by slope changes at splices between units.

CAMBER ATTAINMENT EXAMPLES:

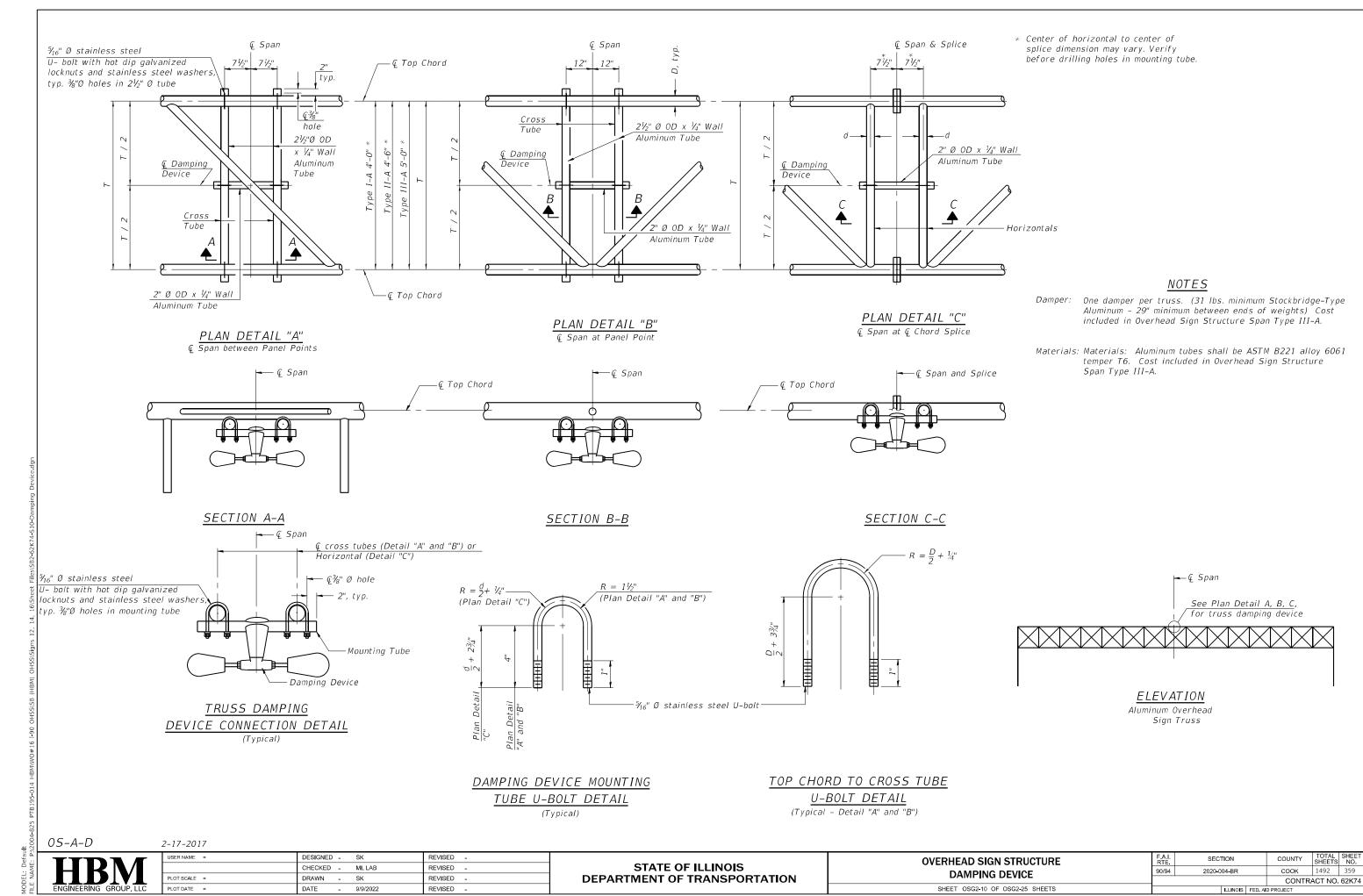


Camber shown is for fabrication only, measured with truss fully supported. (No-load condition)

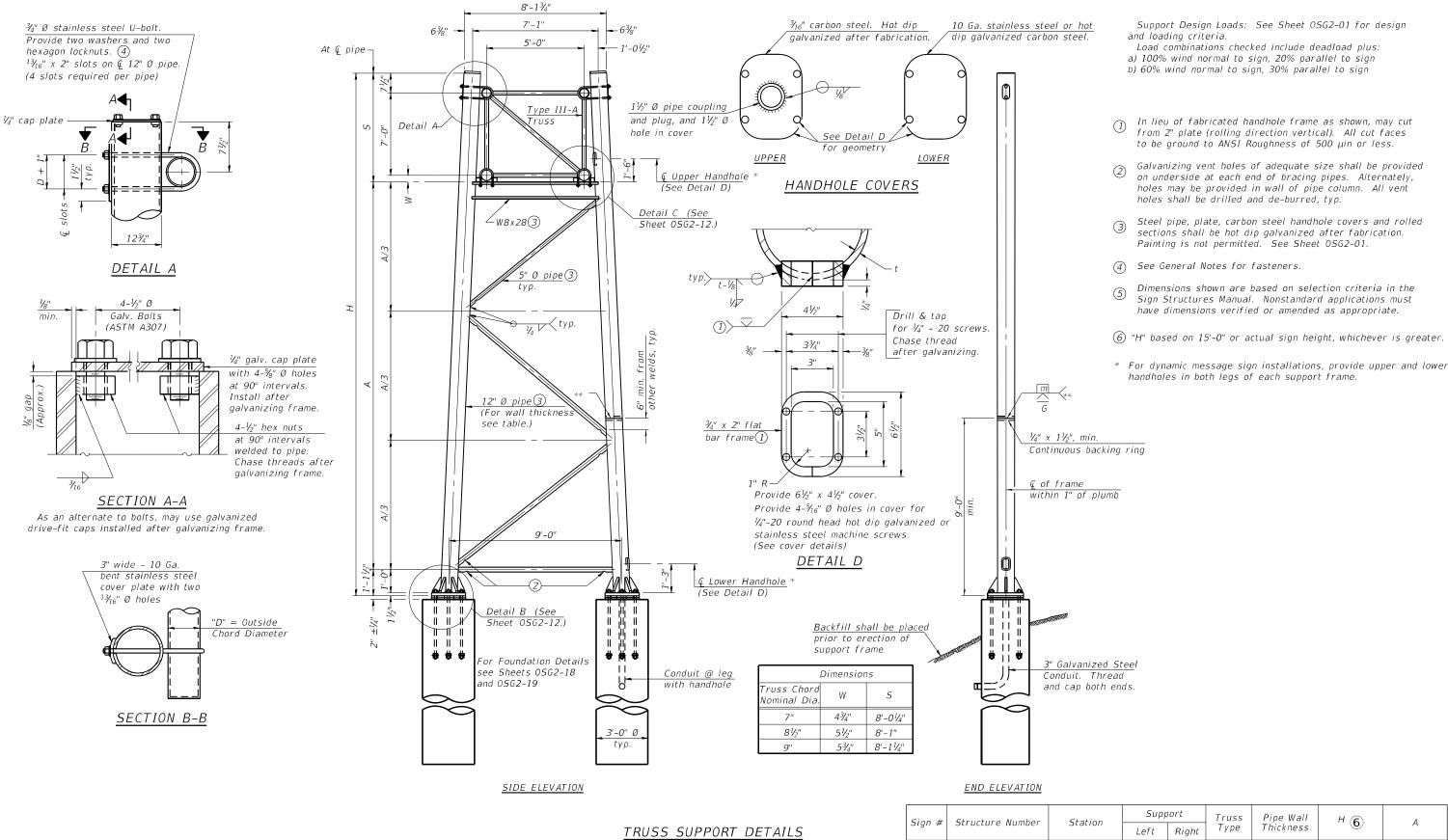
2-17-2017

USER NAME =	DESIGNED	-	SK	REVISED	-
	CHECKED	-	MI, LAB	REVISED	-
PLOT SCALE =	DRAWN	-	SK	REVISED	-
PLOT DATE =	DATE	-	9/9/2022	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION** **OVERHEAD SIGN STRUCTURES - ALUMINUM TRUSS DETAILS** SECTION COUNTY 90/94 2020-004-BR COOK 1492 358 FOR TRUSS TYPES III-A CONTRACT NO. 62K74 SHEET OSG2-09 OF OSG2-25 SHEETS



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(12" Ø Pipe-Type III-A Truss

** One butt welded joint is allowed only on one post per support frame. If used, weld procedure must be preapproved by Engineer and joint shall receive 100% RT or UT (tension criteria) at Contractor's expense.

Sign #	Structure Number	Station	Support		Truss	Pipe Wall	H (6)	A
Jigii #	Structure Number	Station	Left	Right	Туре	Thickness		A
Cian 16	15016I094R049.6	344+53.87	X		III-A	0.33"	26'-4 1/4"	17'-2 1/2"
Sign 16				Χ	III-A	0.33"	23'-6 7/8"	14'-5 1/8"
Sign 12	1S016I094R045.4	568+94.52	X		III-A	0.33"	28'-1 1/2"	18'-11 7/8"
Sign 12				Χ	III-A	0.33"	27'-5 1/2"	18'-3 7/8"
Cian 11	1S016I094R044.6	611+60.67	X		III-A	0.33"	27'-0 5/8"	17'-10 7/8"
Sign 14				X	III-A	0.33"	28'-1 1/4"	18'-11 1/2"

054-A-8a

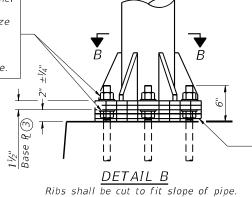
2-17-2017

DESIGNED - SK REVISED -CHECKED - MI, LAB REVISED -REVISED REVISED . - 9/9/2022

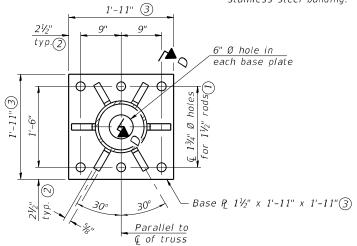
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES SUPPORT FRAME FOR TYPE III-A ALUMINUM TRUSS SHEET OSG2-11 OF OSG2-25 SHEETS

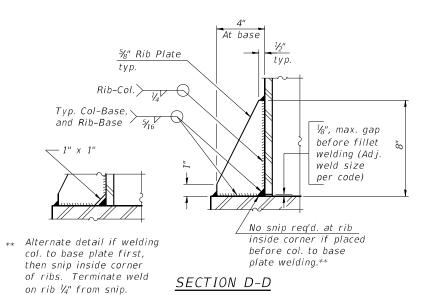
SECTION COUNTY 90/94 2020-004-BR COOK 1492 360 CONTRACT NO. 62K74

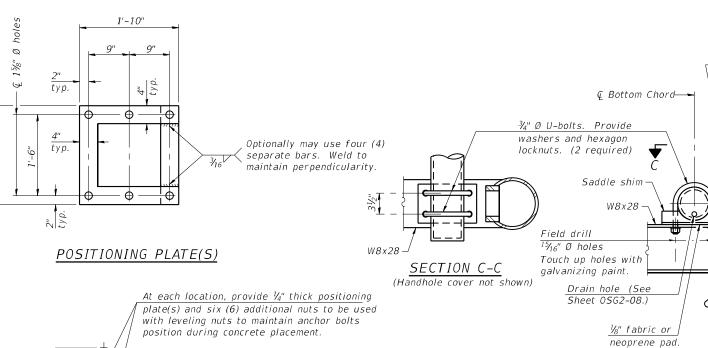


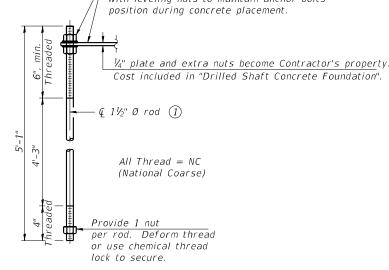
Stainless Steel Standard Grade Wire Cloth, 3" wide, 1/4" maximum opening with a minimum wire diameter of AWG. No. 16 with a minimum 2" lap. Secure to base plate after erection with ¾" stainless steel banding.



SECTION B-B







ANCHOR ROD DETAIL

Anchor rods shall conform to ASTM F1554 Grade 105 Galvanize upper 12" minimum per AASHTO M232. No welding shall be permitted on rods.

TYPE III-A TRUSS 12" Ø PIPE SUPPORT FRAME DETAILS

For Type III-A Truss spans greater than 150 ft, and up to 160 ft.:

- 1 1 ¾ g rod, 2" Ø holes
- (2) 2¾" edge distance
- ③ Base P₂ 15/8" x 1'-111/2" x 1'-111/2"

russ Chora

Iominal Dia.

81/2"

typ.

11/4"

For W, see Sheet #SAT2. SADDLE SHIM DETAIL ASTM B26 Alloy 356-F

 $*R = \frac{\nu}{2} + \frac{1}{32}$ at 90° D = Outside Diameter of Chord.

DETAIL C

 $D + 3\frac{1}{3}$

for U-bolts

ASTM B209 Alloy 6061-T651 (4 required per sign truss)

054-A-8aA

2-17-2017

HBM	
ENGINEERING GROUP, LLC	

USER NAME =	DESIGNED	-	SK	REVISED	-
	CHECKED	-	MI, LAB	REVISED	-
PLOT SCALE =	DRAWN	-	SK	REVISED	-
PLOT DATE =	DATE	-	9/9/2022	REVISED	-

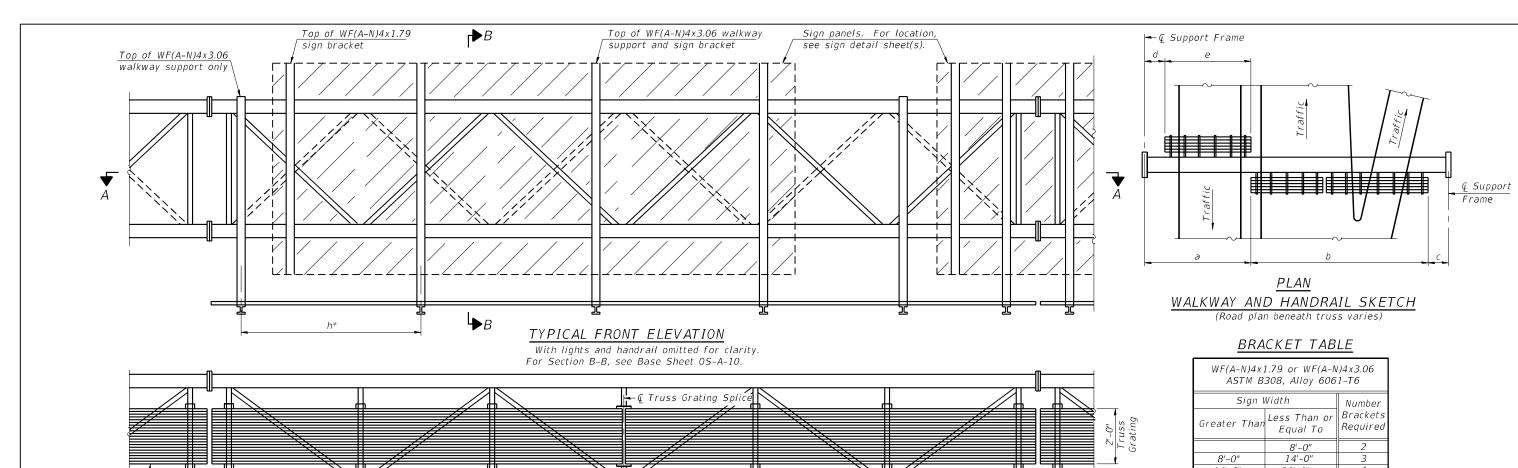
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

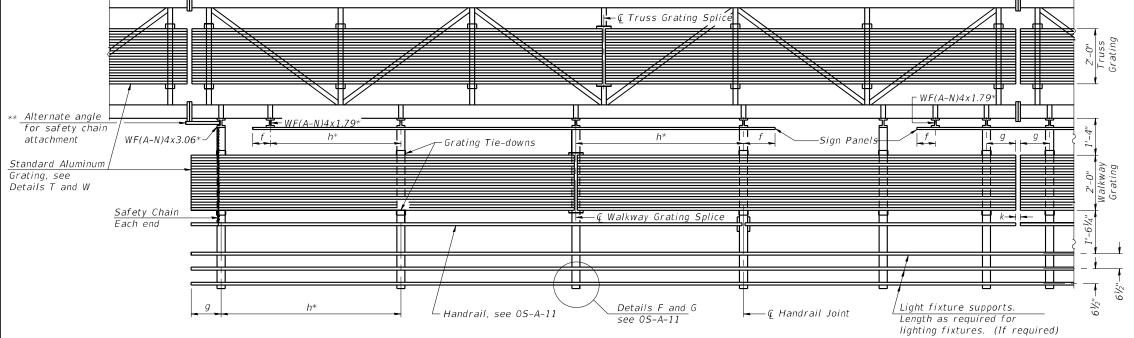
OVERHEAD SIGN STRUCTURES SUPPORT FRAME FOR TYPE III-A ALUMINUM TRUSS SHEET OSG2-12 OF OSG2-25 SHEETS

SECTION COUNTY 2020-004-BR COOK 1492 361 CONTRACT NO. 62K74

 $1\frac{1}{2}$ " Ø pipe coupling for conduit attachment (plug for shipping)

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SECTION A-A

Handrail and walkway shall span a minimum of three brackets between splices and/or gap joints. Place all sign and walkway brackets as close to panel points as practical. Handrail joints, grating, and light support splices placed as needed.

Sign #	Structure Number	Station	а	b	С	Ls	Walkway Grating and Handrail Lengths
Sign 12	1S016I094R045.4	568+94.52	1'-6"	17'-11 1/2"	20'-2 1/2"	29'-1"	39'-0"

	WF(A-N)4x1.79 or WF(A-N ASTM B308, Alloy 606							
Sign \	Nidth	Number						
Greater Than	Less Than or Equal To	Brackets Required						
	8'-0"	2						
8'-0"	14'-0"	3						
14'-0"	20'-0"	4						
20'-0"	26'-0"	5						
26'-0"	32'-0"	6						

- Space walkway brackets WF(A-N)4x3.06 and sign brackets WF(A-N)4x1.79 for efficiency and within limits shown:
- f = 12" maximum, 4" minimum (End of sign to Q of nearest bracket) g = 12" maximum, 4" minimum (End of walkway grating to Q of nearest support bracket)
- h=6'-0'' maximum ($\c to \c sign and/or walkway support brackets,$ WF(A-N)4x1.79 or WF(A-N)4x3.06)
- k = 2" maximum gap between adjacent walkway grating sections and handrail ends
- ** If walkway bracket at safety chain location is behind sign, add angle to bracket, see Alternate Safety Chain Attachment on Base Sheet OS-A-11.

For Details T and W, Section B-B and Grating Splice Details see Base Sheet OS-A-10.

For Handrail Details see Base Sheet OS-A-11.

Truss grating to facilitate inspection shall run full length (center to center of support frames) ± 12 " on overhead trusses. Cost of truss grating is included in "Overhead Sign Structure".

> Walkway and Truss Grating width dimensions are nominal and may vary $\pm \frac{1}{2}$ " based on available standard widths.

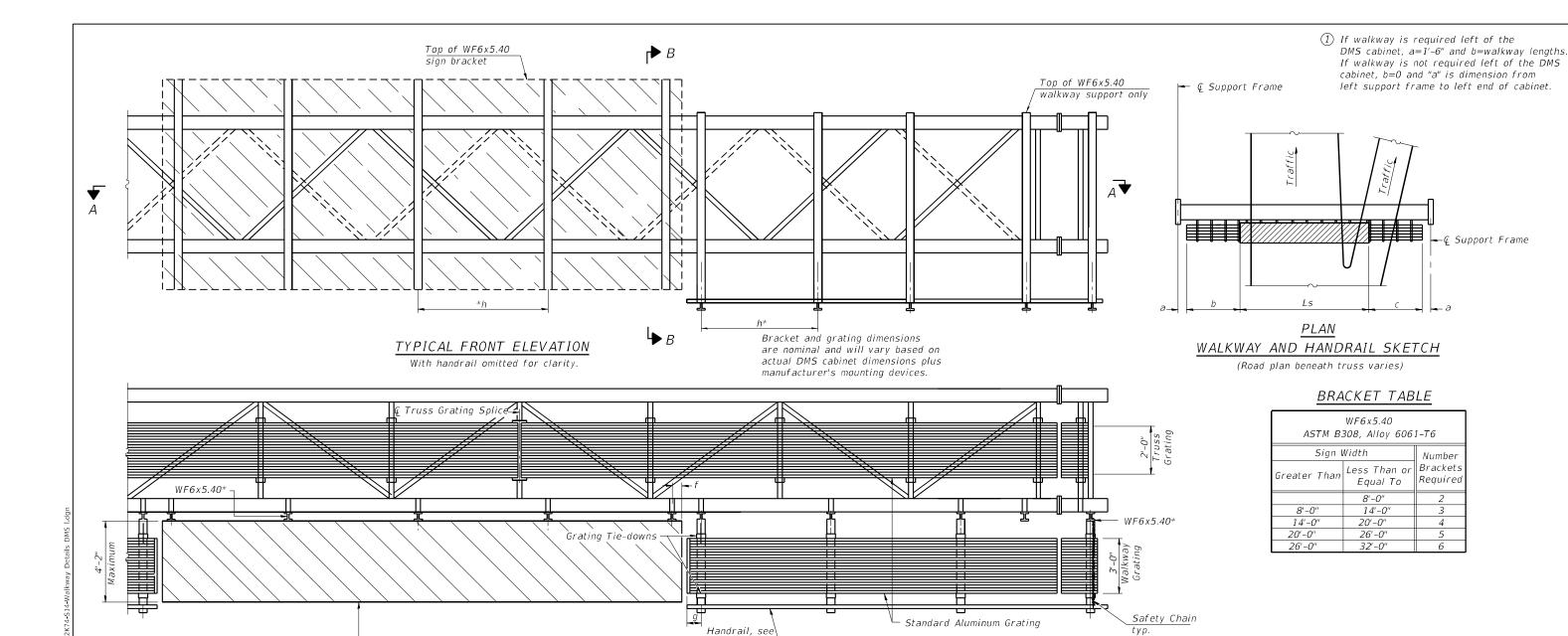
2-17-2017

USER NAME =	DESIGNED	-	SK	REVISED	-
	CHECKED	-	MI, LAB	REVISED	-
PLOT SCALE =	DRAWN	-	SK	REVISED	-
PLOT DATE =	DATE	-	9/9/2022	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES ALUMINUM WALKWAY DETAILS SHEET OSG2-13 OF OSG2-25 SHEETS

SECTION COUNTY 90/94 2020-004-BR COOK 1492 362 CONTRACT NO. 62K74



05-A-11-DMS

SECTION A-A

Handrail and walkway shall span a minimum of three brackets between splices and/or gap joints. Place all sign and walkway brackets as close to panel points as practical. Grating and handrail splices placed as needed.

	Sign #	Structure Number	Station	а	b	С	Ls	Walkway Grating and Handrail Lengths
Г	Sign 16	1S016I094R049.6	344+53.87	1'-6"	17'-11 1/2"	20'-2 1/2"	29'-1"	39'-0"
Г	Sign 14	150161094R044.6	611+60.67	1'-6"	17'-1 1/2"	23'-7"	29'-1"	41'-0"

Truss grating to facilitate inspection shall run full length

(center to center of support frames) ± 12 " on overhead trusses.

Cost of truss grating is included in "Overhead Sign Structure".

Notes:

* Space walkway brackets WF6x5.40 for efficiency and within limits shown:

f=12" maximum, 4" minimum (End of sign to \mathbb{Q} of nearest bracket)

g=12" maximum, 4" minimum (End of walkway grating to $\mbox{\it Q}$ of nearest support bracket)

h=6'-0" maximum (Q to Q sign and/or walkway support brackets, WF6x5.40

Maximum DMS weight = 5000 lbs. 4'-2'' maximum cabinet depth includes depth of cabinet plus connection to WF6x5.40.

For Section B-B and Grating Splice Details, see Sheet 0SG2-16. For Handrail Splice Details, see Sheet 0SG2-17.

05-4-9-DMS

2-17-2017



Walkway and Truss Grating

width dimensions are nominal

and may vary $\pm \frac{1}{2}$ " based on

available standard widths.

USER NAME =	DESIGNED	-	SK	REVISED	-
	CHECKED	-	MI, LAB	REVISED	-
PLOT SCALE =	DRAWN	-	SK	REVISED	-
PLOT DATE =	DATE	-	9/9/2022	REVISED	-

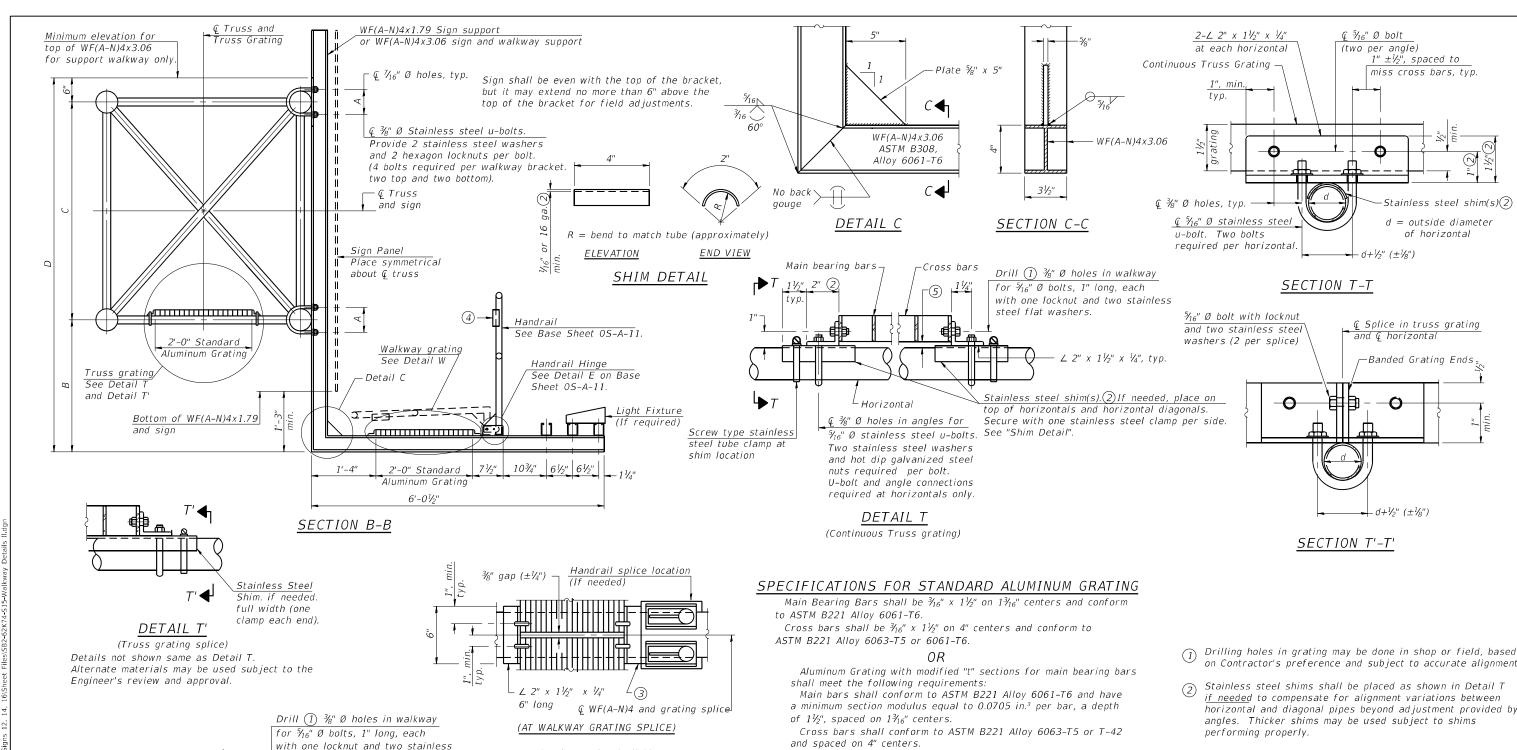
└ Dynamic Message Sign Cabinet

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES
ALTERNATE ALUMINUM WALKWAY DETAILS FOR DMS I

SHEET OSG2-14 OF OSG2-25 SHEETS

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and spaced on 4" centers.

- on Contractor's preference and subject to accurate alignment.
- horizontal and diagonal pipes beyond adjustment provided by
- $\stackrel{\frown}{3}$ If Handrail Joint present, weld angle to WF(A-N)4 and $\stackrel{\lor}{4}$ " extension bars. (See Base Sheet OS-A-11.)
- (4) P₁ 1/8" x 1/2" x 2" welded to handrail posts to protect locations that contact grating.
- 5 Tube to grating gap may vary from 0 to ½", max. to align walkway, allow for camber, etc.
- 6 Based on actual height of tallest sign given on OS-A-1.

Sign #	Structure Number	Station	Α	⑥ ^B	С	6 D
Sign 12	15016I094R045.4	568+94.52	7 3/8"	3'-4 3/4"	7'-0"	10'-10 3/4"

2-17-2017

Grating width plus 1/8", 2"

DETAIL W

(Walkway grating)

DESIGNED - SK REVISED CHECKED - MI, LAB REVISED . REVISED REVISED -- 9/9/2022

 $2\frac{1}{2}$ " long at continuous grating,

6" long at grating splices.

steel flat washers

L 2" x 1½" x ¼"

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

Continuous handrail hinge

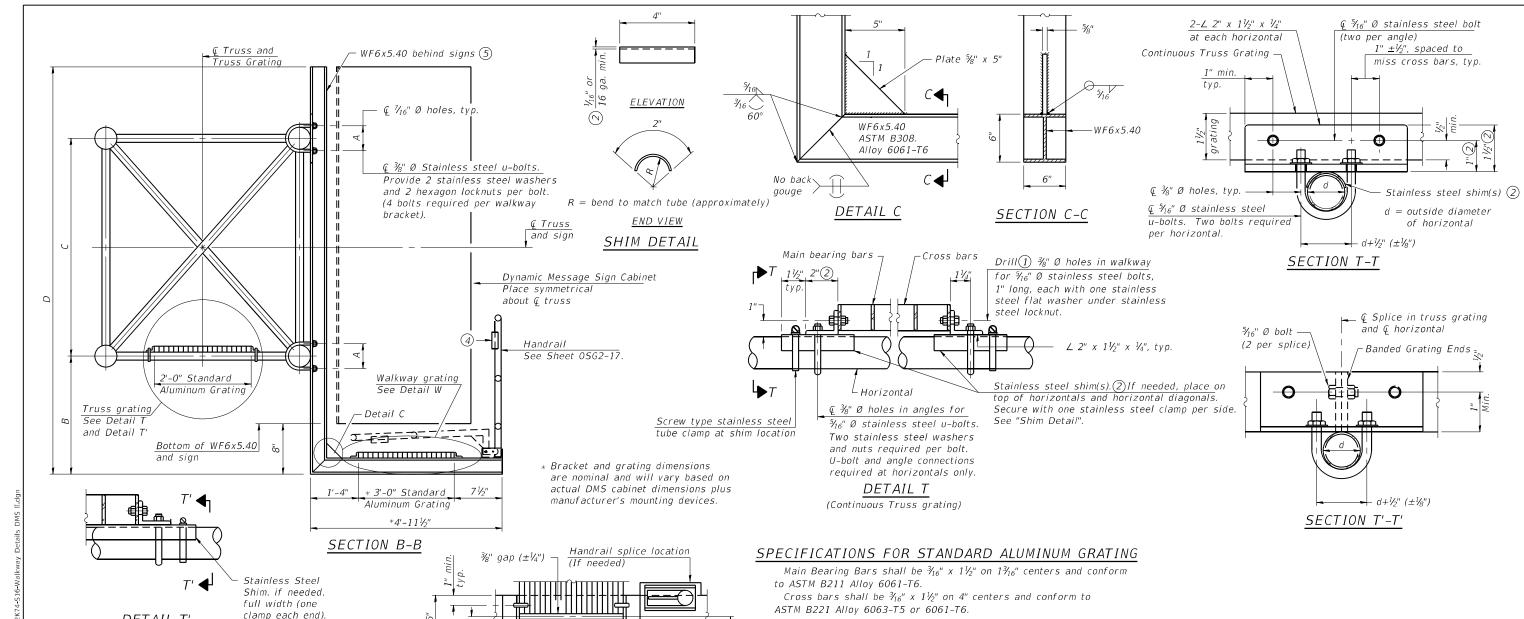
(CONTINUOUS WALKWAY GRATING)

SECTION W-W

 $\angle 2'' \times 1\frac{1}{2}'' \times \frac{1}{4}''$

OVERHEAD SIGN STRUCTURES ALUMINUM WALKWAY DETAILS SHEET OSG2-15 OF OSG2-25 SHEETS

SECTION COUNTY 90/94 2020-004-BR COOK 1492 364 CONTRACT NO. 62K74



ASTM B221 Alloy 6063-T5 or 6061-T6.

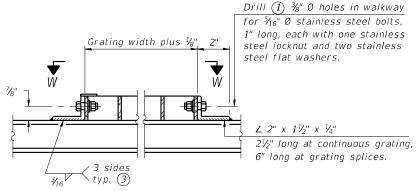
Aluminum Grating with modified "t" sections for main bearing bars shall meet the following requirements:

Main bars shall conform to ASTM B221 Alloy 6061-T6 and have a minimum section modulus equal to 0.0705 in.3 per bar, a depth of $1\frac{1}{2}$ ", spaced on $1\frac{3}{16}$ " centers.

Cross bars shall conform to ASTM B221 Alloy 6063-T5 or T-42 and spaced on 4" centers.

- (1) Drilling holes in grating may be done in shop or field, based on Contractor's preference and subject to accurate alignment.
- ② Stainless steel shims shall be placed as shown in Detail T if needed to compensate for alignment variations between horizontal and diagonal pipes beyond adjustment provided by angles. Thicker shims may be used subject to shims performing properly.
- $\stackrel{\textstyle \frown}{\mbox{\footnotesize (3)}}$ If Handrail Joint present, weld angle to WF(A-N)4 and ${\mbox{\it V}}_4{\mbox{\it "}}$ extension bars. (See Base Sheet OS-A-11.)
- (4) PL %" x 1/2" x 2" welded to handrail posts to protect locations that contact grating.
- (5) Cabinet manufacturer must design and supply hardware for connection of cabinet to WF6's. Bolts must be stainless steel or hot dip galvanized high strength per IDOT specifications.
- (6) Based on actual height of tallest sign given on Sheet 0SG2-01.

Sign #	Structure Number	Station	Α	⑥ ^B	С	⑥ D
Sign 16	15016I094R049.6	344+53.87	7 3/8"	1'-1 1/2"	7'-0"	8'-7"
Sign 14	1S016I094R044.6	611+60.67	7 3/8"	1'-1 1/2"	7'-0"	8'-7"



DETAIL W (Walkway grating)

DETAIL T'

Engineer's review and approval.

Details not shown same as Detail T.

Alternate materials may be used subject to the

(Truss grating splice)

0S-A-10-DMS

-A-10-DM3	2-17-2017			
	USER NAME =	DESIGNED -	SK	REVISED -
		CHECKED -	MI, LAB	REVISED -
	PLOT SCALE =	DRAWN -	SK	REVISED -
IGINEERING GROUP, LLC	PLOT DATE =	DATE -	9/9/2022	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

and grating

splice

 \angle 2" x 1\frac{1}{2}" x \frac{1}{4}"

∠ 2" x 1½" x ¼"

21/5" Iona

(AT WALKWAY GRATING SPLICE)

(CONTINUOUS WALKWAY GRATING)

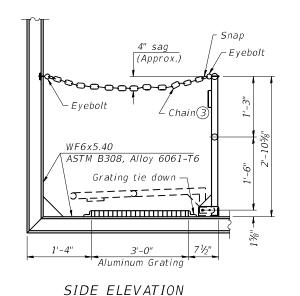
SECTION W-W

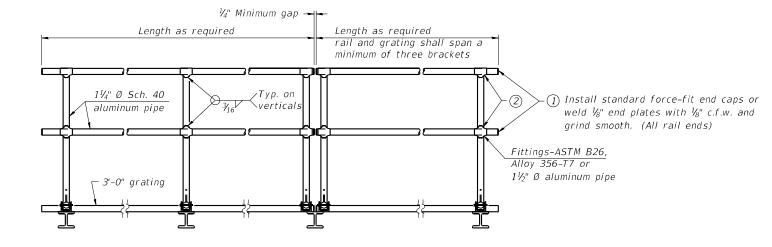
Continuous handrail hinge

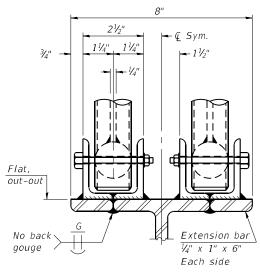
SECTION COUNTY **OVERHEAD SIGN STRUCTURES** 90/94 2020-004-BR COOK 1492 365 ALTERNATE ALUMINUM WALKWAY DETAILS FOR DMS II CONTRACT NO. 62K74 SHEET OSG2-16 OF OSG2-25 SHEETS

ENG

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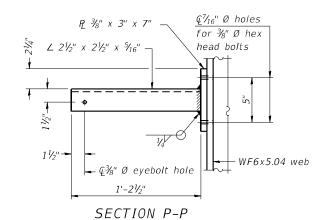


ELEVATION AT HANDRAIL JOINT 4

(Showing safety chain w/o sign)

HANDRAIL DETAILS

Handrail pipe shall be ASTM B241, Alloy 6063-T6 or Alloy 6061-T6.



Drill and ream for $\frac{3}{8}$ " Ø stainless steel bolt with washer and hexagon locknut.

PLAN

DETAIL E HANDRAIL HINGE

2-17-2017

Drill 5/16" Ø hole for

Etyp.

¼" Ø ring-grip quick release

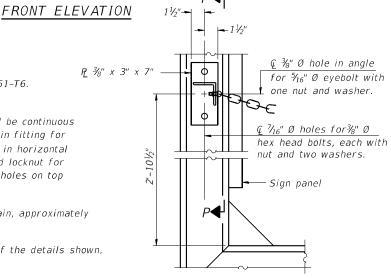
self-locking stainless steel pin

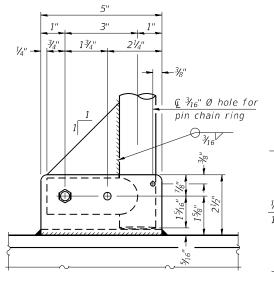
(2) Horizontal handrail member shall be continuous thru fitting. Provide V_{16} " Ø hole in fitting for %" Ø bolt. Field drill V_{16} " Ø hole in horizontal rail member. Provide washer and locknut for bolt. (Use V_{16} " eyebolts in V_{16} " Ø holes on top rail at ends only.)

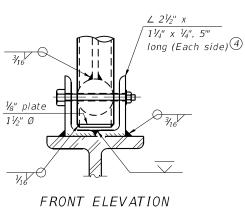
PLAN AT HANDRAIL JOINT

Details not shown same as "PLAN"

- 3 y_{16} " type 304L stainless steel chain, approximately 12 links per foot.
- Extrusions may be used in lieu of the details shown, with approval of the Engineer.







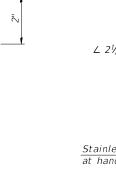
<u>SIDE ELEVATION</u>

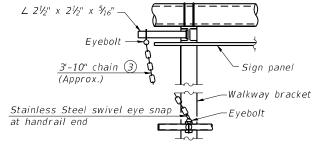
See "ELEVATION" at right for dimensions.

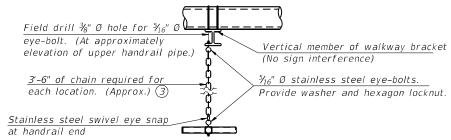
ALTERNATE SAFETY CHAIN ATTACHMENT

(With Sign Present)
Items not shown same as "Side Elevation" of "Handrail Details"

Items not shown same as "Side Elevation" of "Ha1/2" x $2^{1}/2$ " x 5/16" —







ALTERNATE SAFETY CHAIN ATTACHMENT

Details not shown similar to "Safety Chain" Details (Walkway omitted for clarity)

SAFETY CHAIN

One required for each end of each walkway.

0S-A-11-DMS

HBM ENGINEERING GROUP LIC

11/4" Ø Sch. 40 aluminum pipe

keeper hole typ.

¾"Ø pin

 $\frac{V_{16}"}{6"}$ stainless steel chain, 6" long, with $\frac{V_{16}"}{16}$ stainless

steel ring each end

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

MODEL: Default FILE NAME: P:\2004-825 PTB195-014 HBM\

UNIONE SESSES A DM

	Bar	Number	Size	Length	Shape
V-	4(E)	24	#9	F less 5"	
#	≠4 ba	ar spiral	(E) - see	Side Eleva	tion

NOTES

The foundation dimensions shown are based on the presence of mostly cohesive soils with an average Unconfined Compressive Strength (Qu) of at least 1.25 tsf, which must be determined by previous soil investigations at the jobsite. When other conditions are indicated, the boring data will be included in the plans and the foundation dimensions shown will be the result of site specific designs.

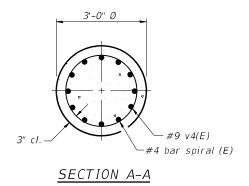
If the conditions encountered are different than those indicated, the Contractor shall notify the Engineer to determine if the foundation dimensions need to be modified. If dimensions "B" or "F" are revised by more than 12" by the Contractor, "as-built" plans shall be prepared and submitted to the District Bureau of Operations for future reference.

No sonotubes or decomposable forms shall be used below the lower conduit entrance. Permanent metal forms or other shielding may not be left in place below that elevation without the Engineer's written permission.

Concrete shall be placed monolithically, without construction joints.

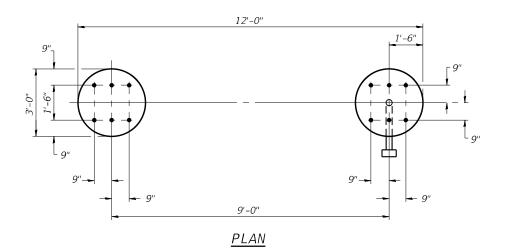
Backfill shall be placed per Article 502 of Standard Specification and prior to erection of support column.

A normal surface finish followed by a Concrete Sealer application will be required on concrete surfaces above the lowest elevation 6" below finished ground line. Cost included in Drilled Shaft Concrete Foundation.



DETAILS FOR 12" Ø SUPPORT FRAME TYPE III-A TRUSS

## ## ## ## ## ## ## ## ## ## ## ## ##	Approved clamps for grounding* #6 copper wire or cable ground rod driven into ground 9-0". Cost of rod, cable, conduit, caps and clamps shall be included in Drilled Shaft Concrete Foundations. SIDE ELEVATION	Elevation (Top) 3'' 0 Galvanized Steel Conduit. Thread and cap both ends.
3 hoops minimum top and bottom		
81 top and bottom		



For anchor rod size and placement, see Support Frame Detail Sheet.

* Anchor rod shall be ground or filed to bright metal at clamp and cable connection location.

			Left Foundation					Right Foundation					
Sign # Structure Number	Station	Elevation Top	Elevation Bottom	А	В	F	Elevation Top	Elevation Bottom	А	В	F	Class DS Concrete (Cu. Yds.)	
Sign 16 1S016I094R049.6	344+53.87						586.59	538.09	3'-6"	45'-0"	48'-6"	25.4	
Sign 12 15016I094R045.4	568+94.52						600.17	574.67	3'-6"	22'-0"	25'-6"	13.4	
Sign 14 15016I094R044.6	611+60.67						609.04	570.54	3'-6"	35'-0"	38'-6"	20.2	

054-F4

2-17-2017

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USER NAME =	DESIGNED	-	SK	REVISED	-
	CHECKED	-	MI, LAB	REVISED	-
PLOT SCALE =	DRAWN	-	SK	REVISED	-
PLOT DATE =	DATE	-	9/9/2022	REVISED	-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES								
DRILLED SHAFT DETAILS I								
SHEET OSCS 18 OF OSCS 25 SHEETS								

 F.A.I. RTE.
 SECTION
 COUNTY SHEETS NO.
 TOTAL SHEETS NO.

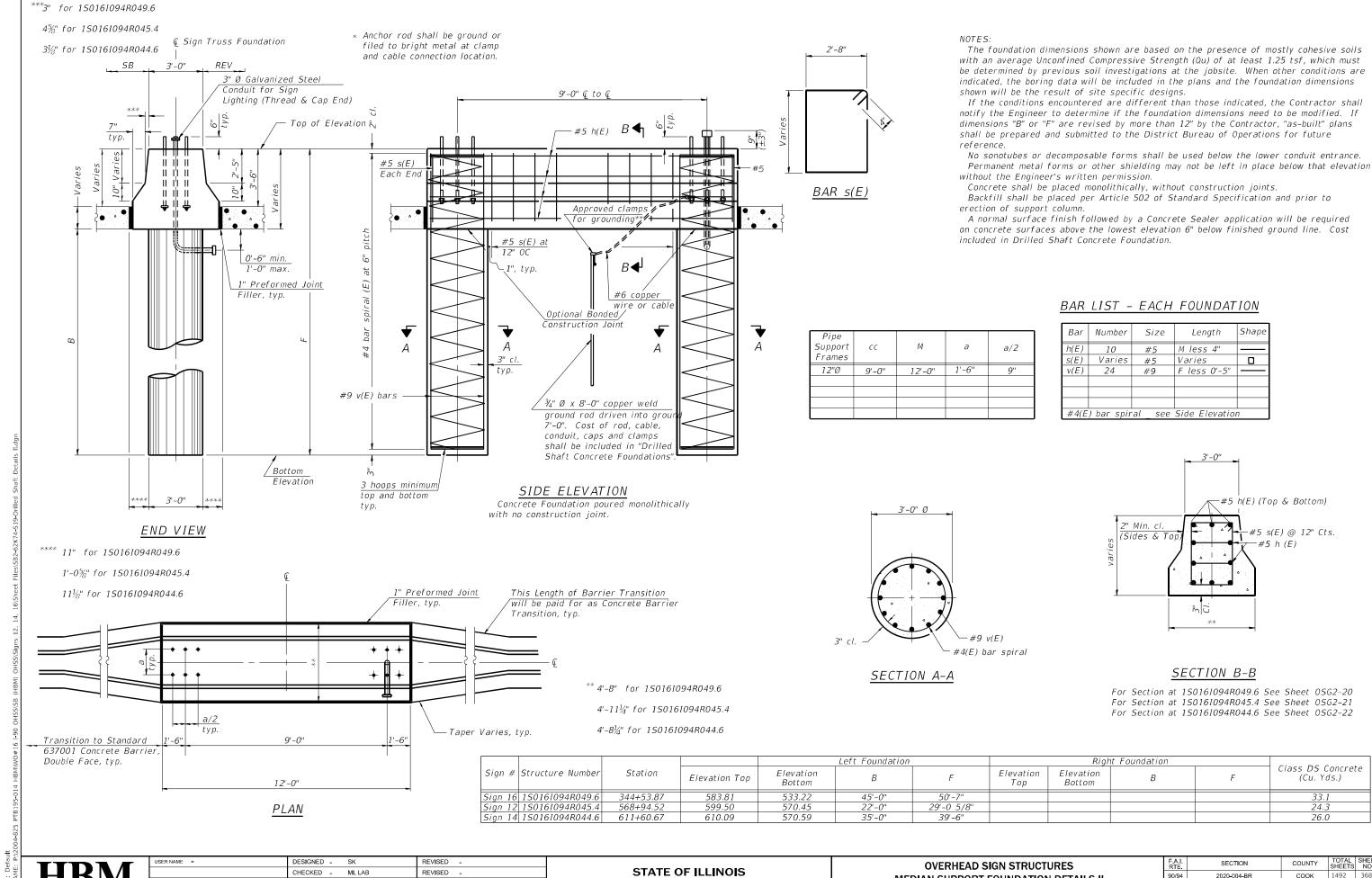
 90/94
 2020-004-BR
 COOK
 1492
 367

 CONTRACT NO. 62K74

 ILLINOIS FED. AID PROJECT

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3M\WO#16 I-90 OH5S\SB (HBM) OH5S\Signs 12, 14, 16\Sheet Files\SB2-62K74-518-Drilled



DEPARTMENT OF TRANSPORTATION

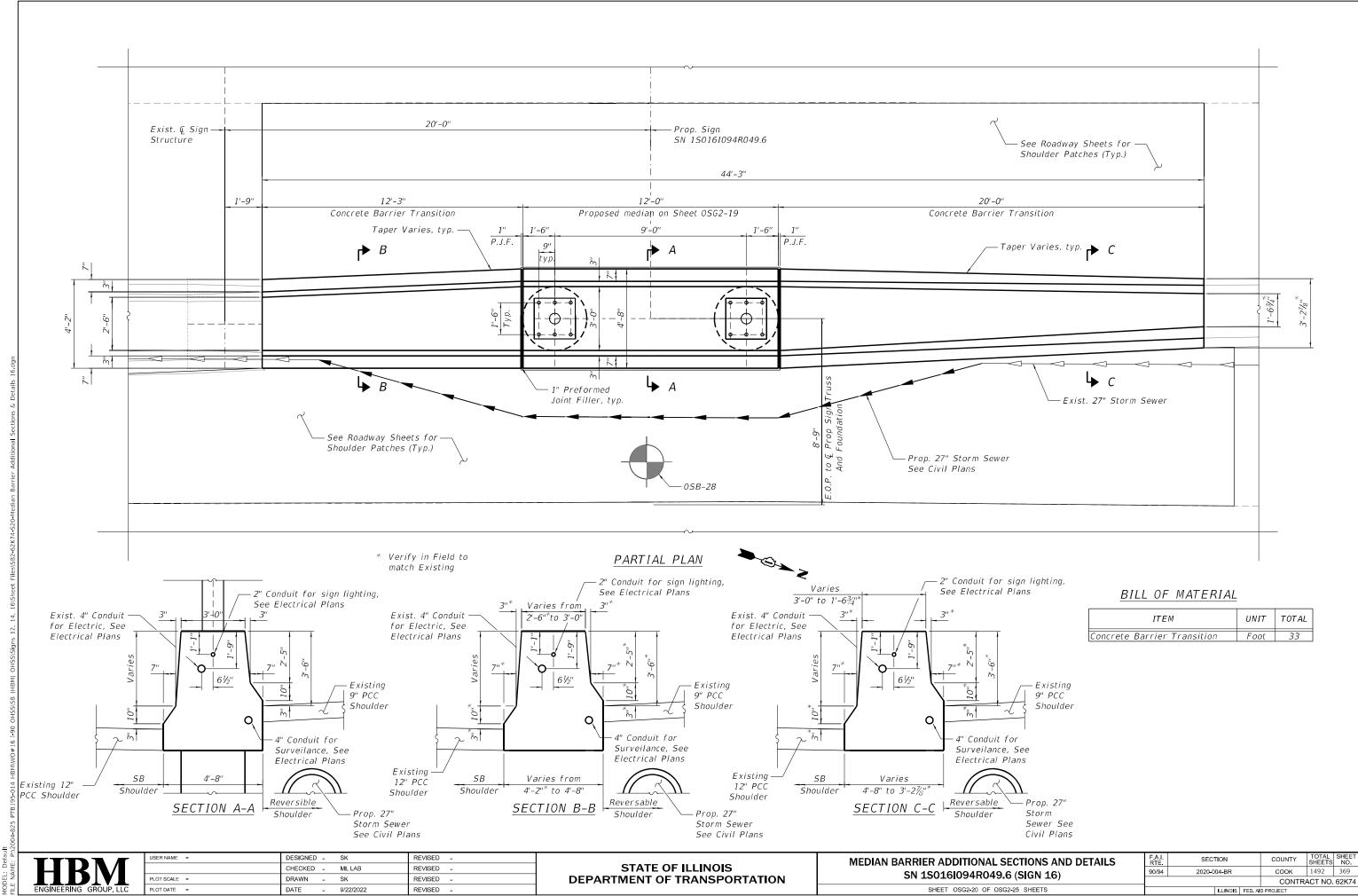
REVISED

- 9/22/2022

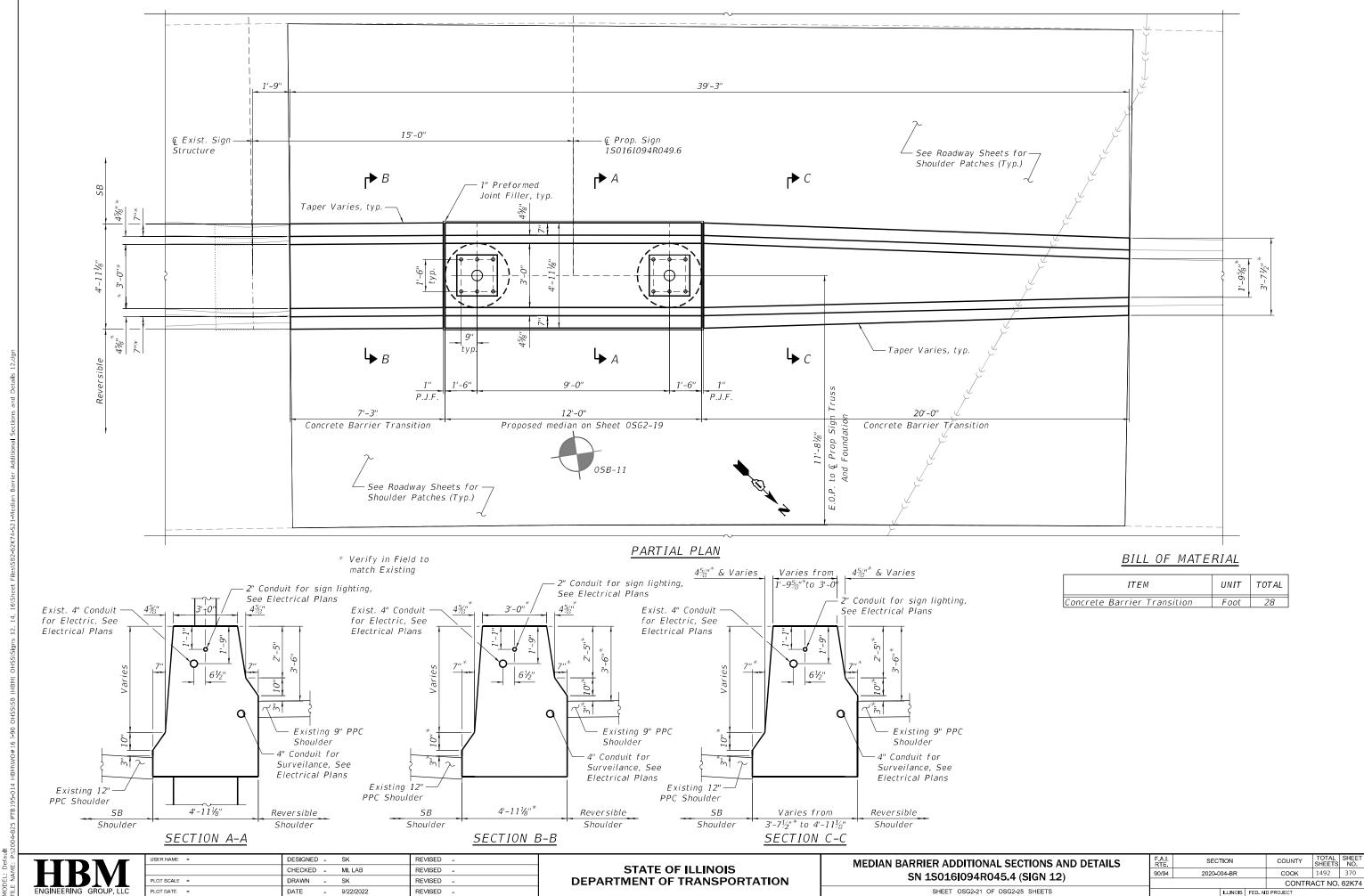
REVISED -

MEDIAN SUPPORT FOUNDATION DETAILS II SHEET OSG2-19 OF OSG2-25 SHEETS

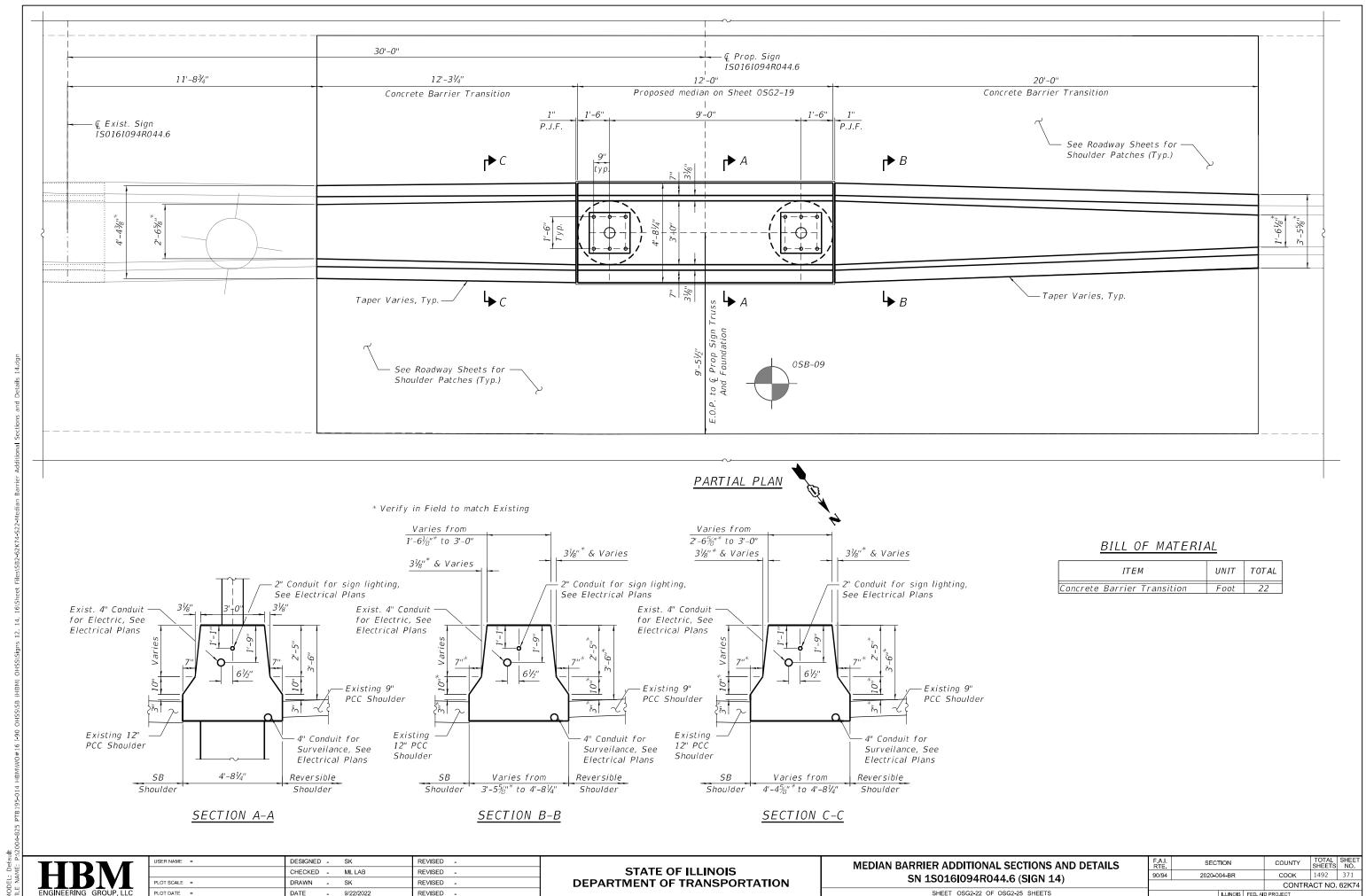
F.A.I. RTE	SECTION		COUNTY	TOTAL SHEETS	SHEE NO.
90/94	2020-004-BR		соок	1492	368
			CONTRA	ACT NO.	62K7
	ILLINOIS	FED. Al	D PROJECT		



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9/22/2022 10:41:58



GSI Job No. __19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date __11/11/21_

PTB 185-012, WO #32 **PROJECT** I-90 & I-94 Tollway LOCATION COUNTY **DRILLING METHOD** Hollow Stem Auger HAMMER TYPE **CME Automatic** D B М Surface Water Elev. CLIENT E L СО Stream Bed Elev. E С P O SI S **BORING NO.** OSB-27 T W S Groundwater Elev.: T W 1906971 Qu T Northing H S First Encounter 562.3 ft ▼ H S Qu T 1167344 **Easting Upon Completion** ft Ground Surface Elev. 580.3 (ft) (/6") (tsf) (%) (pcf) After Hrs. ft (ft) (/6") (tsf) (%) (pcf) CLAY-gray-soft to medium dense 12.0" CONCRETE (continued) CRUSHED ASPHALT & STONE-very dense (Fill) 2.10 12 2 0.60 25 2 B CLAY LOAM-gray-medium stiff to 4 1.50 15 2 0.25 25 Р 1 P 3 0.50 17 1 0.25 25 1 P В CLAY-gray-soft to medium dense 0.50 25 1 0.50 24 В 1 B End Of Boring @ -30.0'. Boring backfilled with cuttings. 0.50 25 В 2 0.60 25 В 2 0.60 17 В 0.60 21 В

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



GSI Job No. <u>19079-B</u>

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date <u>10/21/21</u>

PROJECT	PTB 185-012	, WO #32													
LOCATION	I-90 & I-94 To	ollway													
COUNTY	Cook	DRILLING	ME.	тно	D _		Hol	low Stem Auger	HAMMER T	YPE .	C	CME	Auto	mati	с
Northing Easting		.6 ft	D E P T H	B L O W S	U C S Qu		DRY DWZW-TY	Surface Water Elev. Stream Bed Elev. Groundwater Elev.: First Encounter Upon Completion After Hrs.	Dry	_ ft _ ft	D E P T H	B L O W S	U C s Qu		DRY DWZW-TY
10.0" CONCR			(11)	(/6")	(tst)	(%)	(pcf)	CLAY-gray-soft to med	dium stiff	11	(II)	(/6")	(tsf)	(70)	(pcf)
CRUSHED ST	ONE-loose (Fill)	578.8		5 2 3		3		(continued)			_	2 2 9	0.80 B	22	
27	1: (:cc.)	576.6	_							,	10-				
stiff CLAY LOAM-Q	gray-medium stiff t	0	_	1	1.80	14					_		0.80	15	
79-B_LC			5	4	В					554.1	-25	7	Р		
)GS/190			_	2				CLAY LOAM-gray-ver	y stiff			4			
SORING LO			_	3	1.40 B	16				9	_	5 7	3.10 B	16	
CLAY-gray-so	ft to medium stiff	571.6	_	1						8	_	4			
.73&62K74			-10	2	0.80 B	24				549.6	-30	Ω	2.00 P	19	
JECTS 62k			_	1				End Of Boring @ -30.0 backfilled with cuttings	or. Boring S.	3	ş. 				
NEDY PRO				1 2	0.40 B	25				ā	-				
O#15 KEN			_	1						,	-				
192-007-W			-15	2	0.80 B	25					-35				
9079-B EFK, IDOT			_	2 2 3	0.80 B	24					2				
Cyprolects2019079-B EFK, IDOT 192-007-WO#15 KENNEDY PROLECTS 62K73862K74419079-B BORING LOGS/19079-B LOG, GPU 129/21 A A A A A A A A A A A A A A A A A A A			-20	2 2 3	0.80 B	24				e S					

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



USER NAME =	DESIGNED -	SK	REVISED -	
	CHECKED -	MI, LAB	REVISED -	
PLOT SCALE =	DRAWN -	SK	REVISED -	
PLOT DATE =	DATE -	9/9/2022	REVISED -	



GSI Job No. <u>19079-B</u>

SOIL BORING LOG

Page $\underline{1}$ of $\underline{1}$

Date ___11/8/21__

LOCATION 1-90	0 & I-94 Tollway													
COUNTY Coo	k DRILLI	NG MI	ЕТНО	D _		Hol	low Stem Auger	HAMMER T	YPE		CME	Auto	mati	С
CLIENT HBN BORING NO. Northing Easting Ground Surface Ele	OSB-10 1922177 1152177	D E P T H	B L O W s	U C S Qu		DEY CHEWATER	Surface Water Elev Stream Bed Elev Groundwater Elev.: First Encounter _ Upon Completion _ After Hrs.	Dry	_ ft _ ft	D E P T H	BLOWs	U C S Qu	M O I S T	DEY DWZW-TY
12.0" CONCRETE		(11)	(/6")	(tst)	(%)	(pcf)	Drillers Observation: Co	oncrete Fill	11	(ft)	(/6")	(tsf)	(%)	(pc
CRUSHED STONE-m dense (Fill)	590 nedium	5.1 -	16 10 5		5		(continued)			_	50/0"		NR	
			9 20		9						50/0"		NR	
CLAY LOAM-brown & very stiff	gray-stiff to	.6	5 5				Auger Refusal @ -26.0	' End of	571.1	-25 —				
			7	2.30 B	21		Boring. 3 Attempts Mac backfilled with cuttings.	le. Boring						
			٦ 3	1.50 P	24					30				
		_	2 3 3	1.00 B	23									
becoming gray @ -13.	.0'	- - -1	⊣ ,	1.50 B	12					-35				
SILTY GRAVEL with Sand-gray-very dense	581		35		5									
Drillers Observation: 0	579 Concrete Fill	 0.1	50/2"							_				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



GSI Job No. <u>19079-B</u>

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date <u>10/19/21</u>

PROJECT	PTB 185-012,	WO #32													_
LOCATION	I-90 & I-94 Tol	lway													
COUNTY	Cook E	RILLING	MET	ГНОІ	D _		Hol	low Stem Auger	_ HAMMER T	YPE	(OME	Auto	mati	С
BORING NO Northing Easting _ Ground Surface	1922232		D E P T H	B L O W S	U C S Qu (tsf)		DRY DEENS-TY (pcf)	Upon Completion	580.0	_ ft		B L O W s	U C S Qu (tsf)		DRY DWZW-TY (pc
10.0" CONCRE	TE	504.0						SILTY GRAVEL & ST	ONE-very		-		200		Anna
CRUSHED STO	NE-medium	594.2		31				dense (Fill) (continued	1)			50/0"			
dense to dense			-	34		4		2			-	30/0		NR	
		÷		25		(4)				572.5					
			-5	7 10 3		5		Auger Refusal @ -22. Attempts Made. End 0 Boring backfilled with	Of Boring.	012.0					
		-	_								_				
				8							_				
				13 20		11									
			-												
		-													
				6 5		6									
			-10	7		0					-30				
		584.5									50				
CLAY LOAM-gra hard(Fill)	ay-medium stiff to			4											
nara(r iii)			-		0.50	24					-				
		-		5	Р										
			_	5							13				
					4.50	13									
		2	▼ -15		Ρ						-35				
CII TV CDAVEL	9 CTONE years	579.5									_				
SILTY GRAVEL dense (Fill)	. & STONE-very			50/0"											
						NR		,			-				
											_				
			-	50/1"							-				
						18									
			-20								-40				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)

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ENGINEERING GROUP LLC

USER NAME =	DESIGNED	-	SK	REVISED -
	CHECKED	-	MI, LAB	REVISED -
PLOT SCALE =	DRAWN	-	SK	REVISED -
PLOT DATE =	DATE	-	9/9/2022	REVISED -

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GSI Job No. 19079-B

SOIL BORING LOG

Page <u>1</u> of <u>1</u>

Date ___11/1/21__

	PROJECT	PTB 185-012, V	/O #32													_
	LOCATION	I-90 & I-94 Tolly	Vay													
	COUNTY	Cook DF	RILLING	ME	THO	D _		Hol	low Stem Auger	HAMMER T	YPE	(CME	Auto	mati	с
	CLIENT	HBM OSB-08		D E P T	B L O W	U C S	M O I S	DRY DWZW-TY	Surface Water Elev. Stream Bed Elev. Groundwater Elev.:		ft	D E P T	B L O W	U C S	M O I S	<
	Northing	1925245	_	Ĥ	s	Qu		Ş	First Encounter	Dry		H	s	Qu	1.000	Ş
		1149424 ce Elev. 606.0	— _{ft}	(64)	1000000		,,,,	1.5	Upon Completion After Hrs.	Dry	ft ft	(54)			(0/)	
î	12.0" CONCRET	2		(π)	(/6")	(tsf)	(%)	(pcf)	After Hrs CLAY LOAM-brown &		π	(π)	(/6")	(tsf)	(%)	(pcf)
	12.0 CONCRET	I E	605.0	_					gray-medium stiff to st			-				
Ì		NE-dense to very	003.0		16				(continued)				5			
	dense (Fill)		-		27		8					_		1.20	19	
_				-	18							_	6	В		
2/9/21			-						CLAY-gray-soft to med	dium stiff	583.0					
PJ 1				<u>-</u>	50/5"				gray controlline			-	3			
)G.G			-				7					_		0.60	22	
-B_C			-	-5								-25	3	В		
9079	CINDERS-black	-loose (Fill)	600.5	_	-							_				
)GS/1	ON IDENTIFICATION	10000 (1 111)	-		5							-	2			
1G LC			_	-	3		23							0.25	24	
30RII				_	2							_	2	Р		
9-B	CLAY LOAM-bro	wn &	598.0		-				CLAY LOAM-gray-ver	v etiff	578.0					
/1907	gray-medium stif	ff to stiff			3				OL/(1 Lo/(ivi-gray-ver	y Still		-	4			
2K74			-		1	1.30	21		+					2.00	21	
7386				-10	4	В			E 10(B : 0.00)		576.0	-30	7	Р		
S 62K				_					End Of Boring @ -30.0 backfilled with cuttings			-				
ECT			-		2					•		1				
PRO,				_	4	1.50	23					-				
ED			-		5	Р										
ENN					1											
#151				_	2							_				
-W0			ğ			1.50	20					-				
2-00				-15	5	В						-35				
DT 18			-									_				
Ž.	becoming gray @	② -15.5'	-		3							-				
B EF				-		1.10	21					-				
::/PROJECTS\2019\19079-B EFK, IDOT 192-007-WO#15 KENNEDY PROJECTS 62K73&62K74\19079-B BORING LOGS\19079-B_LOG.GPJ 12\9\21			-		5	В										
119/1			-									_				
15/20					_											
SEC			-		3	1.00	20									
\PRC				-20	4	1.00 B	20					-40				

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
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BBS, from 137 (Rev. 8-99)



GSI Job No. <u>19079-B</u>

SOIL BORING LOG

Page 1 of 1

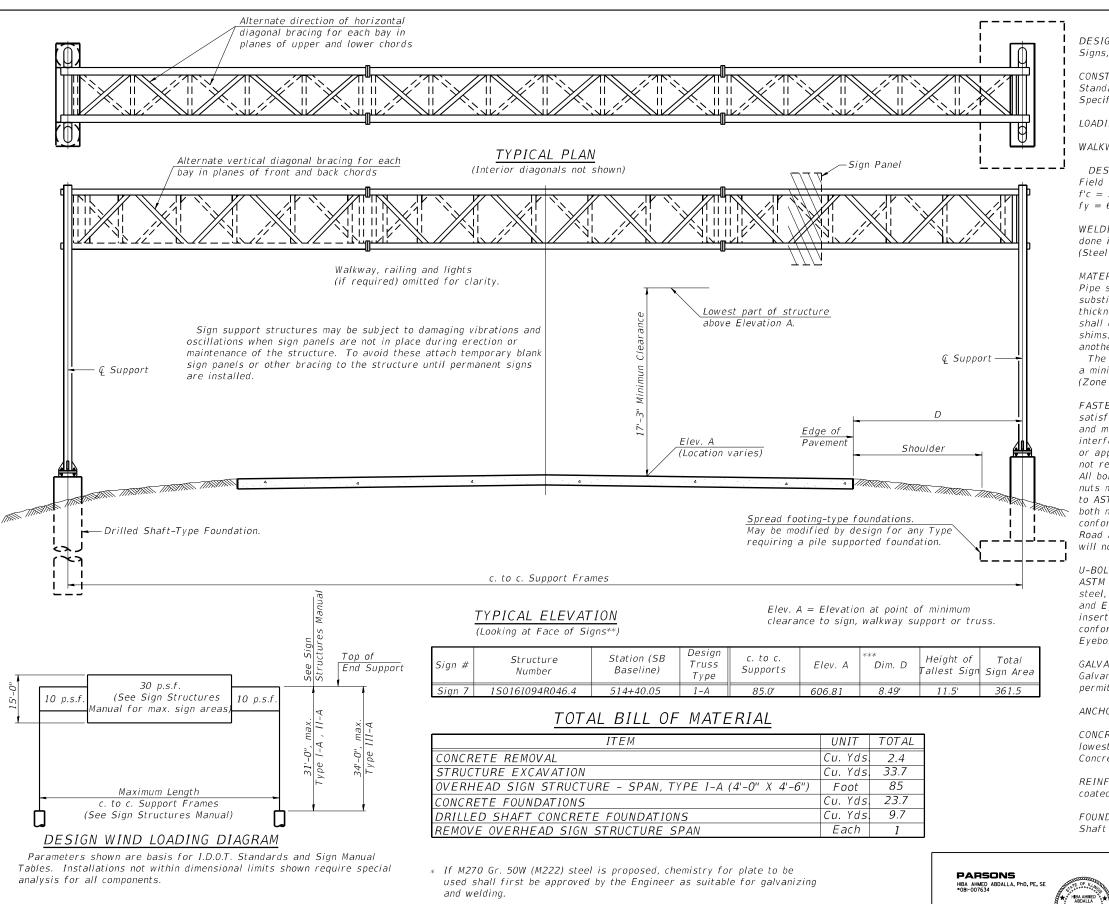
Date 10/19/21

LOCATION	1-90 & 1-94 7			T 110			Literi		LIAMANAED			2045	Λ		
COUNTY	Cook	DRILLING	3 ME	THO	Ь_	Í	Hol	low Stem Auger	HAMMER	IYPE		JME	Auto	matı	С
CLIENT	НВМ		D E	B L	C	M O	D R Y	Surface Water Elev. Stream Bed Elev.		_ ft _ ft	D E	B L	C	M O	
Northing Easting	OSB-09 1925309 1149460		P T H	O W S	S Qu	S T	DIEZW-I-Y	Groundwater Elev.: First Encounter Upon Completion	592.4	_ ft <u>▼</u> _ ft	P T H	o W s	S Qu	I S T	:2
Ground Surfa	ice Elev. 60	5.9 ft	(ft)	(/6")	(tsf)	(%)	(pcf)			ft	(ft)	(/6")	(tsf)	(%)	(p
10.0" CONCRE	TE	605.0						CLAY LOAM-gray-med stiff (continued)	dium stiff to						
CRUSHED STO	ONE-medium	605.0	_	7				sun (conunuea)				4			
dense (Fill)				12		2		•					1.00	24	T
			_	10							_	4	Р	,	
CLAY LOAM-bi	rown & gray	602.9													
spotted black-m stiff (Fill)	nedium stiff to v	ery		2	0.50	40						3	0.75		
·,			_	5	2.50 P	18					-25	3	0.75 P	23	
											23				
				7							21	4			
			_	4	2.00	21					_	5	0.80	22	
				8	В							5	В		
				1							<u> </u>				
				4								4			
			_	- 5	2.50 B	22				F7F 0	_	6	0.80 B	22	
			10					End Of Boring @ -30.0	D'. Boring	575.9	-30				
				2				backfilled with cuttings	i.]			
			_		0.60	27					_				
				2	В										
CLAY LOAM-ar	ray-medium stiff	592.9	_	-											
stiff	ay-mediam sun	110	<u>¥</u> _	3											
			_	1	1.00	23					_]			
			15	-	Р						-35				
				1							_=				
			_	3	1.25	24		•			_				
			-	5	P	27									
			-	2							_				
				3	0.80	20						-			
			-20	3	В						-40]			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
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BBS, from 137 (Rev. 8-99)

HBM
ENGINEERING GROUP LLC

USER NAME =	DESIGNED -	SK	REVISED -	
	CHECKED -	MI, LAB	REVISED -	
PLOT SCALE =	DRAWN -	SK	REVISED -	
PLOT DATE =	DATE -	9/9/2022	REVISED -	



GENERAL NOTES

DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions. ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

DESIGN STRESSES: Field Units f'c = 3,500 p.s.i. fy = 60,000 p.s.i. (reinforcement)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Specificiations.

MATERIALS: Aluminum Alloys as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer.

The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to ASTM F1554 Gr. 105.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final ground line at each foundation shall be cleaned and coated with Concrete Sealer in accordance with the Standard Specifications.

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

FOUNDATIONS: The contract unit price for Concrete Foundations and Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.

**Looking upstation for structures with signs both sides.

***For Sign 7 Layout See Sheet 2

PARSONS
HBA ARMED ABDALLA, PhD, PE, SE
*08-0076534

**DATE: 12/05/2022

**EXPRATION DATE: 130-2024

SIGNATURE AND SEAL APPLY TO SIGN STRUCTURE NO. 7 DRAWINGS

0S-A-1

2-17-2017

PARSONS
PARSONS
TRANSPORTATION GROUP
PROBLETS & PLANSETS
OCHOROS 1. 60000
PROBLETS
P

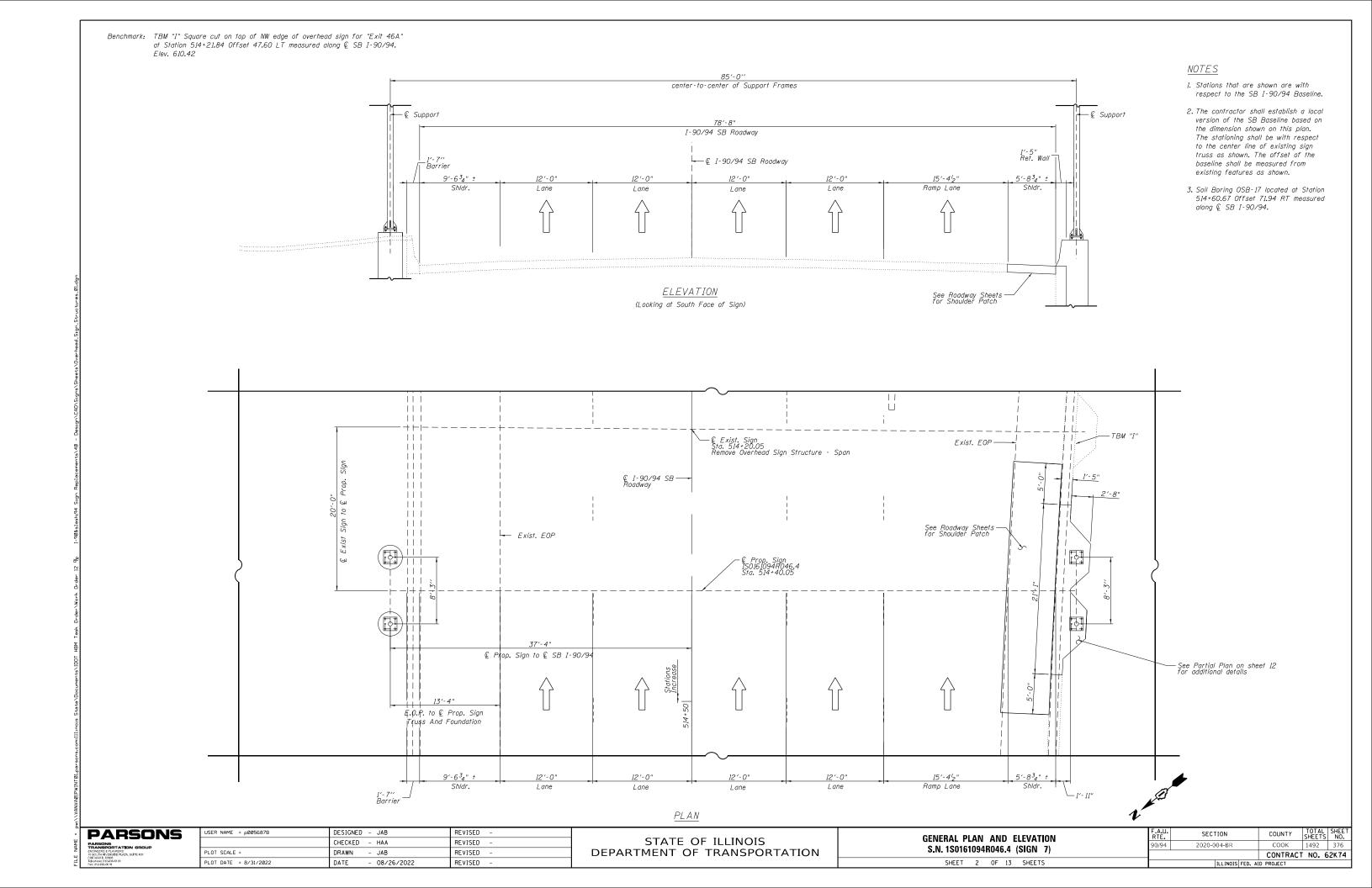
USER NAME = p009959A	DESIGNED - JAB	REVISED -
	CHECKED - HAA	REVISED -
PLOT SCALE =	DRAWN - JAB	REVISED -
PLOT DATE = 12/5/2022	DATE - 08/26/2022	REVISED -

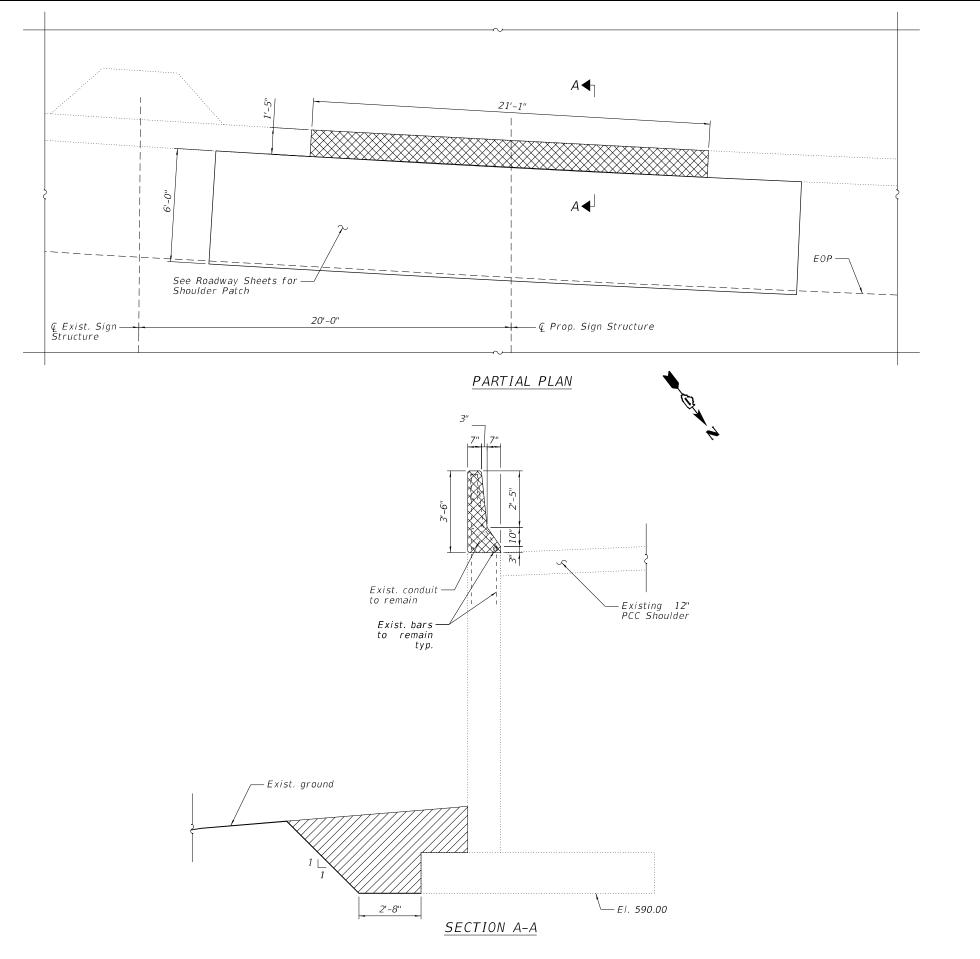
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD SIGN STRUCTURES - GENERAL PLAN & ELEVATION - ALUMINUM TRUSS & STEEL SUPPORTS

SHEET 1 0F 13 SHEETS

.A.U.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.
0/94	2020-004-BR		COOK	1492	375
			CONTRACT	NO. 6	2K74
	ILLINOIS	FED. AI	D PROJECT		





BILL OF MATERIAL

Item	Unit	Total
Concrete Removal	Cu. Yd.	2.4
Structure Excavation	Cu. Yd.	16.9
Remove Overhead Sign Structure Span	Each	1

<u>NOTES:</u>

- 1. Existing reinforcement extended into the removal area shall be cleaned, straightened and incorporated into the new construction. Any reinforcement bars that are damaged during concrete removal operations shall be replaced using an approved bar splicer or anchorage system at the Contractor's expense.
- The Contractor is responsible to protect the existing conduit embedded in the barrier during removal and construction. Any damage to the existing conduit shall be repaired by the Contractor at his or her expense at no charge to IDOT.
- 3. The Contractor shall verify all existing dimensions and details affecting new construction and removals and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work.

<u>LEGEND</u>



Concrete Removal



Structure Excavation

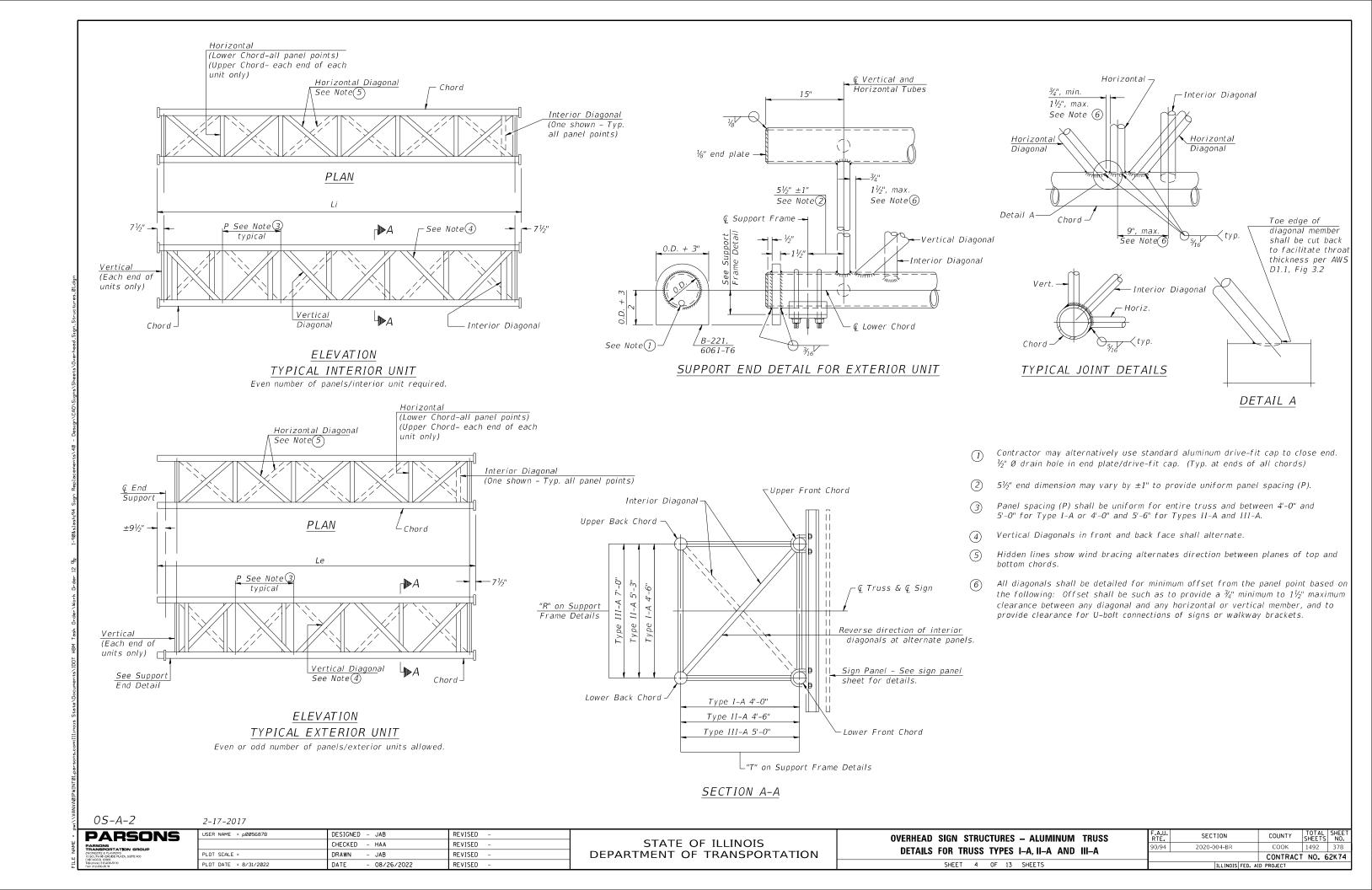
	PARSONS
Ĭ	PARSONS
Ŧ	TRANSPORTATION GROUP
∠	ENGINEERS & PLANNERS
	10 SOUTH RIVERSIDE PLAZA, SUITE 400

USER NAME = p005687B	DESIGNED - JAB	REVISED -
	CHECKED - HAA	REVISED -
PLOT SCALE =	DRAWN - JAB	REVISED -
PLOT DATE = 8/31/2022	DATE - 08/26/2022	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

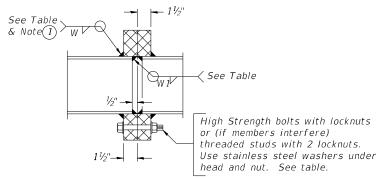
REMOVAL DETAILS
S.N. 1S0161094R046.4 (SIGN 7)

SHEET 3 OF 13 SHEETS



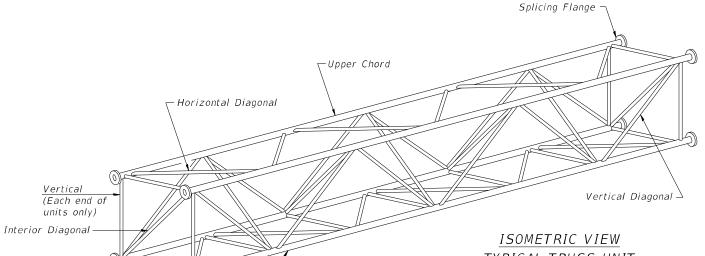
TRUSS UNIT TABLE

Structure	Chahian	Station	Station	Station	Station	Station	Station	Design Truss		erior Unit:				or Unit		Upper &		Vertical,	Horizontals; Horizontal,	Camber at			Splicing			
Number	Station	Type	No. Panels		Panel		No. Panels		Panel	L			or Diagonals	Midspan		Bolts Weld Si			A	B						
			1 '	per Unit	Lgth.(Le)	Lgth.(P)	Req'd.	per Unit	Lgth.(Li)	Lgth.(P)	0.D.	Wall	0.D.	Wall		No./Splice	Dia.	W	W 1							
15016I094R046.4	514+40.05	I-A	6	29'-11/2"	4'-61/2"	1	6	28'-6"	4'-61/2"	5"	5∕ ₁₆ "	21/2"	<i>⁵</i> ⁄ ₁₆ "	21/2"	6	⁷ /8"	5/16"	1/4"	8¾"	113/4"						



SECTION B-B

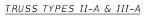
1 Splicing Flanges shall be attached to each truss unit with the truss shop assembled to camber shown. Truss units shall be in proper alignment and flange surfaces shall be shop bolted into full contact before welding. Sufficient external welds or tacks shall be made to secure flanges until remaining welds are made after disassembly. Adjacent flanges shall be "match marked" to insure proper field assembly.



Note:

TYPICAL TRUSS UNIT ASTM B221 Alloy 6061 Temper T6

Units shall be shipped individually with adequate provision to prevent detrimental motion during transport. This may require ropes between horizontals and diagonals or energy dissipating (elastic) ties to the vehicle. The Contractor is responsible for maintaining the configuration and protection of the units.



*Flange I.D

Bolt Circle Ø =

Flange O.D. = B

*Flange I.D

TRUSS TYPES I-A, II-A, & III-A

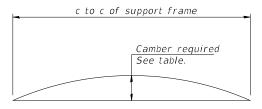
 $B \blacktriangleleft$

SPLICING FLANGES

Drill 6 holes 1/₁₆" larger than bolt diameter.

Drill 8 holes ½₁₆" larger than bolt diameter.

ASTM B221, Alloy 6061-T6 or ASTM B209, Alloy 6061-T651 *To fit O.D. of Chord with maximum gap of $\frac{1}{16}$ ".



(Upper Chord - each end of each unit only)

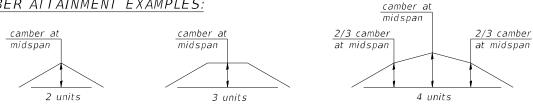
(Lower Chord - all panel points)

CAMBER DIAGRAM

Camber curve shown is theoretical. Actual camber attained by slope changes at splices between units.

CAMBER ATTAINMENT EXAMPLES:

/ Horizontal



Camber shown is for fabrication only, measured with truss fully supported. (No-load condition)

-Lower Chord

054-A-2

2-17-2017

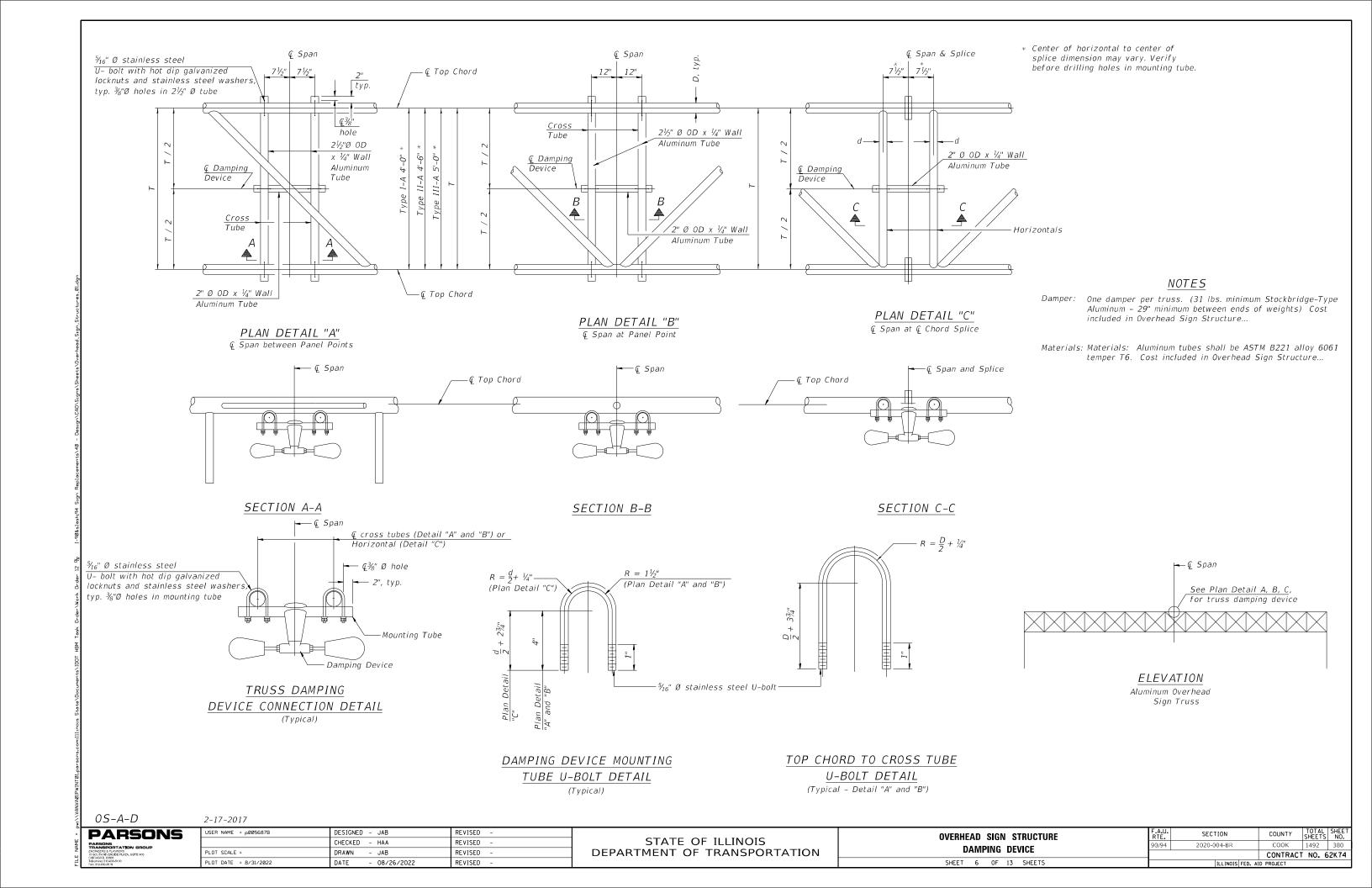
P	1B	SC	NC	IS
PARSON	IS ORTAT i O	N GROU	P	
ENGINEERS &	PLANWERS		-	
10 SOUTH RIV	ERSIDE PLAZA.	SUITE 400		
CHICAGO IL 6	8086			
Telephones 31:	636-5100			

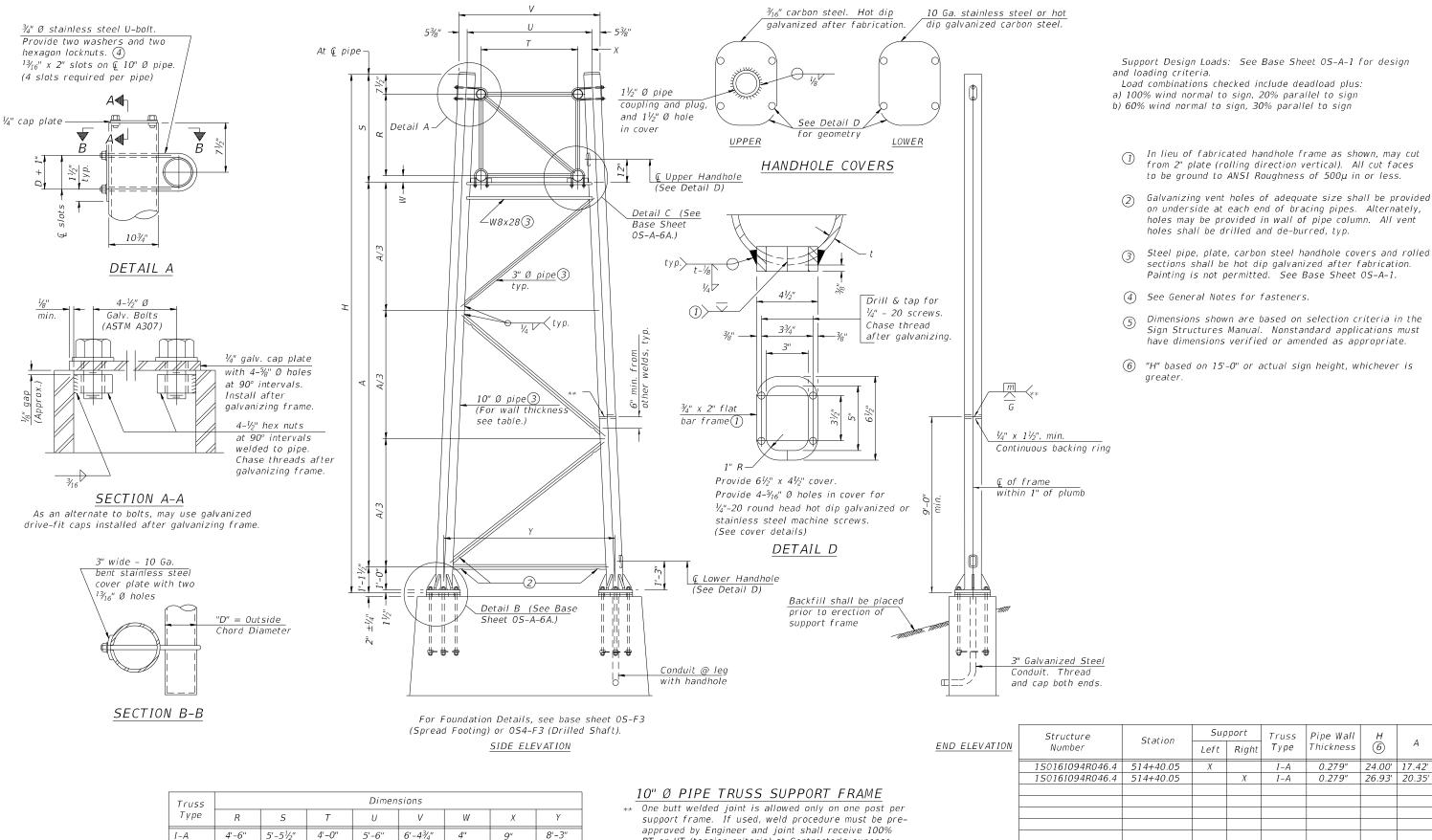
USER NAME = p0056878	DESIGNED - JAB	REVISED -
	CHECKED - HAA	REVISED -
PLOT SCALE =	DRAWN - JAB	REVISED -
PLOT DATE = 8/31/2022	DATE - 08/26/2022	REVISED -

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

OVERHEAD	 				JMINUM -A AND	 DETAILS	
	SHEET	5	0F	13	SHEETS		

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.			
90/94	2020-004-BR	соок	1492	379			
CONTRACT NO. 62K74							
ILLINOIS FED. AID PROJECT							





approved by Engineer and joint shall receive 100% RT or UT (tension criteria) at Contractor's expense.

	Structure	Structure Station		Support		Pipe Wall	Н	
<u> </u>	Number	Station	Left	Right	Туре	Thickness	6	A
	1S016I094R046.4	514+40.05	Χ		I-A	0.279"	24.00'	17.42'
	15016I094R046.4	514+40.05		Χ	I-A	0.279"	26.93'	20.35'
-								
-								
-								
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ł								
ł								
ł								
Į						1		

In lieu of fabricated handhole frame as shown, may cut

from 2" plate (rolling direction vertical). All cut faces

Galvanizing vent holes of adequate size shall be provided on underside at each end of bracing pipes. Alternately,

holes may be provided in wall of pipe column. All vent

sections shall be hot dip galvanized after fabrication.

Dimensions shown are based on selection criteria in the

Sign Structures Manual. Nonstandard applications must

have dimensions verified or amended as appropriate.

Painting is not permitted. See Base Sheet OS-A-1.

holes shall be drilled and de-burred, typ.

to be ground to ANSI Roughness of 500μ in or less.

0S-A-6

2-17-2017

II-A (5)

5'-3"

6'-31/4"

PARSONS

USER NAME = p005687B	DESIGNED - JAB	REVISED -
	CHECKED - HAA	REVISED -
PLOT SCALE =	DRAWN - JAB	REVISED -
PLOT DATE = 8/31/2022	DATE - 08/26/2022	REVISED -

4'-6"

6'-1"

6'-11¾"

43/4"

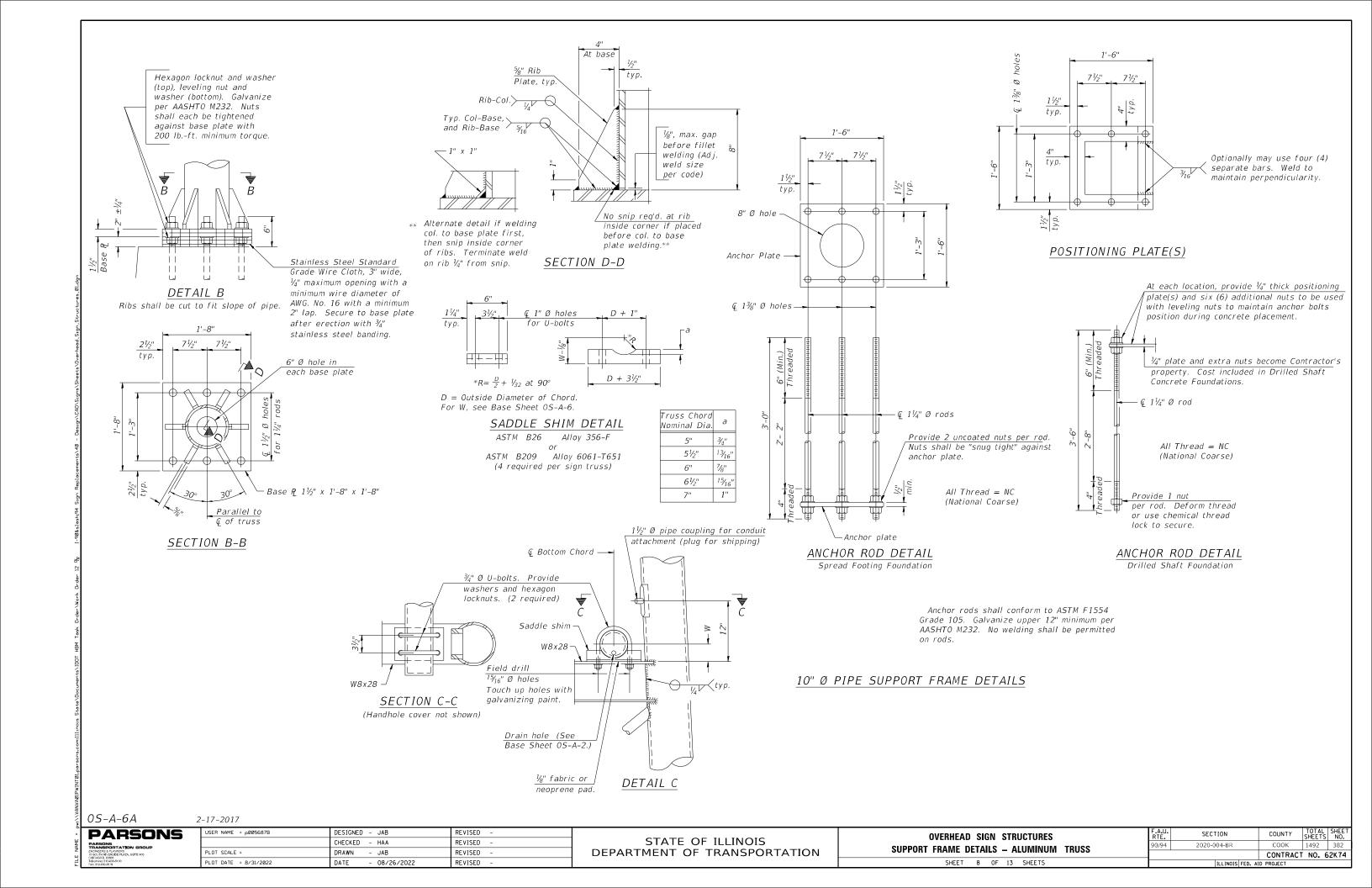
91/5"

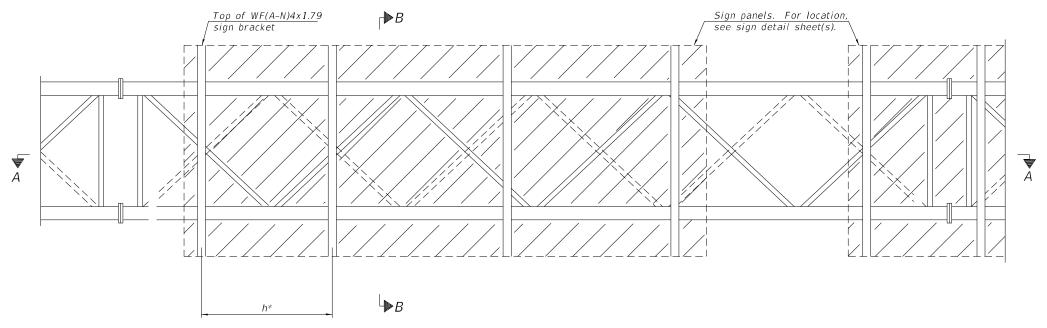
8'-3"

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

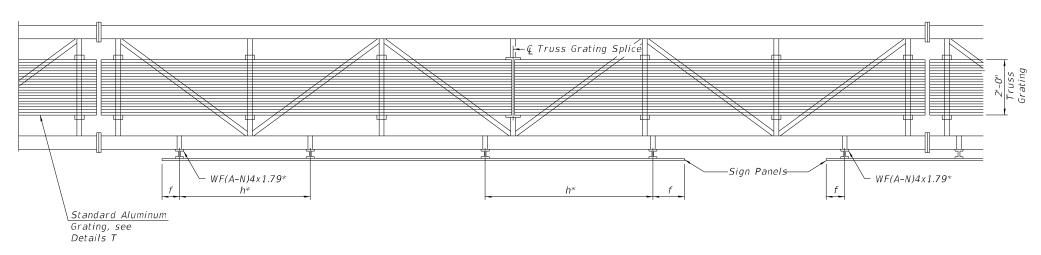
ov	ERHEAD)	SIGN	ST	RUCTURE	S
SUPPORT	FRAM	E	FOR	ALU	MINUM	TRUSS
	SHEET	7	OF	13	SHEETS	

A.U. TE.	SECTION		COUNTY	TOTAL SHEETS	SHEET NO.			
/94	2020-004-BR		соок	1492	381			
	CONTRACT NO. 62K74							
	ILL INOIS FE	D. AI	D PROJECT					





TYPICAL FRONT ELEVATION



BRACKET TABLE

#F(A-N)4x1.79 ASTM B308, Alloy 6061-T6 Sign Width				
Greater Than Less Than or Equal To Brackets Required 8'-0" 2 8'-0" 14'-0" 3 14'-0" 20'-0" 4 20'-0" 26'-0" 5			1-T6	
Required 8'-0" 2 8'-0" 14'-0" 14'-0" 20'-0" 20'-0" 4 20'-0" 5	Г	Sign \	Number	
8'-0" 14'-0" 3 14'-0" 20'-0" 4 20'-0" 26'-0" 5		Greater Than	Less Than or Equal To	Brackets Required
14'-0" 20'-0" 4 20'-0" 26'-0" 5			8'-0"	2
20'-0" 26'-0" 5		8'-0"	14'-0"	3
20 0 20 0		14'-0"	20'-0"	4
261 011 221 011 6		20'-0"	26'-0"	5
20-0 32-0 6		26'-0"	32'-0"	6

SECTION A-A

Place all sign brackets as close to panel points as practical.

* Space sign brackets WF(A-N)4x1.79 for efficiency and within limits

f=12" maximum, 4" minimum (End of sign to $\mathcal Q$ of nearest bracket) h=6'-0" maximum ($\mathcal Q$ to $\mathcal Q$ sign support brackets, WF(A-N)4x1.7

For Detail T and Section B-B, see Base Sheet OS-A-10-NW. Truss grating to facilitate inspection shall run full length (center to center of support frames) 12"± on overhead trusses. Cost of truss grating is included in "Overhead Sign Structure". Truss Grating width dimensions are nominal and may vary $\frac{1}{2}$ " \pm

based on available standard widths.

05-A-9-NW

4-1-2020

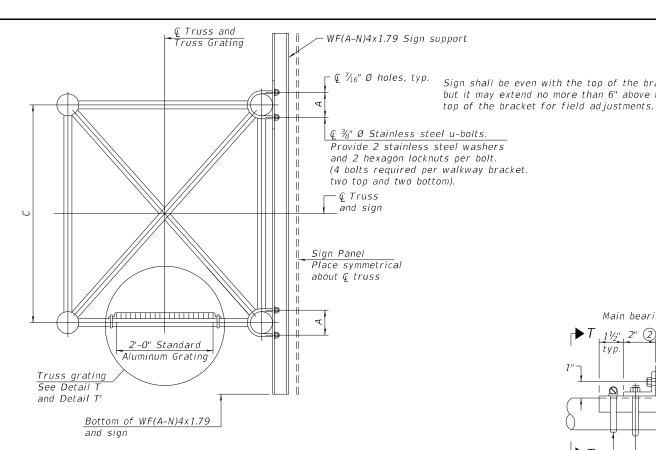
PARSONS

USER NAME = p005687B	DESIGNED - JAB	REVISED -
	CHECKED - HAA	REVISED -
PLOT SCALE =	DRAWN - JAB	REVISED -
PLOT DATE = 8/31/2022	DATE - 08/26/2022	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

	SHEET	9	ΩF	13	SHEETS
ALUMIN	IUM V	VALI	$\langle W \rangle$	Y	DETAILS
OVERHE	EAD S	SIGN	S	TRL	ICTURES

F.A.U. RTE.	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
90/94	2020-004-BR	соок	1492	383
		CONTRACT	NO. 6	2K74
	ILLINOIS FED. A	ID PROJECT		



SECTION B-B

SPECIFICATIONS FOR STANDARD ALUMINUM GRATING

Main Bearing Bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on $1\frac{3}{16}$ " centers and conform to ASTM B221 Alloy 6061-T6.

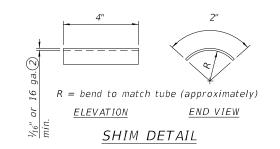
Cross bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on 4" centers and conform to ASTM B221 Alloy 6063-T5 or 6061-T6.

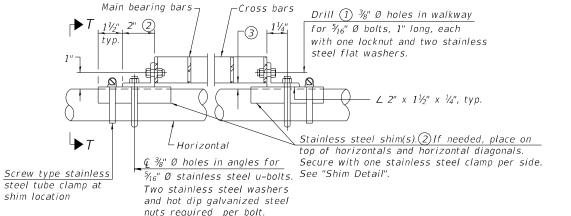
Aluminum Grating with modified "t" sections for main bearing bars shall meet the following requirements:

Main bars shall conform to ASTM B221 Alloy 6061-T6 and have a minimum section modulus equal to 0.0705 in.3 per bar, a depth of $1\frac{1}{2}$ ", spaced on $1\frac{3}{16}$ " centers.

Cross bars shall conform to ASTM B221 Alloy 6063-T5 or T-42 and spaced on 4" centers.

Structure Number	Station	A	С
15016I094R046.4	514+40.05	5½16"	4'-6"





DETAIL T (Continuous Truss grating)

U-bolt and angle connections

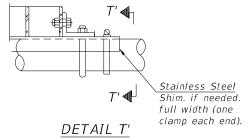
required at horizontals only.

Sign shall be even with the top of the bracket, but it may extend no more than 6" above the

> 2-L 2" x 1½" x ½" Q 5/16" Ø bolt at each horizontal (two per angle) $1'' \pm \frac{1}{2}''$, spaced to Continuous Truss Grating miss cross bars, typ. typ. Q ¾" Ø holes, typ. --Stainless steel shim(s)(2) € 5/16" Ø stainless steel d = outside diameter u-bolt. Two bolts of horizontal required per horizontal $\perp d + \frac{1}{2}$ " ($\pm \frac{1}{8}$ ")

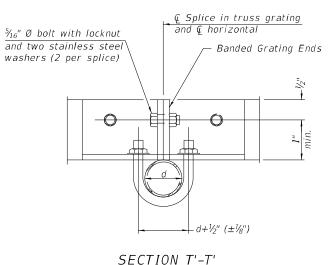
> > SECTION T-T

- (1) Drilling holes in grating may be done in shop or field, based on Contractor's preference and subject to accurate alignment.
- 2) Stainless steel shims shall be placed as shown in Detail T if needed to compensate for alignment variations between horizontal and diagonal pipes beyond adjustment provided by angles. Thicker shims may be used subject to shims performing properly.
- $\begin{tabular}{ll} \hline \end{tabular}$ Tube to grating gap may vary from 0 to $\ens{tabular}$, max. to align walkway, allow for camber, etc.



(Truss grating splice) Details not shown same as Detail T. Alternate materials may be used subject to the

Engineer's review and approval.



0S-A-10-NW 4-1-2020

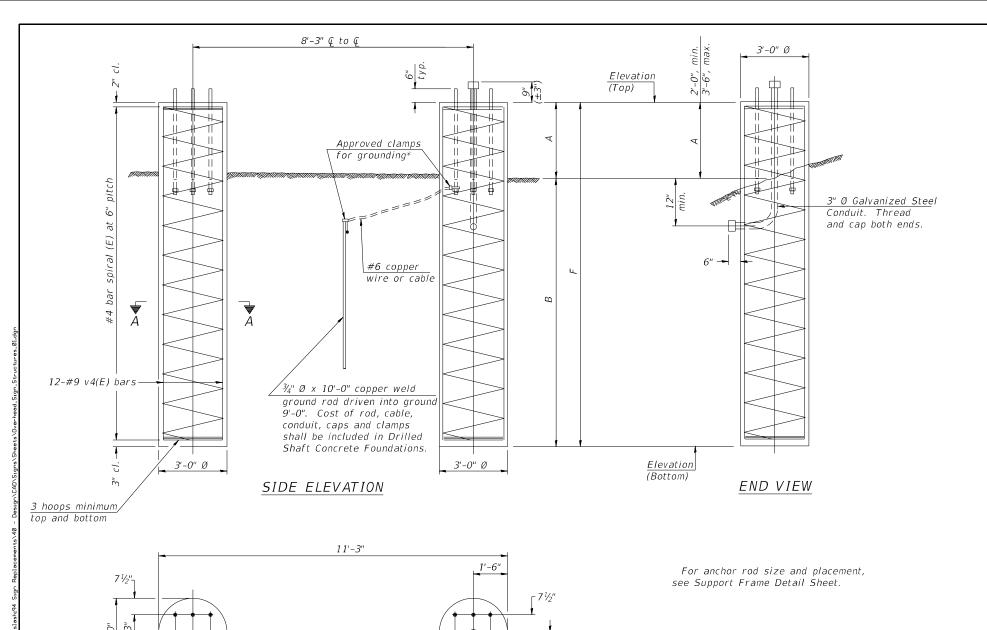
PARSONS

USER NAME = p005687B	DESIGNED - JAB	REVISED -
C	CHECKED - HAA	REVISED -
PLOT SCALE = D	DRAWN - JAB	REVISED -
PLOT DATE = 8/31/2022 D	DATE - 08/26/2022	REVISED -

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

OVERHEAD	SIC	SN ST	TRUCTURES
ALUMINUM	WΑ	LKWA	Y DETAILS
SHEET	10	0F 13	SHEETS

F.A.U. RTE.	SECTION		COUNTY	TOTAL SHEETS	SHEE.
90/94	2020-004-BR		соок	1492	384
			CONTRACT	NO. 6	2K74
	ILLINOIS	FED. Al	ID PROJECT		



BAR LIST - EACH FOUNDATION

Bar	Number	Size	Length	Shape
v4(E)	24	#9	F less 5"	
#4 ba	ar spiral	(E) - see	Side Eleva	tion

The foundation dimensions shown are based on the presence of mostly cohesive soils with an average Unconfined Compressive Strength (Qu) of at least 1.25 tsf, which must be determined by previous soil investigations at the jobsite. When other conditions are indicated, the boring data will be included in the plans and the foundation dimensions shown will be the result of site specific designs.

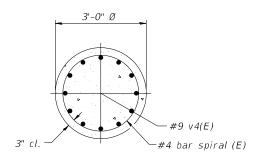
If the conditions encountered are different than those indicated, the Contractor shall notify the Engineer to determine if the foundation dimensions need to be modified. If dimensions "B" or "F" are revised by more than 12" by the Contractor, "as-built" plans shall be prepared and submitted to the District Bureau of Operations for future reference.

No sonotubes or decomposable forms shall be used below the lower conduit entrance. Permanent metal forms or other shielding may not be left in place below that elevation without the Engineer's written permission.

Concrete shall be placed monolithically, without construction joints.

Backfill shall be placed per Article 502 of Standard Specification and prior to erection

A normal surface finish followed by a Concrete Sealer application will be required on concrete surfaces above the lowest elevation 6" below finished ground line. Cost included in Drilled Shaft Concrete Foundation.



SECTION A-A

DETAILS FOR 10" Ø SUPPORT FRAME TYPE I-A or II-A TRUSS

Structure Statio. Number				Left Fo	oundation			Right Fou	ındation			Class DS
	Station	Elevation Top	Elevation Bottom	А	В	F	Elevation Top	Elevation Bottom	А	В	F	Concrete (Cu. Yds.)
1S016I094R046.4	514+40.05	610.02	591.52	0.50'	18.00'	18.50'	-	-	-	-	-	10.0

0*S*4-*F*3

2-17-2017 **PARSONS**

7½"-

- 7½"

USER NAME = p005687B DESIGNED - JAB REVISED CHECKED - HAA REVISED PLOT SCALE = REVISED PLOT DATE = 8/31/2022 DATE - 08/26/2022 REVISED

8'-3"

PLAN

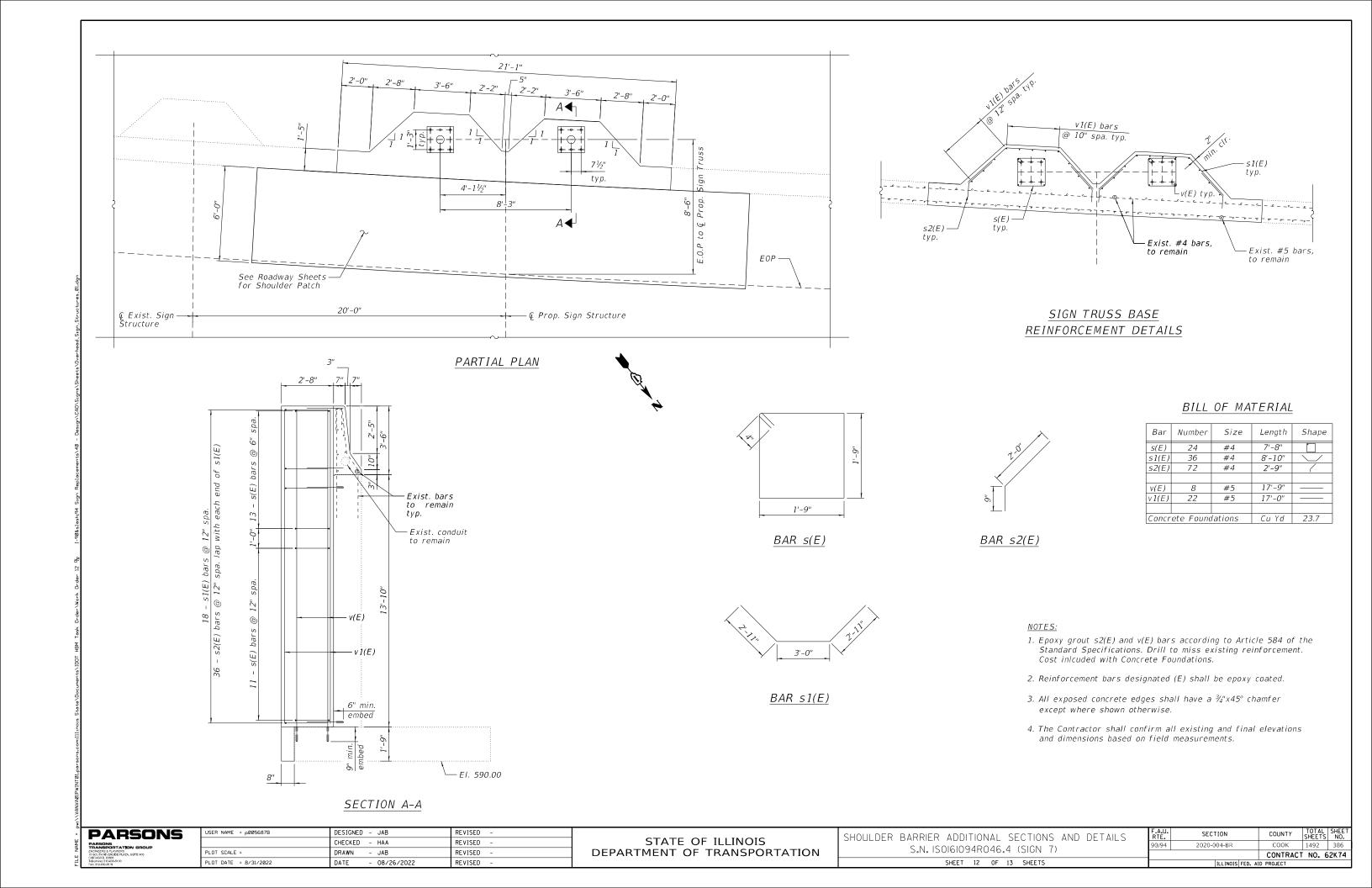
71/2"-

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

* Anchor rod shall be ground or filed to bright metal at clamp and cable connection location.

> OVERHEAD SIGN STRUCTURES DRILLED SHAFT DETAILS SHEET 11 OF 13 SHEETS

SECTION COUNTY 2020-004-BR COOK 1492 385 CONTRACT NO. 62K74





GSI Job No. __19079-B

SOIL BORING LOG

Page $\underline{1}$ of $\underline{2}$

Date __10/20/21

	PROJECT	PTB 185-012,	WO #32													_
	LOCATION	I-90 & I-94 To	llway													
	COUNTY	Cook	DRILLING	ME	тно	D _		Hol	low Stem Auger	_ HAMMER 1	TYPE _	(CME	Auto	mati	С
	CLIENT	НВМ		D E	B L	U	M	D R Y	Surface Water Elev. Stream Bed Elev.		_ ft _ ft	D E	B L	U	M	D R Y
	BORING NO Northing Easting	1919278 1156742		P T H	O W S	S Qu		DEZØ-FY	Groundwater Elev.: First Encounter Upon Completion	586.5	_ft _	P T H	o W s	S Qu	I S T	DEZS-TY
	Ground Surfaction 10.0" CONCRET		.0 ft	(ft)	(/6")	(tsf)	(%)	(pcf)	After HrsCLAY LOAM-brown &	gray-stiff	ft	(ft)	(/6")	(tsf)	(%)	(pcf
	actividated on sandcontinuing gomens		604.2	_					(continued)	1	584.5	-				
	CRUSHED STO	NE-dense (Fill)		_	25 23				CLAY-gray-soft to me	dium stiff		_	3	0.50	0.5	
					10		6						4	0.50 B	25	
3/21			602.0													
J 12/9	CLAY LOAM-dai spotted black-sti		ill)	_	5							_	3			
BORING LOGS/19079-B_LOG.GPJ					4	1.50	17						2	0.25	26	
P-B	ı			-5	4	В						-25	3	В		
19079				_	1							-				
-0GS					4								2			
RINGL					3 5	2.00 B	26						3	0.25 B	24	
B B0				_												
9079				_	2							_	0			
PROJECTS 62K73&62K74\19079-B				_		1.50	22							0.25	24	
<7386				-10	4	Р						-30	1	В		
S 62k				_	-							_				
DJECT					4											
					8 6	2.20 B	18									
NNEDY			592.0	-								_				
#15 KE	CRUSHED STO SAND-brown-me		I)	_	30								1			
#OM-200-			,	_	18		19				,		- 8	0.25	26	
92-00				-15	5							-35	1	Р		
DOT 1	CLAY LOAM-bro	own & grav-stiff	589.5	_	-							_				
EFK.	.I				4											
9-6/C					5 7	1.80 B	20		CLAY LOAM-gray-ver	rv stiff	568.0					
PROJECTS\2019\19079-B EFK									J SEATT ESTANT GIAY-VEI	, 500		_				
:TS/20				Y _	5								7			
OJEC						1.00	23							3.00	13	
R				_	6	ь						-	10	B		

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)



GSI Job No. __19079-B

SOIL BORING LOG

Page <u>2</u> of <u>2</u>

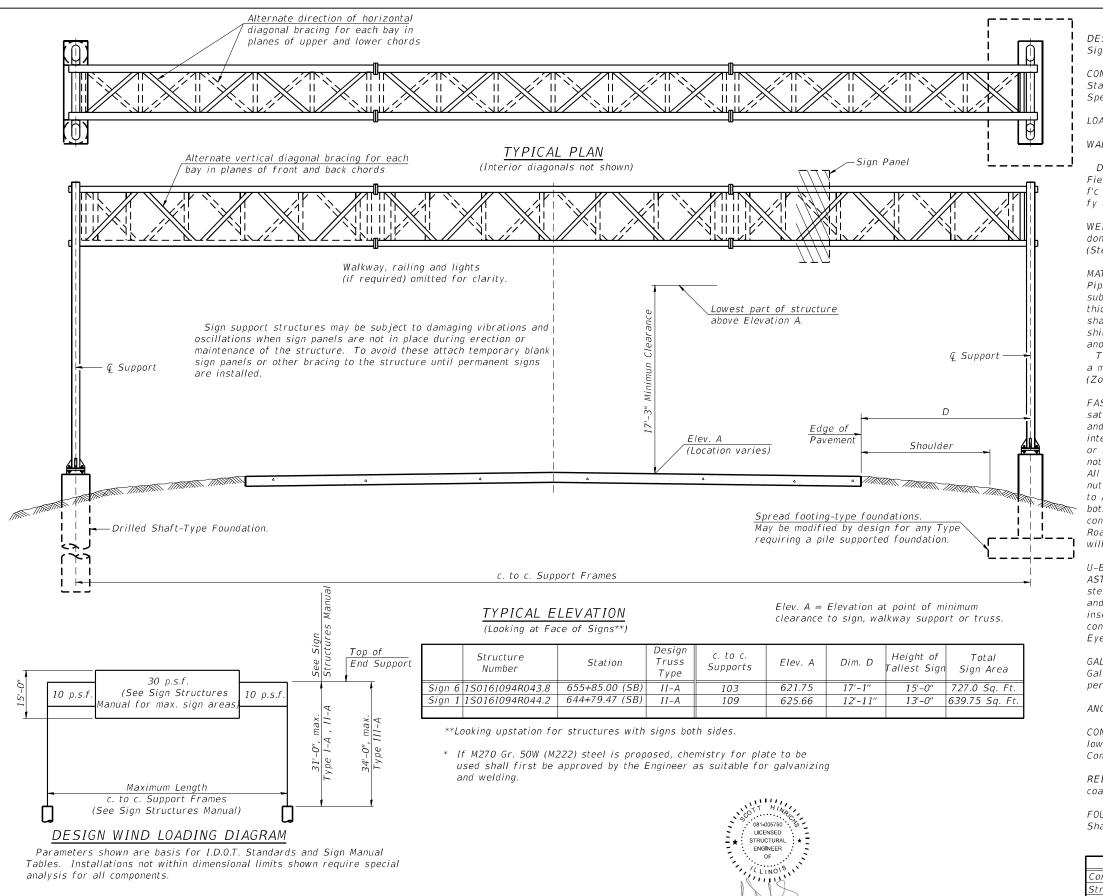
Date __10/20/21_

PROJECT	PTB 185-012	2, WO #32								
LOCATION	I-90 & I-94 T	ollway								
COUNTY	Cook	DRILLING	METH	DD _		Hol	low Stem Auger	HAMMER TYP	PECME Automati	c
CLIENT	HBM OSB-17		D B L P O T W	U C S	M O I S	DRY DWZ	Surface Water Elev. Stream Bed Elev. Groundwater Elev.:	fi		
Northing Easting	1919278 1156742 ice Elev. 608	5.0 ft	H S	Qu	Т	DHZW-FY	First Encounter Upon Completion After Hrs.	586.5 fi	t ⁻	
E.IPROJECTS/2019/19079-B EFK, IDOT 192-007-WO#15 KENNEDY PROJECTS 62X73862X74/19079-B BORING LOGS/19079-B BORING LOGS/1907-B BORING LOG	@ -40.0'. Boring	-		(tsf.	(%)	(pcf)	After Hrs.			

The Unconfined Compressive Strength (UCS) Failure Mode is indicated by (B-Bulge, S-Shear, P-Penetrometer)
The SPT (N value) is the sum of the last two blow values in each sampling zone (AASHTO T206), GP-Geoprobe Hand Auger
BBS, from 137 (Rev. 8-99)

PARSONS
PARSONS TRANSPORTATION GROUP
ENGINEERS & PLANNERS 10 SOUTH RIVERSIDE PLAZA, SUITE 400

USER NAME = p005687B	DESIGNED - JAB	REVISED -
	CHECKED - HAA	REVISED -
PLOT SCALE =	DRAWN - JAB	REVISED -
PLOT DATE = 8/31/2022	DATE - 08/26/2022	REVISED -



GENERAL NOTES

DESIGN: AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals. ("AASHTO Specifications")

CONSTRUCTION: Current (at time of letting) Illinois Department of Transportation Standard Specifications for Road and Bridge Construction, Supplemental Specifications and Special Provisions. ("Standard Specifications")

LOADING: 90 M.P.H. WIND VELOCITY

WALKWAY LOADING: Dead load plus 500 lbs. concentrated live load.

DESIGN STRESSES: Field Units f'c = 3,500 p.s.i.

fy = 60,000 p.s.i. (reinforcement)

WELDING: All welds to be continuous unless otherwise shown. All welding to be done in accordance with current AWS D1.1 and D1.2 Structural Welding Codes (Steel and Aluminum) and the Standard Specificiations.

MATERIALS: Aluminum Allovs as shown throughout plans. All Structural Steel Pipe shall be ASTM A53 Grade B or A500 Grade B or C. If A500 pipe is substituted for A53, then the outside diameter shall be as detailed and wall thickness greater than or equal to A53. All Structural Steel Plates and Shapes shall conform to AASHTO M270 Gr. 36, Gr. 50 or Gr. 50W*. Stainless steel for shims, sleeves and handhole covers shall be ASTM A240, Type 302 or 304, or another alloy suitable for exterior exposure and acceptable to the Engineer.

The steel pipe and stiffening ribs at the base plate for the column shall have a minimum longitudinal Charpy V-Notch (CVN) energy of 15 lb.-ft. at 40° F. (Zone 2) before galvanizing.

FASTENERS FOR ALUMINUM TRUSSES: All bolts noted as "high strength" must satisfy the requirements of AASHTO M164 (ASTM A325), or approved alternate, and must have matching lock nuts. Threaded studs for splices (if Members interfere) must satisfy the requirements of ASTM A449, ASTM A193, Grade B7, or approved alternate, and must have matching lock nuts. Bolts and lock nuts not required to be high strength must satisfy the requirements of ASTM A307. All bolts and lock nuts must be hot dip galvanized per AASHTO M232. The lock nuts must have nylon or steel inserts. A stainless steel flat washer conforming to ASTM A240 Type 302 or 304, is required under both head and nut or under both nuts where threaded studs are used. High strength bolt installation shall conform to Article 505.04 (f) (2)d of the IDOT Standard Specifications for Road and Bridge Construction. Rotational capacity ("ROCAP") testing of bolts will not be required.

U-BOLTS AND EYEBOLTS: U-Bolts and Eyebolts must be produced from ASTM A276 Type 304, 304L, 316 or 316L, Condition A, cold finished stainless steel, or an equivalent material acceptable to the Engineer. All nuts for U-Bolts and Eyebolts must be lock nuts equivalent to ASTM A307 with nylon or steel inserts and hot dip galvanized per AASHTO M232. A stainless steel flat washer conforming to ASTM A240, Type 302 or 304, is required under each U-Bolt and Eyebolt lock nut.

GALVANIZING: All Steel Grating, Plates, Shapes and Pipe shall be Hot Dip Galvanized after fabrication in accordance with AASHTO M111. Painting is not permitted.

ANCHOR RODS: Shall conform to ASTM F1554 Gr. 105.

CONCRETE SURFACES: All concrete surfaces above an elevation 6" below the lowest final ground line at each foundation shall be cleaned and coated with Concrete Sealer in accordance with the Standard Specifications.

REINFORCEMENT BARS: Reinforcement Bars designated (E) shall be epoxy coated in accordance with the Standard Specifications.

FOUNDATIONS: The contract unit price for Concrete Foundations and Drilled Shaft Concrete Foundations shall include reinforcement bars complete in place.

TOTAL BILL OF MATERIAL

ITEM	UNIT	TOTAL
Concrete Barrier Removal	Foot	95.0
Structure Excavation	Cu Yd	44.5
Temporary Soil Retention System	Sq Ft	280
Concrete Barrier Transition	Foot	72.5
Overhead Sign Structure Span Type II-A (4'-6" x 5'-3")	Foot	212.0
Drilled Shaft Concrete Foundations	Cu Yd	83.9
Remove Overhead Sign Structure - Span	Each	2.0
•		

05-A-1

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2-17-2017

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STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

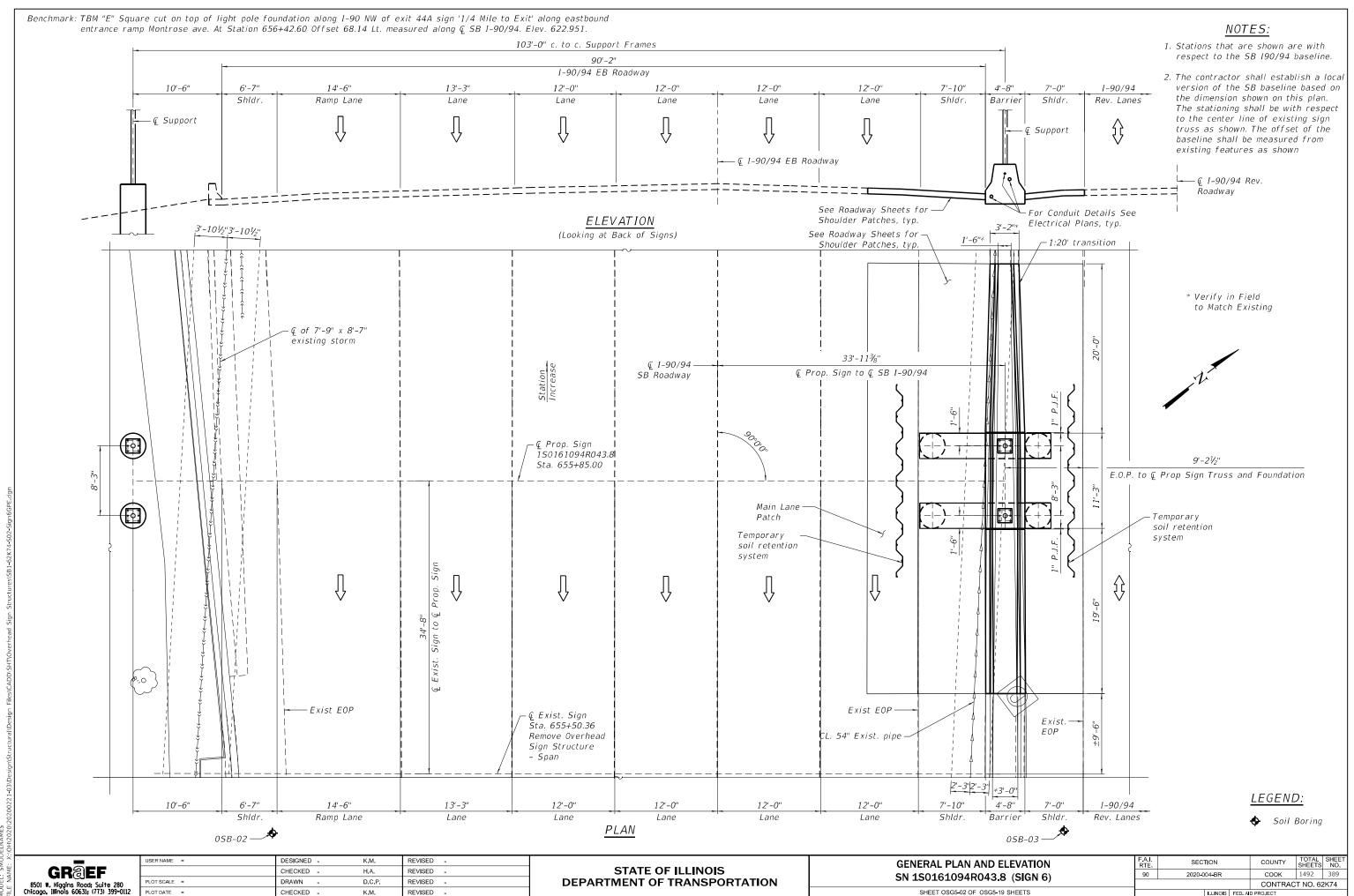
Illinois Registered Engineer No. 081-005750

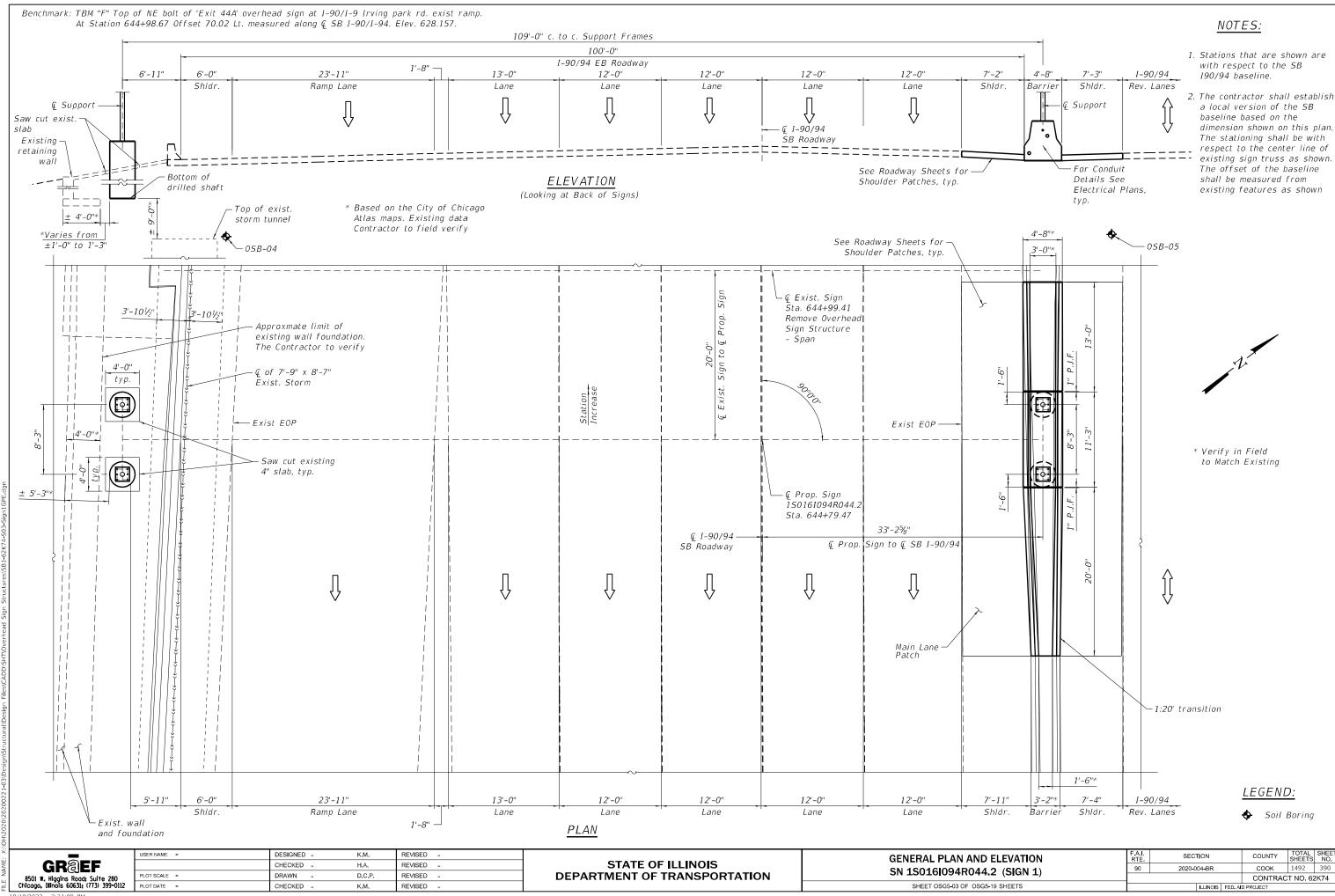
Registration Expires 11. 30, 2024

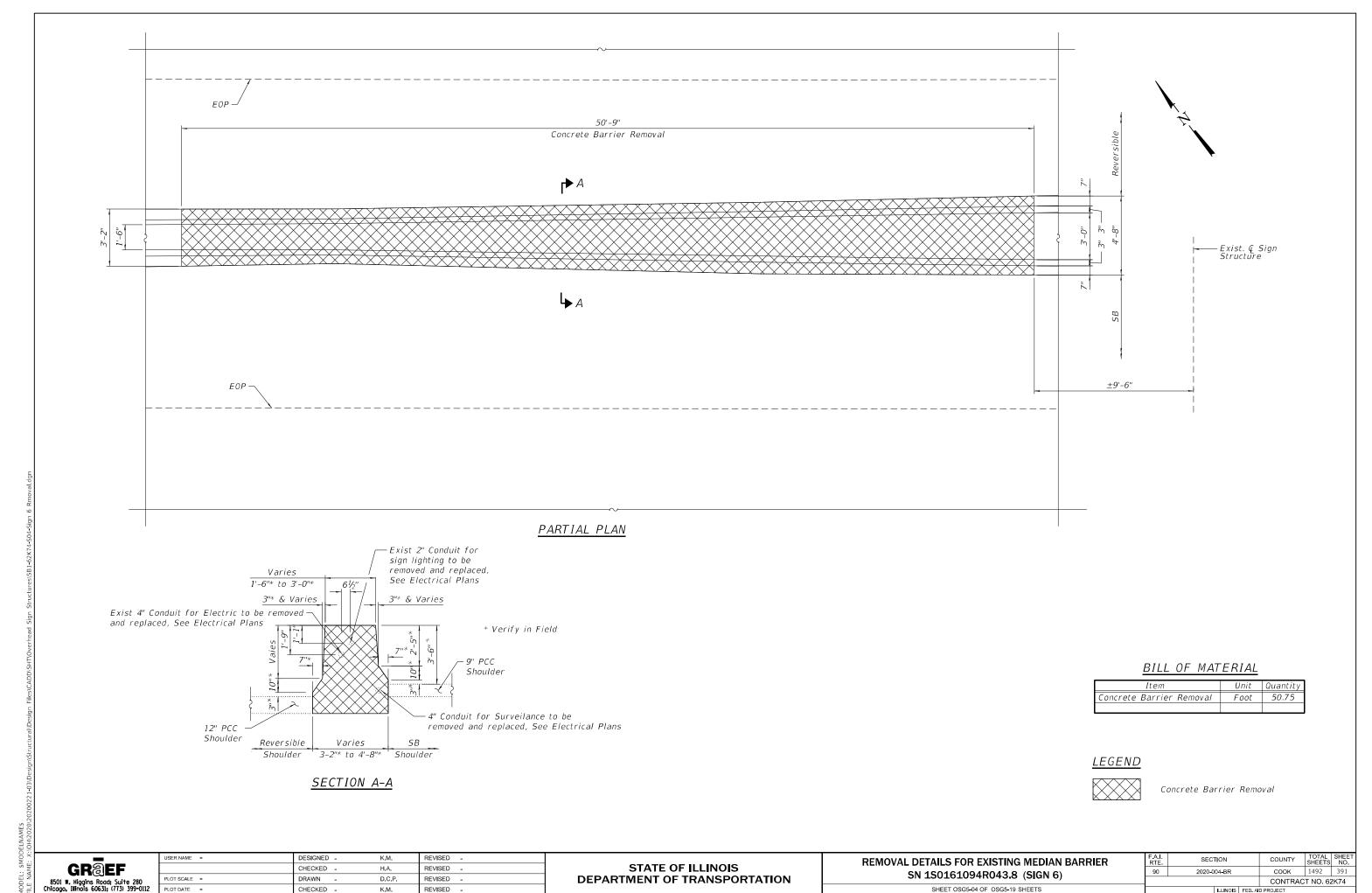
Engineer Full Name: Scott Hinrichs Date: 10-20-2022

SECTION **OVERHEAD SIGN STRUCTURES - GENERAL PLAN &** 2020-004-BR **ELEVATION - ALUMINUM TRUSS & STEEL SUPPORTS**

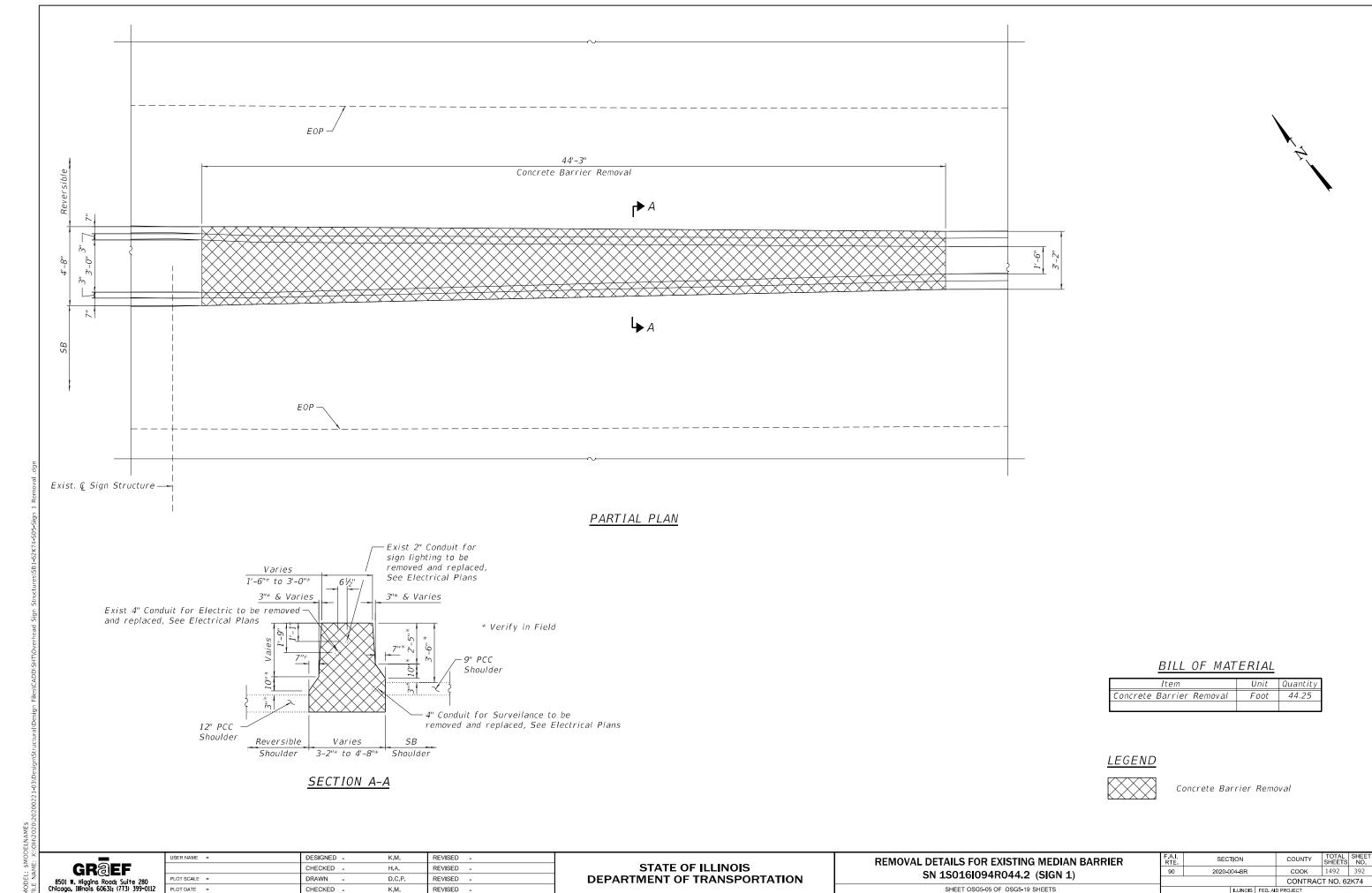
COUNTY COOK 1492 388 CONTRACT NO. 62K74 SHEET OSG5-01 OF OSG5-19 SHEETS



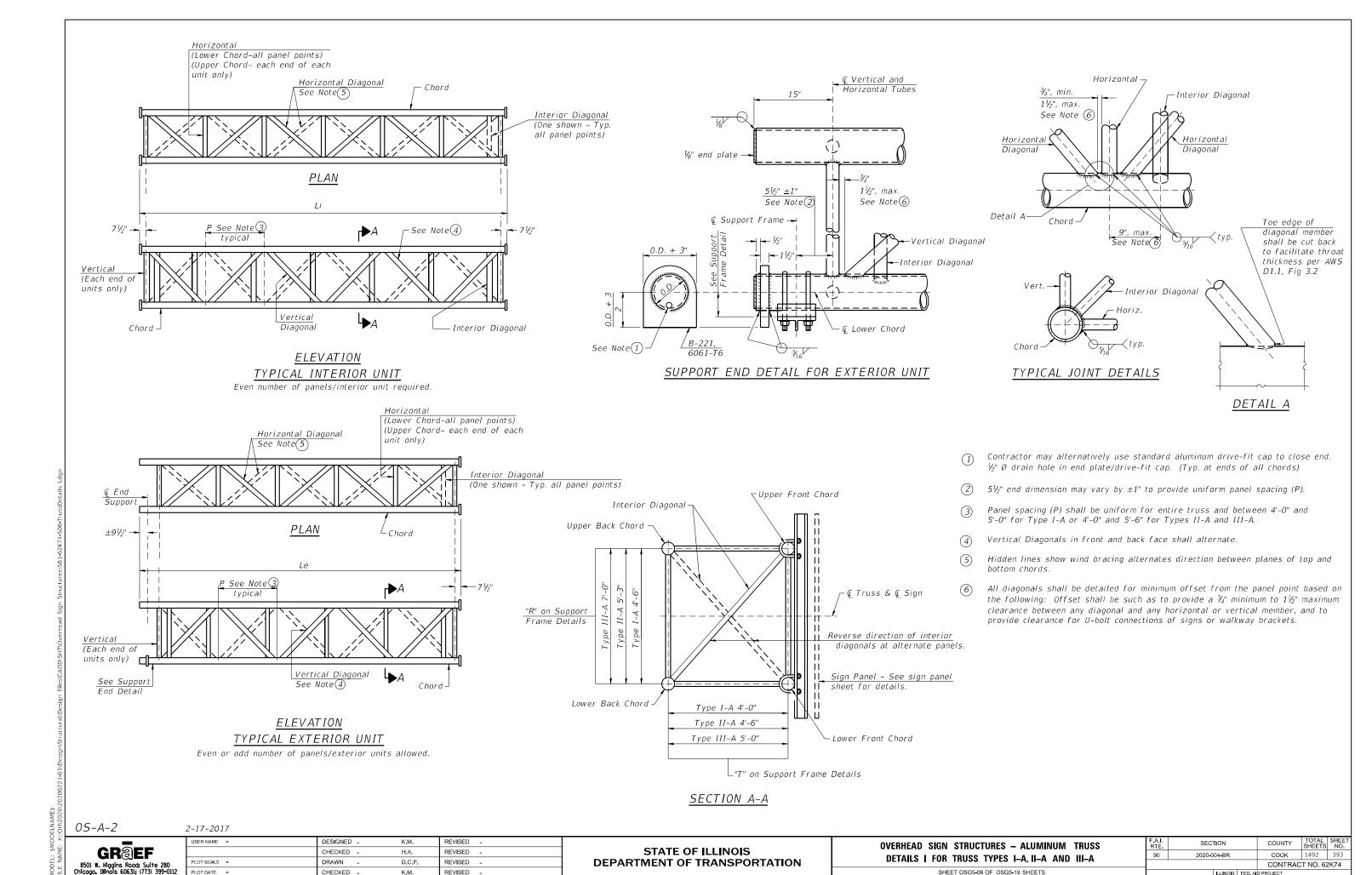




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SHEET OSG5-06 OF OSG5-19 SHEETS

PLOT DATE =

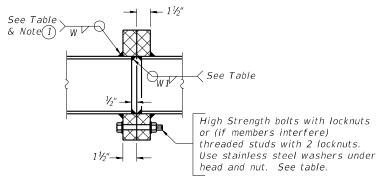
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K.M.

REVISED

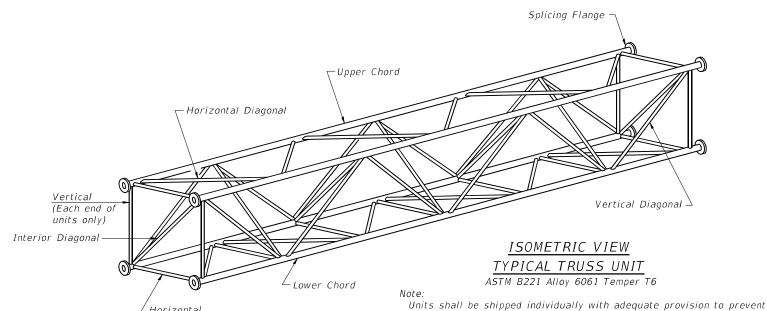
TRUSS UNIT TABLE

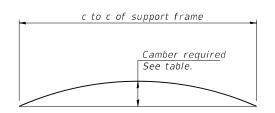
Structure		Structure $\mid \mid \mid \mid \mid \mid \mid \mid \mid \mid $		tructure C		icture Truss		ructure G Tru		ructure C		tructure G		tructure C T.								Upper & Lower Chord		vertical, Horizontal,		Camber at	Dolts Wold Gines					
	Number	Station	Туре	No. Panels per Unit	Unit Lgth.(Le)	Panel Lgth.(P)	No. Reg'd.	No. Panels per Unit	Unit Lgth.(Li)	Panel Lgth.(P)	0.D.	Wall	0.D.	Wall	Midspan	No./Splice		Weld	W1	Α	В											
Sign 6	1S016I094R043.8	655+85.00 (SB)	II-A			4'-11 3/4"	1	6	31'-1 1/2"		6 1/2"	5/16"	3"	5/16"	3 1/4"	6	1"	3/8"	1/4"	11"	14 1/2"											
Sign 1	1S016I094R044.2	644+79.47 (SB)	II-A	7	38'-9 1/4"	5'-3 1/4"	1	6	32'-10 1/2''	5'-3 1/4"	6 1/2"	5/16"	3"	5/16"	3 5/8"	6	1"	3/8"	1/4"	11"	14 1/2"											
																					+											



SECTION B-B

1) Splicing Flanges shall be attached to each truss unit with the truss shop assembled to camber shown. Truss units shall be in proper alignment and flange surfaces shall be shop bolted into full contact before welding. Sufficient external welds or tacks shall be made to secure flanges until remaining welds are made after disassembly. Adjacent flanges shall be "match marked" to insure proper field assembly.





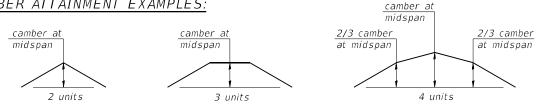
(Upper Chord - each end of each unit only)

(Lower Chord - all panel points)

CAMBER DIAGRAM

Camber curve shown is theoretical. Actual camber attained by slope changes at splices between units.

CAMBER ATTAINMENT EXAMPLES:



Camber shown is for fabrication only, measured with truss fully supported. (No-load condition)

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	CHECKED	-	H.A.	REVISED	-
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PLOT DATE =	CHECKED	-	K.M.	REVISED	-

STATE OF ILLINOIS **DEPARTMENT OF TRANSPORTATION**

 STRUCTURES – ALUMINUM TRUSS DETAILS II TRUSS TYPES I–A, II–A AND III–A
SHEET OSG5-07 OF OSG5-19 SHEETS

detrimental motion during transport. This may require ropes between

The Contractor is responsible for maintaining the configuration and

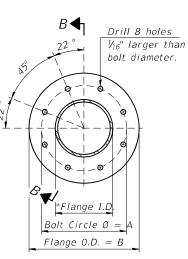
protection of the units.

horizontals and diagonals or energy dissipating (elastic) ties to the vehicle.

F.A.I. RTE	SEC.	TION	COUNTY	TOTAL SHEETS	SHEE NO.	
90	2020-0	04 - BR		соок	1492	394
			CONTRAC	T NO. 62	2K74	
		ILLINOIS	D PROJECT			

Drill 6 holes 1/₁₆" larger than bolt diameter. *Flange I.D

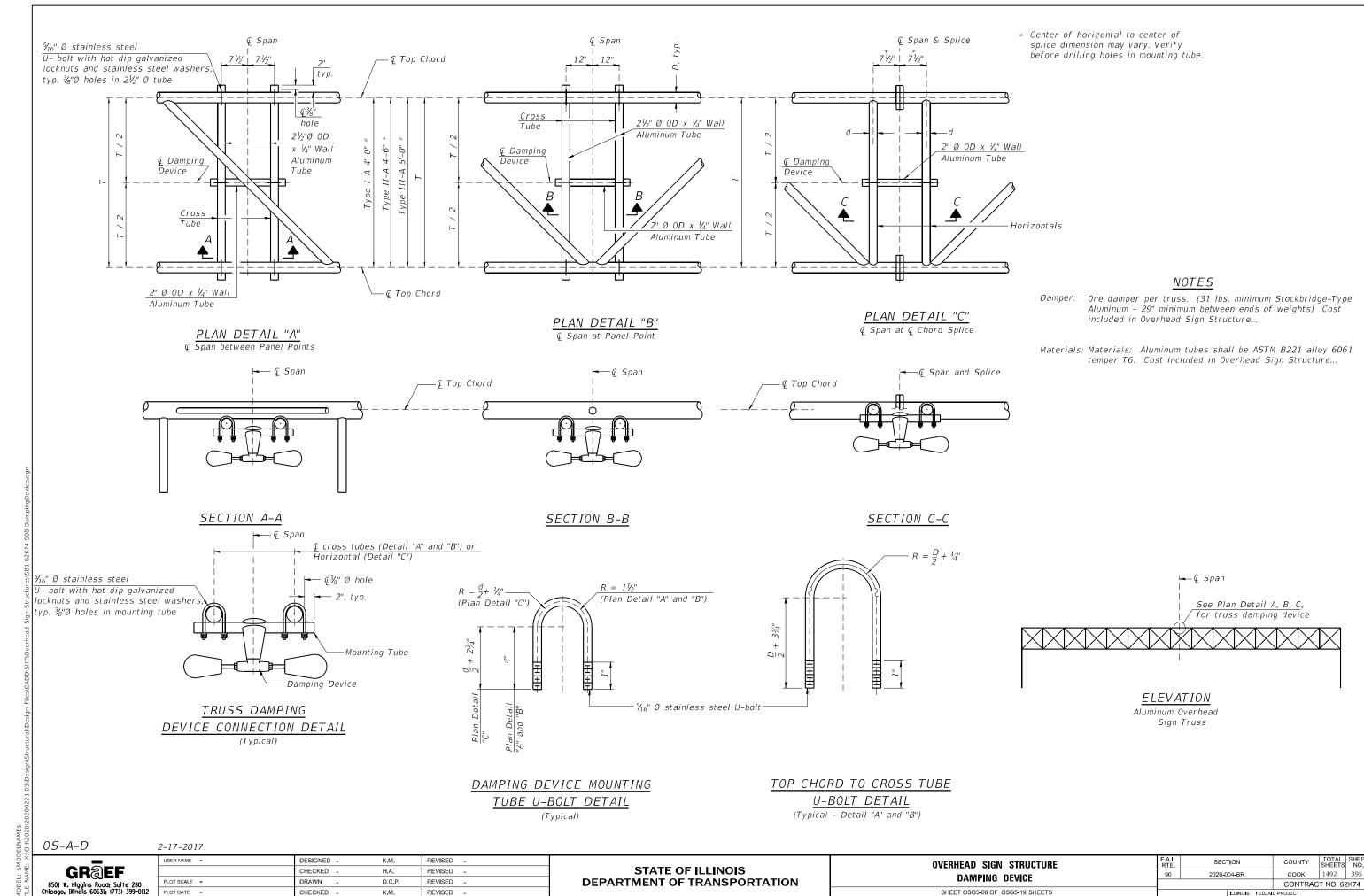
TRUSS TYPES I-A, II-A, & III-A



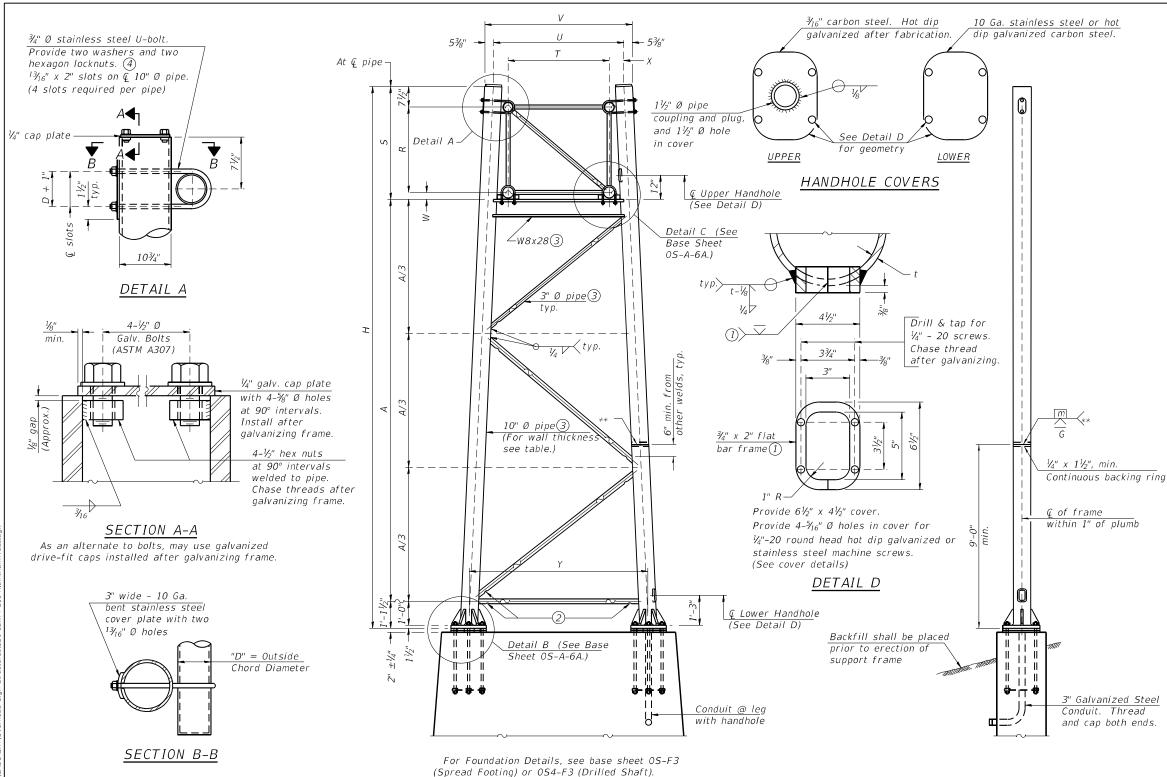
TRUSS TYPES II-A & III-A

SPLICING FLANGES

ASTM B221, Alloy 6061-T6 or ASTM B209, Alloy 6061-T651 *To fit O.D. of Chord with maximum gap of V_{16} ".



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SIDE ELEVATION

Support Design Loads: See Base Sheet OS-A-1 for design and loading criteria.

Load combinations checked include deadload plus:

a) 100% wind normal to sign, 20% parallel to sign b) 60% wind normal to sign, 30% parallel to sign

- ① In lieu of fabricated handhole frame as shown, may cut from 2" plate (rolling direction vertical). All cut faces to be ground to ANSI Roughness of 500μ in or less.
- @ Galvanizing vent holes of adequate size shall be provided on underside at each end of bracing pipes. Alternately, holes may be provided in wall of pipe column. All vent holes shall be drilled and de-burred, typ.
- 3 Steel pipe, plate, carbon steel handhole covers and rolled sections shall be hot dip galvanized after fabrication. Painting is not permitted. See Base Sheet OS-A-1.
- (4) See General Notes for fasteners.
- Dimensions shown are based on selection criteria in the Sign Structures Manual. Nonstandard applications must have dimensions verified or amended as appropriate.
- (6) "H" based on 15'-0" or actual sign height, whichever is greater.

END ELEVATION

Truss				Dimen	sions			
Туре	R	S	T	U	V	W	X	Υ
I-A	4'-6"	5'-5½"	4'-0"	5'-6"	6'-43/4"	4"	9"	8'-3"
II-A (5)	5'-3"	6'-31/4"	4'-6"	6'-1"	6'-11¾"	4¾"	91/2"	8'-3"

10" Ø PIPE TRUSS SUPPORT FRAME

** One butt welded joint is allowed only on one post per support frame. If used, weld procedure must be preapproved by Engineer and joint shall receive 100% RT or UT (tension criteria) at Contractor's expense.

	Structure	Station	Sup		Truss	Pipe Wall	Н	
	Number	Station	Left	Right	Туре	Thickness	6	А
Sign 6	15016I094R043.8	655+85.00	Х		II-A	0.365	25'- 6 1/4"	18'- 1 1/2"
				Χ	II-A	0.365	29'- 1 5/8"	21'- 8 7/8'
Sign 1	15016I094R044.2	644+79.47	X		II-A	0.365	25'- 6''	18'- 1 1/4"
				Χ	II-A	0.365	29'- 0 3/4"	21'-8"

05-A-6

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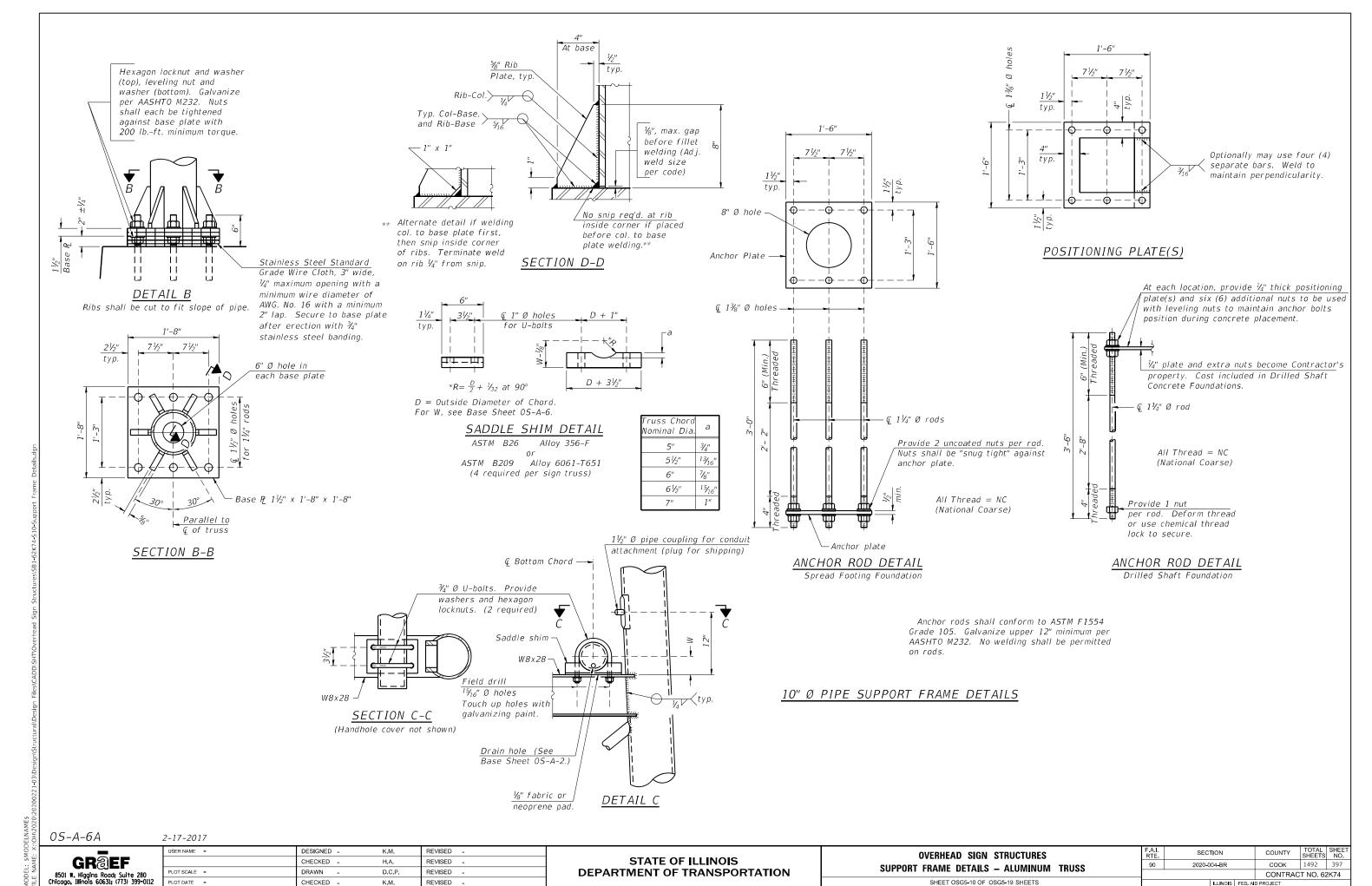
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PLOT SCALE =	DRAWN -	D.C.P.	REVISED -	l
PLOT DATE =	CHECKED -	K.M.	REVISED -	

STATE OF ILLINOIS
DEPARTMENT OF TRANSPORTATION

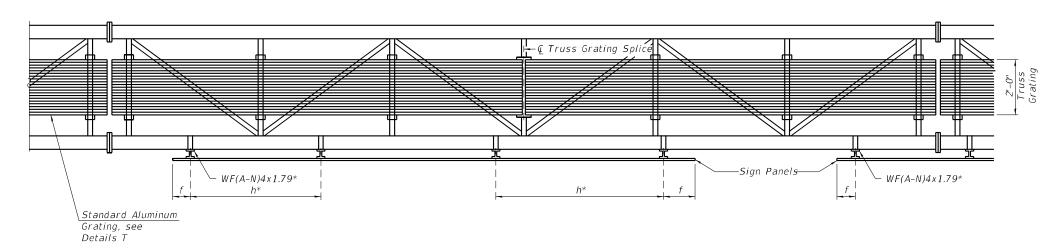
OVERHEAD SIGN STRUCTURES SUPPORT FRAME FOR ALUMINUM TRUSS	F.A.I. RTE	SECTION	COUNTY	TOTAL SHEETS	SHEET NO.
	90	2020-004-BR	соок	1492	396
SUFFURI TRAINE TUR ALUMINUM TRUSS			CONTRAC	T NO. 62	2K74
SHEET OSG5-09 OF OSG5-19 SHEETS		ILLINOIS FED A	D PROJECT		

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TYPICAL FRONT ELEVATION



BRACKET TABLE

WF(A-N)4x1.79 ASTM B308, Alloy 6061-T6									
Sign V	Number								
Greater Than	Less Than or Equal To	Brackets Required							
	8'-0"	2							
8'-0"	14'-0"	3							
14'-0"	20'-0"	4							
20'-0"	5								
26'-0"	32'-0"	6							

SECTION A-A

Place all sign brackets as close to panel points as practical.

* Space sign brackets WF(A-N)4x1.79 for efficiency and within limits shown:

f=12" maximum, 4" minimum (End of sign to $\mathcal Q$ of nearest bracket) h=6'-0" maximum ($\mathcal Q$ to $\mathcal Q$ sign support brackets, WF(A-N)4x1.7

Notes:

For Detail T and Section B-B, see Base Sheet 0S-A-10-NW.
Truss grating to facilitate inspection shall run full length
(center to center of support frames) 12"± on overhead trusses.
Cost of truss grating is included in "Overhead Sign Structure".

Truss Grating width dimensions are nominal and may vary V_2 "± based on available standard widths.

05-A-9-NW

4-1-2020

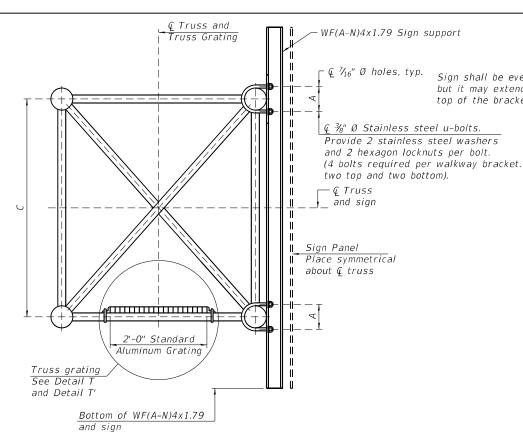


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PLOT DATE =	CHECKED	-	K.M.	REVISED	-

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DEPARTMENT OF TRANSPORTATION

OVERHEAD	SIGN	STRI	UCTURES	
ALUMINUM	WALKW	VAY	DETAILS	I
SHEET OSC	E 11 OF O	CC 5 10	O CHEETC	

F.A.I. RTE	SEC.	TION	COUNTY	TOTAL SHEETS	SHE	
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		CONTRAC	T NO. 62	2K74		
		D PROJECT				



SECTION B-B

SPECIFICATIONS FOR STANDARD ALUMINUM GRATING

Main Bearing Bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on $1\frac{3}{16}$ " centers and conform to ASTM B221 Alloy 6061-T6.

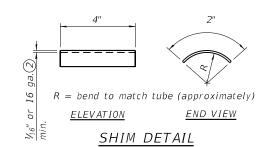
Cross bars shall be $\frac{3}{16}$ " x $1\frac{1}{2}$ " on 4" centers and conform to ASTM B221 Alloy 6063-T5 or 6061-T6.

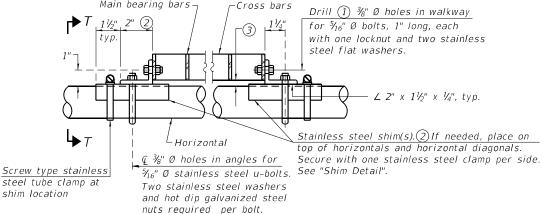
Aluminum Grating with modified "t" sections for main bearing bars shall meet the following requirements:

Main bars shall conform to ASTM B221 Alloy 6061-T6 and have a minimum section modulus equal to 0.0705 in.3 per bar, a depth of $1\frac{1}{2}$ ", spaced on $1\frac{3}{16}$ " centers.

Cross bars shall conform to ASTM B221 Alloy 6063-T5 or T-42 and spaced on 4" centers.

	Structure Number	Station	А	С
Sign 6	15016I094R043.8	655+85.00	7"	5'-3''
Sign 1	15016I094R044.2	644+79.47	7"	5'-3''





DETAIL T

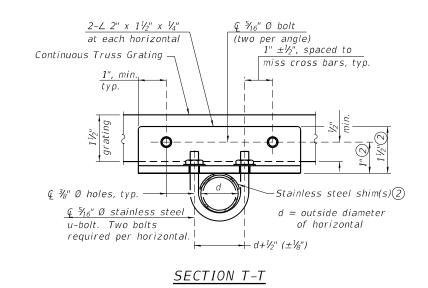
U-bolt and angle connections

required at horizontals only.

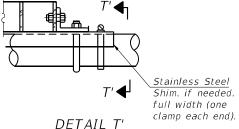
Main bearing bars -

Sign shall be even with the top of the bracket, but it may extend no more than 6" above the top of the bracket for field adjustments.

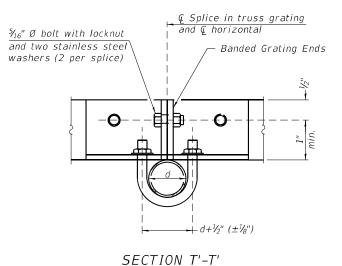
(Continuous Truss grating)



- 1) Drilling holes in grating may be done in shop or field, based on Contractor's preference and subject to accurate alignment.
- (2) Stainless steel shims shall be placed as shown in Detail T <u>if needed</u> to compensate for alignment variations between horizontal and diagonal pipes beyond adjustment provided by angles. Thicker shims may be used subject to shims performing properly.
- 3 Tube to grating gap may vary from 0 to ½", max. to align walkway, allow for camber, etc.



(Truss grating splice) Details not shown same as Detail T. Alternate materials may be used subject to the Engineer's review and approval.



05-A-10-NW

4-1-2020

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STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

SECTION **OVERHEAD SIGN STRUCTURES ALUMINUM WALKWAY DETAILS II** SHEET OSG5-12 OF OSG5-19 SHEETS

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COUNTY 2020-004-BR COOK 1492 399 CONTRACT NO. 62K74

BAR LIST - EACH FOUNDATION

Bar	Number	Size	Length	Shape	
v4(E)	24	#9	F less 5"		
#4 ba	ar spiral	(E) - see	Side Eleva	tion	

The foundation dimensions shown are based on the presence of mostly cohesive soils with an average Unconfined Compressive Strength (Qu) of at least 1.25 tsf, which must be determined by previous soil investigations at the jobsite. When other conditions are indicated, the boring data will be included in the plans and the foundation dimensions shown will be the result of site specific designs.

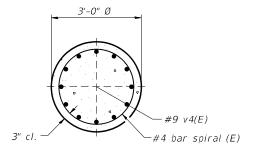
If the conditions encountered are different than those indicated, the Contractor shall notify the Engineer to determine if the foundation dimensions need to be modified. If dimensions "B" or "F" are revised by more than 12" by the Contractor, "as-built" plans shall be prepared and submitted to the District Bureau of Operations for future reference.

No sonotubes or decomposable forms shall be used below the lower conduit entrance. Permanent metal forms or other shielding may not be left in place below that elevation without the Engineer's written permission.

Concrete shall be placed monolithically, without construction joints.

Backfill shall be placed per Article 502 of Standard Specification and prior to erection of support column.

A normal surface finish followed by a Concrete Sealer application will be required on concrete surfaces above the lowest elevation 6" below finished ground line. Cost included in Drilled Shaft Concrete Foundation.



SECTION A-A

DETAILS FOR 10" Ø SUPPORT FRAME

TYPE I-A or II-A TRUSS

3" Ø Galvanized Steel

Conduit. Thread

and cap both ends.

	Structure				Left Fo	oundation			Right F	oundation			Class DS
	Number	Station	Elevation Top	Elevation Bottom	А	В	F	Elevation Top	Elevation Bottom	А	В	F	Concrete (Cu. Yds.)
Sign 6	1S016I094R043.8	655+85.00						620.94	596.94	3'-6"	20.5'	24'-0''	12.6
Sign 1	15016I094R044.2	644+79.47						624.93	600.93	3'-6"	20.5'	24'-0''	12.6

054-F3

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#4 bar spiral (E) at 6" pitch

12-#9 v4(E) bars

3 hoops minimum, top and bottom

3'-0" Ø

71/2"-

2-17-2017

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8'-3" (to (

Approved clamps

#6 copper wire or cable

′¾" Ø x 10'-0" copper weld ground rod driven into groun 9'-0". Cost of rod, cable, conduit, caps and clamps

shall be included in Drilled Shaft Concrete Foundations.

11'-3"

8'-3"

PLAN

SIDE ELEVATION

for grounding*

_3'-0" Ø

7½"—

Elevation (Top)

> Elevation (Bottom)

END VIEW

For anchor rod size and placement, see Support Frame Detail Sheet.

* Anchor rod shall be ground or filed to bright metal at clamp and cable connection location.

STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION **OVERHEAD SIGN STRUCTURES** DRILLED SHAFT DETAILS I SHEET OSG5-13 OF OSG5-19 SHEETS

SECTION COUNTY 2020-004-BR COOK 1492 400 CONTRACT NO. 62K74